

Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483

843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 41613

JOB: 23-6774-R01

JOB NAME: LOT 12 PROVIDENCE CREEK

Wind Code: 37

Wind Speed: Vult= 120mph

Exposure Category: B

Mean Roof Height (feet): 35

These truss designs comply with IRC 2015 as well as IRC 2018.

20 Truss Design(s)

Trusses:

R01, R01, R02, R03, R04, R05, R05A, R06, R07, R08, R09, R10, V01, V02, V03, V04, V05,
V06 V07 V08



10/10/2023

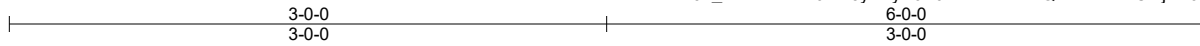
Mark Morris

Warning !—Verify design parameters and read notes before use.

This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to

Job 23-6774-R01	Truss P01	Truss Type Piggyback	Qty 21	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	-------------------------	-----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:07 2023 Page 1
ID:av29u_vm2cwLXF0Wc5ybwyV6X0-dh2kt1VbZQBWWDE2Cmj4rtiJl1_Z194QnN6daNyUjJY



Scale = 1:11.6

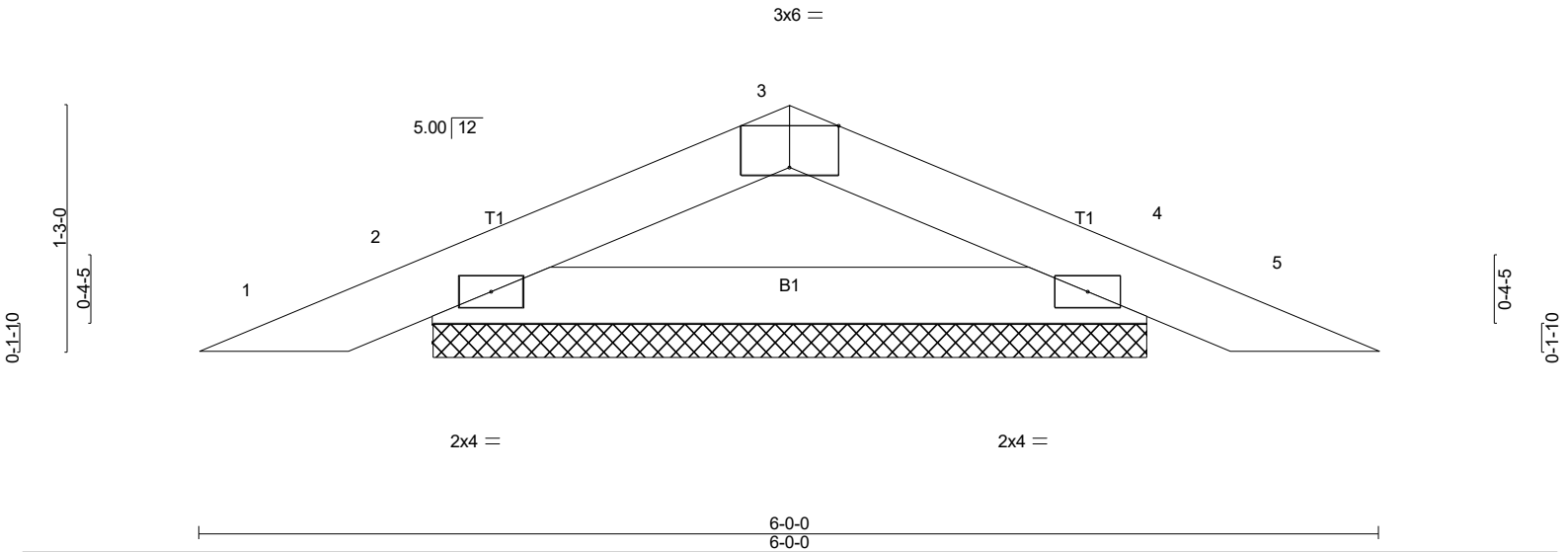


Plate Offsets (X,Y)-- [3-0-3-0_Edge]		SPACING-		CSI.		DEFL.				PLATES		GRIP	
LOADING (psf)		2-0-0		TC	0.07	in	(loc)	l/defl	L/d	MT20	244/190		
TCLL (roof)	20.0	Plate Grip DOL	1.15	BC	0.28	Vert(LL)	0.00	4	n/r	180			
Snow (Pf)	20.0	Lumber DOL	1.15	WB	0.00	Vert(CT)	0.00	4	n/r	80			
TCDL	10.0	Rep Stress Incr	YES	Matrix-P		Horz(CT)	0.00	4	n/a	n/a			
BCLL	0.0 *	Code IRC2021/TPI2014									Weight: 15 lb		FT = 20%
BCDL	10.0												

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

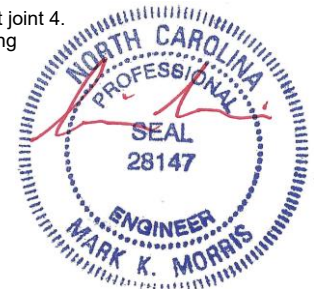
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=194/3-7-10 (min. 0-1-8), 4=194/3-7-10 (min. 0-1-8)
Max Horz 2=16(LC 14)
Max Uplift 2=-35(LC 14), 4=-35(LC 15)
Max Grav 2=233(LC 21), 4=233(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 2 and 35 lb uplift at joint 4.
 - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard



10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss R01	Truss Type Piggyback Base Supported Gable	Qty 1	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	--	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:10 2023 Page 1
ID:av29u_vm2cwLxTF0Wc5ybwV6X0-2GjtV3ZTsLa5NhzzduuHnTWJqnE36ETZITLLHBIyUjV

0-10-8 26-0-0 32-0-0 58-0-0 58-10-8
0-10-8 26-0-0 6-0-0 26-0-0 0-10-8

Scale = 1:99.6

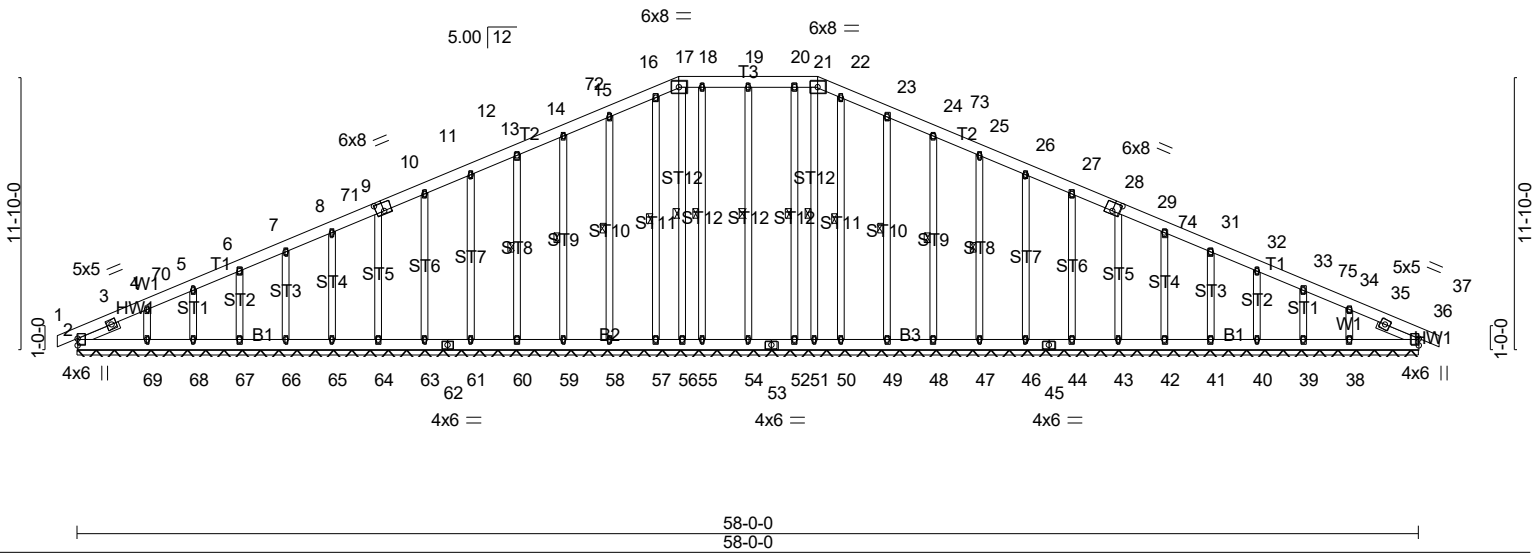


Plate Offsets (X,Y)-- [10:0-4-0,0-4-4], [28:0-4-0,0-4-4]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.06	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.03	Vert(LL) 0.00 36 n/r 180		
TCDL 10.0	Lumber DOL 1.15	WB 0.21	Vert(CT) 0.00 36 n/r 80		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.01 36 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014			Weight: 585 lb	FT = 20%

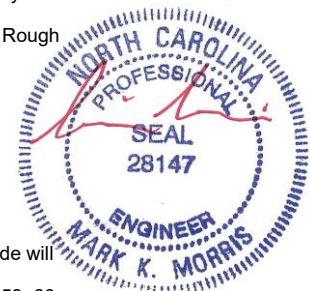
LUMBER-
TOP CHORD 2x6 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 - 1-11-0, Right 2x4 SP No.3 - 1-11-0

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 19-54, 18-55, 16-57, 15-58, 14-59, 13-60, 17-56, 20-52, 22-50, 23-49, 24-48, 25-47, 21-51

REACTIONS. All bearings 58-0-0.
(lb) - Max Horz 2=-167(LC 19)
Max Uplift All uplift 100 lb or less at joint(s) 2, 54, 57, 58, 59, 60, 61, 63, 64, 65, 66, 67, 68, 56, 49, 48, 47, 46, 44, 43, 42, 41, 40, 39, 51, 38 except 69=-108(LC 14)
Max Grav All reactions 250 lb or less at joint(s) 2, 55, 57, 65, 66, 67, 68, 56, 52, 50, 42, 41, 40, 39, 51, 36, 69, 38 except 54=305(LC 44), 58=297(LC 45), 59=287(LC 45), 60=287(LC 45), 61=286(LC 45), 63=289(LC 45), 64=276(LC 45), 49=297(LC 45), 48=287(LC 45), 47=287(LC 45), 46=286(LC 45), 44=289(LC 45), 43=276(LC 45)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 14-72=-120/272, 15-72=-115/276, 15-16=-135/311, 16-17=-138/322, 17-18=-134/315, 18-19=-134/315, 19-20=-134/315, 20-21=-134/315, 21-22=-138/322, 22-23=-135/311, 23-73=-115/276, 24-73=-120/272

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 21-0-0, Corner(3R) 21-0-0 to 37-0-0, Exterior(2N) 37-0-0 to 54-0-14, Corner(3E) 54-0-14 to 58-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 54, 57, 58, 59, 60, 61, 63, 64, 65, 66, 67, 68, 56, 49, 48, 47, 46, 44, 43, 42, 41, 40, 39, 51, 38 except (jt=lb) 69=108.



10/10/2023

LOAD CASE(S) & BRACING parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss R02	Truss Type Piggyback Base	Qty 4	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	------------------------------	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:12 2023 Page 1
ID:av29u_vm2cwLXF0Wc5ybywV6X0-_frdlwakOyqpd_70?JFYxPOZ2ZkiBCAwfqOGbyUjT

-0-10-8 0-10-8	8-10-10 8-10-10	17-5-3 8-6-8	26-0-0 8-6-13	32-0-0 6-0-0	40-6-13 8-6-13	49-1-6 8-6-8	58-0-0 8-10-10	58-10-8 0-10-8
-------------------	--------------------	-----------------	------------------	-----------------	-------------------	-----------------	-------------------	-------------------

Scale = 1:99.0

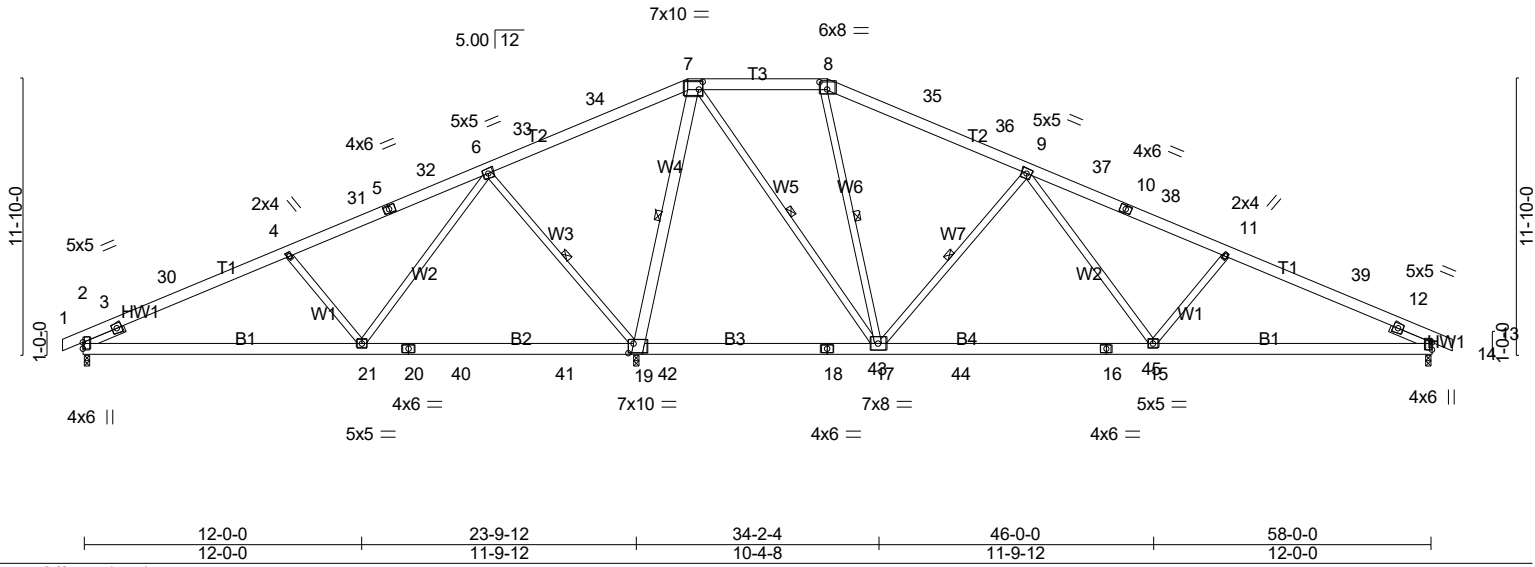


Plate Offsets (X,Y)-- [7:0-2-0,0-4-0], [8:0-4-0,0-3-13], [19:0-2-12,0-5-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.68	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.79	Vert(LL) -0.29 15-17 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.97	Vert(CT) -0.42 15-17 >965 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.03 13 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014			Weight: 425 lb	FT = 20%

LUMBER-
 TOP CHORD 2x6 SP No.2
 BOT CHORD 2x6 SP No.2 *Except*
 B2: 2x6 SP DSS
 WEBS 2x4 SP No.3 *Except*
 W4: 2x6 SP No.2
 SLIDER Left 2x4 SP No.3 -° 1-11-0, Right 2x4 SP No.3 -° 1-11-0

BRACING-
 TOP CHORD Structural wood sheathing directly applied.
 BOT CHORD Rigid ceiling directly applied.
 WEBS 1 Row at midpt 6-19, 7-19, 7-17, 8-17, 9-17
 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=647/0-3-8 (min. 0-1-8), 19=2932/0-3-8 (min. 0-1-12), 13=1166/0-3-8 (min. 0-1-9)
 Max Horz 2=-167(LC 15)
 Max Uplift 2=-128(LC 14), 19=-271(LC 14), 13=-216(LC 15)
 Max Grav 2=753(LC 41), 19=3877(LC 45), 13=1299(LC 39)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-606/0, 3-30=-933/166, 4-30=-869/184, 4-31=-634/131, 5-31=-539/135, 5-32=-532/136,
 6-32=-445/152, 6-33=-18/1088, 33-34=0/1206, 7-34=0/1321, 7-8=-624/262, 8-35=-608/231,
 35-36=-611/216, 9-36=-739/201, 9-37=-1672/354, 10-37=-1763/338, 10-38=-1777/337,
 11-38=-1881/334, 11-39=-2049/383, 12-39=-2173/355, 12-13=-851/0
 BOT CHORD 2-21=-247/802, 20-21=-437/235, 20-40=-437/235, 40-41=-437/235, 19-41=-437/235,
 19-42=-655/252, 42-43=-655/252, 18-43=-655/252, 17-18=-655/252, 17-44=-71/1250,
 44-45=-71/1250, 16-45=-71/1250, 15-16=-71/1250, 13-15=-260/1927
 WEBS 4-21=-558/246, 6-21=-90/930, 6-19=-1339/323, 7-19=-2480/284, 7-17=-220/1942,
 8-17=-347/80, 9-17=-1300/321, 9-15=-78/780, 11-15=-464/233

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-10-8 to 3-11-2, Interior(1) 3-11-2 to 19-2-9, Exterior(2R) 19-2-9 to 38-9-7, Interior(1) 38-9-7 to 54-0-14, Exterior(2E) 54-0-14 to 58-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Bearing at joint(s) 19 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=128, 19=271, 13=216.



10/10/2023

Continued on Page 2 Design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE
23-6774-R01	R02	Piggyback Base	4	1	Job Reference (optional) # 41613

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:12 2023 Page 2
ID:av29u_vm2cwLTXF0Wc5ybyV6X0-_frdlakOyqpd_70?JFYxP0Z2ZkiBCAwfqOGbyUjjT

NOTES-

11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

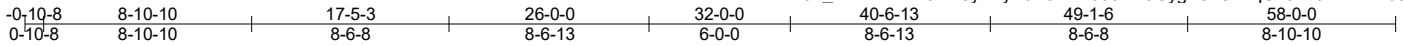


10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss R03	Truss Type PIGGYBACK BASE	Qty 2	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	------------------------------	----------	----------	---

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:13 2023 Page 1
ID: av29u_vm2cwLxXF0Wc5ybwyV6X0-SrP085bM9GygE8hCZ1qU48xBJSvzReTJ9JZxo1yUjJS



Scale = 1:98.9

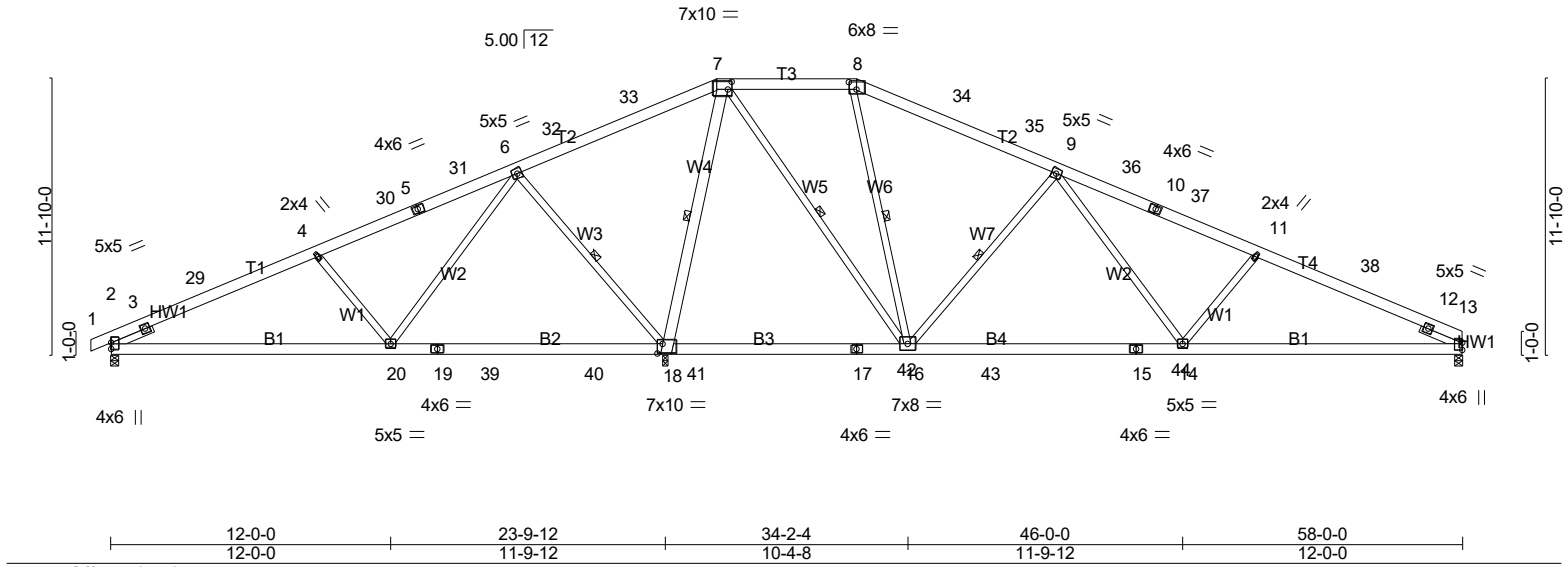


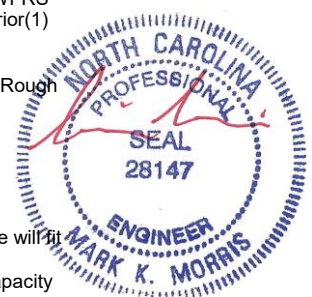
Plate Offsets (X,Y)-- [7:0-2-0,0-4-0], [8:0-4-0,0-3-13], [18:0-2-12,0-5-0]					
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.68	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.79	Vert(LL) -0.29 14-16 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.97	Vert(CT) -0.42 14-16 >967 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.03 13 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014			Weight: 423 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied.
BOT CHORD 2x6 SP No.2 *Except* B2: 2x6 SP DSS	BOT CHORD Rigid ceiling directly applied.
WEBS 2x4 SP No.3 *Except* W4: 2x6 SP No.2	WEBS 1 Row at midpt 6-18, 7-18, 7-16, 8-16, 9-16
SLIDER Left 2x4 SP No.3 -° 1-11-0, Right 2x4 SP No.3 -° 1-11-0	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=648/0-3-8 (min. 0-1-8), 18=2931/0-3-8 (min. 0-1-12), 13=1114/0-3-8 (min. 0-1-8)
Max Horz 2=172(LC 18)
Max Uplift 2=-127(LC 14), 18=-273(LC 14), 13=-197(LC 15)
Max Grav 2=752(LC 41), 18=3876(LC 45), 13=1247(LC 39)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-606/0, 3-29=-931/164, 4-29=-867/182, 4-30=-632/129, 5-30=-538/133, 5-31=-530/133,
6-31=-446/150, 6-32=-20/1086, 32-33=-2/1204, 7-33=-0/1319, 7-8=-626/259,
8-34=-610/228, 34-35=-613/214, 9-35=-742/198, 9-36=-1676/352, 10-36=-1769/336,
10-37=-1783/335, 11-37=-1887/332, 11-38=-2056/381, 12-38=-2179/353, 12-13=-862/0
BOT CHORD 2-20=-250/800, 19-20=-435/228, 19-39=-435/228, 39-40=-435/228, 18-40=-435/228,
18-41=-653/244, 41-42=-653/244, 17-42=-653/244, 16-17=-653/244, 16-43=-80/1254,
43-44=-80/1254, 15-44=-80/1254, 14-15=-80/1254, 13-14=-269/1933
WEBS 4-20=-558/246, 6-20=-90/929, 6-18=-1339/323, 7-18=-2478/289, 7-16=-222/1942,
8-16=-347/81, 9-16=-1301/321, 9-14=-79/782, 11-14=-466/234

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-10-8 to 3-11-2, Interior(1) 3-11-2 to 19-2-9, Exterior(2R) 19-2-9 to 38-9-7, Interior(1) 38-9-7 to 53-2-6, Exterior(2E) 53-2-6 to 58-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=127, 18=273, 13=197.



10/10/2023

Continued on Page 2 Design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE
23-6774-R01	R03	PIGGYBACK BASE	2	1	Job Reference (optional) # 41613

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:13 2023 Page 2
ID:av29u_vm2cwLxXF0Wc5ybwyV6X0-SrP085bM9GygE8hCZ1qU48xBJSvzReTJ9JZxo1yUjjs

NOTES-

11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

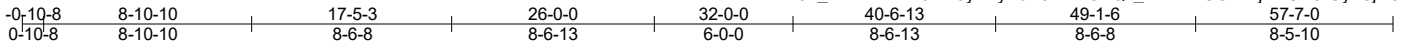


10/10/2023

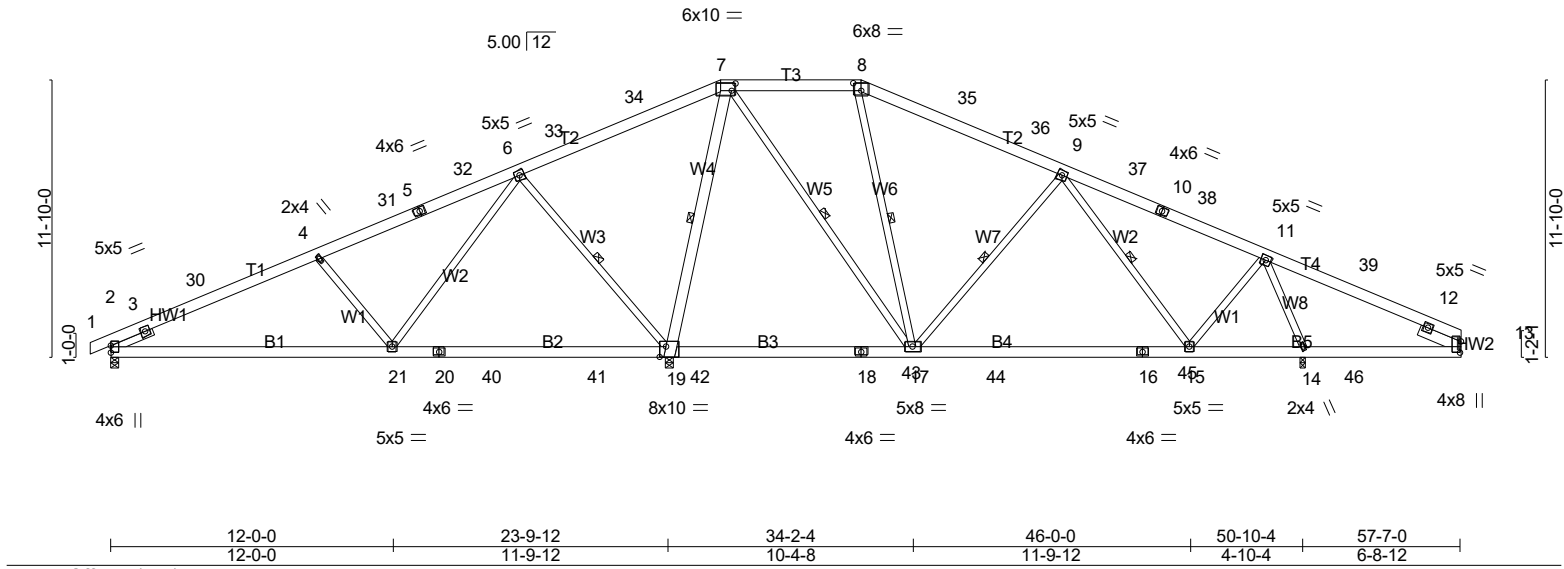
Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss R04	Truss Type PIGGYBACK BASE	Qty 3	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	------------------------------	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:14 2023 Page 1
ID:av29u_vm2cwLxXFOWc5ybwyV6X0-w2zOLQc_wa4XsIGO7kLjdMUN8rGyA9jSOzJUKTyUjJR



Scale = 1:98.3



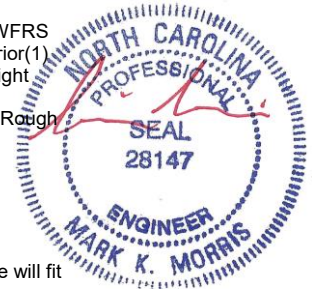
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.61	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.74	Vert(LL) -0.24 19-21 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.71	Vert(CT) -0.33 15-17 >975 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.03 14 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014			Weight: 429 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied.
BOT CHORD 2x6 SP No.2 *Except* B2: 2x6 SP DSS	BOT CHORD Rigid ceiling directly applied.
WEBS 2x4 SP No.3 *Except* W4: 2x6 SP No.2	WEBS 1 Row at midpt 6-19, 7-19, 7-17, 8-17, 9-17, 9-15
SLIDER Left 2x4 SP No.3 -° 1-11-0, Right 2x6 SP No.2 -° 1-11-0	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 0-3-8 except (jt=length) 13=Mechanical.
(lb) - Max Horz 2=175(LC 14)
Max Uplift All uplift 100 lb or less at joint(s) except 2=-125(LC 14), 19=-283(LC 14), 13=-107(LC 11), 14=-171(LC 15)
Max Grav All reactions 250 lb or less at joint(s) except 2=864(LC 39), 19=3218(LC 45), 13=330(LC 55), 14=1369(LC 39)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-678/0, 3-30=-1195/160, 4-30=-1129/179, 4-31=-886/124, 5-31=-776/137, 5-32=-769/138, 6-32=-662/154, 6-33=-24/570, 33-34=-5/690, 7-34=-3/804, 7-8=-676/273, 8-35=-688/244, 35-36=-689/229, 9-36=-800/214, 9-37=-907/280, 10-37=-986/263, 10-38=-990/263, 11-38=-1071/250, 11-39=-140/287, 12-39=-236/269
BOT CHORD 2-21=-251/1042, 20-21=-47/268, 20-40=-47/268, 40-41=-47/268, 19-41=-47/268, 19-42=-266/181, 42-43=-266/181, 18-43=-266/181, 17-18=-266/181, 17-44=-81/982, 44-45=-81/982, 16-45=-81/982, 15-16=-81/982, 14-15=-205/610
WEBS 4-21=-527/247, 6-21=-90/885, 6-19=-1326/323, 7-19=-1823/246, 7-17=-150/1411, 8-17=-335/70, 9-17=-756/255, 11-15=0/553, 11-14=-1352/195

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 19-2-9, Exterior(2R) 19-2-9 to 38-9-7, Interior(1) 38-9-7 to 52-9-6, Exterior(2E) 52-9-6 to 57-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.



10/10/2023

Continued on page 2 Design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE
23-6774-R01	R04	PIGGYBACK BASE	3	1	Job Reference (optional) # 41613

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:14 2023 Page 2
ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-w2zOLQc_wa4XsIGO7kLjdMUN8rGyA9jSOzJUKTyUjJR

NOTES-

- 10) Bearing at joint(s) 19 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 125 lb uplift at joint 2, 283 lb uplift at joint 19, 107 lb uplift at joint 13 and 171 lb uplift at joint 14.
- 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

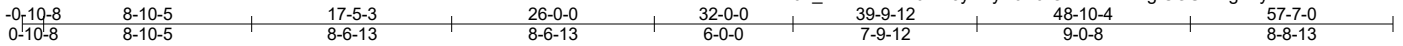


10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss R05	Truss Type Piggyback Base	Qty 3	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	------------------------------	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:15 2023 Page 1
ID:av29u_vm2cwLXF0Wc5ybwyV6X0-OEXmZmdcgtCOUSrbgSsyAZ1XdFY2vYxcdd22swyUjJQ



Scale = 1:98.3

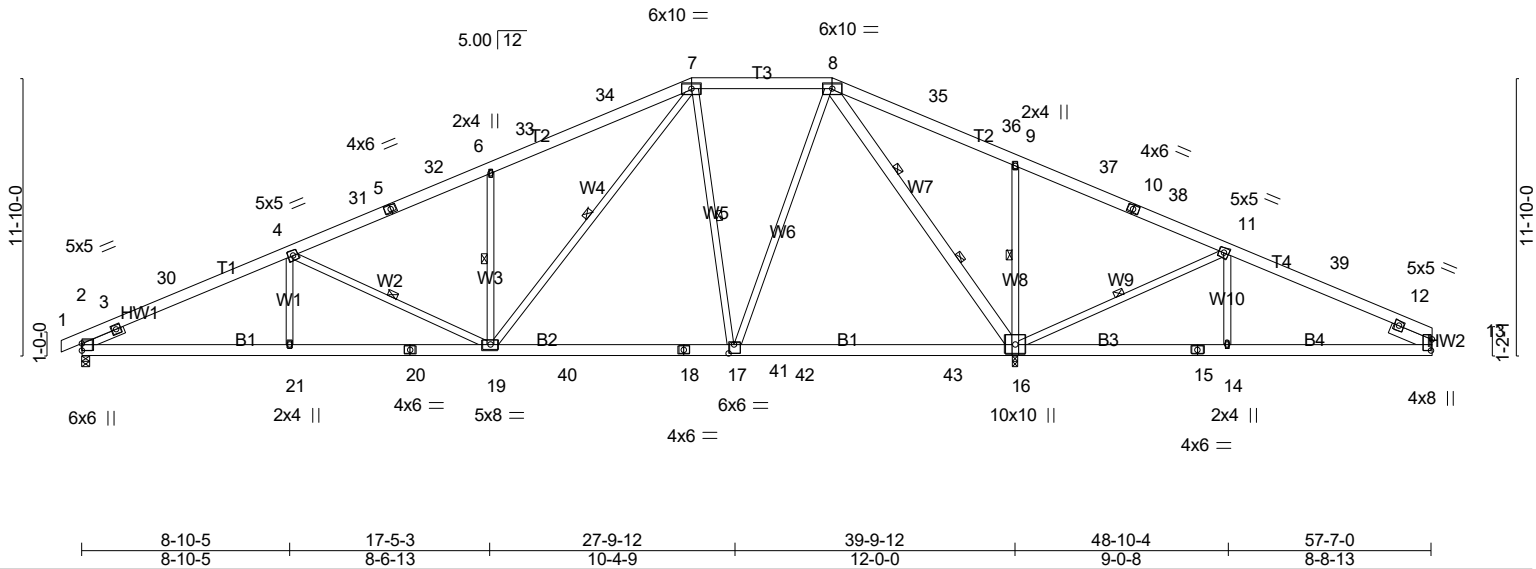


Plate Offsets (X,Y)-- [13:0-5-10,0-0-8], [17:0-2-12,0-4-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.69	Vert(LL) -0.34	16-17	>999	240	MT20	244/190
Snow (Pf) 20.0	Lumber DOL 1.15		BC 0.94	Vert(CT) -0.49	16-17	>984	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.97	Horz(CT) 0.07	16	n/a	n/a		
BCLL 0.0 *	Code IRC2021/TPI2014		Matrix-AS						
BCDL 10.0								Weight: 437 lb	FT = 20%

LUMBER-
TOP CHORD 2x6 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3 *Except*
W7: 2x6 SP No.2
SLIDER Left 2x4 SP No.3 -° 1-11-0, Right 2x6 SP No.2 -° 1-11-0

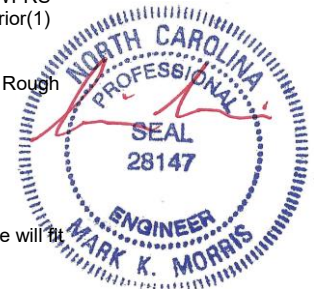
BRACING-
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 4-19, 6-19, 7-19, 7-17, 9-16, 11-16
2 Rows at 1/3 pts 8-16

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=1488/0-3-8 (min. 0-2-0), 13=358/Mechanical, 16=2813/0-3-8 (min. 0-2-7)
Max Horz2=175(LC 14)
Max Uplift2=-230(LC 14), 13=-110(LC 15), 16=-225(LC 15)
Max Grav2=1700(LC 39), 13=461(LC 55), 16=3544(LC 39)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1089/42, 3-30=-3079/375, 4-30=-2977/394, 4-31=-2466/302, 5-31=-2385/306,
5-32=-2371/307, 6-32=-2269/323, 6-33=-2532/433, 33-34=-2387/448, 7-34=-2375/463,
7-8=-1141/286, 8-35=0/1087, 35-36=0/948, 9-36=0/821, 9-37=-16/1133, 10-37=-29/963,
10-38=-31/944, 11-38=-47/857, 11-39=-370/253, 12-39=-447/199, 12-13=-263/0
BOT CHORD 2-21=-446/2748, 20-21=-446/2748, 19-20=-446/2748, 19-40=-49/1277, 18-40=-49/1277,
18-41=-49/1277, 17-41=-49/1277, 17-42=0/666, 42-43=0/666, 16-43=0/666, 15-16=-184/357,
14-15=-184/357, 13-14=-184/357
WEBS 4-21=0/295, 4-19=-662/208, 6-19=-928/280, 7-19=-351/1771, 7-17=-995/242,
8-17=-127/1505, 8-16=-2532/183, 9-16=-885/274, 11-16=-1049/243, 11-14=0/368

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 19-2-9, Exterior(2R) 19-2-9 to 38-9-7, Interior(1) 38-9-7 to 52-9-6, Exterior(2E) 52-9-6 to 57-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Bearing at joint(s) 16 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.



10/10/2023

Continued on Page 2 Design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE
23-6774-R01	R05	Piggyback Base	3	1	Job Reference (optional) # 41613

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:15 2023 Page 2
ID:av29u_vm2cwLtxF0Wc5ybwyV6X0-OEXmZmdcgtCOUSrbgSsyAZ1XdFY2vYxocd22swyUjJQ

NOTES-

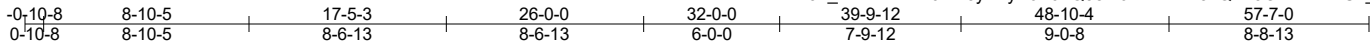
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 2, 110 lb uplift at joint 13 and 225 lb uplift at joint 16.
- 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Scale = 1:100.2

Plate Offsets (X,Y)--	[13:0-5-10,0-0-8], [16:0-5-0,0-5-0], [21:0-4-0,0-4-12]
-----------------------	--

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15		TC 0.68	Vert(LL) -0.50	19-20	>955	240	MT20	244/190
Snow (Pf) 20.0	Lumber DOL 1.15		BC 0.86	Vert(CT) -0.72	19-20	>665	180		
TCDL 10.0	Rep Stress Incr YES		WB 0.96	Horz(CT) 0.06	16	n/a	n/a		
BCLL 0.0 *	Code IRC2021/TPI2014		Matrix-AS						
BCDL 10.0								Weight: 453 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied.
BOT CHORD 2x6 SP No.2 *Except* B1: 2x6 SP DSS, B3: 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied. Except: 10-0-0 oc bracing: 18-20
WEBS 2x4 SP No.3 *Except* W7: 2x6 SP DSS	WEBS 1 Row at midpt 4-23, 6-23, 7-23, 7-21, 9-16, 11-16 2 Rows at 1/3 pts 8-18
SLIDER Left 2x4 SP No.3 -° 1-11-0, Right 2x6 SP No.2 -° 1-11-0	

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=1522/0-3-8 (min. 0-1-12), 13=358/Mechanical, 16=3003/0-3-8 (min. 0-2-11)
 Max Horz 2=175(LC 14)
 Max Uplift 2=-215(LC 14), 13=-111(LC 15), 16=-129(LC 15)
 Max Grav 2=1735(LC 39), 13=455(LC 55), 16=3963(LC 45)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1203/28, 3-34=-3156/342, 4-34=-3102/361, 4-35=-2645/269, 5-35=-2564/272,
 5-36=-2551/273, 6-36=-2448/289, 6-37=-2712/400, 37-38=-2566/415, 7-38=-2554/430,
 7-8=-1362/243, 8-39=0/1072, 39-40=0/933, 9-40=0/806, 9-41=-8/1116, 10-41=-21/947,
 10-42=-23/928, 11-42=-39/840, 11-43=-350/282, 12-43=-427/228, 12-13=-284/0
 BOT CHORD 2-25=-416/2823, 24-25=-416/2823, 23-24=-416/2823, 23-44=-12/1481, 22-44=-12/1481,
 22-45=-12/1481, 21-45=-12/1481, 21-46=0/608, 17-46=0/608, 17-47=0/608, 16-47=0/608,
 15-16=-210/340, 14-15=-210/340, 13-14=-210/340, 20-48=-32/271, 19-48=-32/271,
 19-49=-32/271, 18-49=-32/271
 WEBS 4-25=0/296, 4-23=-660/208, 6-23=-926/279, 7-23=-362/1746, 7-21=-941/263,
 20-21=-120/1709, 8-20=-81/1711, 8-18=-2791/128, 16-18=-2712/95, 9-16=-881/274,
 11-16=-1032/251, 11-14=0/337, 17-19=-384/0

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 19-2-9, Exterior(2R) 19-2-9 to 38-9-7, Interior(3) 38-9-7 to 52-9-6, Exterior(2E) 52-9-6 to 57-7-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



10/10/2023

9) Refer to order(s) for truss to truss connections.
 Continued on page 2 Design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE
23-6774-R01	R05A	Piggyback Base	7	1	Job Reference (optional) # 41613

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:16 2023 Page 2
ID:av29u_vm2cwLTXF0Wc5ybywV6X0-tQ58m6dERBKE5cQnE9OBinZiXfvUe_FirHobPMYUjJP

NOTES-

- 10) Bearing at joint(s) 16 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 11) Provide metal plate or equivalent at bearing(s) 16 to support reaction shown.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 215 lb uplift at joint 2, 111 lb uplift at joint 13 and 129 lb uplift at joint 16.
- 13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

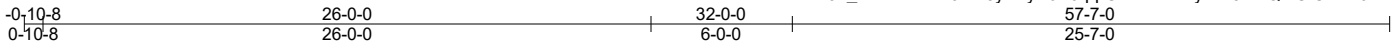


10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss R06	Truss Type Piggyback Base Supported Gable	Qty 1	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	--	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:18 2023 Page 1
ID:av29u_vm2cwLTXF0Wc5ybywV6X0-ppCvBofVzoayLva9MaQfnCfCIToz64X2JbHiTFyUjJN



Scale = 1:98.6

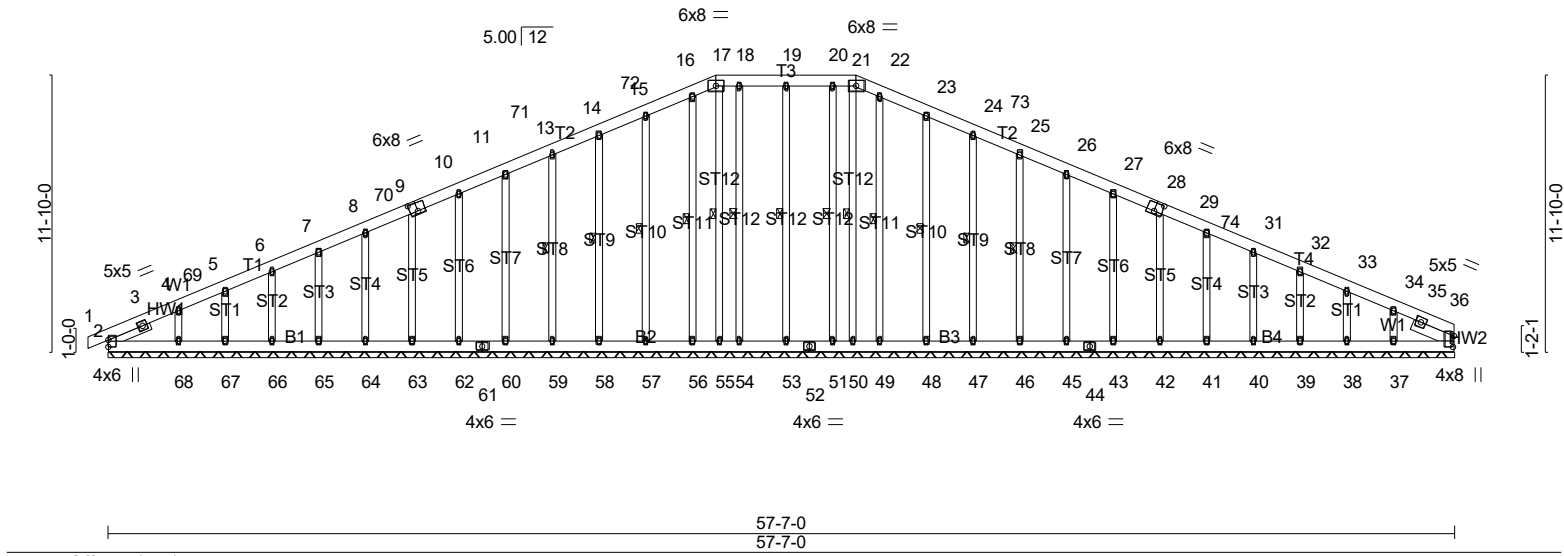


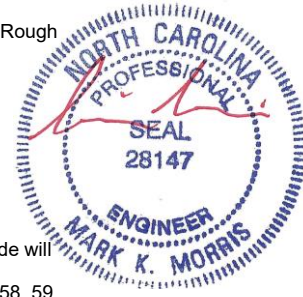
Plate Offsets (X,Y)-- [10:0-4-0,0-4-4], [28:0-4-0,0-4-4], [36:0-5-10,0-0-8]					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15	TC 0.06	Vert(LL) -0.00 1 n/r 180	MT20	244/190
Snow (Pf) 20.0	Lumber DOL 1.15	BC 0.03	Vert(CT) 0.00 1 n/r 80		
TCDL 10.0	Rep Stress Incr YES	WB 0.21	Horz(CT) 0.01 36 n/a n/a		
BCLL 0.0 *	Code IRC2021/TPI2014	Matrix-S			
BCDL 10.0					Weight: 583 lb FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 19-53, 18-54, 16-56, 15-57, 14-58, 13-59, 17-55, 20-51, 22-49, 23-48, 24-47, 25-46, 21-50
OTHERS 2x4 SP No.3	
SLIDER Left 2x4 SP No.3 -° 1-11-0, Right 2x6 SP No.2 -° 1-11-0	

REACTIONS. All bearings 57-7-0.
(lb) - Max Horz 2=-171(LC 19)
Max Uplift All uplift 100 lb or less at joint(s) 2, 53, 56, 57, 58, 59, 60, 62, 63, 64, 65, 66, 67, 55, 48, 47, 46, 45, 43, 42, 41, 40, 39, 38, 50, 37 except 68=-108(LC 14)
Max Grav All reactions 250 lb or less at joint(s) 2, 54, 56, 64, 65, 66, 67, 55, 51, 49, 41, 40, 39, 38, 50, 36, 68, 37 except 53=305(LC 44), 57=297(LC 45), 58=287(LC 45), 59=287(LC 45), 60=286(LC 45), 62=289(LC 45), 63=276(LC 45), 48=297(LC 45), 47=287(LC 45), 46=287(LC 45), 45=286(LC 45), 43=289(LC 45), 42=276(LC 45)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 14-72=-118/279, 15-72=-111/283, 15-16=-133/318, 16-17=-136/328, 17-18=-132/322, 18-19=-132/321, 19-20=-132/321, 20-21=-132/322, 21-22=-136/328, 22-23=-133/318, 23-73=-112/283, 24-73=-118/279

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 21-0-0, Corner(3R) 21-0-0 to 37-0-0, Exterior(2N) 37-0-0 to 52-9-6, Corner(3E) 52-9-6 to 57-7-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 53, 56, 57, 58, 59, 60, 62, 63, 64, 65, 66, 67, 55, 48, 47, 46, 45, 43, 42, 41, 40, 39, 38, 50, 37 except (jt=lb) 68=108.



LOAD CASE(S) & BRACING parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss R07	Truss Type Common	Qty 1	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	----------------------	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:20 2023 Page 1
ID:av29u_vm2cwLTXF0Wc5ybywV6X0-IBKfcUgIVQqgaDkYT?S7tdkTiGMya?kLmvpY7yUjjL



Scale = 1:27.4

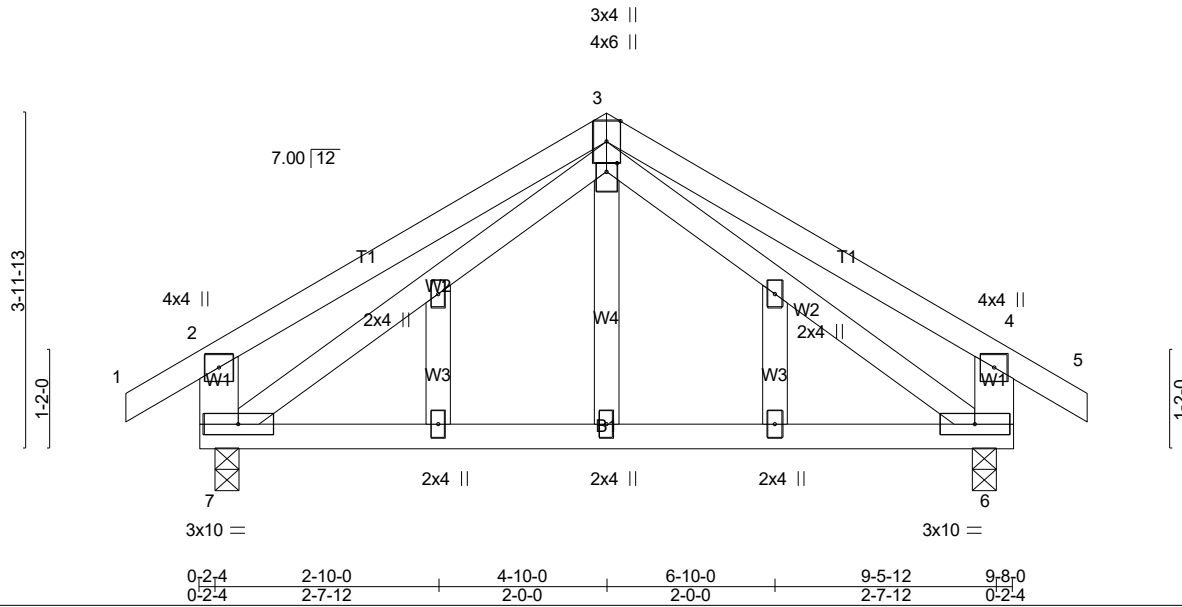


Plate Offsets (X,Y)-- [3:0-1-4,0-1-8]

LOADING (psf)	SPACING-	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.35	Vert(LL) 0.32	6-7	>344	240		MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.57	Vert(CT) -0.31	6-7	>352	180			
TCDL 10.0	Lumber DOL 1.15	WB 0.16	Horz(CT) 0.00	6	n/a	n/a			
BCLL 0.0 *	Rep Stress Incr YES	Matrix-AS							
BCDL 10.0	Code IRC2021/TPI2014							Weight: 64 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except*
W1: 2x6 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 6=435/0-3-8 (min. 0-1-8), 7=435/0-3-8 (min. 0-1-8)
Max Horz 7=-105(LC 12)
Max Uplift 6=-62(LC 15), 7=-62(LC 14)
Max Grav 6=529(LC 22), 7=529(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-434/479, 3-4=-434/479, 2-7=-467/359, 4-6=-467/359
WEBS 3-6=-263/152, 3-7=-263/152

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCCL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Exterior(2R) 3-11-2 to 5-8-14, Exterior(2E) 5-8-14 to 10-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 7.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss R08	Truss Type DUAL RIDGE GABLE	Qty 1	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	--------------------------------	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:21 2023 Page 1
ID:av29u_vm2cwLxF0Wc5ybwyV6X0-DOu1pqhNGjyXCNJk1izMPqHeVgnpJOOU?ZVM4ayUjJK

-0-10-8 0-10-8	4-10-0 4-10-0	14-3-0 9-5-0	28-6-0 14-3-0	29-4-8 0-10-8
-------------------	------------------	-----------------	------------------	------------------

Scale = 1:60.0

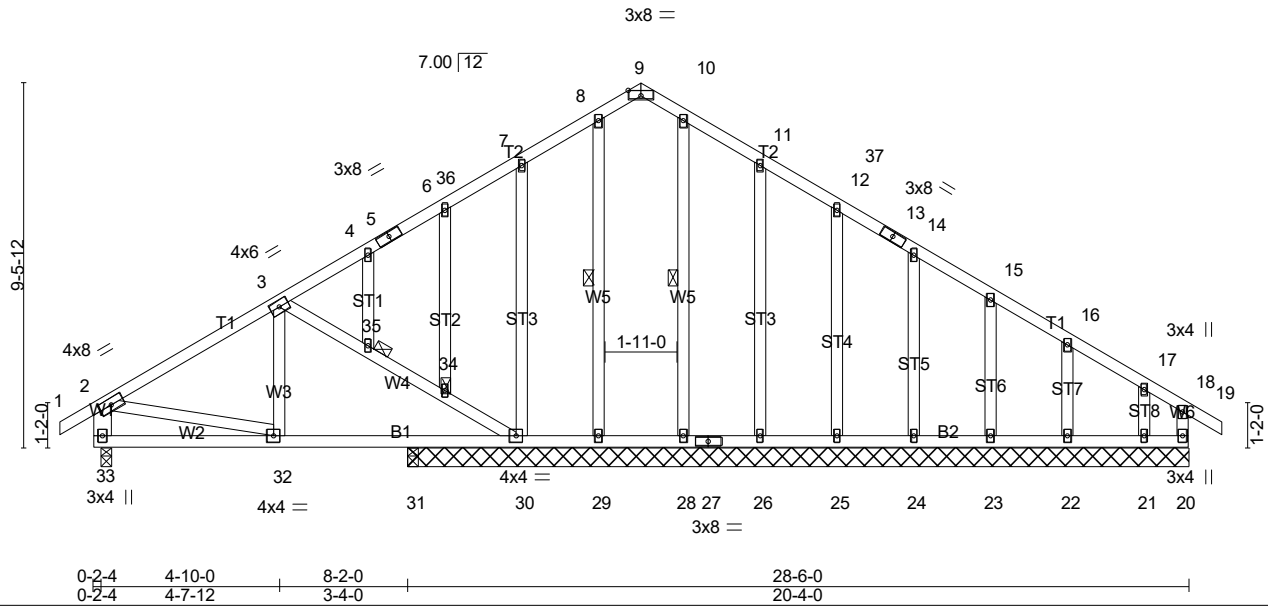


Plate Offsets (X,Y)-- [9:0-4-0,Edge]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.35	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.15	Vert(LL) 0.02 32-33 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.39	Vert(CT) -0.02 32-33 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.01 20 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014			Weight: 203 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except*
W1: 2x6 SP No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 10-28, 8-29
JOINTS 1 Brace at Jt(s): 34, 35

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 20-4-0 except (jt=length) 33=0-3-8, 31=0-3-8.
(lb) - Max Horz 33=-225(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 33, 26, 25, 24, 23, 22, 20 except 30=262(LC 14), 21=-181(LC 15)
Max Grav All reactions 250 lb or less at joint(s) 23, 22, 21, 20, 29, 31 except 33=471(LC 21), 30=639(LC 21), 26=282(LC 6), 25=253(LC 25), 24=261(LC 25), 28=299(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-426/233, 2-33=-422/196
BOT CHORD 31-32=-145/342, 30-31=-145/342
WEBS 3-35=-360/278, 34-35=-372/284, 30-34=-390/290, 7-30=-377/183

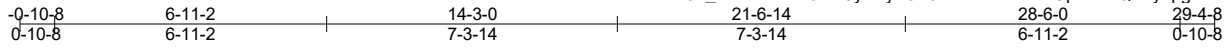
- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TC DL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 9-5-6, Exterior(2R) 9-5-6 to 19-0-10, Interior(1) 19-0-10 to 24-6-14, Exterior(2E) 24-6-14 to 29-4-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 33, 26, 25, 24, 23, 22, 20 except (jt=lb) 30=262, 21=181.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Scale = 1:58.3

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.91	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 1.00	Vert(LL) -0.38 11-13 >895 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.47	Vert(CT) -0.47 11-13 >711 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.05 10 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014				Weight: 167 lb FT = 20%

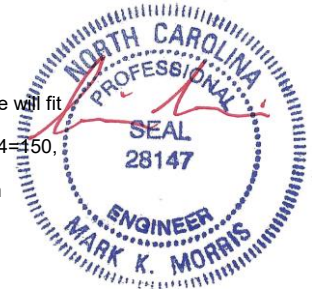
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 *Except* W1: 2x6 SP No.2	BRACING- TOP CHORD Structural wood sheathing directly applied, except end verticals. BOT CHORD Rigid ceiling directly applied. WEBS 1 Row at midpt 3-14, 7-10 <div style="border: 1px solid black; padding: 5px; margin-top: 5px;"> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>
--	--

REACTIONS. (lb/size) 14=1188/0-3-8 (min. 0-1-8), 10=1188/0-3-8 (min. 0-1-8)
 Max Horz 14=-226(LC 12)
 Max Uplift 14=-150(LC 14), 10=-150(LC 15)
 Max Grav 14=1276(LC 24), 10=1276(LC 25)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-503/133, 3-4=-1530/232, 4-15=-1461/242, 5-15=-1431/266, 5-16=-1431/266,
 6-16=-1462/242, 6-7=-1530/232, 7-8=-503/133, 2-14=-464/152, 8-10=-463/152
 BOT CHORD 14-17=-195/1468, 17-18=-195/1468, 13-18=-195/1468, 13-19=-24/1038, 12-19=-24/1038,
 12-20=-24/1038, 11-20=-24/1038, 11-21=-92/1315, 21-22=-92/1315, 10-22=-92/1315
 WEBS 5-11=-114/661, 7-11=-323/231, 5-13=-113/661, 3-13=-323/231, 3-14=-1274/126,
 7-10=-1273/126

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 9-5-6, Exterior(2R) 9-5-6 to 19-0-10, Interior(1) 19-0-10 to 24-6-14, Exterior(2E) 24-6-14 to 29-4-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=150, 10=150.
 - 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

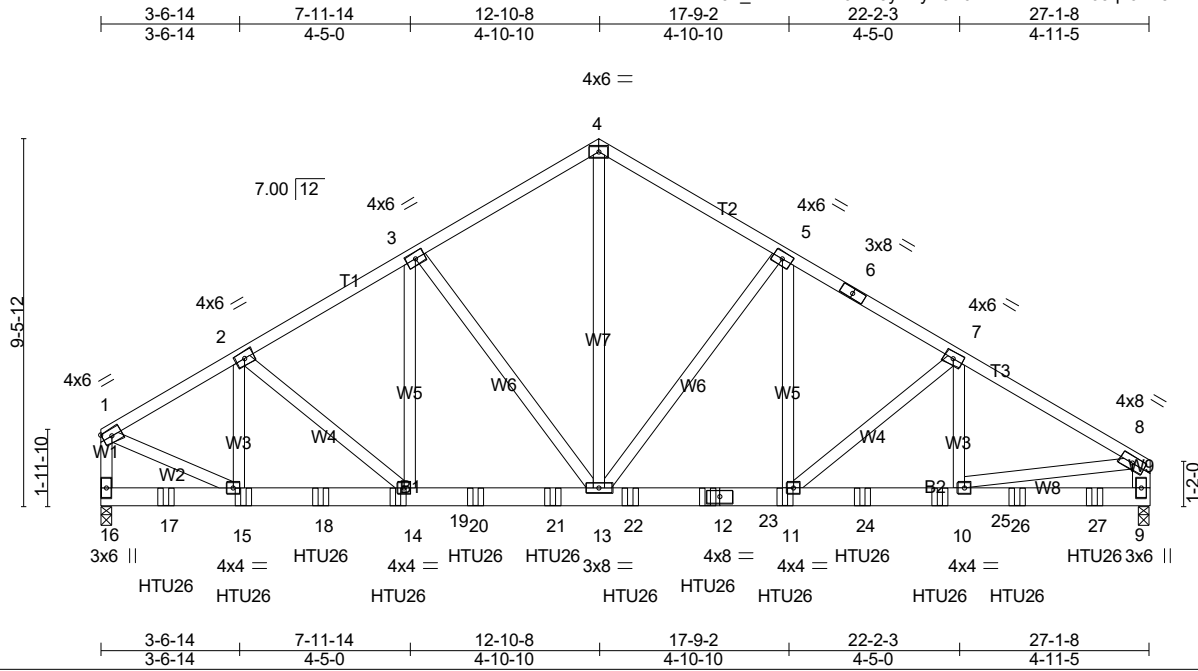


10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss R10	Truss Type Common Girder	Qty 1	Ply 3	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	-----------------------------	----------	----------	---

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:24 2023 Page 1
ID:av29u_vm2cwLxXF0Wc5ybwyV6X0-ezaARrkFZeK63q1JirX31TvBdtllWjGxhXk0huyUjjH



Scale = 1:59.5

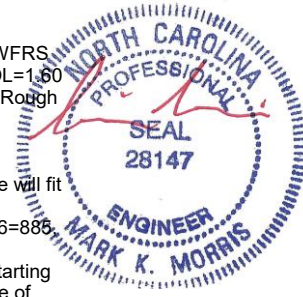
LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.23	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.42	Vert(LL) -0.06 11-13 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.51	Vert(CT) -0.10 11-13 >999 180		
BCLL 0.0 *	Rep Stress Incr NO	Matrix-MSH	Horz(CT) 0.02 9 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014				Weight: 612 lb FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3 *Except* W9: 2x6 SP No.2	

REACTIONS. (lb/size) 16=3426/0-3-8 (min. 0-1-8), 9=3743/0-3-8 (min. 0-1-9)
Max Horz 16=-218(LC 8)
Max Uplift 16=-885(LC 12), 9=-920(LC 13)
Max Grav 16=3537(LC 2), 9=3898(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-3602/902, 2-3=-4062/1012, 3-4=-3447/906, 4-5=-3447/898, 5-6=-4432/1109,
6-7=-4515/1098, 7-8=-5025/1190, 1-16=-3331/837, 8-9=-3345/806
BOT CHORD 15-18=-821/3067, 18-19=-821/3067, 14-19=-821/3067, 14-20=-835/3455, 20-21=-835/3455,
13-21=-835/3455, 13-22=-828/3846, 22-23=-828/3846, 12-23=-828/3846, 11-12=-828/3846,
11-24=-981/4274, 24-25=-981/4274, 10-25=-981/4274, 10-26=-167/619, 26-27=-167/619,
9-27=-167/619
WEBS 2-15=-774/162, 2-14=-103/511, 3-14=-232/754, 3-13=-946/351, 4-13=-811/3111,
5-13=-1538/474, 5-11=-365/1460, 7-11=-584/207, 7-10=-214/430, 1-15=-803/3311,
8-10=-829/3722

- NOTES-**
- 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCCL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCCL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 16=885, 9=920.
 - Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 1-8-4 from the left end to 25-8-4 to connect truss(es) R04 (1 ply 2x6 SP), R05A (1 ply 2x6 SP), R05 (1 ply 2x6 SP) to back face of bottom chord.
 - Fill all nail holes where hanger is in contact with lumber.



Continued on Page 2
Design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss R10	Truss Type Common Girder	Qty 1	Ply 3	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	-----------------------------	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:24 2023 Page 2
ID:av29u_vm2cwLxXF0Wc5ybwyV6X0-ezaARrkFZeK63q1JirX31TvBdtllWjGxhXk0huyUjH

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-60, 4-8=-60, 9-16=-20

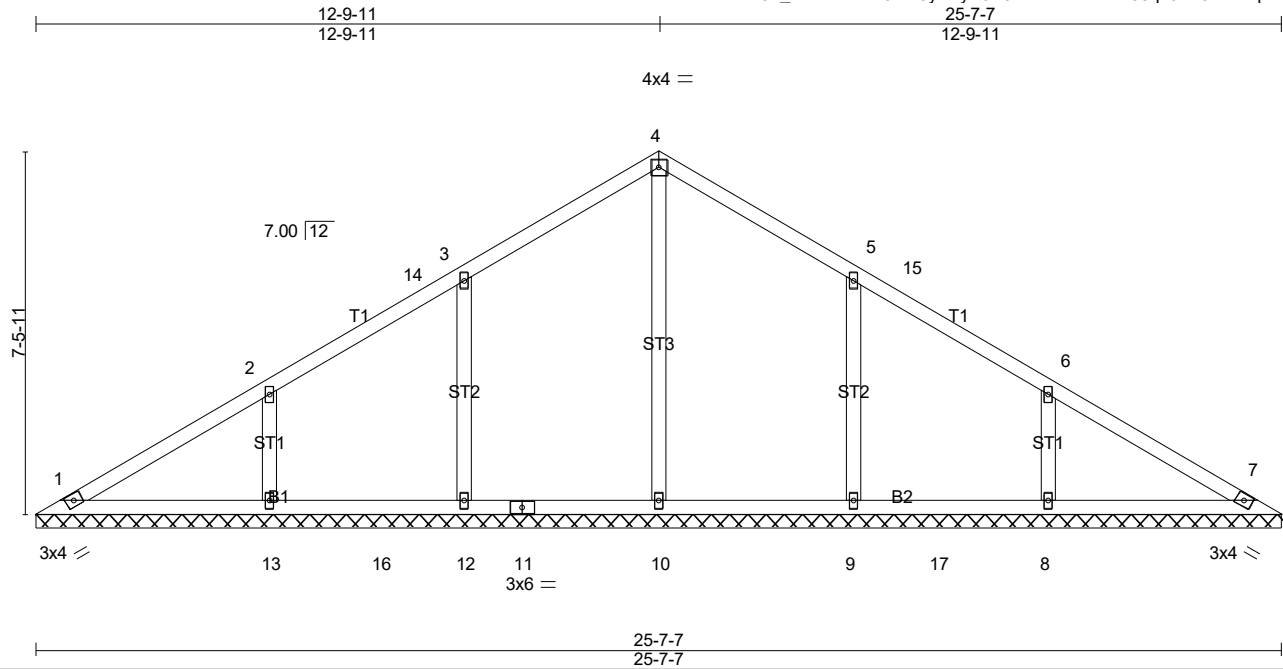
Concentrated Loads (lb)

Vert: 15=-303(B) 11=-410(B) 17=-303(B) 18=-303(B) 19=-410(B) 20=-410(B) 21=-410(B) 22=-410(B) 23=-410(B) 24=-410(B) 25=-416(B) 26=-416(B) 27=-416(B)



10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D*Onofrio Drive, Madison, WI 53719.



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.28	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.39	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.20	Vert(CT) n/a - n/a 999		
BCDL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 7 n/a n/a		
TCDL 10.0	Code IRC2021/TPI2014				
				Weight: 111 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 25-7-7.
(lb) - Max Horz 1=-158(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 1 except 12=-110(LC 14), 13=-121(LC 14), 9=-110(LC 15), 8=-121(LC 15)
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=456(LC 26), 12=523(LC 5), 13=392(LC 23), 9=523(LC 6), 8=392(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-12=-361/150, 2-13=-283/161, 5-9=-361/150, 6-8=-283/161

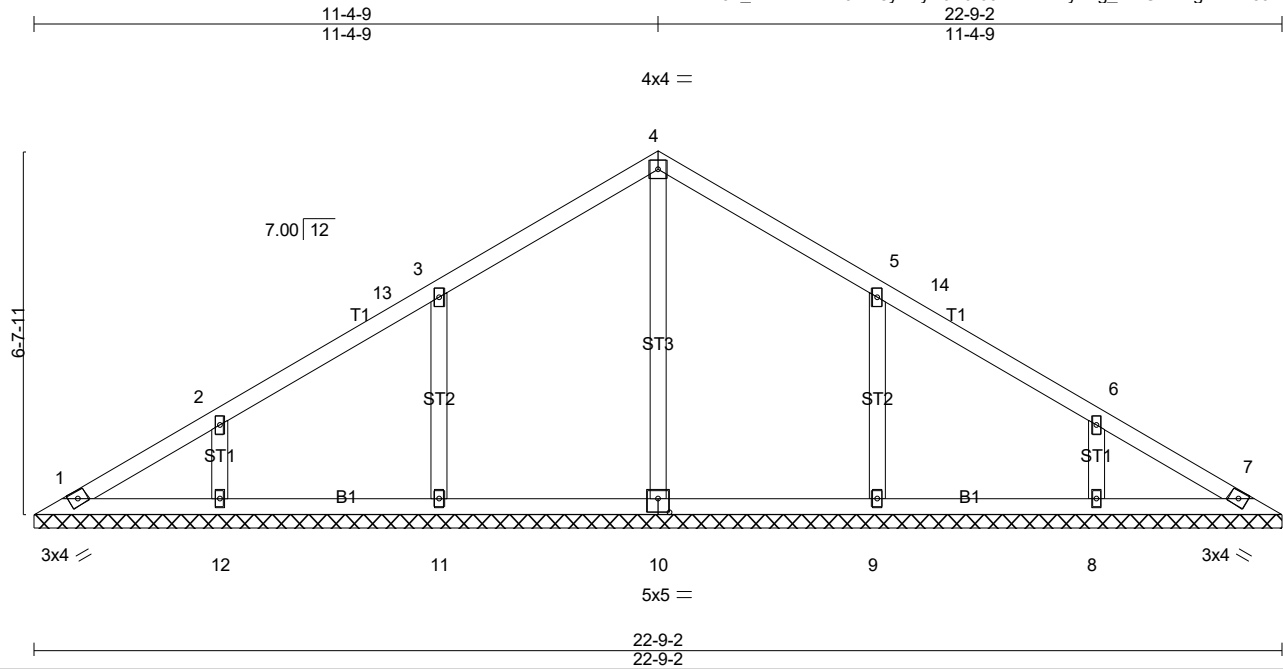
- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-6-8 to 5-4-1, Interior(1) 5-4-1 to 8-0-2, Exterior(2R) 8-0-2 to 17-7-5, Interior(1) 17-7-5 to 20-3-6, Exterior(2E) 20-3-6 to 25-0-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 12=110, 13=121, 9=110, 8=121.

LOAD CASE(S) Standard



10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D’Onofrio Drive, Madison, WI 53719.



Scale = 1:42.1

Plate Offsets (X,Y)-- [10:0-2-8,0-3-0]

LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.29	Vert(LL) n/a	-	n/a	999	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.43	Vert(CT) n/a	-	n/a	999		
TCDL 10.0	Lumber DOL 1.15	WB 0.16	Horz(CT) 0.00	7	n/a	n/a		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S						
BCDL 10.0	Code IRC2021/TPI2014							
							Weight: 96 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 22-9-2.
 (lb) - Max Horz 1=-139(LC 10)
 Max Uplift All uplift 100 lb or less at joint(s) 1, 12, 8 except 11=-117(LC 14), 9=-117(LC 15)
 Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 10=458(LC 23), 11=471(LC 5), 12=299(LC 1), 9=472(LC 6), 8=299(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-11=-376/158, 5-9=-376/158

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TC DL=5.0psf; BC DL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-6-8 to 5-4-1, Interior(1) 5-4-1 to 6-7-0, Exterior(2R) 6-7-0 to 16-2-3, Interior(1) 16-2-3 to 17-5-1, Exterior(2E) 17-5-1 to 22-2-11 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) All plates are 2x4 MT20 unless otherwise indicated.
 - 6) Gable requires continuous bottom chord bearing.
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 12, 8 except (jt=lb) 11=117, 9=117.

LOAD CASE(S) Standard

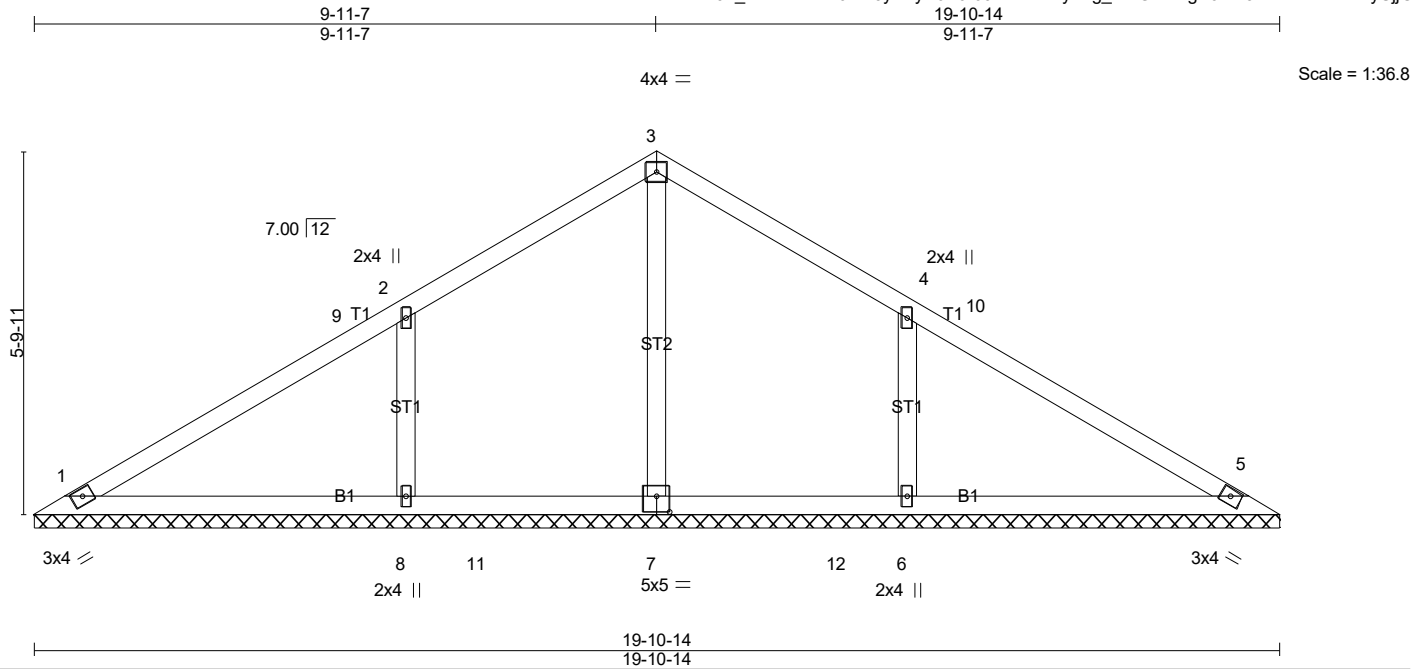


10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss V03	Truss Type Valley	Qty 1	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	----------------------	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:25 2023 Page 1
ID:av29u_vm2cwLxXF0Wc5ybywV6X0-697YfBkuKySzg_cWGY2lagRJDH5uFHi4wBTaDLyUjJG



LOADING (psf)		SPACING-		CSI.		DEFL.				PLATES	GRIP
TCLL (roof)	20.0	2-0-0		TC	0.43	in	(loc)	l/defl	L/d	MT20	244/190
Snow (Pf)	20.0	Plate Grip DOL	1.15	BC	0.40	Vert(LL)	n/a	-	n/a		
TCDL	10.0	Lumber DOL	1.15	WB	0.12	Vert(CT)	n/a	-	n/a		
BCLL	0.0 *	Rep Stress Incr	YES	Matrix-S		Horz(CT)	0.00	5	n/a		
BCDL	10.0	Code IRC2021/TPI2014								Weight: 79 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2x4 SP No.3	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2x4 SP No.3	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 19-10-14.
 (lb) - Max Horz 1=121(LC 13)
 Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-156(LC 14), 6=-156(LC 15)
 Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=389(LC 5), 8=577(LC 20), 6=577(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=-452/202, 4-6=-452/202

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-6-8 to 5-4-1, Exterior(2R) 5-4-1 to 14-6-13, Exterior(2E) 14-6-13 to 19-4-6 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=156, 6=156.

LOAD CASE(S) Standard

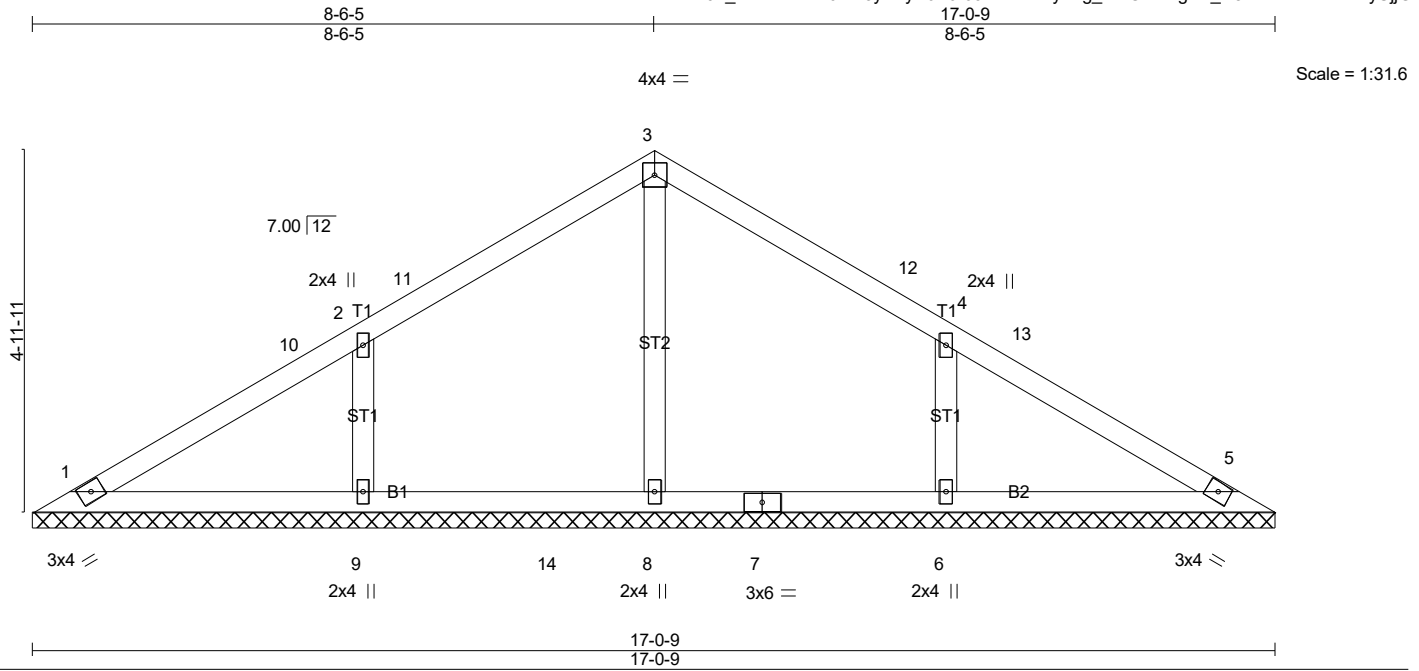


10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss V04	Truss Type Valley	Qty 1	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	----------------------	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:25 2023 Page 1
ID:av29u_vm2cwLtxF0Wc5ybwyV6X0-697YfBkuKySzg_cWGY2lagRL_H8YFHB4wBTaDLyUjJg



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.31	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.23	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.09	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 5 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014			Weight: 66 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

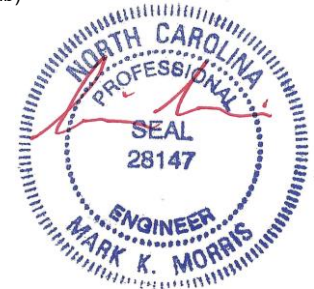
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 17-0-9.
(lb) - Max Horz 1=103(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=128(LC 14), 6=128(LC 15)
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=356(LC 23), 9=490(LC 20), 6=490(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-9=-395/167, 4-6=-395/167

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-6-8 to 5-4-1, Exterior(2R) 5-4-1 to 11-8-8, Exterior(2E) 11-8-8 to 16-6-2 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 9=128, 6=128.

LOAD CASE(S) Standard

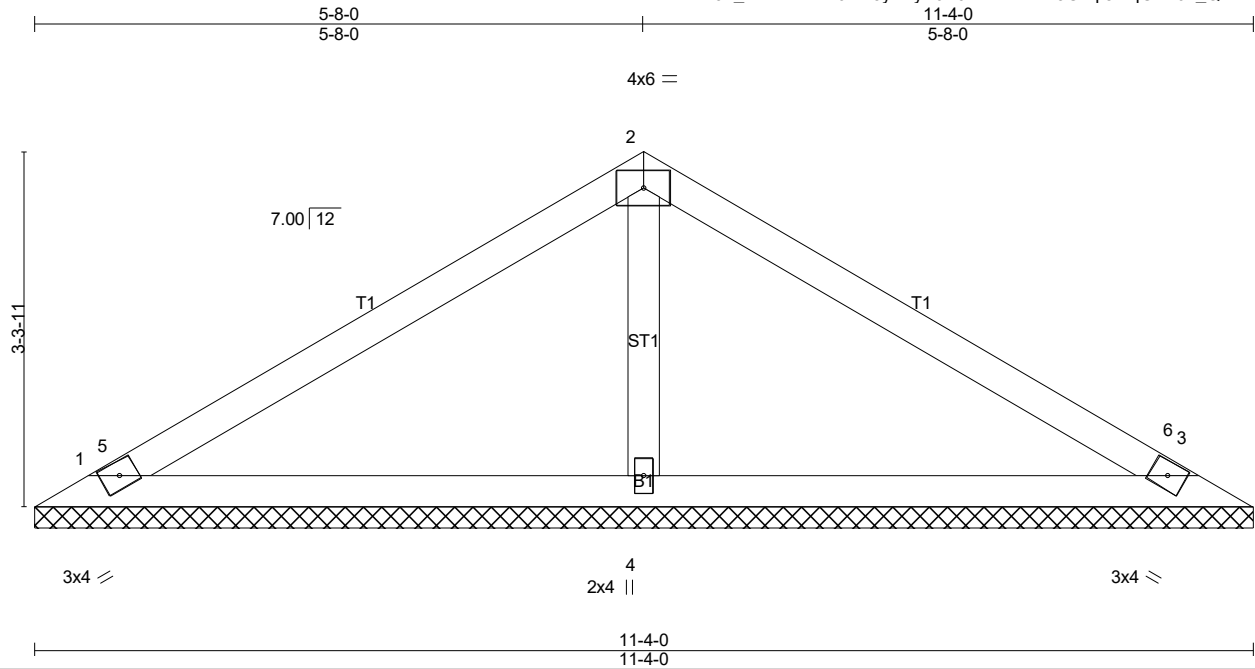


10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss V06	Truss Type Valley	Qty 1	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	----------------------	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:26 2023 Page 1
ID:av29u_vm2cwLxF0Wc5ybywV6X0-aLhwsXlW5GbqI8BiqGZX6u_QFhRQ_kcD8rD7InyUjJf



Scale = 1:21.4

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.60	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.44	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.08	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.00 3 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014			Weight: 39 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 1=194/11-4-0 (min. 0-1-8), 3=194/11-4-0 (min. 0-1-8), 4=433/11-4-0 (min. 0-1-8)
Max Horz 1=66(LC 13)
Max Uplift 1=-37(LC 14), 3=-46(LC 15), 4=-23(LC 14)
Max Grav 1=273(LC 20), 3=273(LC 21), 4=453(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-4=-301/105

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-6-8 to 5-4-1, Exterior(2R) 5-4-1 to 5-11-15, Exterior(2E) 5-11-15 to 10-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard

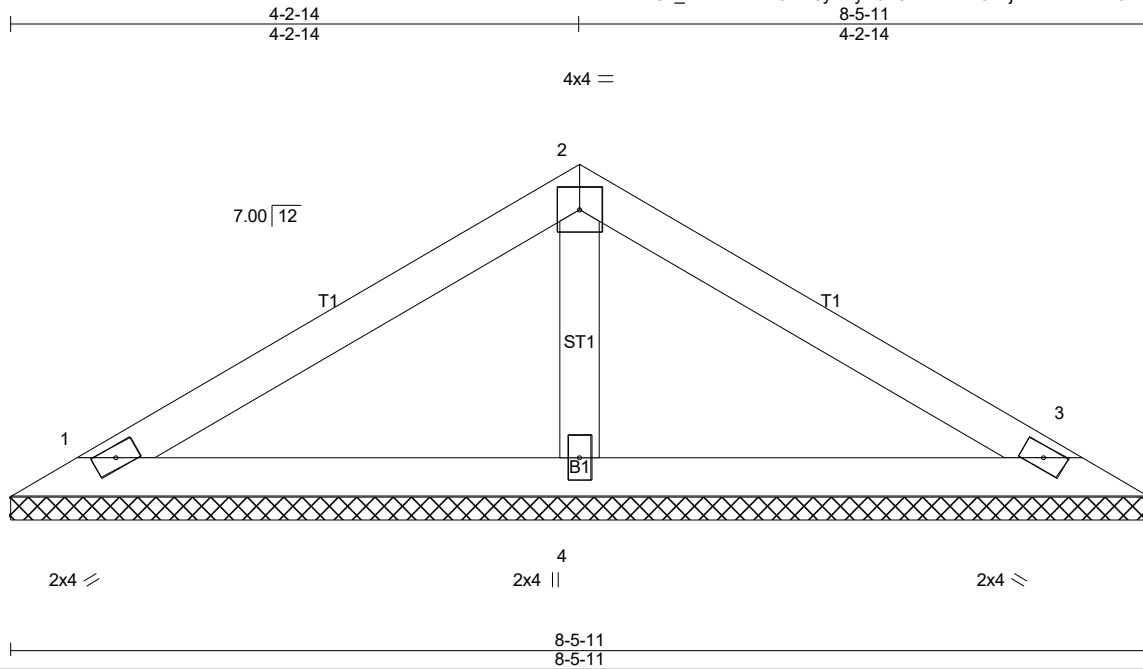


10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D*Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss V07	Truss Type Valley	Qty 1	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	----------------------	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:27 2023 Page 1
ID:av29u_vm2cwLTXF0Wc5ybwYV6X0-2YFI4tm8sZjhwlmNz4mf5XeE5qvjCONNvYgIDyUjJE



Scale = 1:17.1

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.39	in (loc) l/defl L/d	MT20	244/190
Snow (Pf) 20.0	Plate Grip DOL 1.15	BC 0.23	Vert(LL) n/a - n/a 999		
TCDL 10.0	Lumber DOL 1.15	WB 0.04	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-P	Horz(CT) 0.00 3 n/a n/a		
BCDL 10.0	Code IRC2021/TPI2014			Weight: 28 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 1=154/8-5-11 (min. 0-1-8), 3=154/8-5-11 (min. 0-1-8), 4=283/8-5-11 (min. 0-1-8)
Max Horz 1=-48(LC 12)
Max Uplift 1=-34(LC 14), 3=-40(LC 15), 4=-2(LC 14)
Max Grav 1=212(LC 20), 3=212(LC 21), 4=288(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCCL=5.0psf; BCCL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard

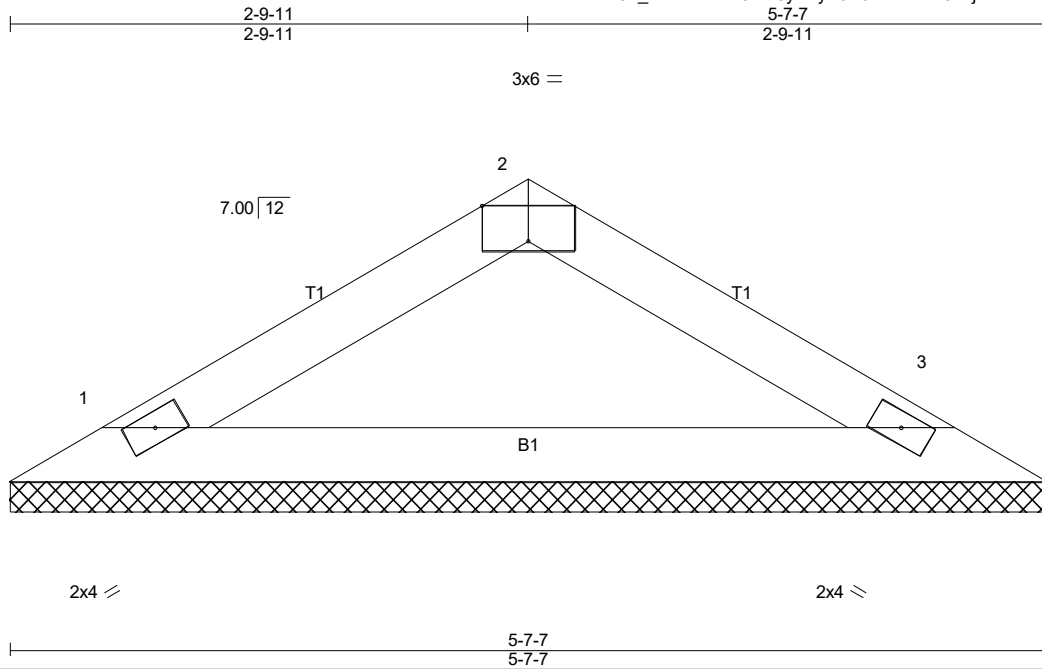


10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D*Onofrio Drive, Madison, WI 53719.

Job 23-6774-R01	Truss V08	Truss Type Valley	Qty 1	Ply 1	LOT 12 PROVIDENCE CREEK 290 DAVINHALL DRIVE Job Reference (optional) # 41613
--------------------	--------------	----------------------	----------	----------	--

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Tue Oct 10 20:11:27 2023 Page 1
ID:av29u_vm2cwLxXF0Wc5ybwyV6X0-2YF14tm8szJhwImuNz4mf5XjP5nfjC1NNVygIDyUjJE



Scale = 1:12.5

LOADING (psf)		SPACING-		CSI.	DEFL.				PLATES	GRIP	
TCLL (roof)	20.0	2-0-0	Plate Grip DOL	1.15	TC	0.13	in (loc)	l/defl	L/d	MT20	244/190
Snow (Pf)	20.0	1.15	Lumber DOL	1.15	BC	0.44	n/a	-	n/a		
TCDL	10.0	YES	Rep Stress Incr	YES	WB	0.00	n/a	-	n/a		
BCLL	0.0 *	Code IRC2021/TPI2014	Matrix-P		Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0									Weight: 16 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-7-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 1=182/5-7-7 (min. 0-1-8), 3=182/5-7-7 (min. 0-1-8)
Max Horz 1=29(LC 13)
Max Uplift 1=-21(LC 14), 3=-21(LC 15)
Max Grav 1=207(LC 20), 3=207(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TC DL=5.0psf; BC DL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TC LL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



10/10/2023

Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.