# Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 41420 JOB: 23-6776-R01

JOB NAME: LOT 50 PROVIDENCE CREEK

Wind Code: 37

Wind Speed: Vult= 120mph

**Exposure Category: B** 

Mean Roof Height (feet): 23

These truss designs comply with IRC 2015 as well as IRC 2018.

18 Truss Design(s)

## Trusses:

J01, J02, R01, R02, R02A, R03, R04, R05, R06, R07, R08, R09, SP01, SP02, V01, V02, V03, V04



### Warning !—Verify design parameters and read notes before use.

This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to

.lob Truss Truss Type LOT 50 PROVIDENCE CREEK | 199 WINDSWEPT WAY FUQUAY-VARINA, NC 23-6776-R01 J01 Jack-Closed # 41420 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MITek Industries, Inc. Thu Sep 28 08:43:48 2023 Page 1 ID:zSnl\_VDJTy\_hu?pmOjJgrKyZRJv-VlcmfxTuaV9r6vxBSGQpPEftKbc5Vr5CR9GoBXyZA?\ -0-10-8 0-10-8 5-8-8 Scale = 1:17.5 2x4 || 3 5.00 12 2-10-13 W1 0-6-4 В1 4 2x4 || 4x4 =5-6-4 LOADING (psf) SPACING-GRIP CSI. DEFL. **PLATES** 2-0-0 (loc) I/defl L/d TCLL (roof) 20.0 244/190 Plate Grip DOL 1.15 TC 0.47 Vert(LL) 0.044-7 >999 240 MT20 Snow (Pf) 20.0 вс Lumber DOL 1.15 0.34 Vert(CT) -0.054-7 >999 180 **TCDL** 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) n/a n/a 0.0 **BCLL** Code IRC2021/TPI2014 FT = 20% Matrix-AS Weight: 27 lb BCDL 10.0 BRACING-LUMBER-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied, except end verticals. BOT CHORD

BOT CHORD 2x6 SP No.2 WFBS 2x4 SP No.3

Rigid ceiling directly applied.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS.** (lb/size) 4=218/Mechanical, 2=279/0-3-8 (min. 0-1-8)

Max Horz 2=84(LC 13)

Max Uplift4=-47(LC 10), 2=-64(LC 10) Max Grav 4=303(LC 21), 2=364(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 4 and 64 lb uplift at joint 2.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord. 10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates

that the member must be braced. 11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the

structural design of the truss to support the loads indicated. 12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling,

Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing 13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

SEAL 28147

NONEE NORMANIA Warning !--Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

.lob Truss Truss Type LOT 50 PROVIDENCE CREEK | 199 WINDSWEPT WAY FUQUAY-VARINA, NC 23-6776-R01 J02 Jack-Closed # 41420 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu Sep 28 08:43:49 2023 Page 1 ID:zSnI\_VDJTy\_hu?pmOjJgrKyZRJv-zy98tHUWLpHik3WN0\_x2xSC24?yKEILLgp0LjzyZA?u 5-8-8 -0-10-8 0-10-8 3-8-8 2-0-0 Scale = 1:17.5 2x4 || 3 2x4 || 5.00 12 2-10-13 W2 W1 0-6-4 B1 2x4 || 4 2x4 || 4x4 =5-6-4 LOADING (psf) SPACING-GRIP CSI. DEFL. **PLATES** 2-0-0 I/defl L/d (loc) TCLL (roof) 20.0 244/190 Plate Grip DOL 1.15 TC 0.47 Vert(LL) 0.044-9 >999 240 MT20 Snow (Pf) 20.0 вс Lumber DOL 1.15 0.34 Vert(CT) -0.054-9 >999 180 **TCDL** 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) n/a n/a 0.0 **BCLL** Code IRC2021/TPI2014 FT = 20% Matrix-AS Weight: 29 lb BCDL 10.0 BRACING-LUMBER-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied, except end verticals. BOT CHORD 2x6 SP No.2 BOT CHORD Rigid ceiling directly applied. WFBS 2x4 SP No.3 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Installation guide.

**REACTIONS.** (lb/size) 4=218/Mechanical, 2=279/0-3-8 (min. 0-1-8)

Max Horz 2=84(LC 13)

Max Uplift4=-47(LC 10), 2=-64(LC 10) Max Grav 4=303(LC 21), 2=364(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-16: Vult=120mph (3-second gust) Vasd=95mph: TCDL=5.0psf: BCDL=5.0psf: h=23ft: Cat. II: Exp B: Enclosed: MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 4 and 64 lb uplift at joint 2.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord. 10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates

that the member must be braced. 11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the

structural design of the truss to support the loads indicated. 12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling,

Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing 13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

Job Truss Truss Type LOT 50 PROVIDENCE CREEK | 199 WINDSWEPT WAY FUQUAY-VARINA, NC 23-6776-R01 R01 GABLE # 41420 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MITek Industries, Inc. Thu Sep 28 08:43:52 2023 Page 1 ID:zSnl\_VDJTy\_hu?pmOjJgrKyZRJv-OWrGVJWOekfHbWEyh6UIZ4qdXC1NRd4nMnE?KIyZA?ı 19-0-0 37-8-8 0-10-8 19-0-0 18-8-8 Scale = 1:67.0 5x6 = 6.00 12 12 13 14 45 15 3x8 / 16 3x8 < 17 6 18 19 5 SH8 <sup>20</sup>46 SHE 21 3x4 || 3x4 || 22 1-1-12 1-0-0 43 42 41 40 39 38 37 36 35 34 33 32 31 30 29 28 27 26 25 24 23 3x4 II 3x8 =3x8 =3x4 || 37-8-8 LOADING (psf) GRIP SPACING-CSI. DEFL. PLATES 2-0-0 I/defl L/d (loc) TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.14 Vert(LL) -0.00 n/r 180 MT20 244/190 Snow (Pf) 20.0 вс Lumber DOL 1.15 0.13 Vert(CT) -0.00n/r 80 TCDL 10.0 Rep Stress Incr YES WB 0.13 Horz(CT) 23 n/a n/a **BCLL** 0.0 Code IRC2018/TPI2014 Weight: 267 lb Matrix-R FT = 20% BCDL 10.0 BRACING-Structural wood sheathing directly applied or 6-0-0 oc purlins, except

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WFBS 2x4 SP No.3 2x4 SP No.3 OTHERS

TOP CHORD

**BOT CHORD** WFBS

end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing 12-33, 11-34, 10-35, 13-32, 14-31 1 Row at midpt

Installation guide.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

REACTIONS. All bearings 37-8-8.

- Max Horz 43=142(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 43, 23, 34, 35, 36, 38, 39, 40, 41, 32, 31, 30, 28, 27, 26, 25, 24 except 42=-107(LC 14)

Max Grav All reactions 250 lb or less at joint(s) 43, 23, 36, 38, 39, 40, 41, 42, 30, 28, 27, 26, 25, 24 except 33=285(LC 27), 34=290(LC 5), 35=276(LC 5), 32=290(LC 6), 31=276(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 9-44=-120/251, 10-44=-112/259, 10-11=-139/299, 11-12=-156/332, 12-13=-156/332, TOP CHORD 13-14=-139/299, 14-45=-112/259, 15-45=-120/251

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph, TCDL=5.0psf; BCDL=5.0psf; h=24ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-0-0, Exterior(2N) 3-0-0 to 15-0-0, Corner(3R) 15-0-0 to 23-0-0, Exterior(2N) 23-0-0 to 33-6-12, Corner(3E) 33-6-12 to 37-6-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

  4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

standard ANSI/TPI 1.

- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

  5) Unbalanced snow loads have been considered for this design.

  6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

  7) All plates are 2x4 MT20 unless otherwise indicated.

  8) Gable requires continuous bottom chord bearing.

  9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

  10) Gable studs spaced at 2-0-0 oc.

  11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

  12) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

  13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 43, 23, 34, 35, 36, 38, 39, 40, 41, 32, 31, 30, 28, 27, 26, 25, 24 except (jt=lb) 42=107.

  14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced

9/26/2023

LOAD TO CASE SCIENCE TO PARAMETERS and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job Truss Truss Type Qtv LOT 50 PROVIDENCE CREEK | 199 WINDSWEPT WAY FUQUAY-VARINA, NC 23-6776-R01 R02 Common 10 # 41420 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu Sep 28 08:43:55 2023 Page 1 ID:zSnl\_VDJTy\_hu?pmOjJgrKyZRJv-o5XP7KYHxf1sS\_zXNF2SBjSyfQuCevEE2kTfxdyZA?o 12-9-3 19-0-0 25-2-13 31-4-11 37-8-8 6-1-14 6-2-13 6-2-13 6-1-14 6-3-13 Scale = 1:70.3 5x8 = 7 2x4 || 24 J2 2x4 II 27 3x8 / 6 3x8 < 6.00 12 2x4 < 2x4 / 10 4x6 < XX/1 XQ 29 30 14 13 4x12 || 7x8 =7x8 =7x8 II 5x5 = 25-2-13 12-5-11 12-9-3 12-5-11 Plate Offsets (X,Y)-- [12:0-4-0,0-6-13], [13:0-4-0,0-4-8], [14:0-4-0,0-4-8] LOADING (psf) DEFL. I/d **PLATES** GRIP SPACING-2-0-0 CSI. in (loc) I/defl TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.91 Vert(LL) -0.58 13-14 >786 240 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 вс 0.82 Vert(CT) -0.82 13-14 >553 180 TCDL 10.0 WB 0.49 Rep Stress Incr YES Horz(CT) 0.12 12 n/a n/a **BCLL** 0.0 Code IRC2018/TPI2014 FT = 20% Matrix-AS Weight: 237 lb **BCDL** 10.0 LUMBER-BRACING-TOP CHORD 2x4 SP No.2 \*Except\* TOP CHORD Structural wood sheathing directly applied. T1: 2x4 SP No.1, T3: 2x4 SP SS BOT CHORD Rigid ceiling directly applied. BOT CHORD 2x6 SP No.1 \*Except\* MiTek recommends that Stabilizers and required cross bracing

B1: 2x6 SP No.2

WFBS 2x4 SP No.3

SLIDER Left 2x6 SP No.2 -° 1-11-0, Right 2x6 SP No.2 -° 1-11-0

REACTIONS. (lb/size) 12=1508/Mechanical, 2=1561/0-3-8 (min. 0-1-13)

Max Horz 2=141(LC 14)

Max Uplift12=-125(LC 15), 2=-141(LC 14)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-881/0, 3-23=-2478/252, 4-23=-2440/275, 4-5=-2262/231, 5-6=-2194/247,

6-24=-2297/319, 24-25=-2247/326, 7-25=-2221/344, 7-26=-2197/344, 26-27=-2249/326,

8-27=-2274/318, 8-9=-2166/246, 9-10=-2233/230, 10-28=-2320/274, 11-28=-2414/252,

11-12=-483/178

**BOT CHORD** 2-14=-277/2130, 14-29=-43/1453, 29-30=-43/1453, 13-30=-43/1453, 12-13=-178/2060 WEBS 7-13=-177/1052, 8-13=-525/198, 6-14=-513/197, 7-14=-181/1091, 4-14=-290/164

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=24ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-1-8, Interior(1) 3-1-8 to 15-0-0, Exterior(2R) 15-0-0 to 23-0-0, Interior(1) 23-0-0 to 33-8-8, Exterior(2E) 33-8-8 to 37-8-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Refer to girder(s) for truss to truss connections.
- 8) Refer to gliue(s) for tascs to task.

  9) Provide mechanical connection (by others) of truss to bearing plate capable or with stationary 100 and 100
- sheetrock be applied directly to the bottom chord.

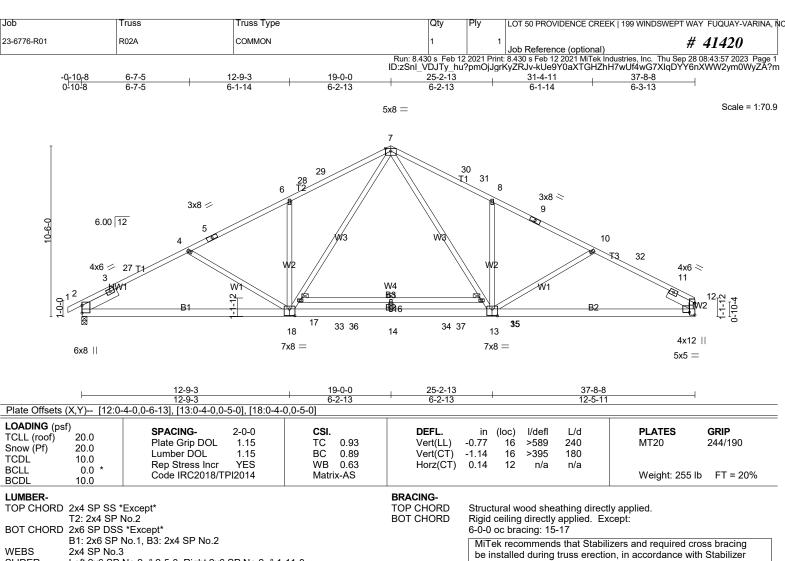
be installed during truss erection, in accordance with Stabilizer

Installation guide

SEAL 28147

26/202

Ved Warning !--Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Installation guide.

2x4 SP No.3

SLIDER Left 2x6 SP No.2 -° 2-5-0, Right 2x6 SP No.2 -° 1-11-0

REACTIONS. (lb/size) 12=1624/Mechanical, 2=1676/0-3-8 (min. 0-2-1)

Max Horz 2=141(LC 14)

Max Uplift12=-67(LC 15), 2=-84(LC 14) Max Grav 12=1734(LC 3), 2=1772(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-758/0, 3-27=-2961/139, 4-27=-2933/159, 4-5=-2788/99, 5-6=-2752/124,

6-28=-2828/195, 28-29=-2804/202, 7-29=-2751/220, 7-30=-2723/219, 30-31=-2777/201, 8-31=-2802/194, 8-9=-2686/123, 9-10=-2754/108, 10-32=-2810/160, 11-32=-2904/138,

11-12=-203/275

**BOT CHORD** 2-18=-178/2552, 18-33=0/1822, 14-33=0/1822, 14-34=0/1822, 34-35=0/1822, 13-35=0/1822, 12-13=-81/2482

7-15=-112/1329, 13-15=-142/1210, 4-18=-262/178, 6-18=-516/193, 17-18=-146/1253, 7-17=-114/1372, 8-13=-533/195, 14-16=-368/0

NOTES-(13)

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=24ft; Cat. II; Exp B; Enclosed: MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-1-8, Interior(1) 3-1-8 to 15-0-0, Exterior(2R) 15-0-0 to 23-0-0, Interior(1) 23-0-0 to 33-8-8, Exterior(2E) 33-8-8 to 37-8-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

(envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-1-8, Interior(1) 3-1-8 to 15-0-0, Exterior(2R) 15-0-0 to 23-0-0, Interior(1) 23-0-0 to 33-8-8, Exterior(2E) 33-8-8 to 37-8-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) All plates are 2x4 MT20 unless otherwise indicated.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with fit between the bottom chord and any other members, with BCDL = 10.0psf.

9) Refer to girder(s) for truss to truss connections.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 2.

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced

9/26/2023

Continuing by period assign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 50 PROVIDENCE CREEK   199 WINDSWEPT WAY FUQUAY-V	ARINA, NC
23-6776-R01	R02A	COMMON	1	1	Job Reference (optional) # 41420	
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu Sep 28 08:43:57 2023 Page 2 ID:zSnI_VDJTy_hu?pmOjJgrKyZRJv-kUe9Y0aXTGHZhH7wUf4wG7XIqDYY6nXWW2ym0WyZA?r						

**NOTES-** (13)

LOAD CASE(S) Standard



<sup>12)</sup> This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

Job Truss Truss Type Qtv LOT 50 PROVIDENCE CREEK | 199 WINDSWEPT WAY FUQUAY-VARINA, NC 23-6776-R01 R03 Common # 41420 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu Sep 28 08:43:59 2023 Page 1 ID:zSnI\_VDJTy\_hu?pmOjJgrKyZRJv-gtmwzicn?tXHwbHlc46OLYcg01Dsah1pzMRt4OyZA?k -0-10<sub>-8</sub> 12-9-3 19-0-0 25-2-13 31-4-11 38-10-8 0-10-8 38-0-0 6-7-5 6-2-13 6-2-13 6-1-14 6-7-5 Scale = 1:68.1 5x8 =6.00 12 7 30 29 3x8 / 3x8 > 31 4x6 < 4x6 / 11 12 3 W4 12 **B3** da: 18 36 32 35 33 36 19 15 14 8x8 || 7x8 = 7x8 =8x8 || 19-0-0 38-0-0 12-9-3 6-2-13 6-2-13 12-9-3 Plate Offsets (X,Y)-- [4:0-0-0,0-0-0], [14:0-4-0,0-5-0], [19:0-4-0,0-5-0] LOADING (psf) DEFL. I/d **PLATES** GRIP SPACING-2-0-0 CSI. in (loc) I/defl TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.82 Vert(LL) -0.7817 >584 240 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 вс 0.90 Vert(CT) -1.1617 >394 180 TCDL 10.0 WB 0.63 Rep Stress Incr YES Horz(CT) 0.12 12 n/a n/a **BCLL** 0.0 Code IRC2018/TPI2014 Weight: 257 lb Matrix-AS FT = 20% **BCDL** 10.0 LUMBER-BRACING-TOP CHORD 2x4 SP No.2 \*Except\* TOP CHORD Structural wood sheathing directly applied. T1: 2x4 SP SS BOT CHORD Rigid ceiling directly applied. Except: BOT CHORD 2x6 SP No.1 \*Except\* 6-0-0 oc bracing: 16-18 B2: 2x6 SP DSS, B3: 2x4 SP No.2 MiTek recommends that Stabilizers and required cross bracing

WFBS 2x4 SP No.3

SLIDER Left 2x6 SP No.2 -° 1-11-0, Right 2x6 SP No.2 -° 1-11-0

REACTIONS. (lb/size) 2=1688/0-3-8 (min. 0-2-2), 12=1688/0-3-8 (min. 0-2-2)

Max Horz 2=133(LC 14)

Max Uplift2=-83(LC 14), 12=-83(LC 15) Max Grav 2=1785(LC 3), 12=1785(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-771/0, 3-28=-2996/389, 4-28=-2958/411, 4-5=-2809/318, 5-6=-2741/332,

6-29=-2849/431, 7-29=-2773/456, 7-30=-2773/456, 8-30=-2849/431, 8-9=-2741/332,

9-10=-2809/318, 10-31=-2958/411, 11-31=-2996/389, 11-12=-527/0

**BOT CHORD** 2-19=-246/2577, 19-32=0/1846, 15-32=0/1846, 15-33=0/1846, 33-34=0/1846, 14-34=0/1846, 12-14=-246/2577

**WEBS** 7-16=-149/1366, 14-16=-181/1247, 8-14=-516/214, 10-14=-266/201, 18-19=-181/1247,

7-18=-149/1366, 6-19=-516/214, 4-19=-266/201, 15-17=-367/0

### (12)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=24ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) '0-10-8 to 3-1-8, Exterior(2N) 3-1-8 to 15-0-0, Corner(3R) 15-0-0 to 23-0-0, Exterior(2N) 23-0-0 to 34-10-8, Corner(3E) 34-10-8 to 38-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design. 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

6) All plates are 2x4 MT20 unless otherwise indicated.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12.

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1

11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

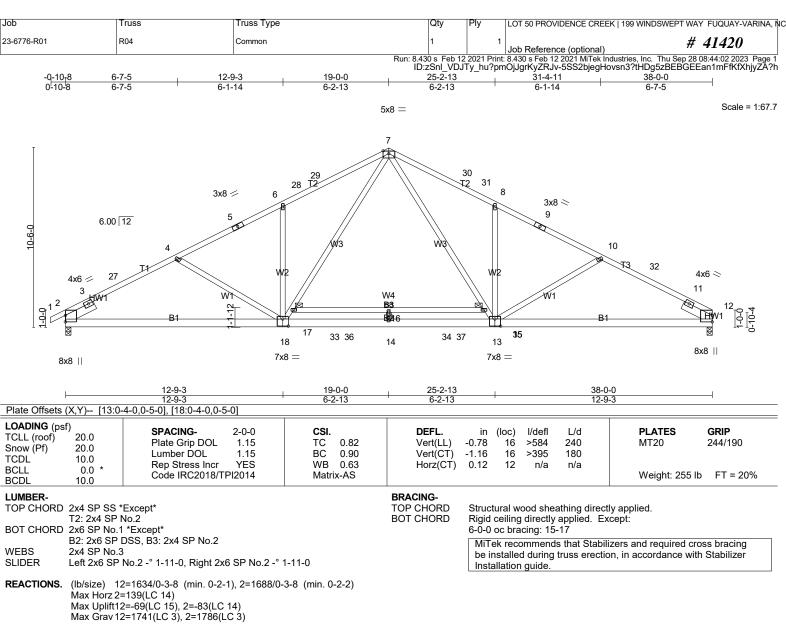
be installed during truss erection, in accordance with Stabilizer

Installation guide.

SEAL 28147

SEAL 28147

YOUNEER STATEMENT OF ESSION AND THE PROPERTY OF ESSION AND THE PROPERTY OF ESSION AND THE PROPERTY OF LOADDOASE(S) r Stranging parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-771/0, 3-27=-2997/137, 4-27=-2959/159, 4-5=-2810/109, 5-6=-2742/124,

6-28=-2850/195, 28-29=-2800/202, 7-29=-2774/220, 7-30=-2775/221, 30-31=-2802/203, 8-31=-2852/196, 8-9=-2744/126, 9-10=-2812/111, 10-32=-2942/164, 11-32=-2999/141,

11-12=-566/0

**BOT CHORD** 2-18=-175/2577, 18-33=0/1847, 14-33=0/1847, 14-34=0/1847, 34-35=0/1847, 13-35=0/1847,

12-13=-81/2581

7-15=-114/1368, 13-15=-145/1250, 8-13=-516/193, 10-13=-269/178, 17-18=-145/1247,

7-17=-114/1366, 6-18=-517/193, 4-18=-266/178, 14-16=-367/0

### NOTES-(12)

**WEBS** 

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=24ft; Cat. II; Exp B; Enclosed: MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-1-8, Interior(1) 3-1-8 to 15-0-0, Exterior(2R) 15-0-0 to 23-0-0, Interior(1) 23-0-0 to 34-0-0, Exterior(2E) 34-0-0 to 38-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-1-8, Interior(1) 3-1-8 to 15-0-0, Exterior(2R) 15-0-0 to 23-0-0, Interior(1) 23-0-0 to 34-0-0, Exterior(2E) 34-0-0 to 38-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) All plates are 2x4 MT20 unless otherwise indicated.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with fit between the bottom chord and any other members, with BCDL = 10.0psf.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 2.

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

standard ANSI/TPI 1.

11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum

9/26/2023

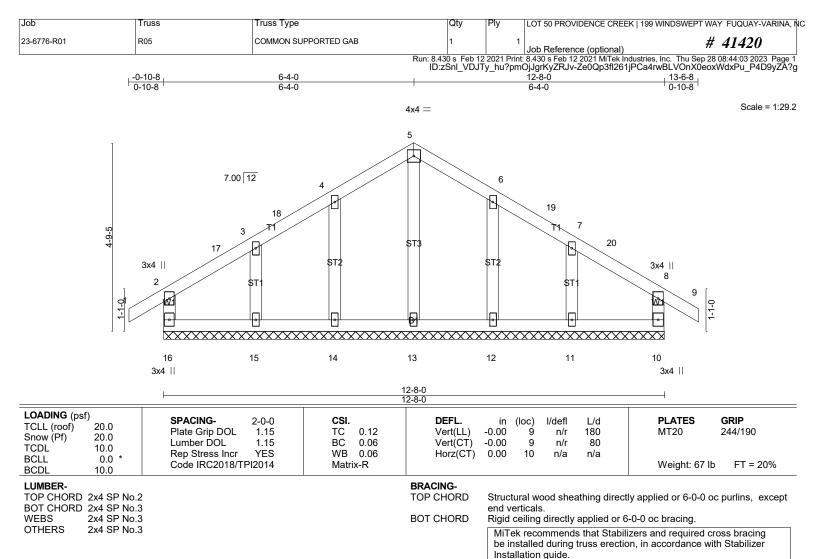
Continuing by period asign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 50 PROVIDENCE CREEK   199 WINDSWEPT WAY FUQUAY-VARINA, N	
23-6776-R01	R04	Common	1	1	Job Reference (optional) # 41420	

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LOAD CASE(S) Standard





REACTIONS. All bearings 12-8-0.

(lb) - Max Horz 16=80(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 15, 12, 11 Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=21ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-1-8, Corner(3R) 3-1-8 to 9-6-8, Corner(3E) 9-6-8 to 13-6-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.

- 8) Gable requires continuous bottom chord bearing.

  9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

  10) Gable studs spaced at 2-0-0 oc.

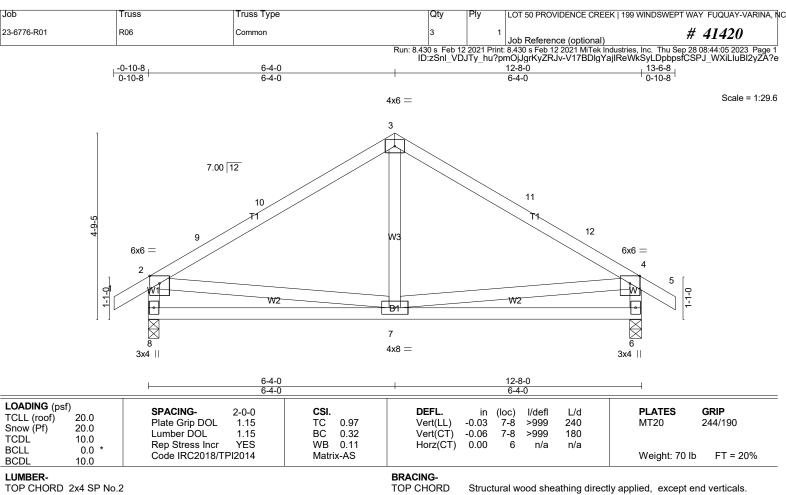
  11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

  12) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

  13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.

  14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

  LOAD CASE(S) Standard



BOT CHORD 2x4 SP No.2 WFBS 2x4 SP No.3 BOT CHORD

Rigid ceiling directly applied.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 8=556/0-3-8 (min. 0-1-8), 6=556/0-3-8 (min. 0-1-8)

Max Horz 8=-80(LC 12)

Max Uplift8=-47(LC 14), 6=-47(LC 15) Max Grav 8=632(LC 21), 6=632(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-9=-600/125, 9-10=-482/137, 3-10=-461/153, 3-11=-461/153, 11-12=-482/137, 4-12=-600/125, 2-8=-574/217, 4-6=-574/217 TOP CHORD

**BOT CHORD** 7-8=-123/364. 6-7=-103/364 2-7=-35/277, 4-7=-39/277 WFBS

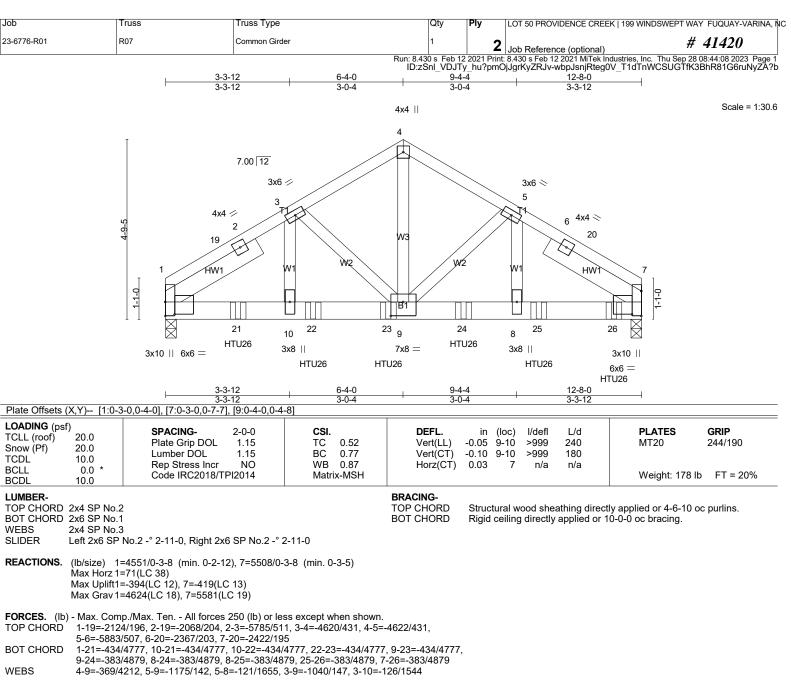
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=21ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-1-8, Corner(3R) 3-1-8 to 9-6-8, Corner(3E) 9-6-8 to 13-6-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.
9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

Warning !--Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



### NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-5-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=21ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 5) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 6) Unbalanced snow loads have been considered for this design.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=394, 7=419
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R5U2.11.1 and R6U2.10.2 and references standard ANSI/TPI 1.

  11) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting 1/10 in 1/2 in 1/2

12) Fill all nail holes where hanger is in contact with lumber.

SEAL 28147

NOMEE NORTH STATE OF THE STATE O Continuing by Design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 50 PROVIDENCE CREEK   199 WINDSWEPT WAY FUQUAY-VARINA, NC
23-6776-R01	R07	Common Girder	1	2	Job Reference (optional) # 41420

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LOAD CASE(S) Standard

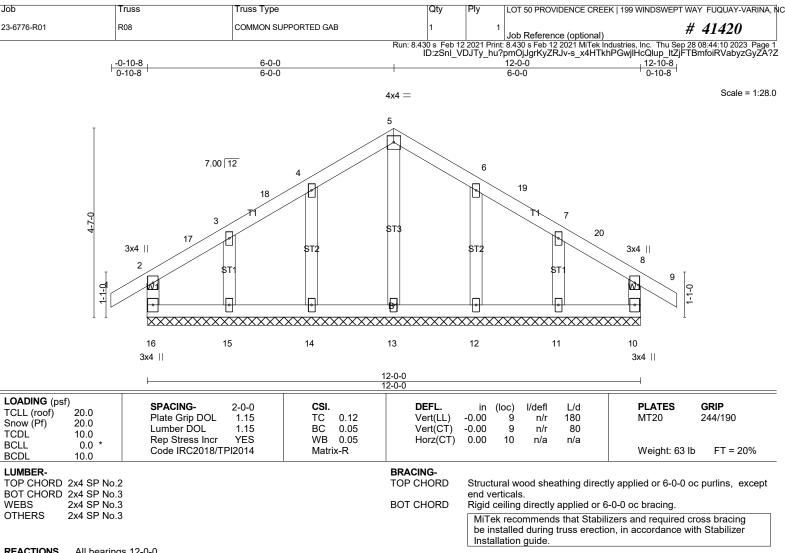
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-4=-60, 4-7=-60, 11-15=-20

Concentrated Loads (lb)

Vert: 21=-1488(B) 22=-1488(B) 23=-1488(B) 24=-1488(B) 25=-1488(B) 26=-1606(B)





REACTIONS. All bearings 12-0-0.

(lb) - Max Horz 16=-76(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 15, 12, 11

Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 15, 11 except 14=252(LC 21), 12=252(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=21ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-1-8, Corner(3R) 3-1-8 to 8-10-8, Corner(3E) 8-10-8 to 12-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.

- 8) Gable requires continuous bottom chord bearing.

  9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

  10) Gable studs spaced at 2-0-0 oc.

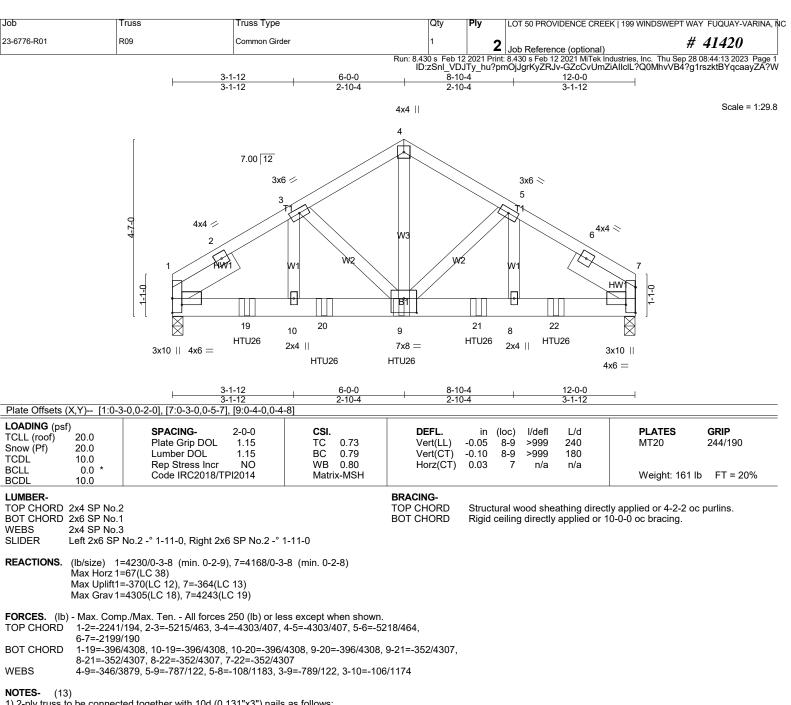
  11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

  12) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

  13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.

  14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

  LOAD CASE(S) Standard



1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=21ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60 5) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

6) Unbalanced snow loads have been considered for this design.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=370, 7=364

10) This truss is designed in accordance with the 2018 International Residential Code sections R5U2.11.1 and R6U2.10.2 and relicions standard ANSI/TPI 1.

11) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting in the compact truss(as) R02 (1 nlv 2x6 SP) to back face of bottom chord.

12) Fill all nail holes where hanger is in contact with lumber.

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Ved Continuing by Design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 50 PROVIDENCE CREEK   199 WINDSWEP	TWAY FUQUAY-VARINA, NC
23-6776-R01	R09	Common Girder	1	2	Job Reference (optional)	# 41420

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu Sep 28 08:44:13 2023 Page 2 ID:zSnI\_VDJTy\_hu?pmOjJgrKyZRJv-GZcCvUmZiAllcIL?Q0MhvVB4?g1rszktBYqcaayZA?W

LOAD CASE(S) Standard

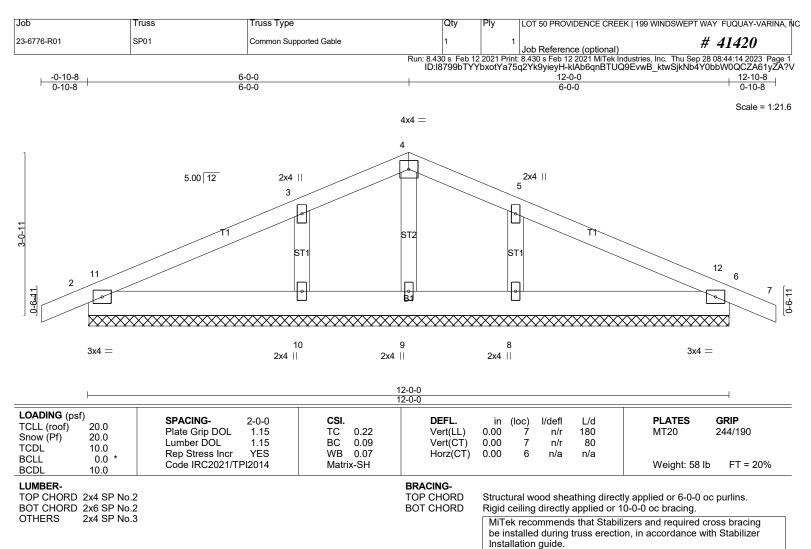
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-4=-60, 4-7=-60, 11-15=-20

Concentrated Loads (lb)

Vert: 9=-1488(B) 19=-1488(B) 20=-1488(B) 21=-1488(B) 22=-1488(B)





All bearings 12-0-0. REACTIONS.

(lb) - Max Horz 2=-38(LC 15)

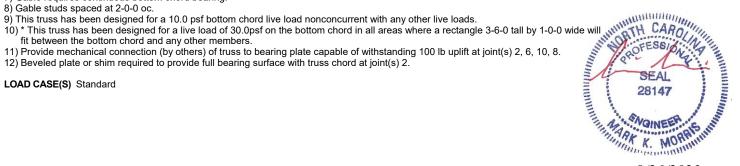
Max Uplift All uplift 100 lb or less at joint(s) 2, 6, 10, 8

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9 except 10=452(LC 21), 8=452(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 3-10=-317/163, 5-8=-317/163

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-0-0, Corner(3R) 4-0-0 to 8-0-0, Corner(3E) 8-0-0 to 12-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2-0-0 oc.



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Job LOT 50 PROVIDENCE CREEK | 199 WINDSWEPT WAY FUQUAY-VARINA, NC Truss Truss Type 23-6776-R01 SP02 Common # 41420 Job Reference (optional) Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Thu Sep 28 08:44:15 2023 Page 1 ID:18799bTYYbxotYa75q2Yk9yieyH-CykzKAoqDoY0r2VNYRP9?wHSLUpsK20AesJjeTyZA?U 6-0-0 12-0-0 12-10-8 -0-10-8 0-10-8 6-0-0 6-0-0 0-10-8 Scale = 1:21.6 4x4 =3 5.00 12 W1 9 10 6 2x4 || 3x6 = 3x6 < 6-0-0 6-0-0 Plate Offsets (X,Y)-- [2:0-0-10,0-1-8], [4:0-0-10,0-1-8] LOADING (psf) DEFL. **PLATES** GRIP SPACING-2-0-0 CSI. in (loc) I/defl I/d TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.61 Vert(LL) -0.034-6 >999 240 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 вс 0.37 Vert(CT) -0.05 4-6 >999 180 TCDL 10.0 WB 0.11 Rep Stress Incr YES Horz(CT) 0.01 n/a n/a **BCLL** 0.0 Code IRC2021/TPI2014 Weight: 54 lb FT = 20% Matrix-SH

LUMBER-

**BCDL** 

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 2x4 SP No.3 WFBS

10.0

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-4-12 oc purlins. Rigid ceiling directly applied or 9-6-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

**REACTIONS.** (lb/size) 2=530/0-3-8 (min. 0-1-8), 4=530/0-3-8 (min. 0-1-8)

Max Horz 2=-38(LC 15)

Max Uplift2=-117(LC 10), 4=-117(LC 11) Max Grav 2=624(LC 21), 4=624(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-7=-775/690, 3-7=-633/701, 3-8=-633/701, 4-8=-775/690 **BOT CHORD** 2-9=-559/594, 6-9=-559/594, 6-10=-559/594, 4-10=-559/594

WFBS 3-6=-379/297

(9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Exterior(2R) 3-11-2 to 8-0-14, Exterior(2E) 8-0-14 to 12-10-8 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

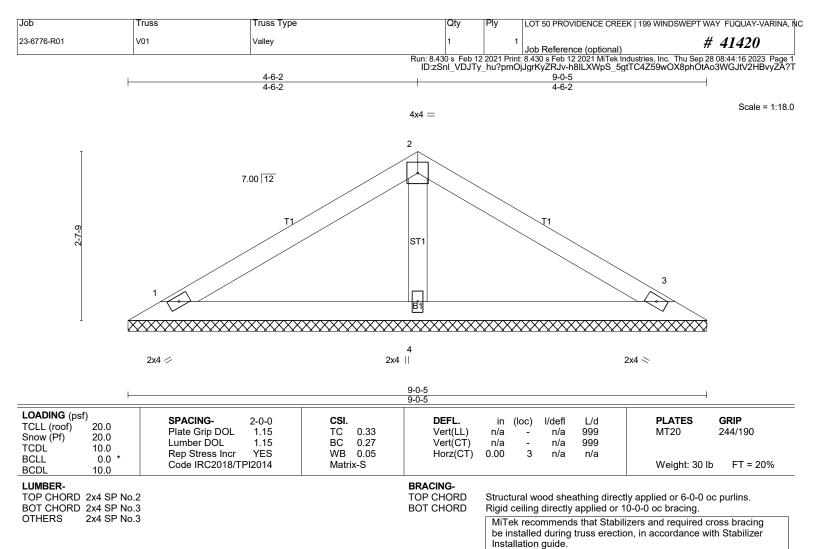
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=117.
4=117.

LOAD CASE(S) Standard

SEAL
28147

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REACTIONS. (lb/size) 1=150/9-0-5 (min. 0-1-8), 3=150/9-0-5 (min. 0-1-8), 4=335/9-0-5 (min. 0-1-8)

Max Horz 1=45(LC 13)

Max Uplift1=-21(LC 14), 3=-27(LC 15), 4=-4(LC 14) Max Grav 1=214(LC 20), 3=214(LC 21), 4=344(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### (10)

1) Unbalanced roof live loads have been considered for this design.

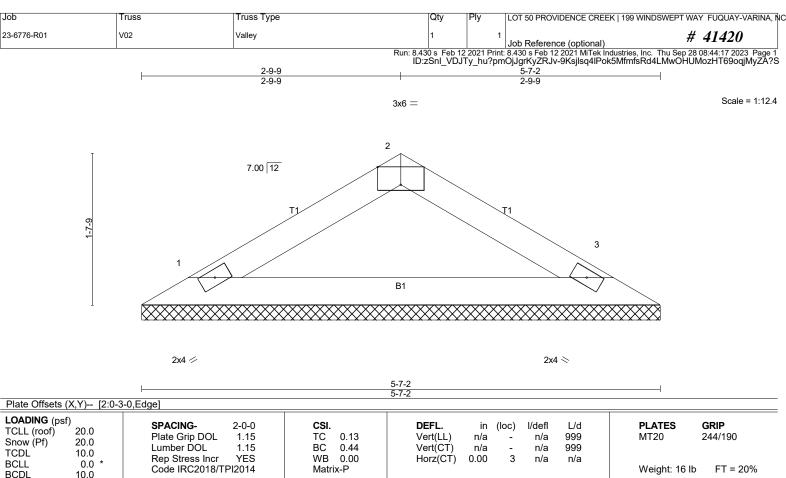
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=22ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



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LUMBER-TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-7-2 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

**REACTIONS.** (lb/size) 1=181/5-7-2 (min. 0-1-8), 3=181/5-7-2 (min. 0-1-8)

Max Horz 1=26(LC 13)

Max Uplift1=-13(LC 14), 3=-13(LC 15) Max Grav 1=206(LC 20), 3=206(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=22ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

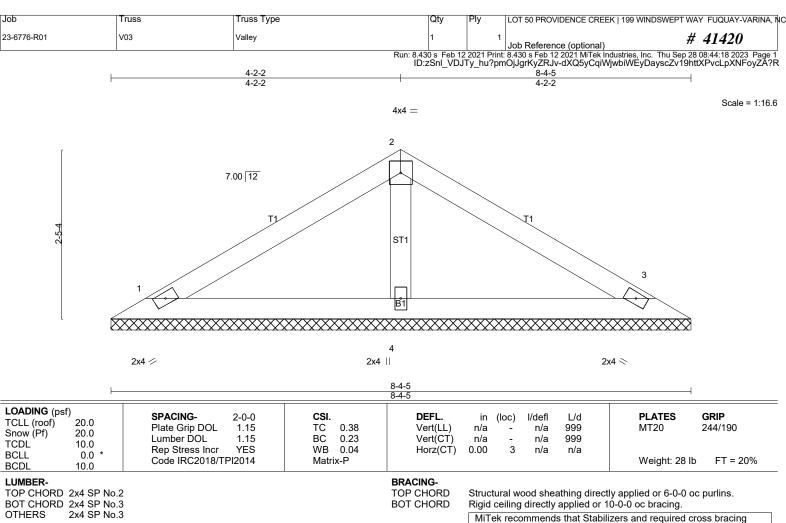
LOAD CASE(S) Standard

SEAL 28147

NONEER K. MORRIMAN

9/26/2023

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be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 1=152/8-4-5 (min. 0-1-8), 3=152/8-4-5 (min. 0-1-8), 4=279/8-4-5 (min. 0-1-8)

Max Horz 1=-41(LC 10)

Max Uplift1=-25(LC 14), 3=-30(LC 15)

Max Grav 1=208(LC 20), 3=208(LC 21), 4=283(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

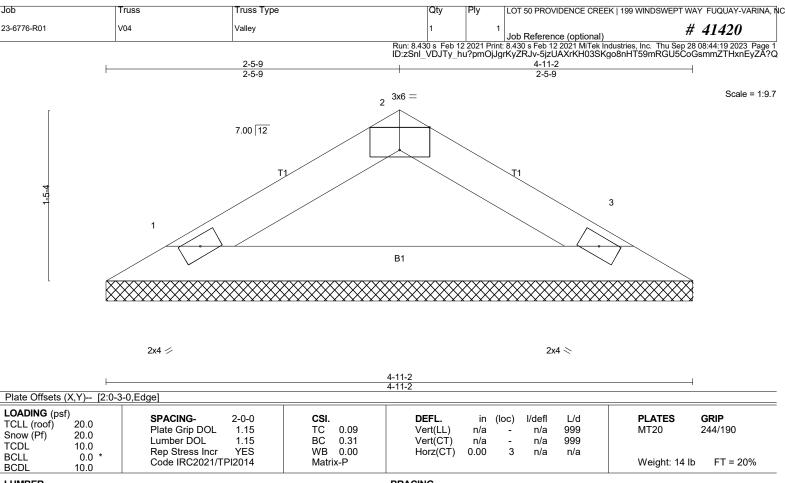
### (10)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=22ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard





LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-11-2 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 1=154/4-11-2 (min. 0-1-8), 3=154/4-11-2 (min. 0-1-8)

Max Horz 1=-22(LC 10) Max Uplift1=-12(LC 14), 3=-12(LC 15) Max Grav 1=173(LC 20), 3=173(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that
- the member must be braced. 10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the
- structural design of the truss to support the loads indicated. 11) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing
- 12) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMEND 🖹 MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

SEAL 28147

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