PLANS DESIGNED TO THE **2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE**

MEAN ROOF HEIGHT: 19'-9	MEAN ROOF HEIGHT: 19'-9"					
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A			
FENESTRATION U-FACTOR	0.35	0.35	0.35			
SKYLIGHT U-FACTOR	0.55	0.55	0.55			
GLAZED FENESTRATION SHGC	0.30	0.30	0.30			
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci			
WALL R-VALUE	15	15	19			
FLOOR R-VALUE	19	19	30			
* BASEMENT WALL R-VALUE	5/13	10/15	10/15			
** SLAB R-VALUE	0	10	10			
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19			

* "10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION ** INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF FOOTING; INSULATION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL

DESIGNED FOR WIND SPEED OF 120 MPH, 3 SECOND GUST (93 FASTEST MILE) EXPOSURE "B"									
COMPONENT	% CLA	DDING	DESIG	NED FO	R THE		WING	LOADS	
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35"-1"	TO 40'	40'-1"	TO 45'	
ZONE 1	14.2	-15.0	14.9	-15.8		-16.4		-16.8	
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2	
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2	
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	17.4	17.4	17.9	
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	21.8	17.4	22.4	
DESIGNED FOR WIND SPEED OF 130 MPH. 3 SECOND GUST (101 FASTEST MILE) EXPOSURE 'B"									
DESIGNED FOR WIN	D SPEED	OF 130 MF	H, 3 SECO	OND GUST	(101 FAS	TEST MILI	E) EXPOSL	IRE 'B"	
DESIGNED FOR WIN									
	& CLA		DESIG 30'-1"	NED FO TO 35'	R THE 35'-1"		WING		
COMPONENT	& CLA	DDING	DESIG 30'-1"	NED FO TO 35' -18.9	35'-1" 18.2	FOLLO TO 40'	WING 40'-1" 18.7	LOADS TO 45' -20.2	
COMPONENT MEAN ROOF	& CLA	DDING O 30'	DESIG 30'-1"	NED FO TO 35'	35'-1" 18.2	FOLLO TO 40'	WING 40'-1" 18.7	LOADS TO 45' -20.2	
COMPONENT MEAN ROOF ZONE 1	& CLA UP T 16.7 16.7 16.7	DDING O 30' -18.0	DESIG 30'-1" 17-5	NED FO TO 35' -18.9 -22.1 -22.1	35' 1" 18.2 18.2 18.2	FOLLO TO 40' -19.6 -22.9 -22.9	WING 40'-1" 18.7 18.7 18.7	LOADS TO 45' -20.2 -23.5 -23.5	
COMPONENT MEAN ROOF ZONE 1 ZONE 2	& CLA UP T 16.7 16.7	DDING O 30' -18.0 -21.0	DESIG 30'-1" 17.5 17.5	NED FO TO 35' -18.9 -22.1	35'-1" 18.2 18.2	FOLLO TO 40' -19.6 -22.9	WING 40'-1" 18.7 18.7	TO 45' -20.2 -23.5	

GUARD RAIL NOTES

SECTION R312

R312.1 Where required. Guards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or *grade* below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a guard.

screening shall not be considered as a guard.

R312, Pleight, Required guards at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads. Exceptions:

1. Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the

Where the top of the guard also serves as a handrail on the open sides of stairs, the top of the quard shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

R312.3 Opening limitations. Required quards shall not have openings from the walking surface to the required *guard* height which allow passage of a sphere 4 inches (102 mm)in diameter.

Exceptions:

 The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153

2. Guards on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter.

ROOF VENTILATION

SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,477 SQ.FT. NET EREE CROSS VENTUATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 16.51 SQ FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 8.26 SQ.FT.

94 HILLWOOD DR, SANFORD, NC

LOT 48 WEST POINTE PHASE III





COMPOSITION SPECIFIED4 SIDING AS

WINDOWS WITH SIDE LOAD **REAR ELEVATION** SCALE 1/8" = 1'-0"

SCALE 1/8" = 1'-0"



RIGHT SIDE ELEVATION

SCALE 1/8" = 1'-0"

SOUARE FOOTAGE HEATED

FIRST FLOOR TOTAL 1791 SQ FT. 1791 SQ FT.

HEATED OPTIONAL CAROLINA ROOM

UNHEATED

188 SQ FT. 469 SQ FT. 657 SQ FT. FRONT PORCH

148 SQ FT.

UNHEATED OPTIONAL

SCREENED PORCH DECK OR PATIO 160 SQ FT. 108 SQ FT. 292 SQ FT 560 SQ FT THIRD GARAGE

AIR LEAKAGE

Section N1102.4

N1102.4.1 Building thermal envelope. The building thermal envelope shall be durably sealed with an air barrier system to limit infiltration. The sealing methods between dissimilar materials shall allow for differential expansion and contraction. For all homes, where present, the following shall be caulked, gasketed, weather stripped or otherwise sealed with an air barrier material or solid material consistent with Appendix E-2.4 of this code: 1. Blocking and sealing floor/ceiling systems and under knee walls

open to unconditioned or exterior space.

2. Capping and sealing shafts or chases, including flue shafts.

SQUARE FOOTAGE HEATED HEATED OPTIONAL UNHEATED JNHEATED OPTIONAL

> Haynes Home Plans, Inc. 4/29/2020

PAGE 1 OF 6

IMENSIONS AND CONDITION FORE CONSTRUCTION BEGIN

EFORE CONSTRUCTION BEGINS HAVINES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND PROCEDURES.

CDDES AND CONDITIONS MAY WARY WITH LOCATION, A LOCAL DESIGNER, ANOTIFECT OR MOINTERS SHOULD BE CONSULTE BEFORE CONSTRUCTION, THESE DRAWING ARE NISTUMENTS OF SERVICE AND AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER.

OPERTY OF THE DESIGNER

Ħ

The Lauren

ELEVATION

WINDOW HEIGHT

9'-1 1/2"

FIRST FLOOR PLATE

© Copyright 2020

200220B



Harnett

LEFT SIDE ELEVATION SCALE 1/8" = 1'-0'

RIDGE VENT AS REQUIRED

OPTIONAL SIDE LOAD

GARAGE DOOR

COMPOSITION SHINGLES AS

VENEER AS SPECIFIED

PURCHASER MUST VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE CONSTRUCTION BEGINS, HAYNES HOME PLANS, INC. ASSUMES NO LLABILITY FOR CONTRACTORS PRACTICES AND PROCEDURES, CODES AND CONDITIONS MAY

PROCEDURES.
CODES AND CONDITIONS MAY
VARY WITH LOCATION A LOCAL
DESIGNER, ARCHITECT OR
KRIMERS HOULD BE CONSULTEL
BEFORE CONSTRUCTION
THESE DERWING ARE
INSTRUMENTS OF SERVICE AND
AS SUCH SHALL REMAIN
PROPERTY OF THE DESIGNER.

SERVICE / SERVICE / RUCH SHALL REMAIN RTY OF THE DESIGNI

MONOLITHIC SLAB PLAN
The Lauren III

WEAVER

HOMES

910.630.2100 • 919.606.4696

SQUARE FOOTAGE
HEARED

SQUARE FOOTAGE
HEATED
FIRST FLOOR 1791 SQ.FF.
TOTAL 189 SQ.FF.
TOTAL

© Copyright 2020 Haynes Home Plans, Inc. 4/29/2020 200220B

200220B PAGE 2 OF 6

EFFORE CONSTRUCTION BEGIN HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AN PROCEDURES. CODES AND CONDITIONS MAY VARY WITH LOCATION, A LOCA DESIGNER, ARCHITECT OR VOINCER SHOULD BE CONSULT BEFORE CONSTRUCTION THESE DRAWING ARE

SQUARE FOOTAGE HEATED FIRST FLOOR 1791 SO.F

© Copyright 2020 Haynes Home Plans, Inc. 4/29/2020

STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans,

Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		-
Guardrail in-fill components	50		-
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless

-- L/360

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fy=285 PSI, E=1.9x106 PSI Paralel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist routs shall be prepared in accordance with this document.
usses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leq vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing

**Sp8* thick for 19.2" on center jost spacing, minimum.

5/8" thick for 19.2" on center jost spacing, and minimum 3/4" iminimum 3/4" per locater also read 6.5 crews. Minimum 3/4" per locater also read 6.5 c minimum 1/2" thick for 16" on center joist spacing, minimum

CONCRETE AND SOILS: See foundation notes.

ROOF TRUSS REQUIREMENTS

(2) 2 X 12

2 JACKS EACH END

(2) SC

TRUSS DESIGN, Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.
KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or insulation. If for any reason the truss manufacturer rais to meet or exceed designated heel helghts, finished knee wall helghts, or finished ceiling helghts shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing

shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

REQUIRED LENGTH OF BRACING: Required brace wall length

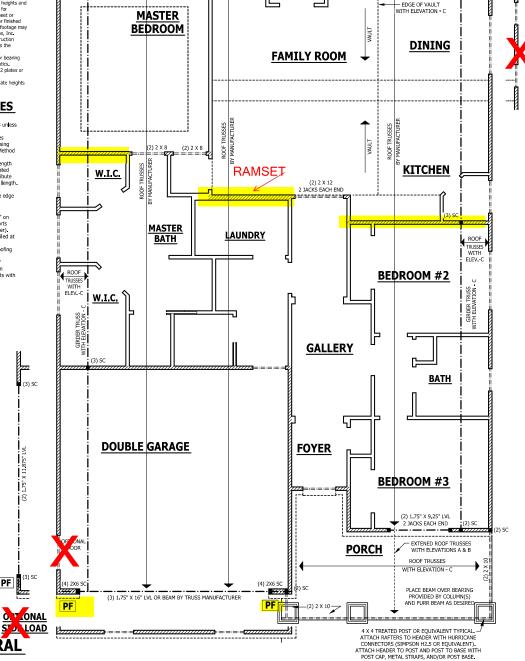
for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length.

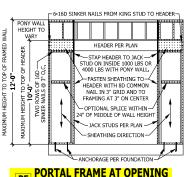
HD: 800 lbs hold down hold down device fastened to the edge of the brace wall panel closets to the corner.

Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter). CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with





(METHOD PF PER FIGURE AND SECTION R602.10.1)

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END - KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END **UNLESS NOTED OTHERWISE** - NON LOAD BEARING HEADERS TO BE

FIRST FLOOR STRUCTURAL

PF

SCALE 1/4" = 1'-0"

FORE CONSTRUCTION BEGI HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR ONTRACTORS PRACTICES AT PROCEDURES. ODES AND CONDITIONS N

(2) 2 X 12

2 JACKS EACH END

STRUCTURAL Ħ Lauren FLOOR The **FIRST**

SQUARE FOOTAGE HEATED FIRST FLOOR 1791 SO.F HEATED OPTIONAL UNHEATED UNHEATED OPTIONAL

© Copyright 2020 Haynes Home Plans, Inc. 4/29/2020 200220B

PAGE 4 OF 6

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to haynes Home Plan, Inc. attention before construction begins.

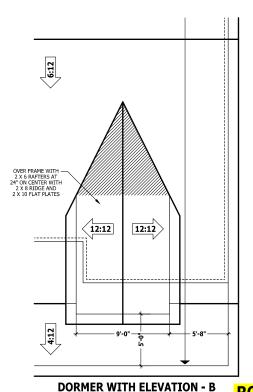
KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and celling heights are shown furred down 10° from roof decking for insulation. If for any reason the truss manufacturer falls to meet or exceed designated heel heights, finished knee wall heights, or finished exceed designated heel heights, finished clines wall heights, or finished clining heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached hefore construction begins. Any variation due to these conditions not being met is the reasonability of the truss manification. ANCHORAGE. All required anchors for trusses due to upfilt or bearing shall meet the requirements as specified on the truss schematics. BEARING, All trusses shall be designed for bearing on SPF #2 plates or

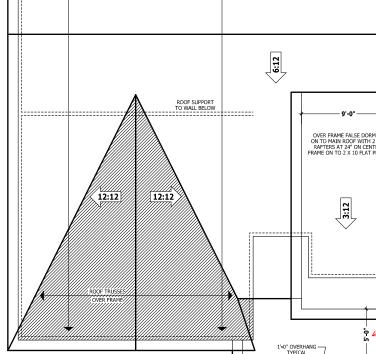
ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

HEEL HEIGHT ABOVE FIRST FLOOR PLATE

HEEL HEIGHT ABOVE SECOND FLOOR PLATE





BEAR TRUSSES ON WALL BELOW

ROOF PLAN WITH ELEVATIONS - A & B

BOXED UP TREY

IN MASTER

ROOF SUPPORT TO WALL BELOW

(E17)

DORMER WITH ELEVATION - A

60" KNEE WALL **HEIGHT**

HEATED
FIRST FLOOR 1791 SQ.FT
TOTAL 1791 SQ.FT
HEATED OPTIONAL
CAROLINA ROOM 148 SQ.FT
TOTAL 148 SQ.FT UNHEATED UNHEATED OPTIONAL

SQUARE FOOTAGE HEATED FIRST FLOOR 1791 SO,FT

MMENSIONS AND CONDITIONS
FORE CONSTRUCTION BEGINS
HAYNES HOME PLANS, INC.
ASSUMES NO LIABILITY FOR
CONTRACTORS PRACTICES AND
PROCEDURES.

PROCEDURES,
AND CONDITIONS MAY
ARY WITH LOCATION, A LOCAL
DESIGNER, ARCHITECT OR
GISHERS HOULD BE CONSULTE
BEFORE CONSTRUCTION,
THESE DRAWING ARE
STRUMENTS OF SERVICE AN
AS SUCH SHALL REMAIN
PROPERTY OF THE DESIGNER.

8

త

ROOF PLAN HELEVATIONS - A 8

WITH

The Lauren III

Haynes Home Plans, Inc. 4/29/2020

200220B

PAGE 5 OF 6

BEAR TRUSSES ON WALL BELOW

FRAME ON TO 2 X 10 FLAT PLATE

OVER FRAME FALSE DORMER ON TO MAIN ROOF WITH 2 X 8 RAFTERS AT 24" ON CENTER

CATHEDRAL CEILING IN

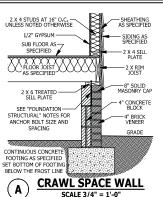
FAMILY ROOM,

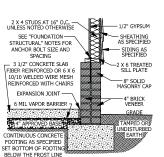
KITCHEN, AND DINING

4:12

CENTER WINDOW

© Copyright 2020





GARAGE STEM WALL

SCALE 3/4" = 1'-0' **DECK STAIR NOTES**

SECTION AM110

AM110.1 Stairs shall be constructed per Figure AM110. Stringer spans shall be no greater than 7 foot span between

supports. Spacing between stringers shall be based upon decking material used per AM107.1. Each Stringer shall have minimum 3 1/2 inches between step cut and back of stringer. If used, suspended headers shall shall be attached with 3/8 inch galvanized bolts with nuts and washers to securely support stringers at the top.

DECK BRACING

SECTION AM100

D

AM109.1 Deck bracing. Decks shall be braced to provide lateral stability. The following are acceptable means to provide lateral stability.

AM109.1.1. When the deck floor height is less than 4'-0"

above finished grade per Figure AM109 and the deck is attached to the structure in accordance with Section AM104, lateral bracing is not required.

AM109.1.2, 4 x 4 wood knee braces may be provided on

each column in both directions. The knee braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be angled between 45 degrees and 60 degrees from the horizontal. Knee braces shall be bolted to the post and the girder/double band with one 5/8 inch hot dipped galvanized bolt with nut and washer at both ends of the

AM109.1.3. For freestanding decks without knee braces or AS SPECIFIED diagonal bracing, lateral stability may be provided by mbedding the post in accordance with Figure AM109.2 nd the following:

POST SIZE	MĀX TRIBUTARY ĀRĒĀ	MAX, POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER				
4 X 4	48 SF	4'-0"	2'-6"	1'-0"				
6 X 6	120 SF	6'-0"	3'-6"	1'-8"				

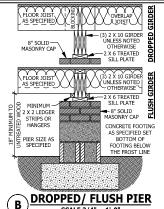
be provided in two perpendicular directions for freestanding decks or parallel to the structure at the exterior column line for attached decks. The 2 x 6's shall be attached to the posts with one 5/8 inch hot dipped galvanized bolt with nut and washer at each end of each bracing member per Figure AM109.3. AM109.1.5. For embedment of piles in Coastal Regions,

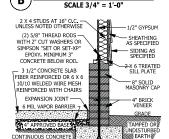
WEEP SCREED SCALE 3/4" = 1'-0"

SHEATHING

SEE FOUNDATION

FOR FOUNDATION





SET BOTTOM OF FOOT BELOW THE EROST LIN <48" GARAGE WING WALL Ε SCALE 3/4" = 1'-0"

CONTINUOUS CONCRET

FOOTING AS SPECIFIED

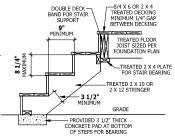


FIGURE AM110 TYPICAL DECK STAIR DETAIL

SCALE 3/4" = 1'-0'

STONE VEENER

AS SPECIFIED

APOR BARRIER

WEEP SCREED

MINIMUM 4" TO

GROUND OR 2'

TO PAVEMENT

GRADE

WEEP SCREEDS

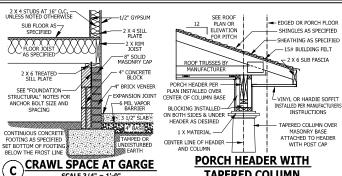
All weep screeds and stone veneer to be installed per manufactures instructions and per the 2012 North Carolina Residential Building code.

the bedrooms.

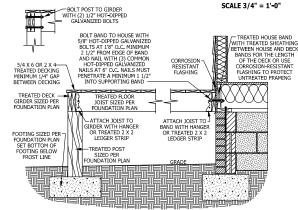
3. On each additional *story* of the *dwelling*, including *basements* and habitable attics (finished) but not including crawl spaces, R703.6.2.1 - A minimum 0.019-inch (0.5 mm) (No. 26 galvanized sheet gage), corrosion-resistant weep screed or plastic weep screed, with a minimum vertical uninhabitable (unfinished) attics and uninhabitable (unfinished) attic-stories. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full *story* attachment flange of 31/2 inches (89 mm) shall be provided at or below the foundation plate line on exterior stud walls below the upper level. in accordance with ASTM C 926. The weep When more than one smoke alarm is required to be installed within screed shall be placed a minimum of 4 an individual diveiling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit.

R314.4 Power source. Smoke alarms shall receive their primary inches (102 mm) above the earth or 2

inches (51 mm) above paved areas and shall be of a type that will allow trapped power from the building wiring when such wiring is served from a water to drain to the exterior of the building. The weather-resistant barrier shall commercial source, and when primary power is interrupted, shall receive nower from a hattery. Wiring shall be permanent and ap the attachment flange. The exterior lath without a disconnecting switch other than those required for overcurrent protection. Smoke alarms shall be interconnected. shall cover and terminate on the attachment flange of the weep screed



TAPERED COLUMN



SCALE 3/4" = 1'-0"

SMOKE ALARMS

equipment provisions of NFPA 72.

listed in accordance with UL 217 and installed in accordance with

R314.2 Smoke detection systems. Household fire alarm systems

R314,2 Smoke detection systems, Household ner alarm systems installed in accordance with NFPA 72 that include smoke alarms, or a combination of smoke detector and audible notification device installed as required by this section for smoke alarms, shall be

permitted. The household fire alarm system shall provide the same

permitted, the nousement interests at the state of the state of smoke detection and alarm as required by this section for smoke alarms. Where a household fire warning system is installed using a combination of smoke detector and audible notification device(s), it shall become a permanent future of the coupancy and owned by the homeowner. The system shall be monitored by an

approved supervising station and be maintained in accordance with

requirements of Section R314.4.

R314.3 Location. Smoke alarms shall be installed in the following

2. Outside each separate sleeping area in the immediate vicinity of

Exception: Where smoke alarms are provided meeting the

the provisions of this code and the household fire warning

SECTION R314

NEPA 72

1. In each sleeping room

DECK ATTACHMENT DETAIL TO FRAMED WALL

SCALE 3/4" TO 1'-0"

STAIRWAY NOTES

R311.7.2 Headroom. The minimum headroom in all parts of the stairway shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the landing or platform on that portion of the stairway.

landing of placorm of that proton of the statistics **ST311.7 A State treads and risers**. Stair treads and risers shall meet the requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners. R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches (210 mm). The riser shall be measured vertically between leading edges of

R311.7.4.2 Tread depth. The minimum tread depth shall be 9 inches (229 mm). The tread depth shall be measured horizontally between the vertical the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a minimum tread depth of 4 inches (102 mm) at any point.

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greate than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stainways with solid

R311.7.7 Handrails. Handrails shall be provided on at least one side of each

restart. It manufacts, naturals statu be provided on at least one side of ear continuous run of treads or flight with four or more risers.

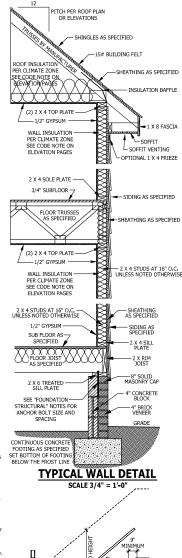
R311.7.1 Height. Handrall height, measured vertically from the sloped plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm).

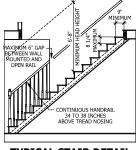
The use of a volute, turnout or starting easing shall be allowed over the

transition between flights, the transition from handrail to quardrail, or used

full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall to a point directly above the lowest reset of the injurit, randical relias shall be returned or shall terminate in newel posts or safety terminals. Handrails adjacent to a wall shall have a space of not less than 11/2 inch (38 mm) between the wall and the handrails.

Two or more separate rails shall be considered continuous if the





TYPICAL STAIR DETAIL

lowest tread.

When handrail fittings or bendings are used to provide continuous. at the start of a flight, the handrall height at the fittings or bendings shall be permitted to exceed the maximum height. R311.7.7.2 Continuity. Handralls for stairways shall be continuous for the

Exceptions:

Handrails shall be permitted to be interrupted by a newel nest 2. The use of a volute, turnout, starting easing or starting newel shall be

allowed over the lowest tread. termination of the rails occurs within 6 inches (152 mm) of each other. It transitioning between a wall-mounted handrail and a guardrail/handrail, the wall-mounted rail must return into the wallSQUARE FOOTAGE HEATED HEATED OPTIONAL UNHEATED

FORE CONSTRUCTION BEGIN HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR ONTRACTORS PRACTICES AN PROCEDURES.

CODES AND CONDITIONS MAY WARY WITH LOCATION, A LOCA DESIGNER, ARCHITECT OR IGINEER SHOULD BE CONSULT REFORE CONSTRUCTION

Ħ

Lauren

The

Ē

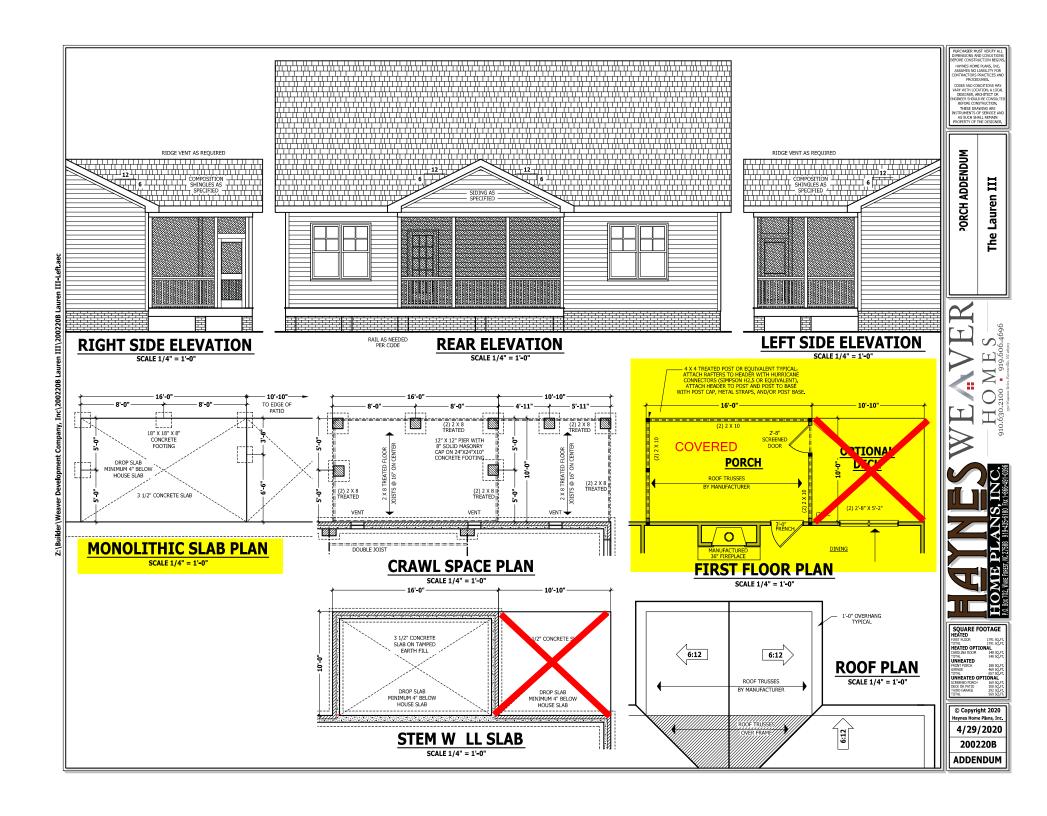
ᆸ

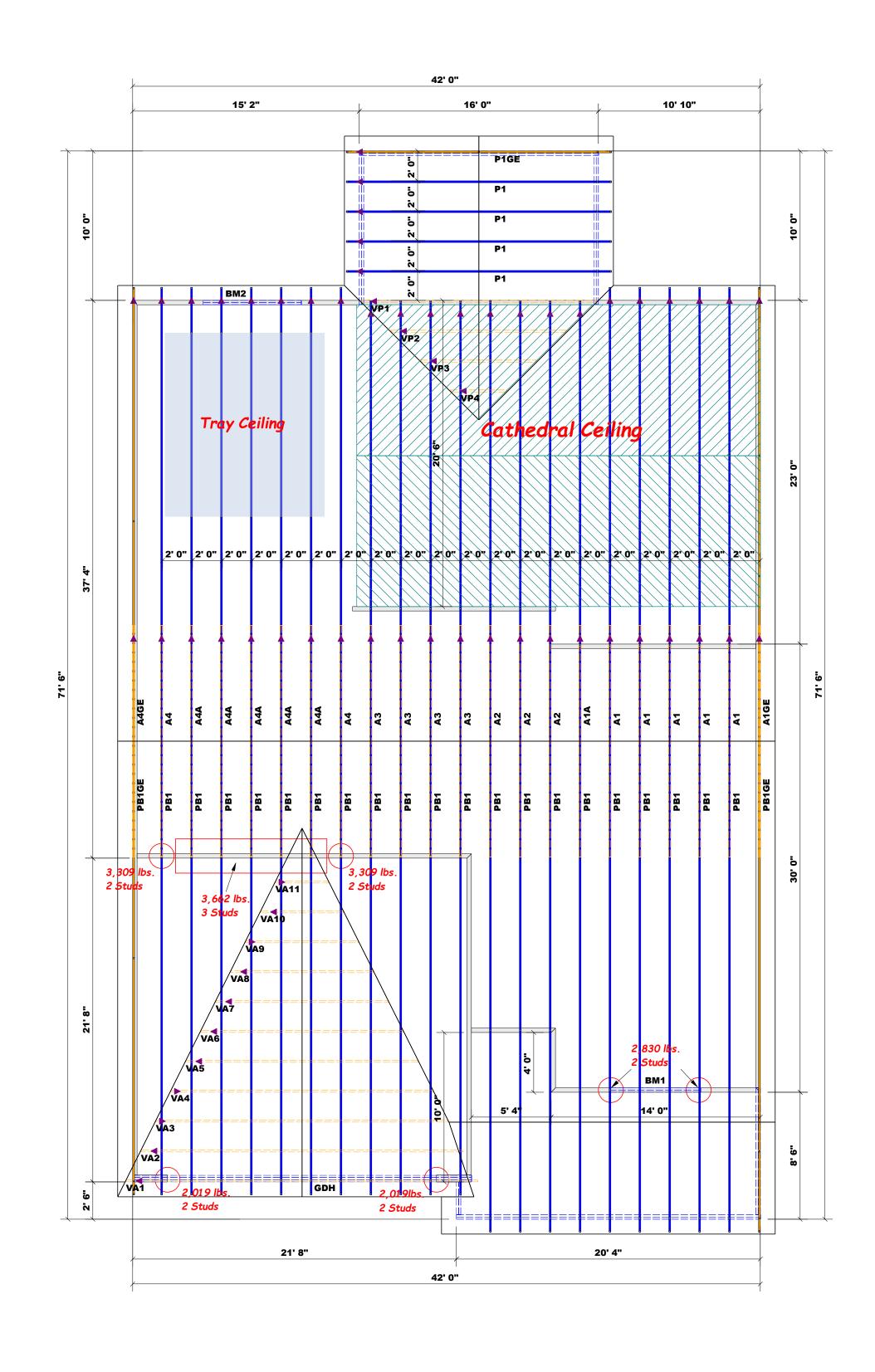
LYPICAL

UNHEATED OPTIONAL © Copyright 2020 Haynes Home Plans, Inc

4/29/2020 200220B

PAGE 6 OF 6





▲ = Denotes Left End of Truss
 (Reference Engineered Truss Drawing)
 Do Not Erect Trusses Backwards

6800 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

5100 2

7650 3

10200 4

12750 5

15300 6

1700 1 3400 2

5100 3

6800 4 8500 5

10200 6

11900 7 13600 8 15300 9 All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

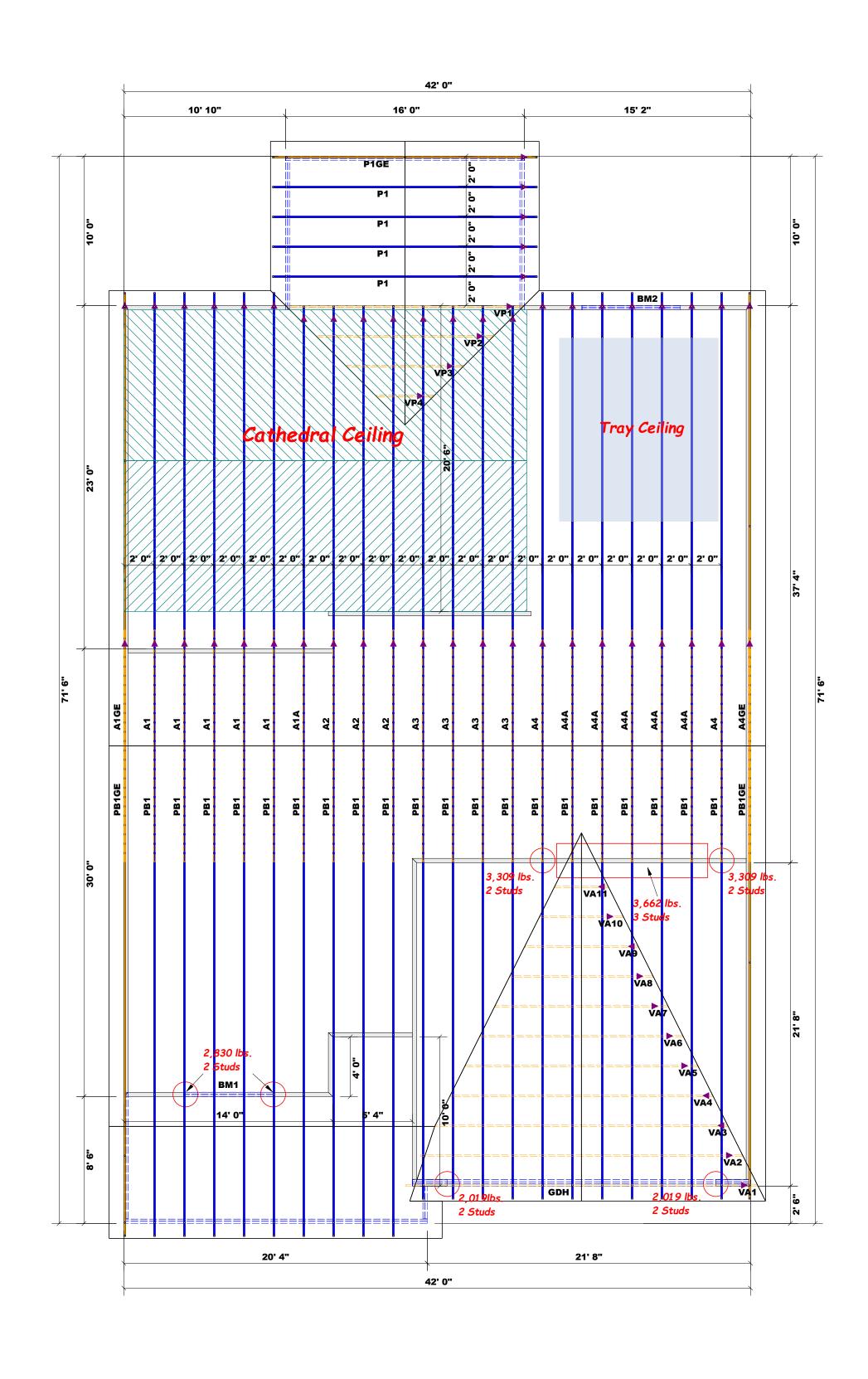
Beam Legend Fab Type PlotID Length Product Plies Net Qty 1-3/4"x 9-1/4" LVL Kerto-S 7' 0" FF BM1 7' 0" FF BM2 2 2 1-3/4"x 9-1/4" LVL Kerto-S GDH 23' 0" 1-3/4"x 16" LVL Kerto-S 3 3 FF

55	es Backwards			SCALE: 3/16" = 1' GDH 23	0" 1-3/4"x 16" LVL Kerto-S 3
	BUILDER	Weaver Development	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer
	JOB NAME	Lot 48 West Pointe	ADDRESS	Lot 48 West Pointe	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package
	PLAN	Lauren III / Elev. A / CP	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables
	SEAL DATE	4/29/20	DATE REV.	09/11/23	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those
	QUOTE#	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.
	JOB#	J0923-5025	SALESMAN	Lenny Norris	Signature Curtis Quick

Truss Placement Plan



Fax: (910) 864-4444



▲ = Denotes Left End of Truss
 (Reference Engineered Truss Drawing)
 Do Not Erect Trusses Backwards

3400 1 6800 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

5100 2 7650 3

10200 4

12750 5

15300 6

10200 6

11900 7 13600 8 15300 9 All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Beam Legend Fab Type PlotID Length Product Plies Net Qty 1-3/4"x 9-1/4" LVL Kerto-S 7' 0" FF BM1 7' 0" FF BM2 2 2 1-3/4"x 9-1/4" LVL Kerto-S GDH 23' 0" 1-3/4"x 16" LVL Kerto-S 3 3 FF

S	es Backwards			SCALE: 3/16" = 1' GDH 23	'0" 1-3/4"x 16" LVL Kerto-S 3
	BUILDER	Weaver Development	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer
	JOB NAME	Lot 48 West Pointe	ADDRESS	Lot 48 West Pointe	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package
	PLAN	Lauren III / Elev. A / CP	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables
	SEAL DATE	4/29/20	DATE REV.	09/11/23	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those
	QUOTE#	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.
	JOB#	J0923-5025	SALESMAN	Lenny Norris	SignatureCurtis Quick

Truss Placement Plan



Phone: (910) 864-8787

Fax: (910) 864-4444



Client: Weaver Development

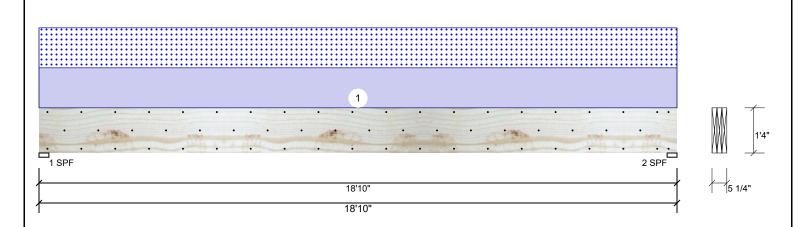
Project: Address: Date:

9/18/2023 Input by: Curtis Quick Job Name: The Lauren III Beams Page 1 of 6

Project #:

1.750" X 16.000" **Kerto-S LVL** 3-Ply - PASSED **GDH**

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Live Wind Type: Application: Floor Brg Direction Dead Snow Const Plies: 3 Design Method: ASD 0 1127 951 0 Vertical 0 1 Moisture Condition: Dry **Building Code:** IBC 2012 2 Vertical 0 1127 951 0 0 Deflection LL: 480 Load Sharing: Yes Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temp <= 100°F Temperature: **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+S 1 - SPF 3.500" Vert 27% 1127 / 951 2078 L

2 - SPF 3.500"

Vert

27%

1127 / 951

2078 L

D+S

Analysis Results

ĺ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	9334 ft-lb	9'5"	62010 ft-lb	0.151 (15%)	D+S	L
	Unbraced	9334 ft-lb	9'5"	10990 ft-lb	0.849 (85%)	D+S	L
	Shear	1744 lb	17'2 1/2"	20608 lb	0.085 (8%)	D+S	L
	LL Defl inch	0.078 (L/2813)	9'5 1/16"	0.460 (L/480)	0.171 (17%)	S	L
	TL Defl inch	0.171 (L/1288)	9'5 1/16"	0.613 (L/360)	0.280 (28%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	101 PLF	0 PLF	101 PLF	0 PLF	0 PLF	A4A
	Self Weight				19 PLF					

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 11/3/2024

isDesign

Client: Weaver Development

Project: Address:

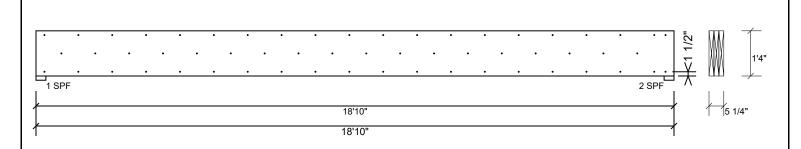
9/18/2023 Input by:

Curtis Quick Job Name: The Lauren III Beams Page 2 of 6

Project #:

Kerto-S LVL 1.750" X 16.000" 3-Ply - PASSED **GDH**

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1 00	

Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







Client: Weaver Development

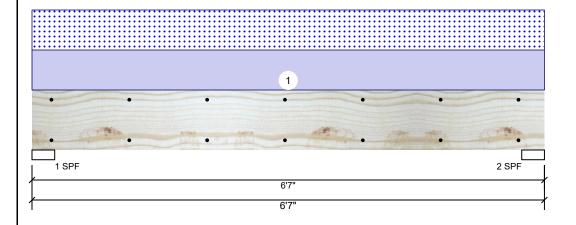
Project: Address: Date: 9/18/2023

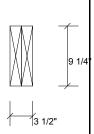
Input by: Curtis Quick Job Name: The Lauren III Beams

Project #:

1.750" X 9.250" 2-Ply - PASSED Kerto-S LVL BM1

Level: Level





Page 3 of 6

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012

Load Sharing: No Deck:

Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1564	1541	0	0
2	Vertical	0	1564	1541	0	0

Bearings

Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	60%	1564 / 1541	3105	L	D+S
2 - SPF	3.500"	Vert	60%	1564 / 1541	3105	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4423 ft-lb	3'3 1/2"	14423 ft-lb	0.307 (31%)	D+S	L
Unbraced	4423 ft-lb	3'3 1/2"	10451 ft-lb	0.423 (42%)	D+S	L
Shear	2108 lb	1' 3/4"	7943 lb	0.265 (27%)	D+S	L
LL Defl inch	0.040 (L/1842)	3'3 1/2"	0.153 (L/480)	0.261 (26%)	S	L
TL Defl inch	0.080 (L/914)	3'3 1/2"	0.204 (L/360)	0.394 (39%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	468 PLF	0 PLF	468 PLF	0 PLF	0 PLF	A1
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



isDesign

Client: Weaver Development

Project: Address: Date: 9/18/2023

Input by: Curtis Quick Job Name: The Lauren III Beams

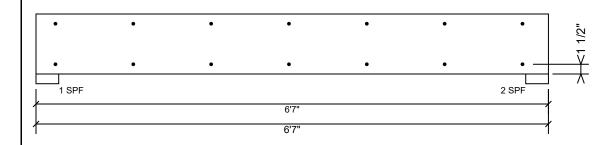
Project #:

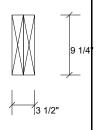
Kerto-S LVL BM1

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 4 of 6

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 11/3/2024



Client: Weaver Development

Project: Address:

Date: 9/18/2023 Input by:

Curtis Quick Job Name: The Lauren III Beams

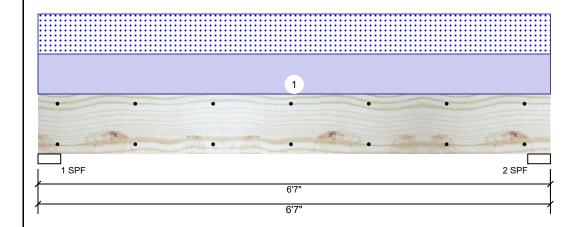
Project #:

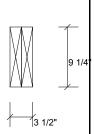
Kerto-S LVL BM₂

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 5 of 6

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No

Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Direction	Live	Dead	Snow	Wind	Const
Vertical	0	955	932	0	0
Vertical	0	955	932	0	0
	Vertical	Vertical 0	Vertical 0 955	Vertical 0 955 932	Vertical 0 955 932 0

Bearings

Bearing	Length	Dir.	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	36%	955 / 932	1887	L	D+S
2 - SPF	3.500"	Vert	36%	955 / 932	1887	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2688 ft-lb	3'3 1/2"	14423 ft-lb	0.186 (19%)	D+S	L
Unbraced	2688 ft-lb	3'3 1/2"	10451 ft-lb	0.257 (26%)	D+S	L
Shear	1283 lb	5'6 1/4"	7943 lb	0.162 (16%)	D+S	L
LL Defl inch	0.024 (L/3046)	3'3 1/2"	0.153 (L/480)	0.158 (16%)	S	L
TL Defl inch	0.049 (L/1504)	3'3 1/2"	0.204 (L/360)	0.239 (24%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	283 PLF	0 PLF	283 PLF	0 PLF	0 PLF	A4A
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 2 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 11/3/2024

isDesign

Client: Weaver Development

Project: Address: Date: 9/18/2023 Input by: Curtis Quick

Job Name: The Lauren III Beams

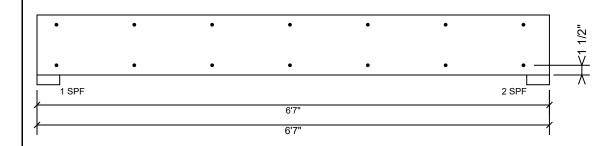
Project #:

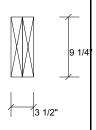
Kerto-S LVL BM₂

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 6 of 6

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

·	
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS

