Mark Morris, P.E.

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The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 41565 JOB: 23-6775-R01 JOB NAME: LOT 24 PROVIDENCE CREEK Wind Code: 37 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. 20 Truss Design(s)

Trusses:

M01, M02, M03, P01, R01, R02, R03, R04, R05, R06, R07, R08, R09, SP01, SP02, V01, V02, V03, V04, V05



Warning !--- Verify design parameters and read notes before use.

This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to



REACTIONS. (lb/size) 4=263/Mechanical, 2=332/0-3-0 (min. 0-1-8) Max Horz 2=82(LC 10) Max Uplift4=-93(LC 10), 2=-106(LC 10) Max Grav 4=354(LC 21), 2=412(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 3-4=-287/160

NOTES- (11)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry



10/5/2023



Max Horz 2=82(LC 10) Max Uplift4=-93(LC 10), 2=-106(LC 10) Max Grav 4=354(LC 21), 2=412(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (9)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left exposed ; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.

3-4=-287/160

- 7) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint 4 and 106 lb uplift at joint 2.

LOAD CASE(S) Standard







10/5/2023



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (12)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4 except (jt=lb) 1=113, 5=113.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard





REACTIONS. All bearings 23-0-0.

(lb) - Max Horz 28=-141(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 28, 16, 23, 24, 25, 26, 27, 21, 20, 19, 18, 17

Max Grav All reactions 250 lb or less at joint(s) 28, 16, 25, 26, 27, 19, 18, 17 except 22=277(LC 27), 23=295(LC

5), 24=279(LC 5), 21=295(LC 6), 20=279(LC 6)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(14)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 6-8-6, Corner(3R) 6-8-6 to 16-3-10, Exterior(2N) 16-3-10 to 19-0-14, Corner(3E) 19-0-14 to 23-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

- non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 10) Gable studs spaced at 2-0-0 co.
 11) This truss has been designed for a 10.0 psf bottom chord inverticed from the bottom chord in all areas where a recommendation of the bottom chord and any other members, with BCDL = 10.0psf.
 12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a recommendation of the bottom chord and any other members, with BCDL = 10.0psf.
 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 28, 16, 23, 24, 25, 26, 27, 21, 20, 19, 18, 17.





of individual web meeters only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 24 PROVIDENCE CREEK 238 WINDSWEPT V	VAY FUQUAY-VARINA, NO
23-6775-R01	R03	COMMON GIRDER	1	3	Job Reference (optional)	# 41565
		Run: 8. ID:s	430 s Feb co?uVwdz	12 2021 Prin N_6Z0rHb	nt: 8.430 s Feb 12 2021 MiTek Industries, Inc. Sat Oct bcoMyuyZQFk-GJEoHljOJYI7DrZJ2UFE7II57T2	7 17:19:01 2023 Page 2 Pdo5ZNShUhWyVIWu

NOTES-(14)

11) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 1-5-4 from the left end to 7-5-4 to connect truss(es) R08 (1 ply 2x6 SP) to front face of bottom chord.

12) Use Simpson Strong-Tie HTU26 (20-16d Girder, 11-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 9-5-4 from the left end to 21-5-4 to connect truss(es) R07 (1 ply 2x6 SP) to front face of bottom chord. 13) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 1-7=-20

Concentrated Loads (lb)

Vert: 10=-2182(F) 8=-2258(F) 13=-2182(F) 14=-2182(F) 15=-2182(F) 16=-2258(F) 18=-2258(F) 20=-2258(F) 21=-2258(F) 22=-2258(F) 23=-2258(F) 2



10/5/2023



LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 20.0 20.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.14 BC 0.06 WB 0.24 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.01	(loc) 29 29 30	l/defl n/r n/r n/a	L/d 180 80 n/a	PLATES MT20 Weight: 377 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS OTHERS	2x4 SP No.2 2x6 SP No.2 2x6 SP No.2 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD WEBS	Structi end ve Rigid o 1 Row	ural wo erticals ceiling at mio	ood shea s. directly dpt	thing direct applied or 6 15-42, 19-39	ly applied or 6-0-0 oc -0-0 oc bracing. 14-43, 13-44, 11-45,	purlins, except 16-41, 17-40,

REACTIONS. All bearings 45-4-0.

(lb) - Max Horz 54=127(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 54, 30, 42, 43, 45, 47, 48, 49, 50, 51,

- 52, 41, 39, 37, 36, 35, 34, 33, 32, 31 except 53=-111(LC 14)
- Max Grav All reactions 250 lb or less at joint(s) 54, 30, 51, 52, 53, 33, 32, 31 except 42=291(LC 44), 43=299(LC 44), 44=274(LC 52), 45=297(LC 47), 47=292(LC 45),
 - 48=292(LC 45), 49=290(LC 45), 50=299(LC 45), 41=299(LC 44), 40=265(LC 50),
 - 39=296(LC 49), 37=292(LC 45), 36=292(LC 45), 35=290(LC 45), 34=299(LC 45)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 11-12=-123/258, 18-19=-123/258

NOTES-(15)

Job

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) 41-4-14 to 46-2-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 13) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Continuing by performance provide and notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Installation guide.

Job	Truss	Truss Type	Qty	Ply	LOT 24 PROVIDENCE CREEK 2	238 WINDSWEPT WAY FUQUAY-VARINA, NC
23-6775-R01	R04	PIGGYBACK BASE SUPPO	1	1	Job Reference (optional)	# 41565
			Run: 8.430 s Feb 1 ID:sco?uVw	2 2021 Prii dzN_6Z0r	nt: 8.430 s Feb 12 2021 MiTek Indu HbcoMyuyZQFk-guwwvmIHcT	ustries, Inc. Sat Oct 7 17:19:04 2023 Page 2 78h4JIujdoxIwNk7gEBqI3?3Qv9HryVIWr

NOTES- (15)

14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 54, 30, 42, 43, 45, 47, 48, 49, 50, 51, 52, 41, 39, 37, 36, 35, 34, 33, 32, 31 except (jt=lb) 53=111.

LOAD CASE(S) Standard





- 10-35=-3764/310, 11-35=-3839/290, 11-12=-1392/47 2-19=-253/3303, 19-36=-149/3053, 36-37=-149/3053, 18-37=-149/3053, 17-18=-149/3053, BOT CHORD 17-38=-72/2560, 38-39=-72/2560, 16-39=-72/2560, 15-16=-145/3053, 15-40=-145/3053, 40-41=-145/3053, 14-41=-145/3053, 12-14=-196/3303
- WEBS 5-19=-10/353, 5-17=-861/186, 6-17=-42/963, 7-17=-308/159, 7-16=-308/159, 8-16=-42/963, 9-16=-861/186, 9-14=-11/353

NOTES-(11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-8-4 to 4-1-5, Interior(1) 4-1-5 to 11-7-1, Exterior(2R) 11-7-1 to 33-8-15, Interior(1) 33-8-15 to 41-2-11, Exterior(2E) 41-2-11 to 46-0-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60
 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 4) Unbalanced snow loads have been considered for this design.
 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 6) Provide adequate drainage to prevent water ponding.
 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with fit between the bottom chord and any other members, with BCDL = 10.0psf.
 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=130, 12=130.
 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum (envelope) gable end zone and C-C Exterior(2E) -0-8-4 to 4-1-5, Interior(1) 4-1-5 to 11-7-1, Exterior(2R) 11-7-1 to 33-8-15, Interior(1)

- THIN BA
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE Standard parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

10/5/2023



D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 24 PROVIDENCE CREEK 238 WINDSWEPT WAY FUQUAY-VARINA, NO
23-6775-R01	R06	PIGGYBACK BASE	1	1	Job Reference (optional) # 41565
		Run: 8.4	430 s Feb 1 D:sco?uV	2 2021 Pri wdzN_6Z	nt: 8.430 s Feb 12 2021 MiTek Industries, Inc. Sat Oct 7 17:19:12 2023 Page 2)rHbcoMyuyZQFk-RROybVrljw8Z1YvQBIyp4ciw3ujSireAugraZNyVIWj

LOAD CASE(S) Standard



10/5/2023



of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designet. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 24 PROVIDENCE CREEK 238 WINDSWEPT WAY FUQUAY-VARINA, NO
23-6775-R01	R07	PIGGYBACK BASE	7	1	Job Reference (optional) # 41565
		Rur ID	: 8.430 s Feb :sco?uVwdzl	12 2021 Pri N_6Z0rHb	nt: 8.430 s Feb 12 2021 MiTek Industries, Inc. Sat Oct 7 17:19:15 2023 Page 2 coMyuyZQFk-r044DXuA0rW7u?e?tRVWiFKRQ6IGvCTdbd4EAiyVIWg

NOTES- (12)

11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard





responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive. Madison. WI 53719.

1	Job	Truss	Truss Type	Qty	Ply	LOT 24 PROVIDENCE CREEK 238 WINDSWE	PT WAY FUQUAY-VARINA, NC
	23-6775-R01	R08	PIGGYBACK BASE	4	1	Job Reference (optional)	# 41565
			Pu	8430 c Eob	12 2021 Dri	nt: 8 430 c Eob 12 2021 MiTok Industrios, Inc., So	t Oct. 7 17:10:10 2023, Page 2

LOAD CASE(S) Standard

000	11055	Trass Type	Guy		LOT 24 FROVIDENCE CILER 230 WINDSW	LET WAT TOQUAT-VAILINA, IN
23-6775-R01	R08	PIGGYBACK BASE	4	1	Job Reference (optional)	# 41565
		Run: 8. ID:sci	130 s Feb 1 o?uVwdzN	2 2021 Prin 6Z0rHbo	nt: 8.430 s Feb 12 2021 MiTek Industries, Inc. S coMyuyZQFk-knKb3vxh441ZNcxm6GaSs	at Oct 7 17:19:19 2023 Page 2 5U50j68r1RCVF2RJTyVIWc

responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/1PI 1 National Design Standard for Me Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive. Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 24 PROVIDENCE CREEK 238 WINDSWEPT WAY FUQUAY-VARINA, NO
23-6775-R01	R09	GABLE	1	1	Job Reference (optional) # 41565
		Run: 8 ID:sc	.430 s Feb o?uVwdzN	12 2021 Pri I_6Z0rHbo	nt: 8.430 s Feb 12 2021 MiTek Industries, Inc. Sat Oct 7 17:19:26 2023 Page 2 coMyuyZQFk-17FFXI04QEvajh_60FC5eZHUaYkN_FVE6rEJ3ZyVIWV

NOTES- (15)

14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 53, 41, 42, 44, 46, 47, 48, 49, 50, 51, 40, 38, 36, 35, 34, 33, 32, 31, 30 except (jt=lb) 52=112.

LOAD CASE(S) Standard

vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

(envelope) gable and c-C Exterior(2E) 0-6-8 to 5-4-1, Exterior(2R) 5-4-1 to 14-6-3, Exterior(2E) 14-6-3 to 19-3-13 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=125, 6=125.

LOAD CASE(S) Standard

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-6-8 to 5-4-1, Exterior(2R) 5-4-1 to 11-1-1, Exterior(2E) 11-1-1 to 15-10-11 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 6.

LOAD CASE(S) Standard

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

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NOTES- (9)

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph, TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

¹⁾ Unbalanced roof live loads have been considered for this design.