\*10)(13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION

\* INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF

FOOTING; INSULATION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL DESIGNED FOR WIND SPEED OF 120 MPH, 3 SECOND GUST (93 FASTEST MILE) EXPOSURE "B"

DESIGHED ON WIND SPEED OF 120 MIN, 1 SECOND 6,57 BY DAYSTEN MILE DIVOSIDE: "P COMPONENTS & CLADOTING DESIGNED FOR THE FOLLOWING LOADS MEAN ROOF! UP TO 30° 130°:1"TO 35° 35°:1" TO 40° 40°-1"TO 45° ZONE 1 14.2 15.0 14.9 1-13.8 15.5 1-16.4 15.9 1-16.8 ZONE 2 14.2 15.0 14.9 1-18.9 15.5 1-15.6 15.9 20.2 ZONE 3 14.2 18.0 14.9 1-18.9 15.5 1-16.6 15.9 20.2 ZONE 4 15.5 1-6.0 16.3 1-6.8 16.9 1-7.4 17.4 17.9 ZONE 5 15.5 12.00 16.3 1-10.1 16.9 12.1 17.4 17.4 17.9

FLOOR R-VALUE

\* BASEMENT WALL R-VALUE

Development Company, Inc\200220B Lauren III\200220B Lauren III.aec

10/10/2023

requirements of Section R802.7

continuous soffit vent only.

NET FREE CROSS VENTILATION NEEDED:

SECTION R806

**ROOF VENTILATION** 

R806.1 Ventilation required. Enclosed attics and enclosed rafter spaces

formed where ceilings are applied directly to the underside of roof rafters

shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4

mm) maximum. Ventilation openings having a least dimension larger than 1/4 inch (6.4 mm) shall be provided with corrosion-resistant wire cloth

the function may shall be provided with consider lessage, whe cloth screening, hardware cloth, or similar material with openings having a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Openings in roof framing members shall conform to the

R806.2 Minimum area. The total net free ventilating area shall not be less than 1/150 of the area of the space ventilated except that reduction of the

total area to 1/300 is permitted provided that at least 50 percent and not

ventilators located in the upper portion of the space to be ventilated at least

3 feet (914 mm) above the eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents. As an alternative, the

net free cross-ventilation area may be reduced to 1/300 when a Class I or II

Enclosed attic/rafter spaces requiring less than 1 square foot (0.0929 m2)

of ventilation may be vented with continuous soffit ventilation only.

2. Enclosed attic/rafter spaces over unconditioned space may be vented with

WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 16.51 SQ.FT.

WITH 50% TO 80% OF VENTING 3'-0" ABOVE FAVE: OR WITH CLASS LOR II

more than 80 percent of the required ventilating area is provided by

vapor retarder is installed on the warm-in-winter side of the ceiling.

SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,477 SQ.FT.

\* CRAWL SPACE WALL R-VALUE

127 HILLWOOD DR SANFORD, NC

NOTE: MONO SLAB - STONE TO RUN TO THE BOTTOM OF WINDOW

STEM WALL - STONE TO FOUNDATION HEIGHT ONLY

WEST POINTE III LOT 6

TOP OF PLATE

SUB FLOOR

WINDOW HEIGHT

9'-1 1/2"

FIRST FLOOR PLATE H

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**ELEVATION** The Lauren

SQUARE FOOTAGE HEATED HEATED OPTIONAL 148 SQ FI 148 SQ FI UNHEATED

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PAGE 1 OF 6

COMPOSITION
SHINGLES AS
SPECIFIED

12 (3) 2'-0" X 3'-0" | FIXED SIDING A

Harnett

10 10 10/15 10/19

STEM WALI

SECTION R312

Exceptions:

the leading edges of the treads.

inches (102 mm)in diameter.

**GUARD RAIL NOTES** 

R312.1 Where required. Guards shall be located along open-sided walking

surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or *grade* below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect expension shift has be openinged as a gradual process.

screening shall not be considered as a *guard*. **R312.2 Height.** Required *guards* at open-sided walking surfaces, including

stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads.

Guards on the open sides of stairs shall have a height not less than 34 inches

(864 mm) measured vertically from a line connecting the leading edges of the

stairs, the top of the *guard* shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting

R312.3 Opening limitations. Required *guards* shall not have openings from the walking surface to the required *guard* height which allow passage of a sphere 4

The triangular openings at the open side of a stair, formed by the riser, tread.

and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153

2. Guards on the open sides of stairs shall not have openings which allow

passage of a sphere 4 3/8 inches (111 mm) in diameter

treads.

2. Where the top of the *quard* also serves as a handrail on the open sides of



# **FRONT ELEVATION - A**

SCALE 1/4" = 1'-0"



SCALE 1/8" = 1'-0"



RIDGE VENT AS REQUIRED COMPOSITION COMPOSITION 1 SIDING AS

SPECIFIED VENEER AS SPECIFIED

**RIGHT SIDE ELEVATION** 

LEFT SIDE ELEVATION PARGE

REAR ELEVATION

PARGE

AIR LEAKAGE

Section N111124 GARAGE 292 SQ. FT.
N1102.4.1 Building thermal envelope. The building thermal envelope shall be durably sealed with an air barrier system to limit infiltration. The sealing methods between dissimilar materials shall allow for differential expansion and contraction. For all homes, where present, the following shall be caulked, gasketed, weather stripped or otherwise sealed with an air barrier material or solid material consistent with Appendix E-2.4 of this code:

1. Blocking and sealing floor/ceiling systems and under knee walls open to unconditioned or exterior space.

SOUARE FOOTAGE

HEATED OPTIONAL

**UNHEATED OPTIONAL** 

1791 SQ FT. 1791 SQ FT.

148 SQ FT

188 SO FT

469 SQ FT. 657 SO FT.

160 SQ.FT. 108 SQ.FT.

HEĂTED

FIRST FLOOR

CAROLINA ROOM

UNHEATED

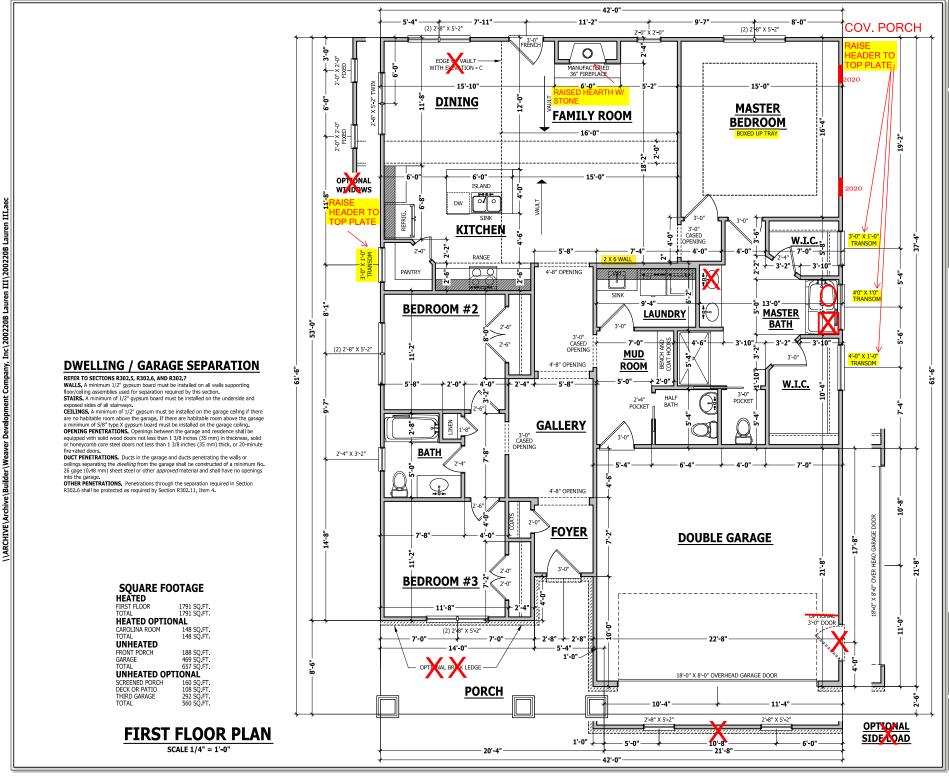
FRONT PORCH

Capping and sealing shafts or chases, including flue shafts.
 Capping and sealing soffit or dropped ceiling areas.

INHEATED OPTIONAL

200220B

**PARGE** 



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FIRST FLOOR PLAN
The Lauren III

WEAVE HOMES— 910.630.2000 • 190.6606.4696

HOME PLANS, INC

SQUARE FOOTAGE
HEATED
FIRST FLOOR
FIRST FL

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PAGE 3 OF 6

42'-4" -

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**Q SLAB** 

Lauren

WALL

EΜ

SQUARE FOOTAGE HEATED FIRST FLOOR 1791 SQ.FT.
TOTAL 1791 SQ.FT. **HEATED OPTIONAL** CAROLINA ROOM TOTAL UNHEATED FRONT PORCH GARAGE UNHEATED OPTIONAL

SCREENED PORCH DECK OR PATIO THIRD GARAGE

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## **STRUCTURAL NOTES**

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION	
USE	(PSF)	(PSF)	(LL)	
Attics without storage	10	10	L/240	
Attics with limited storage	20	10	L/360	
Attics with fixed stairs	40	10	L/360	
Balconies and decks	40	10	L/360	
Fire escapes	40	10	L/360	
Guardrails and handrails	200	-	-	
Guardrail in-fill components	50	-	_	
Passenger vehicle garages	50	10	L/360	
Rooms other than sleeping	40	10	L/360	
Sleeping rooms	30	10	L/360	
Stairs	40	-	L/360	

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

### ENGINEERED WOOD BEAMS

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1,9x106 PSI Paralel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Instal all connections per manufacturers instructions

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise.

FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4"

PF: Portal fame per figure R602.10.1

ROOF SHEATHING: OSB or CDX roof sheathing minimum

3/8" thick.

CONCRETE AND SOILS: See foundation notes.

**ROOF TRUSS REQUIREMENTS** 

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.

KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE, All required anchors for trusses due to uplift or bearing

shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights

### **BRACE WALL PANEL NOTES**

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

REQUIRED LENGTH OF BRACING: Required brace wall length

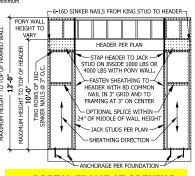
for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length Method PE contributes 1.5 times its actual length

HD: 800 lbs hold down hold down device fastened to the edge of the brace wall panel dosets to the corner.

### Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter). CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws



### **PORTAL FRAME AT OPENING** ( METHOD PF PER FIGURE AND SECTION R602.10.1 ) SCALE 1/4" = 1'-0"

## **EXTERIOR HEADERS**

(2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE - KING STUDS EACH END PER TABLE BELOW

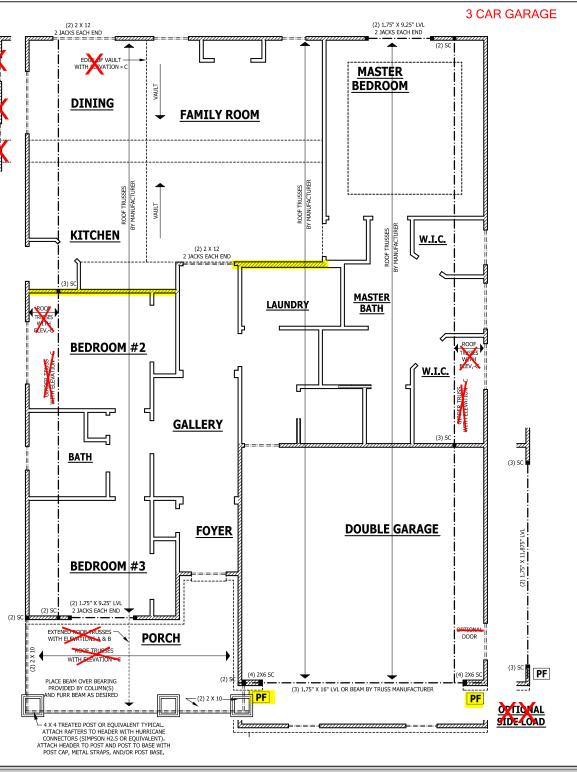
HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16 KING STUD(S) 1 2 3 5 6

# INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE LADDER FRAMED

# FIRST FLOOR STRUCTURAL

SCALE 1/4" = 1'-0'



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The Lauren

STRUCTURAL

FLOOR

**FIRST** 

SQUARE FOOTAGE HEATED

INHEATED OPTIONAL

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200220B

PAGE 4 OF 6

148 SQ.FI 148 SQ.FI

HEATED OPTIONAL

UNHEATED

# **ROOF TRUSS REQUIREMENTS**

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins, KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

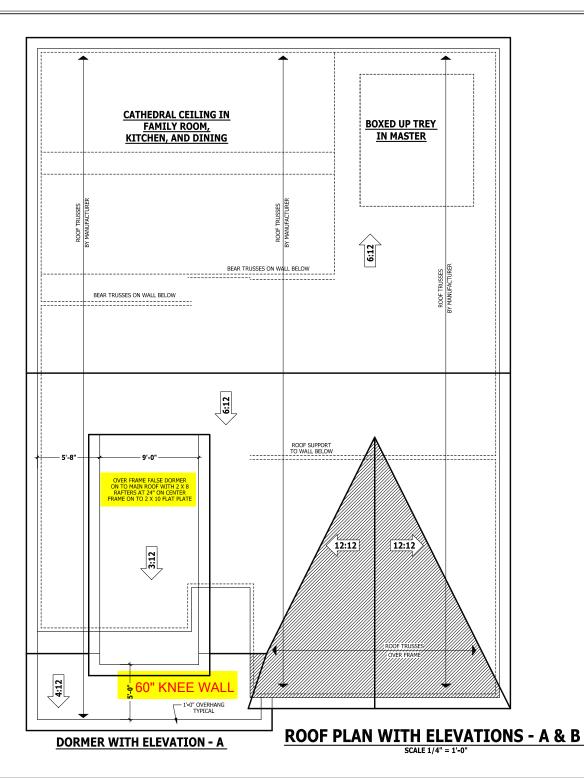
ANCHORAGE, All required anchors for trusses due to uplift or bearing

shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

HEEL HEIGHT ABOVE

HEEL HEIGHT ABOVE SECOND FLOOR PLATE



DIMENSIONS AND CONDITIONS
EFORE CONSTRUCTION BEGIN
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8 య The Lauren III ROOF PLAN

SQUARE FOOTAGE HEATED OTAL UNHEATED INHEATED OPTIONAL

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PAGE 5 OF 6

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Development

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2 X 4 STUDS AT 16" O.C

SHEATHING

AS SPECIFIED

2 X 4 SILL PLATE

2 X RIM JOIST

- 8" SOLID MASONRY CAP

" CONCRETE BLOCK

4" BRICK

GRADE

1/2" GYPSUM

SHEATHING AS SPECIFIED

SIDING AS SPECIFIED

— 8" SOLID MASONRY CAP

4" BRICK VENEER

TAMPED OF

INDISTURBE

S FARTH

# **DECK STAIR NOTES**

AM110.1 Stairs shall be constructed per Figure AM110. Stringer spans shall be no greater than 7 foot span between supports. Spacing between stringers shall be based upon decking material used per AM107.1. Each Stringer shall have

minimum 3 1/2 inches between step cut and back of stringer If used, suspended headers shall shall be attached with 3/8 inch galvanized bolts with nuts and washers to securely support stringers at the top.

### **DECK BRACING**

SECTION AM109

AM109.1 Deck bracing. Decks shall be braced to provide lateral stability. The following are acceptable means to rovide lateral stability.

AM109.1.1. When the deck floor height is less than 4'-0' above finished grade per Figure AM109 and the deck is attached to the structure in accordance with Section AM104, lateral bracing is not required.

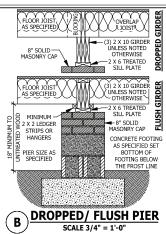
AM109.1.2. 4 x 4 wood knee braces may be provided on each column in both directions. The knee braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be angled between 45 degrees and 60 degrees from the horizontal. Knee braces shall be bolted to the post and the girder/double band with one 5/8 inch hot dipped galvanized bolt with nut and washer at both ends of the brace per Figure AM109.1

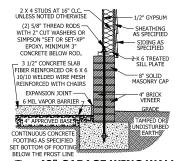
AM109.1.3. For freestanding decks without knee braces or diagonal bracing, lateral stability may be provided by

and the following:							
POST SIZE	MAX TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER			
4 X 4	48 SF	4'-0"	2'-6"	1'-0"			
6 X 6	120 SF	6'-0"	3'-6"	1'-8"			

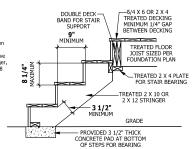
AM109.1.4. 2 x 6 diagonal vertical cross bracing may be provided in two perpendicular directions for freestanding decks or parallel to the structure at the exterior column line for attached decks. The 2 x 6's shall be attached to the posts with one 5/8 inch hot dipped galvanized bolt with nut and washer at each end of each bracing member per Figure AM109 3

AM109.1.5. For embedment of piles in Coastal Regions, see Chapter 45.





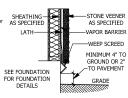
<48" GARAGE WING WALL Ε SCALE 3/4" = 1'-0'



### FIGURE AM110 TYPICAL DECK STAIR DETAIL

SCALE 3/4" = 1'-0"

## WEEP SCREEDS



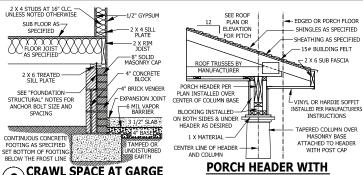
**WEEP SCREED** SCALE 3/4" = 1'-0"

All weep screeds and stone veneer to be installed ner manufactures instructions and per the 2012 North Carolina Residential

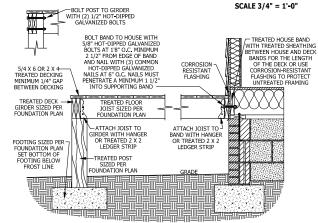
C

Building code. R703.6.2.1 - A minimum 0.019-inch (0.5 mm) (No. 26 galvanized sheet gage), corrosion-resistant weep screed or plastic veep screed, with a minimum vertical attachment flange of 31/2 inches (89 mm) shall be provided at or below the foundation plate line on exterior stud walls in accordance with ASTM C 926. The weep

screed shall be placed a minimum of 4 inches (102 mm) above the earth or 2 inches (51 mm) above paved areas and shall be of a type that will allow trapped water to drain to the exterior of the building. The weather-resistant barrier shal Ian the attachment flance. The exterior lath shall cover and terminate on the attachment flange of the weep screed.



CRAWL SPACE AT GARGE **TAPERED COLUMN** SCALE 3/4" = 1'-0"



# **DECK ATTACHMENT DETAIL TO FRAMED WALL**

SCALE 3/4" TO 1'-0"

**SMOKE ALARMS** 

SECTION R314 R314.1 Smoke detection and notification. All smoke alarms shall be listed in accordance with UL 217 and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72.

R314.2 Smoke detection systems. Household fire alarm systems installed in accordance with NFPA 72 that include smoke alarms, or a combination of smoke detector and audible notification device installed as required by this section for smoke alarms, shall be permitted. The household fire alarm system shall provide the same level of smoke detection and alarm as required by this section for smoke alarms. Where a household fire warning system is installed using a combination of smoke detector and audible notification device(s), it shall become a permanent fixture of the occupancy and owned by the homeowner. The system shall be monitored by an approved supervising station and be maintained in accordance with NEDA 72

Exception: Where smoke alarms are provided meeting the requirements of Section R314.4.

R314.3 Location. Smoke alarms shall be installed in the following

 In each sleeping room. 2. Outside each separate sleeping area in the immediate vicinity of

the bedrooms.

3. On each additional *story* of the *dwelling*, including *basements* and habitable attics (finished) but not including crawl spaces. uninhabitable (unfinished) attics and uninhabitable (unfinished) attic-stories. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full *story* 

below the upper level. When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of

the alarms in the individual unit. **R314.4 Power source.** Smoke alarms shall receive their primary power from the building wiring when such wiring is served from a commercial source, and when primary power is interrupted, shall receive power from a battery. Wiring shall be permanent and without a disconnecting switch other than those required for overcurrent protection. Smoke alarms shall be interconnected.

## STAIRWAY NOTES

shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the landing or platform on that portion of the stainway.

R311.7.4 Stair treads and risers. Stair treads and risers shall meet the requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners. R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches

(210 mm). The riser shall be measured vertically between leading edges of the adiacent treads. P311 7.4.2 Tread denth. The minimum tread denth shall be 9 inches (229 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a

minimum tread depth of 4 inches (102 mm) at any point.

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stairways with solid

R311.7.7 Handrails. Handrails shall be provided on at least one side of each continuous run of treads or flight with four or more risers.

R311,7,7,1 Height, Handrail height, measured vertically from the sloped plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm). Exceptions:

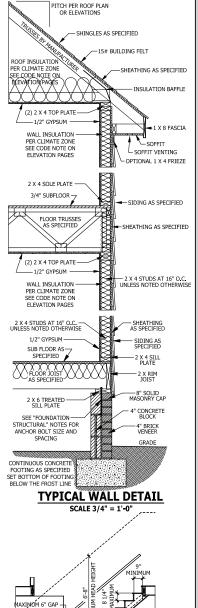
1. The use of a volute, turnout or starting easing shall be allowed over the lowest tread.

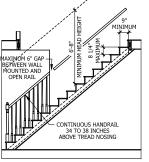
 When handrail fittings or bendings are used to provide continuous transition between flights, the transition from handrall to guardrall, or used at the start of a flight, the handrall height at the fittings or bendings shall be permitted to exceed the maximum height.

R311.7.7.2 Continuity. Handrails for stairways shall be continuous for the full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails adjacent to a wall shall have a space of not less than 11/2 inch (38 mm) between the wall and the handrails.

1. Handrails shall be permitted to be interrupted by a newel post.

The use of a volute, turnout, starting easing or starting newel shall be allowed over the lowest tread. 3. Two or more separate rails shall be considered continuous if the termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the wall-mounted rail must return into the wall.





TYPICAL STAIR DETAIL

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THESE DRAWING ARE ISTRUMENTS OF SERVICE AND AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER.

S DETAIL Ε Lauren TYPICAL The

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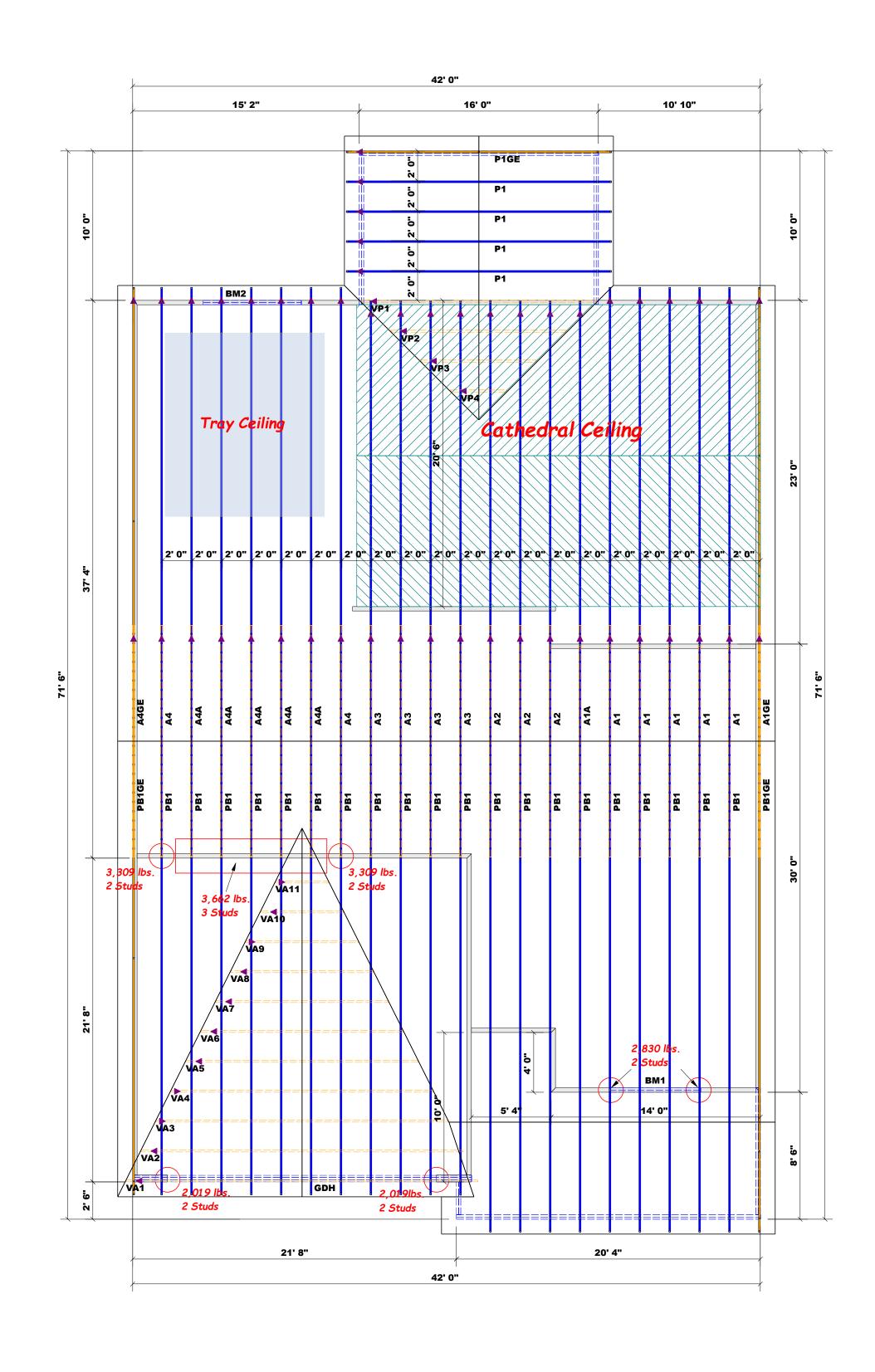
SQUARE FOOTAGE HEATED

HEATED OPTIONAL

Havnes Home Plans, Inc. 2/18/2020

200220B

PAGE 6 OF 6



▲ = Denotes Left End of Truss(Reference Engineered Truss Drawing)Do Not Erect Trusses Backwards

6800 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS PEQUIPED @ EA END OF HEADER/GIRDER

5100 2

7650 3

10200 4

12750 5

15300 6

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

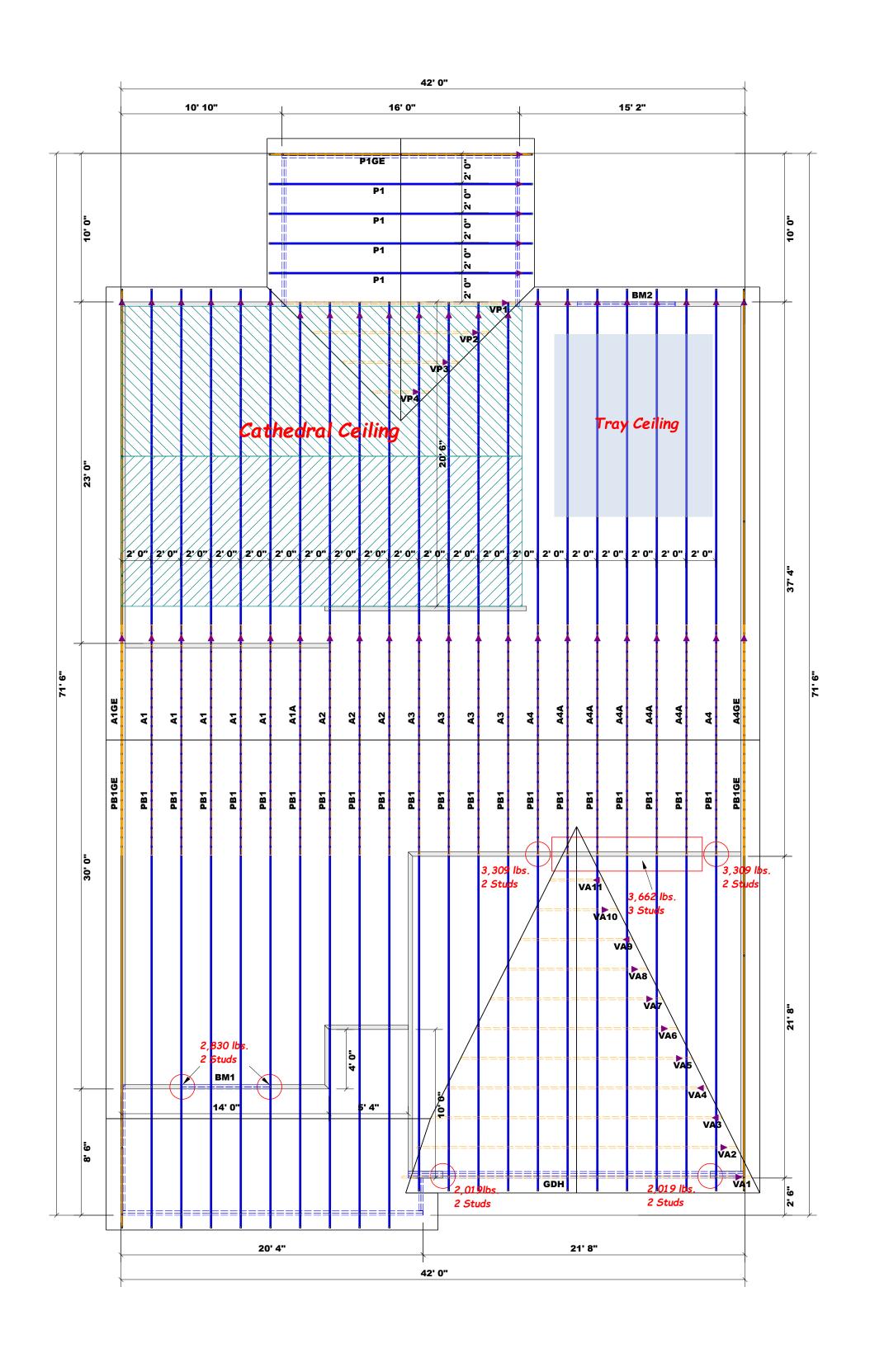
-- Denotes Reaction Greater than 3,000 lbs.

Beam Legend Fab Type PlotID Length Product Plies Net Qty 1-3/4"x 9-1/4" LVL Kerto-S 7' 0" FF BM1 7' 0" FF 2 2 1-3/4"x 9-1/4" LVL Kerto-S BM2 GDH 23' 0" 1-3/4"x 16" LVL Kerto-S 3 3 FF

ses Backwards			Truss Placement Plan SCALE: 3/16" = 1'	GDH	23' 0"	1-3/4"x 9-1/4" LVL Kerto-S 2 1-3/4"x 16" LVL Kerto-S 3			
BUILDER	Weaver Development	Harnett		These the bu	Is A TRUSS PLACEMENT DIAGRAM ONLY.  e trusses are designed as individual building components to be incorporated into uilding design at the specification of the building designer. See individual design for each truss design identified on the placement drawing. The building designer				
JOB NAME	Lot 6 West Pointe	ADDRESS	Lot 6 West Pointe  Roof  Roof  Roy/11/23  Curtis Quick  Lot 6 West Pointe  Is responsible for temporary and the overall structure. The design walls, and columns is the responsible for temporary and the overall structure. The design walls, and columns is the responsible for temporary and the overall structure. The design the support of the overall structure. The design the support of the overall structure. The design state is responsible for temporary and the overall structure. The design state is responsible for temporary and the overall structure. The design state is responsible for temporary and the overall structure. The design state is responsible for temporary and the overall structure. The design state is responsible for temporary and the overall structure. The design state is responsible for temporary and the overall structure. The design state is responsible for temporary and the overall structure. The design state is responsible for the overall structure. The overall structure is the responsible for each structure. The overall structure is the overall structure. The overall structure. The overall structure. The overall structure is the o		ponsible for temporary and permanent bracing of the roof and floor system and for verall structure. The design of the truss support structure including headers, beams, and columns is the responsibility of the building designer. For general guidance ding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package				
PLAN	Lauren III / Elev. A / CP	MODEL			Bearin presc	Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables			
SEAL DATE	2/18/20	DATE REV.			found than 3 be ret	( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those			
QUOTE#	Quote #	DRAWN BY			fied in the attached Tables. A registered design professional shall be ned to design the support system for all reactions that exceed 15000#.				
JOB#	J0923-5027	SALESMAN	Lenny Norris	Signature Curtis Quick					



Phone: (910) 864-8787 Fax: (910) 864-4444



▲ = Denotes Left End of Truss
 (Reference Engineered Truss Drawing)
 Do Not Erect Trusses Backwards

6800 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

5100 2

7650 3

10200 4

12750 5

15300 6

1700 1 3400 2

5100 3

6800 4 8500 5

10200 6

11900 7 13600 8 15300 9 All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Beam Legend Fab Type PlotID Length Product Plies Net Qty 7' 0" 1-3/4"x 9-1/4" LVL Kerto-S FF BM1 7' 0" FF 2 1-3/4"x 9-1/4" LVL Kerto-S 2 BM2 GDH 23' 0" 1-3/4"x 16" LVL Kerto-S 3 3 FF

55	ses Backwards			SCALE: 3/16" = 1'	0" 1-3/4"x 16" LVL Kerto-S		
	BUILDER	Weaver Development	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.  These trusses are designed as individual building components to be incor the building design at the specification of the building designer. See indivis sheets for each truss design identified on the placement drawing. The build		
	JOB NAME	Lot 6 West Pointe	ADDRESS	Lot 6 West Pointe	is responsible for temporary and permanent bracing of the roof and floor the overall structure. The design of the truss support structure including walls, and columns is the responsibility of the building designer. For ger regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss		
	PLAN	Lauren III / Elev. A / CP	MODEL	Roof	or online @ sbcindustry.com  Bearing reactions less than or equal to 3000# are deemed to comprescriptive Code requirements. The contractor shall refer to the		
	SEAL DATE	2/18/20	DATE REV.	09/11/23	( derived from the prescriptive Code requirements ) to determine foundation size and number of wood studs required to support re than 3000# but not greater than 15000#. A registered design profe be retained to design the support system for any reaction that ex		
	QUOTE#	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional retained to design the support system for all reactions that exceed		
	JOB#	J0923-5027	SALESMAN	Lenny Norris	Signature Curtis Quick		

Truss Placement Plan



Phone: (910) 864-8787

Fax: (910) 864-4444



Client: Weaver Development

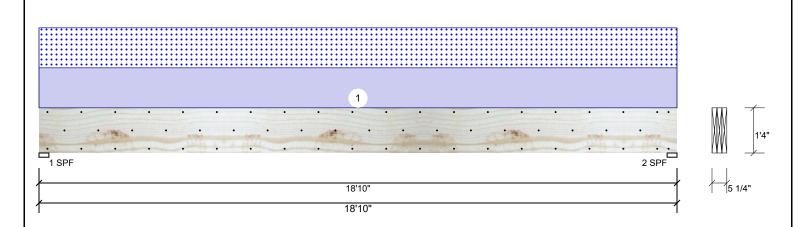
Project: Address: Date:

9/18/2023 Input by: Curtis Quick Job Name: The Lauren III Beams Page 1 of 6

Project #:

1.750" X 16.000" **Kerto-S LVL** 3-Ply - PASSED **GDH** 

Level: Level



### Member Information Reactions UNPATTERNED Ib (Uplift) Live Wind Type: Application: Floor Brg Direction Dead Snow Const Plies: 3 Design Method: ASD 0 1127 951 0 Vertical 0 1 Moisture Condition: Dry **Building Code:** IBC 2012 2 Vertical 0 1127 951 0 0 Deflection LL: 480 Load Sharing: Yes Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temp <= 100°F Temperature: **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+S 1 - SPF 3.500" Vert 27% 1127 / 951 2078 L

2 - SPF 3.500"

Vert

27%

1127 / 951

2078 L

D+S

### Analysis Results

ĺ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	9334 ft-lb	9'5"	62010 ft-lb	0.151 (15%)	D+S	L
	Unbraced	9334 ft-lb	9'5"	10990 ft-lb	0.849 (85%)	D+S	L
	Shear	1744 lb	17'2 1/2"	20608 lb	0.085 (8%)	D+S	L
	LL Defl inch	0.078 (L/2813)	9'5 1/16"	0.460 (L/480)	0.171 (17%)	S	L
	TL Defl inch	0.171 (L/1288)	9'5 1/16"	0.613 (L/360)	0.280 (28%)	D+S	L

## **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	101 PLF	0 PLF	101 PLF	0 PLF	0 PLF	A4A
	Self Weight				19 PLF					

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

  Damaged Beams must not be used Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

**Manufacturer Info** 

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 11/3/2024

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Client: Weaver Development

Project: Address:

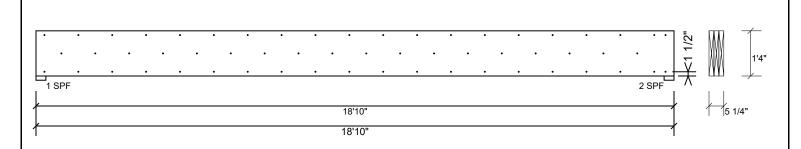
9/18/2023 Input by:

Curtis Quick Job Name: The Lauren III Beams Page 2 of 6

Project #:

**Kerto-S LVL** 1.750" X 16.000" 3-Ply - PASSED **GDH** 

Level: Level



# Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1 00	

### Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

### Handling & Installation

- LVL beams must not be cut or drilled
  Refer to manufacturer's product information
  requirements, multi-ply
  fastening details, beam strength values, and code
  approvals
  Damaged Beams must not be used
- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

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Manufacturer Info

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Client: Weaver Development

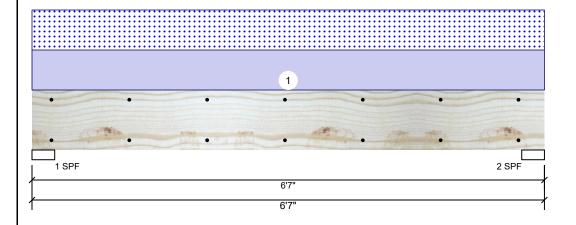
Project: Address: Date: 9/18/2023

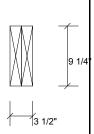
Input by: Curtis Quick Job Name: The Lauren III Beams

Project #:

1.750" X 9.250" 2-Ply - PASSED Kerto-S LVL BM<sub>1</sub>

Level: Level





Page 3 of 6

## **Member Information**

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012

Load Sharing: No Deck:

Not Checked

## Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1564	1541	0	0
2	Vertical	0	1564	1541	0	0

# **Bearings**

Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	60%	1564 / 1541	3105	L	D+S
2 - SPF	3.500"	Vert	60%	1564 / 1541	3105	L	D+S

### Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4423 ft-lb	3'3 1/2"	14423 ft-lb	0.307 (31%)	D+S	L
Unbraced	4423 ft-lb	3'3 1/2"	10451 ft-lb	0.423 (42%)	D+S	L
Shear	2108 lb	1' 3/4"	7943 lb	0.265 (27%)	D+S	L
LL Defl inch	0.040 (L/1842)	3'3 1/2"	0.153 (L/480)	0.261 (26%)	S	L
TL Defl inch	0.080 (L/914)	3'3 1/2"	0.204 (L/360)	0.394 (39%)	D+S	L

## **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	468 PLF	0 PLF	468 PLF	0 PLF	0 PLF	A1
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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Client: Weaver Development

Project: Address: Date: 9/18/2023

Input by: Curtis Quick Job Name: The Lauren III Beams

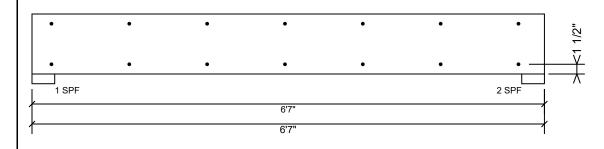
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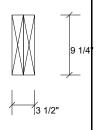
**Kerto-S LVL** BM1

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 4 of 6

# Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

### Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

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This design is valid until 11/3/2024 CSD DESIGN

Manufacturer Info



Client: Weaver Development

Project: Address:

Date: 9/18/2023 Input by:

Curtis Quick Job Name: The Lauren III Beams

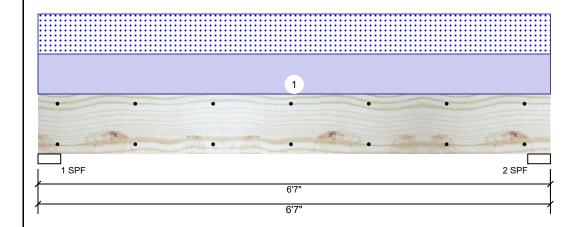
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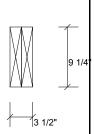
**Kerto-S LVL** BM<sub>2</sub>

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 5 of 6

## **Member Information**

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No

Deck: Not Checked

## Reactions UNPATTERNED Ib (Uplift)

Direction	Live	Dead	Snow	Wind	Const
Vertical	0	955	932	0	0
Vertical	0	955	932	0	0
	Vertical	Vertical 0	Vertical 0 955	Vertical 0 955 932	Vertical 0 955 932 0

# **Bearings**

Bearing	Length	Dir.	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	36%	955 / 932	1887	L	D+S
2 - SPF	3.500"	Vert	36%	955 / 932	1887	L	D+S

### Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2688 ft-lb	3'3 1/2"	14423 ft-lb	0.186 (19%)	D+S	L
Unbraced	2688 ft-lb	3'3 1/2"	10451 ft-lb	0.257 (26%)	D+S	L
Shear	1283 lb	5'6 1/4"	7943 lb	0.162 (16%)	D+S	L
LL Defl inch	0.024 (L/3046)	3'3 1/2"	0.153 (L/480)	0.158 (16%)	S	L
TL Defl inch	0.049 (L/1504)	3'3 1/2"	0.204 (L/360)	0.239 (24%)	D+S	L

## **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	283 PLF	0 PLF	283 PLF	0 PLF	0 PLF	A4A
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

### Handling & Installation

- LVL beams must not be cut or drilled
  Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
  2 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

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This design is valid until 11/3/2024

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Client: Weaver Development

Project: Address: Date: 9/18/2023 Input by: Curtis Quick

Job Name: The Lauren III Beams

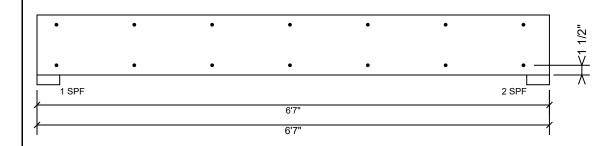
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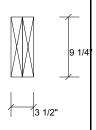
**Kerto-S LVL** BM<sub>2</sub>

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 6 of 6

# Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

·	
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

### Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

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Manufacturer Info

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