

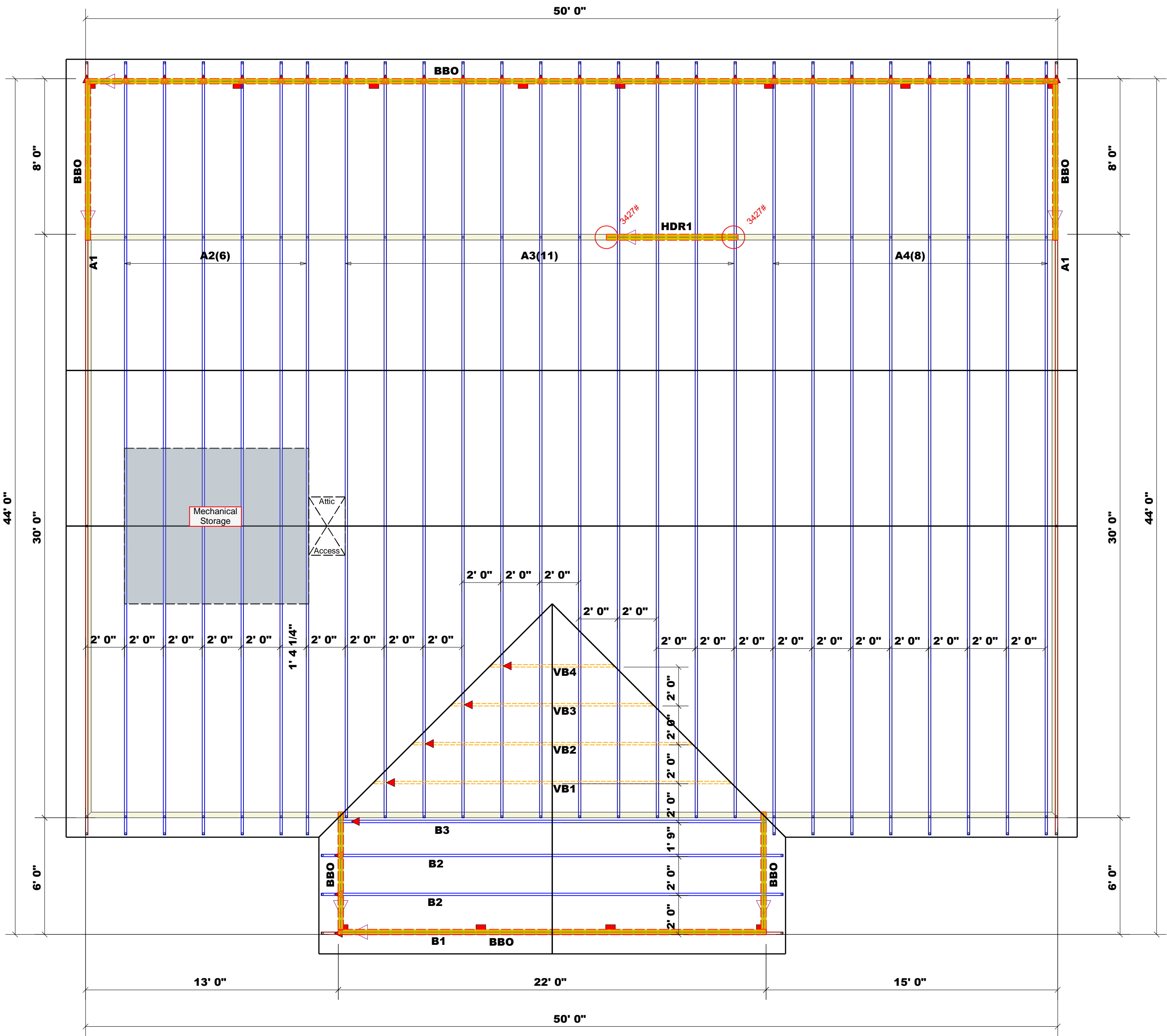


# ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park  
Fayetteville, N.C. 28309  
Phone: (910) 864-8787  
Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables ( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature Neil Baggett  
**Neil Baggett**



Roof Area = 2549.98 sq.ft.  
Ridge Line = 70 ft.  
Hip Line = 0 ft.  
Horiz. OH = 144 ft.  
Raked OH = 120.26 ft.  
Decking = 88 sheets

All Walls Shown Are Considered Load Bearing

1 Truss Placement Plan  
Scale: 1/4"=1'

▲ = Denotes Left End of Truss  
(Reference Engineered Truss Drawing)

Plumbing Drop Notes  
1. Plumbing drop locations shown are NOT exact.  
2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.  
3. Adjust spacing as needed not to exceed 24"oc.

Dimension Notes  
1. All exterior wall to wall dimensions are to face of stud unless noted otherwise  
2. All interior wall dimensions are to face of stud unless noted otherwise  
3. All exterior wall to truss dimensions are to face of stud unless noted otherwise

Hatch Legend  
Padded HVAC  
Drop Beam

Products				
PlotID	Length	Product	Plies	Net Qty
HDR1	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.  
○ -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

### LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))  
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION (UP TO)	REQ. STUDS FOR (1) FT. HEADER	END REACTION (UP TO)	REQ. STUDS FOR (1) FT. HEADER	END REACTION (UP TO)	REQ. STUDS FOR (1) FT. HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #	CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
Old Hickory Enterprises, LLC	Thompson Residence	Plan 1502-RRR	7/1/15	Quote #	J1023-5827	Lillington / Harnett	178 Collins Rd.	Roof	10/17/2023	Neil Baggett	Neil Baggett

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

# Reaction Summary of Order



REQ. QUOTE DATE	/ /	ORDER #	J1023-5827
ORDER DATE	10/17/23	QUOTE #	
DELIVERY DATE	/ /	CUSTOMER ACCT #	0000006631
DATE OF INVOICE	/ /	CUSTOMER PO #	
ORDERED BY	Gary Sealey	INVOICE #	
COUNTY	Harnett	TERMS	
SUPERINTENDANT	Gary Sealey	SALES REP	Neil Baggett
JOBSITE PHONE #	(910) 885--1664	SALES AREA	Neil Baggett

SOLD TO	<b>Old Hickory Enterprises, LLC</b> 233 Tailwinds Ln. St. Pauls, NC 28384 (910) 885--1664	<b>JOB NAME:</b> Thompson Residence <b>MODEL:</b> Roof <b>TAG:</b> Plan 1502-RRR <b>DELIVERY INSTRUCTIONS:</b> 70 miles round trip	<b>LOT #</b> <b>SUBDIV:</b> <b>JOB CATEGORY:</b> WCall - Will Call
	<b>Old Hickory Enterprises, LLC</b> 178 Collins Rd. Lillington, NC 27546	<b>SPECIAL INSTRUCTIONS:</b>	<b>PLAN SEAL DATE:</b> 7/1/15 BY      DATE

<b>BUILDING DEPARTMENT</b>	<b>OVERHANG INFO</b>	<b>HEEL HEIGHT</b>	00-06-08	<b>REQ. LAYOUTS</b>	<b>REQ. ENGINEERING</b>	<b>QUOTE</b>	/ /
Roof Order	<b>END CUT</b>	<b>RETURN</b>				<b>LAYOUT</b>	/ /
	<b>PLUMB</b>	<b>GABLE STUDS</b>	24 IN. OC	<b>JOBSITE</b>	1	<b>CUTTING</b>	NB 10/17/23

<b>ROOF TRUSSES</b>	<b>LOADING INFORMATION</b>	TCLL-TCDL-BCLL-BCDL	STRESS INCR.	<b>ROOF TRUSS SPACING:</b> 24.0 IN. O.C. (TYP.)
		20.0,10.0,0.0,10.0	1.15	

PROFILE	QTY	PITCH		TYPE ID	BASE O/A	LUMBER		OVERHANG		REACTIONS				
		TOP	BOT			TOP	BOT	LEFT	RIGHT	Joint 2	Joint 22	Joint 24	Joint 25	Joint 26
	2	4.00	0.00	GABLE A1	38-00-00 38-00-00	2 X 6	2 X 6	00-10-08	00-10-08	151.7 lbs.	167.8 lbs.	151.5 lbs.	184.3 lbs.	175.3 lbs.
										-42.4 lbs.	-57.0 lbs.	-132.4 lbs.	-93.2 lbs.	-86.5 lbs.
	6	4.00	0.00	ROOF A2	38-00-00 38-00-00	2 X 6	2 X 6	00-10-08	00-10-08	1296.6 lbs.	1978.3 lbs.			
										-77.5 lbs.	-264.8 lbs.			
	11	4.00	0.00	ROOF A3	38-00-00 38-00-00	2 X 6	2 X 6	00-10-08		1256.6 lbs.	1979.1 lbs.			
										-64.9 lbs.	-152.6 lbs.			
	8	4.00	0.00	ROOF A4	38-00-00 38-00-00	2 X 6	2 X 6	00-10-08	00-10-08	1307.1 lbs.	1978.3 lbs.			
										-77.5 lbs.	-152.6 lbs.			
	1	8.00	0.00	GABLE B1	22-00-00 22-00-00	2 X 6	2 X 6	00-10-08	00-10-08	922.1 lbs.	922.1 lbs.			
										-190.6 lbs.	-190.6 lbs.			
	2	8.00	0.00	COMMON B2	22-00-00 22-00-00	2 X 6	2 X 6	00-10-08	00-10-08	922.1 lbs.	922.1 lbs.			
										-135.8 lbs.	-135.8 lbs.			
	1	8.00	0.00	COMMON B3	22-00-00 22-00-00	2 X 6	2 X 6			870.0 lbs.	870.0 lbs.			
										-132.2 lbs.	-132.2 lbs.			
	1	8.00	0.00	VALLEY VB1	18-03-05 18-03-05	2 X 4	2 X 4			168.1 lbs.	168.2 lbs.	489.1 lbs.	401.5 lbs.	493.5 lbs.
										-2.7 lbs.	10.9 lbs.	-125.2 lbs.	50.2 lbs.	-125.3 lbs.
	1	8.00	0.00	VALLEY VB2	14-03-05 14-03-05	2 X 4	2 X 4			106.1 lbs.	93.0 lbs.	340.0 lbs.	259.4 lbs.	340.2 lbs.
										-11.9 lbs.	8.1 lbs.	-97.9 lbs.	48.1 lbs.	-98.1 lbs.
	1	8.00	0.00	VALLEY VB3	10-03-05 10-03-05	2 X 4	2 X 4			186.3 lbs.	186.3 lbs.	378.0 lbs.		
										-23.2 lbs.	-30.5 lbs.	6.5 lbs.		
	1	8.00	0.00	VALLEY VB4	06-03-05 06-03-05	2 X 4	2 X 4			117.1 lbs.	117.1 lbs.	196.4 lbs.		
										-18.5 lbs.	-22.7 lbs.	14.1 lbs.		

QTY	ITEM TYPE	SIZE	LENGTH	PART NUMBER	NOTES
			FT-IN-16		

# Reaction Summary of Order



**ROOF & FLOOR TRUSSES & BEAMS**  
 Reilly Road Industrial Park P.O. Box 40408  
 Fayetteville, N.C. 28309 (910) 864-TRUS

REQ. QUOTE DATE	/ /	ORDER #	J1023-5827
ORDER DATE	10/17/23	QUOTE #	
DELIVERY DATE	/ /	CUSTOMER ACCT #	0000006631
DATE OF INVOICE	/ /	CUSTOMER PO #	
ORDERED BY	Gary Sealey	INVOICE #	
COUNTY	Harnett	TERMS	
SUPERINTENDANT	Gary Sealey	SALES REP	Neil Baggett
JOBSITE PHONE #	(910) 885--1664	SALES AREA	Neil Baggett

**SOLD TO**  
 Old Hickory Enterprises, LLC  
 233 Tailwinds Ln.  
 St. Pauls, NC 28384  
 (910) 885--1664

**JOB NAME:** Thompson Residence **LOT #** **SUBDIV:**  
**MODEL:** Roof **TAG:** Plan 1502-RRR **JOB CATEGORY:** WCall - Will Call  
**DELIVERY INSTRUCTIONS:**  
 70 miles round trip

**SHIP TO**  
 Old Hickory Enterprises, LLC  
 178 Collins Rd.  
 Lillington, NC 27546

**SPECIAL INSTRUCTIONS:**

**PLAN SEAL DATE:** 7/1/15  
**BY** **DATE**

<b>BUILDING DEPARTMENT</b>	<b>OVERHANG INFO</b>	<b>HEEL HEIGHT</b>	00-06-08	<b>REQ. LAYOUTS</b>	<b>REQ. ENGINEERING</b>	<b>QUOTE</b>	/ /
Roof Order	<b>END CUT</b>	<b>RETURN</b>				<b>LAYOUT</b>	/ /
	<b>PLUMB</b>	<b>GABLE STUDS</b>	24 IN. OC		JOBSITE 1	JOBSITE 1	<b>CUTTING</b> NB 10/17/23

## ITEMS

QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
2	LVL Beams (Sized)	LVL, 1-3/4" x 9-1/4" (S)	07-00-00		HDR1

Trenco  
818 Soundside Rd  
Edenton, NC 27932

Re: J1023-5827  
Old Hickory/Thompson Residence/Harnett

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I61450353 thru I61450363

My license renewal date for the state of North Carolina is December 31, 2023.

North Carolina COA: C-0844



October 18, 2023

Gilbert, Eric

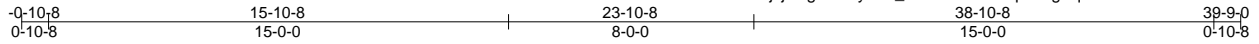
**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job J1023-5827	Truss A1	Truss Type GABLE	Qty 2	Ply 1	Old Hickory/Thompson Residence/Harnett Job Reference (optional)	161450353
-------------------	-------------	---------------------	----------	----------	--------------------------------------------------------------------	-----------

Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Oct 17 07:57:43 2023 Page 1

ID:roCQZwnLu08jxjH0goh?9tySav\_-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcDoi7J4zJC?f



Scale = 1:75.1

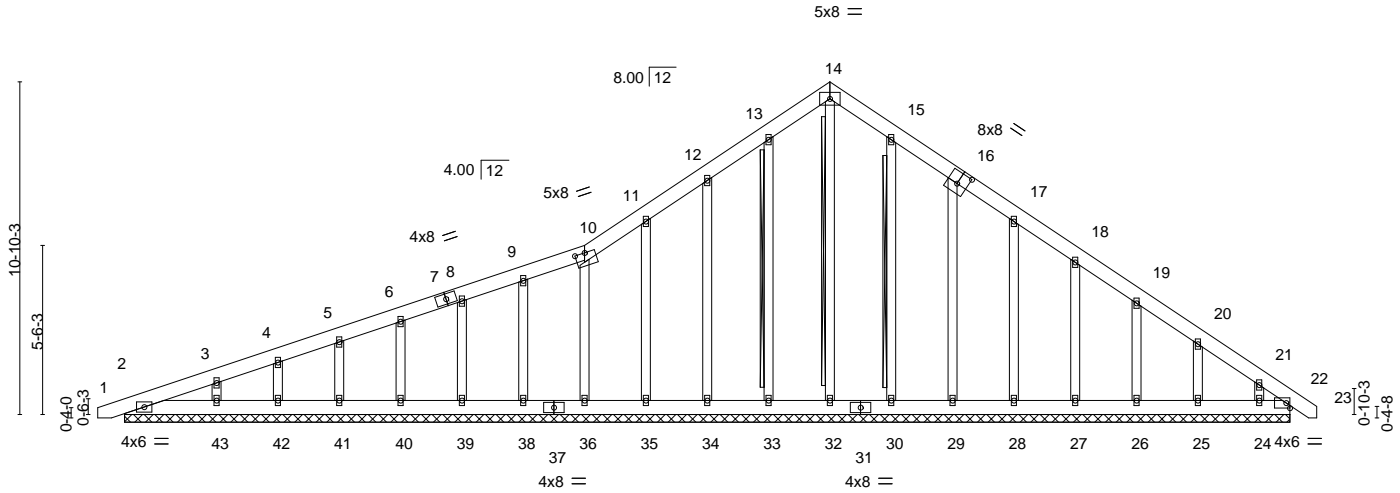


Plate Offsets (X, Y)--	[10:0-4-0-0-0-0], [16:0-4-0-0-4-8]
------------------------	------------------------------------

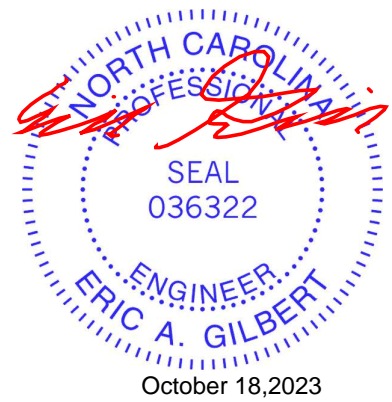
LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) -0.00	22	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.02	Vert(CT) -0.00	22	n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.15	Horz(CT) 0.01	22	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S						
							Weight: 319 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2x4 SP No.2	WEBS T-Brace: 2x4 SPF No.2 - 14-32, 13-33, 15-30
	Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.
	Brace must cover 90% of web length.

**REACTIONS.** All bearings 38-0-0.  
 (lb) - Max Horz 2=343(LC 11)  
 Max Uplift All uplift 100 lb or less at joint(s) 2, 22, 33, 34, 35, 36, 38, 39, 40, 41, 42, 43, 30, 28, 27, 26, 25 except 29=103(LC 13), 24=-132(LC 13)  
 Max Grav All reactions 250 lb or less at joint(s) 2, 22, 32, 33, 34, 35, 36, 38, 39, 40, 41, 42, 43, 30, 29, 28, 27, 26, 25, 24

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-286/64, 12-13=-206/256, 13-14=-242/286, 14-15=-243/287, 21-22=-283/194

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
  - All plates are 2x4 MT20 unless otherwise indicated.
  - Gable requires continuous bottom chord bearing.
  - Gable studs spaced at 2-0-0 oc.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 22, 33, 34, 35, 36, 38, 39, 40, 41, 42, 43, 30, 28, 27, 26, 25 except (jt=lb) 29=103, 24=132.
  - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
  - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.





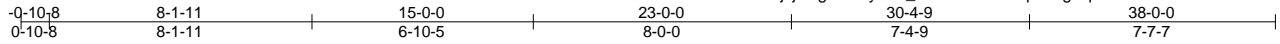


Job J1023-5827	Truss A3	Truss Type ROOF SPECIAL	Qty 11	Ply 1	Old Hickory/Thompson Residence/Harnett Job Reference (optional)	161450355
-------------------	-------------	----------------------------	-----------	----------	--------------------------------------------------------------------	-----------

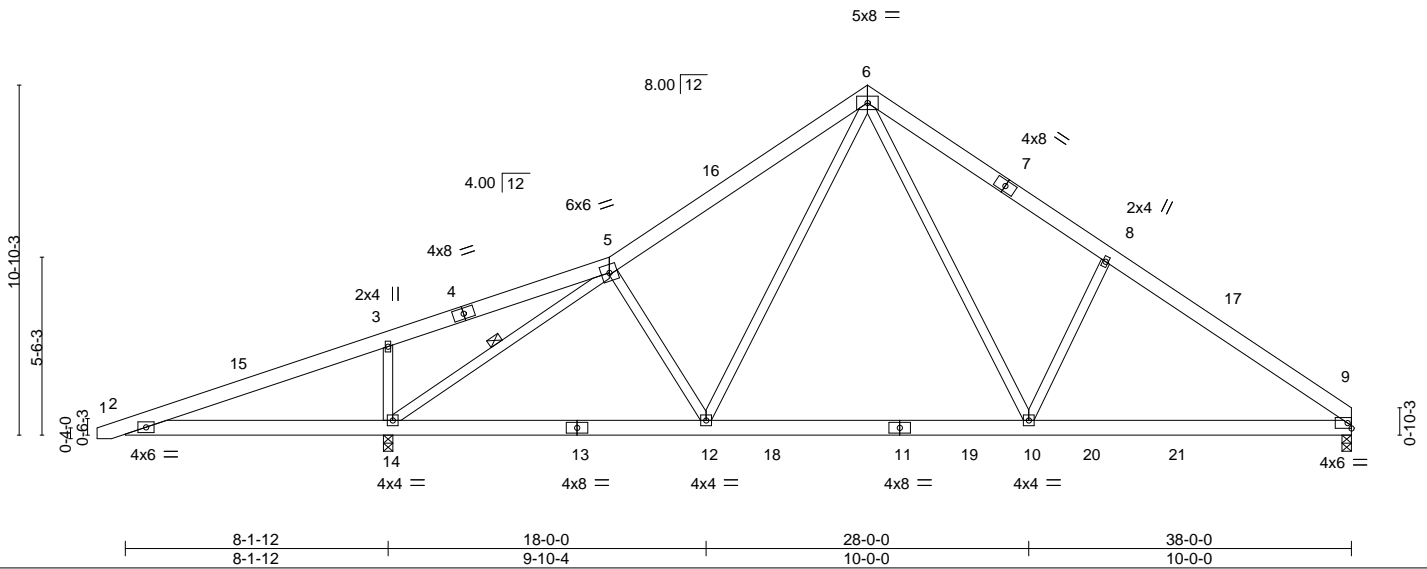
Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Oct 17 07:57:46 2023 Page 1

ID:roCQZwnLu08jxjH0goh?9tySav\_RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



Scale = 1:71.4



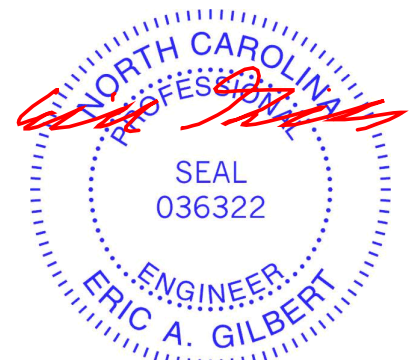
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.41	Vert(LL)	-0.17 10-12	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.49	Vert(CT)	-0.23 10-12	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.70	Horz(CT)	0.03 9	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S	Wind(LL)	0.03 10-12	>999	240		
								Weight: 259 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD Structural wood sheathing directly applied or 5-8-6 oc purlins.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
WEBS 2x4 SP No.2	6-0-0 oc bracing: 2-14.
	WEBS 1 Row at midpt 5-14

**REACTIONS.** (size) 14=0-3-8, 9=0-3-8  
 Max Horz 14=258(LC 9)  
 Max Uplift 14=-153(LC 12), 9=-65(LC 13)  
 Max Grav 14=1979(LC 1), 9=1257(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-407/972, 3-5=-293/931, 5-6=-1318/339, 6-8=-1626/431, 8-9=-1750/330  
 BOT CHORD 2-14=-834/447, 12-14=-73/1094, 10-12=0/910, 9-10=-141/1339  
 WEBS 3-14=-541/259, 5-14=-2080/491, 6-12=-51/389, 6-10=-161/954, 8-10=-467/294

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-13 to 3-9-0, Interior(1) 3-9-0 to 23-0-0, Exterior(2) 23-0-0 to 27-4-13, Interior(1) 27-4-13 to 37-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
  - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 14=153.
  - 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



October 18, 2023

Job J1023-5827	Truss A4	Truss Type ROOF SPECIAL	Qty 8	Ply 1	Old Hickory/Thompson Residence/Harnett Job Reference (optional)	161450356
-------------------	-------------	----------------------------	----------	----------	--------------------------------------------------------------------	-----------

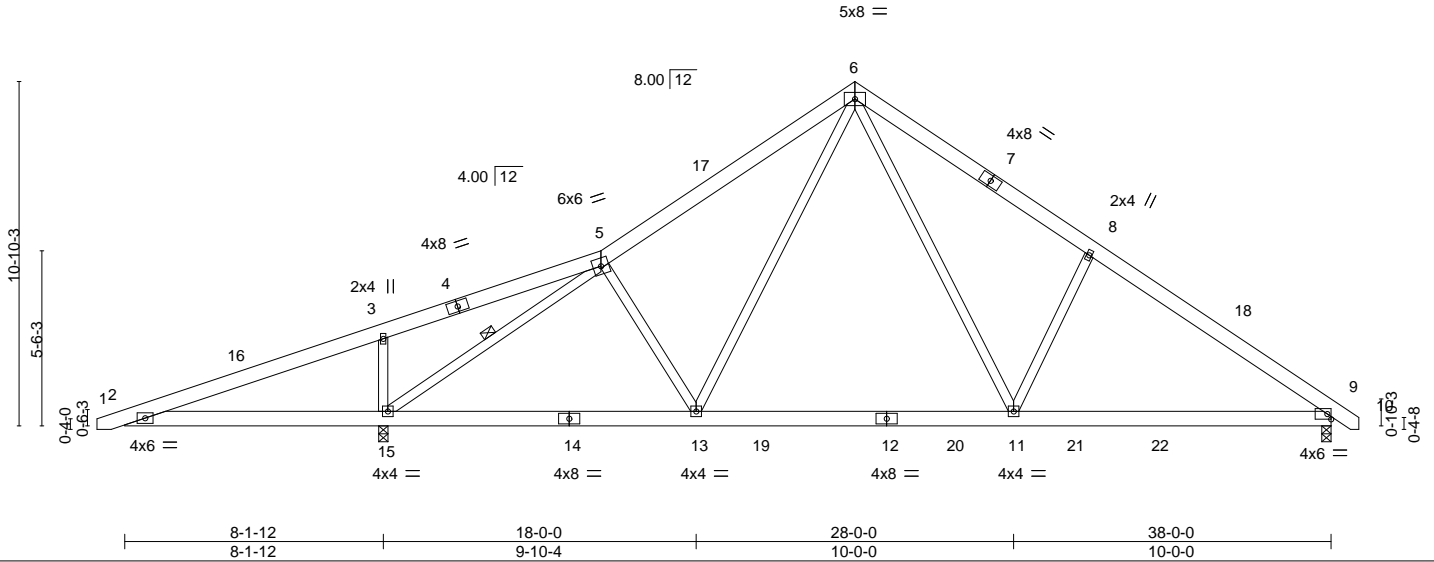
Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Oct 17 07:57:47 2023 Page 1

ID:roCQZwnLu08xjH0goh?9tySav\_-RtC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

-0-10-8	8-1-11	15-0-0	23-0-0	30-4-9	38-0-0	38-10-8
0-10-8	8-1-11	6-10-5	8-0-0	7-4-9	7-7-7	0-10-8

Scale = 1:72.6



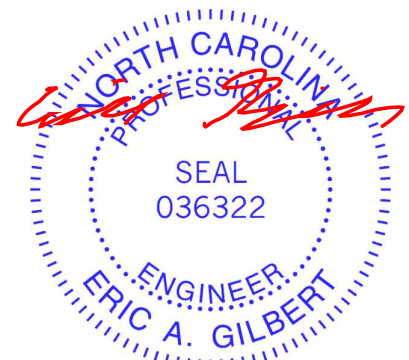
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.41	Vert(LL)	-0.17 11-13	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.49	Horz(CT)	-0.23 11-13	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.70	Wind(LL)	0.03 9-11	>999	240		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S					Weight: 261 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD Structural wood sheathing directly applied or 5-8-12 oc purlins.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
WEBS 2x4 SP No.2	6-0-0 oc bracing: 2-15.
	WEBS 1 Row at midpt 5-15

**REACTIONS.** (size) 15=0-3-8, 9=0-3-8  
 Max Horz 15=259(LC 11)  
 Max Uplift 15=-153(LC 12), 9=-77(LC 13)  
 Max Grav 15=1978(LC 1), 9=1307(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-407/972, 3-5=-293/931, 5-6=-1316/335, 6-8=-1622/421, 8-9=-1747/323  
 BOT CHORD 2-15=-834/447, 13-15=-65/1095, 11-13=0/911, 9-11=-124/1334  
 WEBS 3-15=-541/259, 5-15=-2079/490, 6-13=-50/390, 6-11=-160/950, 8-11=-465/290

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-13 to 3-9-0, Interior(1) 3-9-0 to 23-0-0, Exterior(2) 23-0-0 to 27-4-13, Interior(1) 27-4-13 to 38-8-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
  - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 15=153.
  - 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



October 18, 2023

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

**ENGINEERING BY**  
**TRENCO**  
 A MiTek Affiliate

818 Soundside Road  
 Edenton, NC 27932







Job J1023-5827	Truss B3	Truss Type COMMON	Qty 1	Ply 1	Old Hickory/Thompson Residence/Harnett Job Reference (optional)	I61450359
-------------------	-------------	----------------------	----------	----------	--------------------------------------------------------------------	-----------

Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Oct 17 07:57:51 2023 Page 1  
ID:roCQZwnLu08jxjH0goh?9tySav\_-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

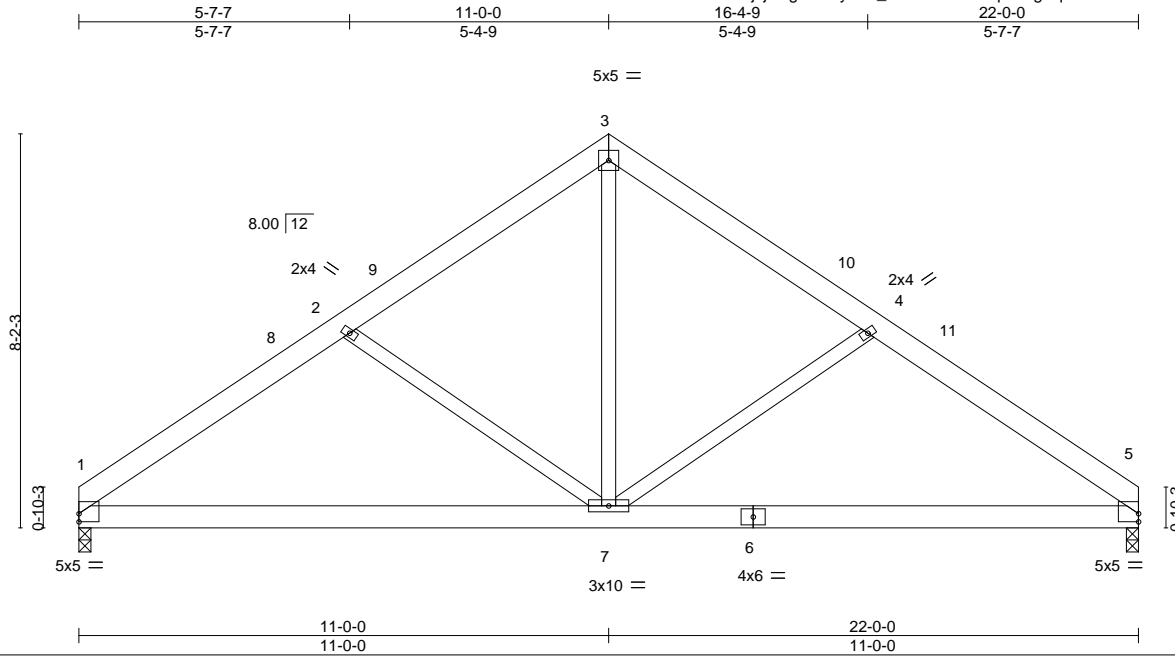


Plate Offsets (X,Y)-- [1:0-0-0,0-2-0], [5:Edge,0-2-0]

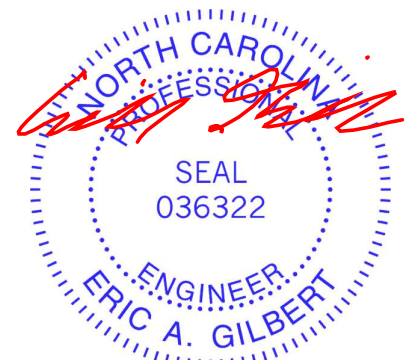
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.41	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.41	Vert(LL) -0.07 1-7 >999 360		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.79	Vert(CT) -0.15 1-7 >999 240		
BCDL 10.0	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.02 5 n/a n/a		
	Code IRC2015/TPI2014		Wind(LL) 0.15 1-7 >999 240	Weight: 144 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 8-11-9 oc bracing.
WEBS 2x4 SP No.2	

**REACTIONS.** (size) 1=0-3-0, 5=0-3-0  
 Max Horz 1=-184(LC 8)  
 Max Uplift 1=-132(LC 9), 5=-132(LC 8)  
 Max Grav 1=870(LC 1), 5=870(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 1-2=-1161/914, 2-3=-898/876, 3-4=-898/876, 4-5=-1161/914  
 BOT CHORD 1-7=-656/888, 5-7=-657/888  
 WEBS 3-7=-810/617, 4-7=-359/229, 2-7=-359/229

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-8 to 4-6-5, Interior(1) 4-6-5 to 11-0-0, Exterior(2) 11-0-0 to 15-4-13, Interior(1) 15-4-13 to 21-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=132, 5=132.

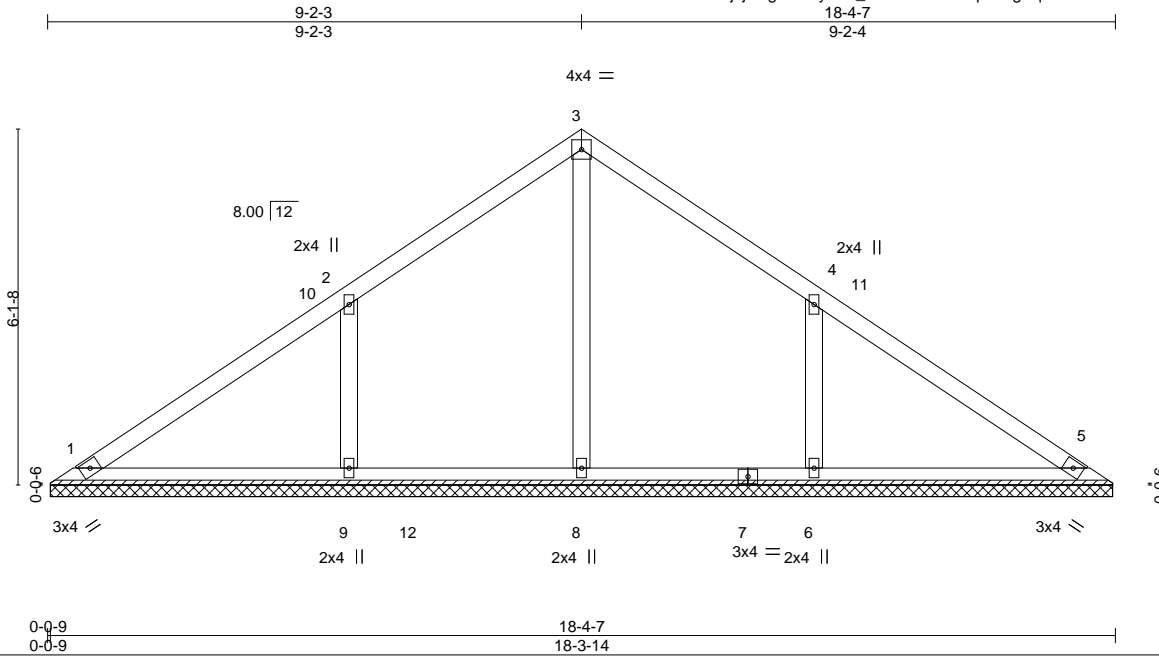


October 18, 2023

Job J1023-5827	Truss VB1	Truss Type VALLEY	Qty 1	Ply 1	Old Hickory/Thompson Residence/Harnett Job Reference (optional)	161450360
-------------------	--------------	----------------------	----------	----------	--------------------------------------------------------------------	-----------

Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Oct 17 07:57:52 2023 Page 1  
ID:roCQZwnLu08jxjH0goh?9tySav\_-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



Scale = 1:39.6

Plate Offsets (X, Y)--	[4:0-0-0,0-0-0]						
<b>LOADING</b> (psf)	<b>SPACING-</b>	2-0-0	<b>CSI.</b>	<b>DEFL.</b>	in (loc)	l/defl	L/d
TCLL 20.0	Plate Grip DOL	1.15	TC 0.21	Vert(LL)	n/a	-	n/a 999
TCDL 10.0	Lumber DOL	1.15	BC 0.16	Vert(CT)	n/a	-	n/a 999
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.09	Horz(CT)	0.00	5	n/a n/a
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S				
							<b>PLATES</b>
							MT20
							<b>GRIP</b>
							244/190
							Weight: 75 lb
							FT = 20%

<b>LUMBER-</b>		<b>BRACING-</b>	
TOP CHORD	2x4 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2x4 SP No.1	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.2		

**REACTIONS.** All bearings 18-3-5.  
 (lb) - Max Horz 1=-139(LC 8)  
 Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-125(LC 12), 6=-125(LC 13)  
 Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=402(LC 19), 9=494(LC 19), 6=489(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 WEBS 2-9=-374/236, 4-6=-374/236

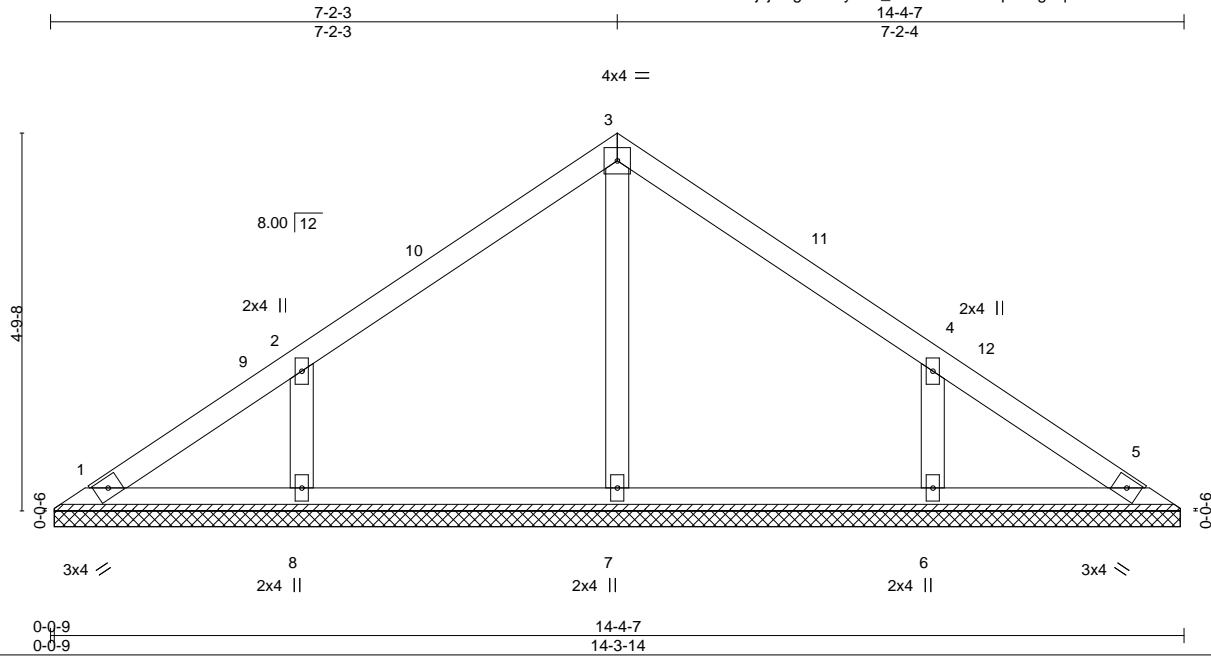
- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 5-2-3, Interior(1) 5-2-3 to 9-2-3, Exterior(2) 9-2-3 to 13-7-0, Interior(1) 13-7-0 to 17-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
  - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=125, 6=125.
  - 6) N/A



Job J1023-5827	Truss VB2	Truss Type VALLEY	Qty 1	Ply 1	Old Hickory/Thompson Residence/Harnett Job Reference (optional)	I61450361
-------------------	--------------	----------------------	----------	----------	--------------------------------------------------------------------	-----------

Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Oct 17 07:57:53 2023 Page 1  
ID:roCQZwnLu08jxjH0goh?9tySav\_-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:29.2

Plate Offsets (X,Y)--	[4:0-0-0,0-0-0]				
<b>LOADING</b> (psf)	<b>SPACING-</b> 2-0-0	<b>CSI.</b>	<b>DEFL.</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 20.0	Plate Grip DOL 1.15	TC 0.13	Vert(LL) n/a - n/a 999	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.09	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.06	Horz(CT) 0.00 5 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 56 lb	FT = 20%

<b>LUMBER-</b>	<b>BRACING-</b>
TOP CHORD 2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2x4 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2x4 SP No.2	

**REACTIONS.** All bearings 14-3-5.  
 (lb) - Max Horz 1=107(LC 9)  
 Max Uplift All uplift 100 lb or less at joint(s) 1, 8, 6  
 Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=259(LC 1), 8=340(LC 19), 6=340(LC 20)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
**WEBS** 2-8=-291/200, 4-6=-291/200

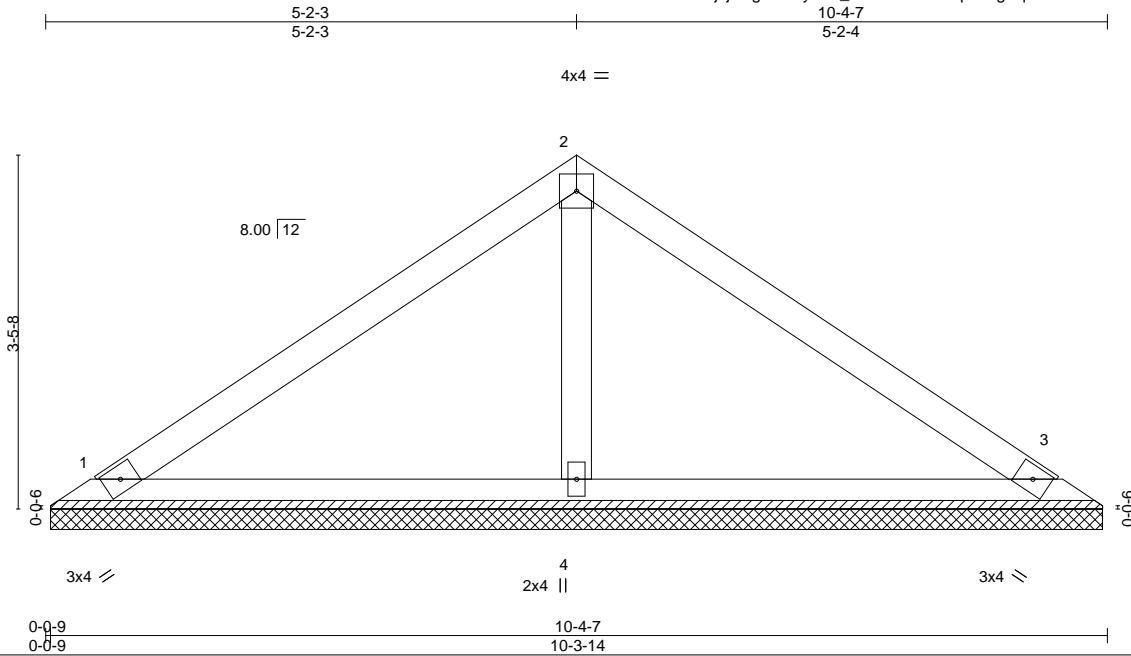
- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TC DL=6.0psf; BC DL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 4-10-12, Interior(1) 4-10-12 to 7-2-3, Exterior(2) 7-2-3 to 11-7-0, Interior(1) 11-7-0 to 13-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8, 6.
  - 6) N/A



Job J1023-5827	Truss VB3	Truss Type VALLEY	Qty 1	Ply 1	Old Hickory/Thompson Residence/Harnett Job Reference (optional)	I61450362
-------------------	--------------	----------------------	----------	----------	--------------------------------------------------------------------	-----------

Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Oct 17 07:57:54 2023 Page 1  
ID:roCQZwnLu08jxjH0goh?9tySav\_-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



Scale = 1:22.5

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.23	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.16	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.05	Horz(CT)	0.00	3	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-S					Weight: 36 lb	FT = 20%
	Code IRC2015/TPI2014							

**LUMBER-**  
TOP CHORD 2x4 SP No.1  
BOT CHORD 2x4 SP No.1  
OTHERS 2x4 SP No.2

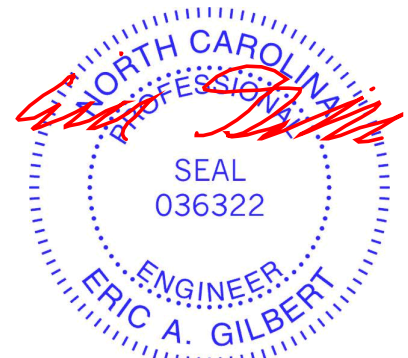
**BRACING-**  
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (size) 1=10-3-5, 3=10-3-5, 4=10-3-5  
Max Horz 1=75(LC 9)  
Max Uplift 1=23(LC 12), 3=31(LC 13)  
Max Grav 1=186(LC 1), 3=186(LC 1), 4=378(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 6) N/A



October 18, 2023

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPH Quality Criteria and DSB-22** available from Truss Plate Institute ([www.tpinst.org](http://www.tpinst.org)) and **BCSI Building Component Safety Information** available from the Structural Building Component Association ([www.sbcacomponents.com](http://www.sbcacomponents.com))



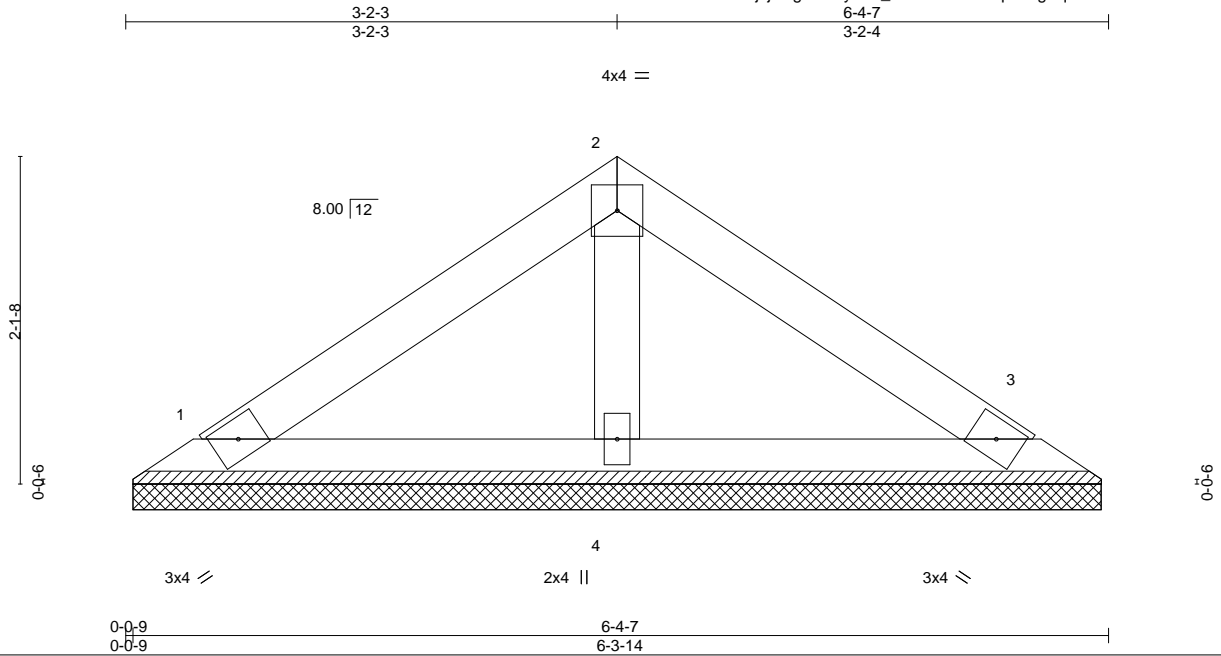
818 Soundside Road  
Edenton, NC 27932



Job J1023-5827	Truss VB4	Truss Type VALLEY	Qty 1	Ply 1	Old Hickory/Thompson Residence/Harnett Job Reference (optional)	I61450363
-------------------	--------------	----------------------	----------	----------	--------------------------------------------------------------------	-----------

Comtech, Inc. Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Oct 17 07:57:55 2023 Page 1  
ID:roCQZwnLu08jxjH0goh?9tySav\_-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



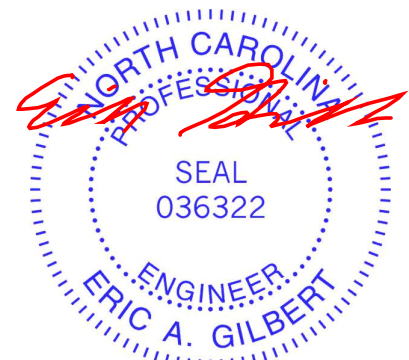
LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.10	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.05	Vert(CT)	n/a	-	n/a		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.02	Horz(CT)	0.00	3	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-P					Weight: 21 lb	FT = 20%
	Code IRC2015/TPI2014							

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2x4 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2x4 SP No.2	

**REACTIONS.** (size) 1=6-3-5, 3=6-3-5, 4=6-3-5  
 Max Horz 1=43(LC 9)  
 Max Uplift 1=-18(LC 12), 3=-23(LC 13)  
 Max Grav 1=117(LC 1), 3=117(LC 1), 4=196(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

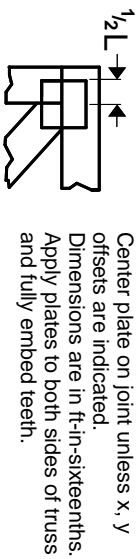
- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
  - 6) N/A



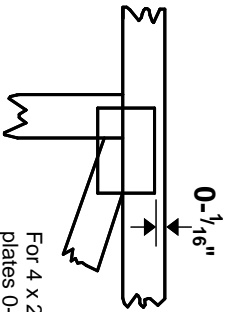
October 18, 2023

# Symbols

## PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16\" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

\* Plate location details available in MITek software or upon request.

## PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

## BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

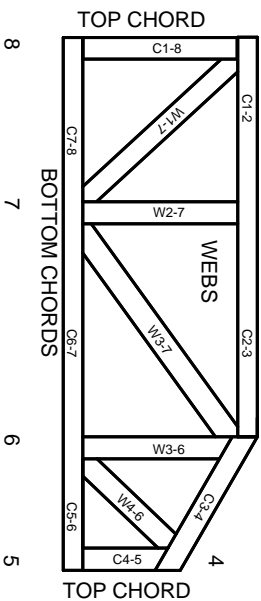
## Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.  
DSB-22: Design Standard for Bracing.  
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

# Numbering System



1 TOP CHORDS  
2 Joint ID  
3 typ.



**JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.**

**CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.**

# Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282  
ESR-4722, ESL-1388

# Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 section 6.3. These truss designs rely on lumber values established by others.

© 2023 MITek® All Rights Reserved

**MITek**

ENGINEERING BY  
**TRENGO**  
A MITek Affiliate

MITek Engineering Reference Sheet: MIL-7473 rev. 1/2/2023

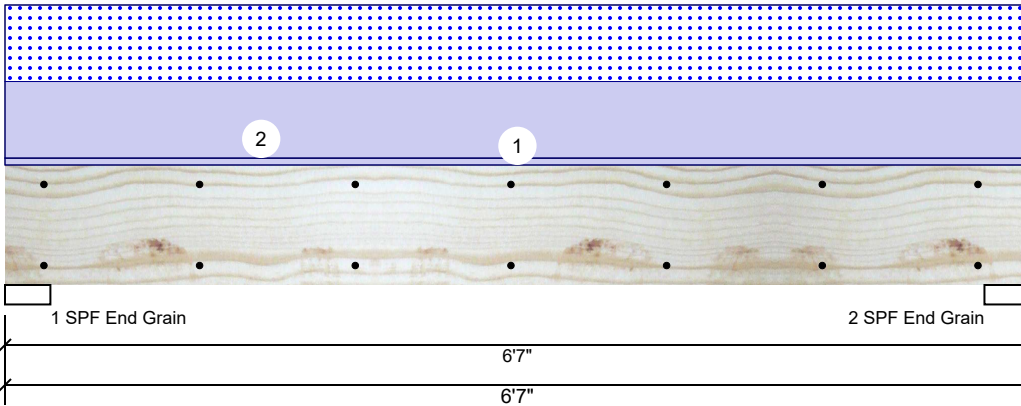
# General Safety Notes

## Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability/bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.

**HDR1 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED**

Level: Level



**Member Information**

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IRC 2018
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

**Reactions UNPATTERNED lb (Uplift)**

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1798	1626	0	0
2	Vertical	0	1798	1626	0	0

**Bearings**

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	33%	1798 / 1626	3424	L	D+S
2 - SPF End Grain	3.500"	Vert	33%	1798 / 1626	3424	L	D+S

**Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4878 ft-lb	3'3 1/2"	14423 ft-lb	0.338 (34%)	D+S	L
Unbraced	4878 ft-lb	3'3 1/2"	10451 ft-lb	0.467 (47%)	D+S	L
Shear	2324 lb	1'3/4"	7943 lb	0.293 (29%)	D+S	L
LL Defl inch	0.042 (L/1745)	3'3 1/2"	0.153 (L/480)	0.275 (28%)	S	L
TL Defl inch	0.089 (L/829)	3'3 1/2"	0.204 (L/360)	0.434 (43%)	D+S	L

**Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	45 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
2	Uniform			Top	494 PLF	0 PLF	494 PLF	0 PLF	0 PLF	A3
	Self Weight				7 PLF					

**Notes**

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

**Lumber**

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

**Handling & Installation**

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

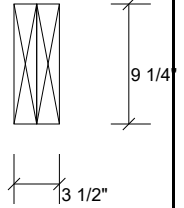
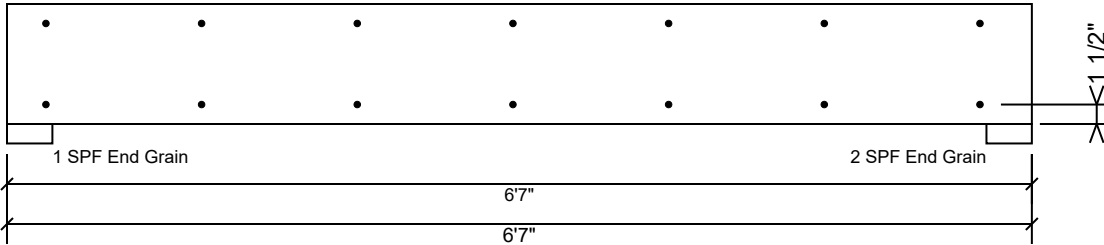
This design is valid until 11/3/2024

**Manufacturer Info**

Metsä Wood  
301 Merritt 7 Building, 2nd Floor  
Norwalk, CT 06851  
(800) 622-5850  
www.metsawood.com/us

**HDR1 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED**

Level: Level



**Multi-Ply Analysis**

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

**Notes**

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

**Lumber**

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

**Handling & Installation**

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

**Manufacturer Info**

Metsä Wood  
301 Merritt 7 Building, 2nd Floor  
Norwalk, CT 06851  
(800) 622-5850  
[www.metsawood.com/us](http://www.metsawood.com/us)