

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0923-5445 The Guilford

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I61035187 thru I61035221

My license renewal date for the state of North Carolina is December 31, 2023.

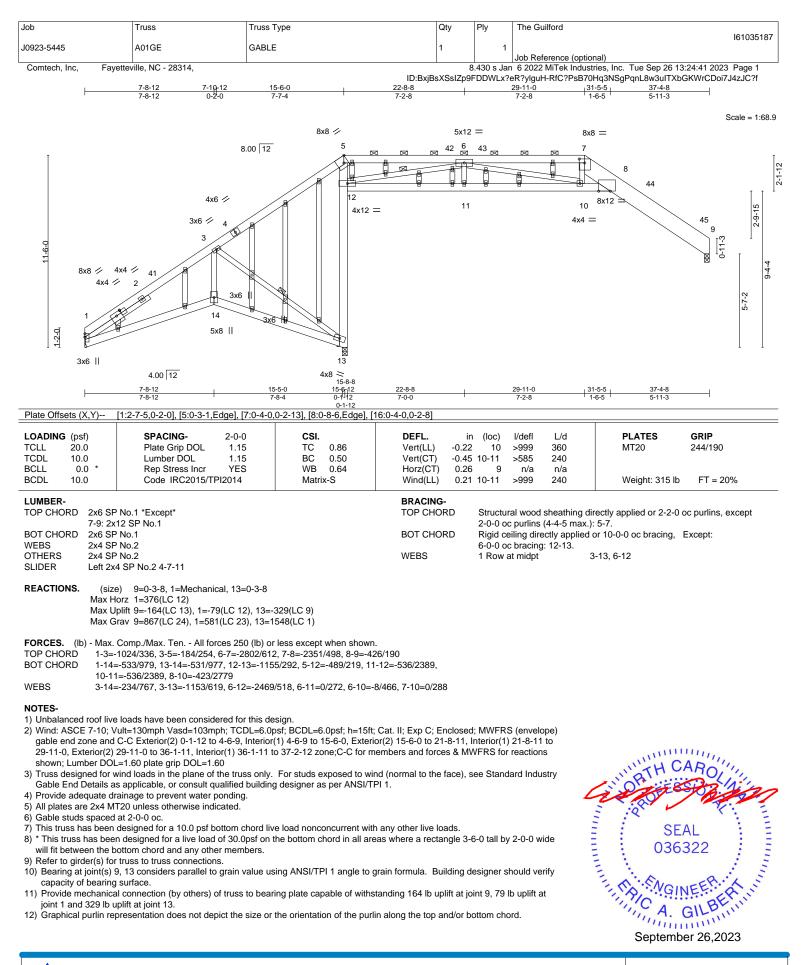
North Carolina COA: C-0844



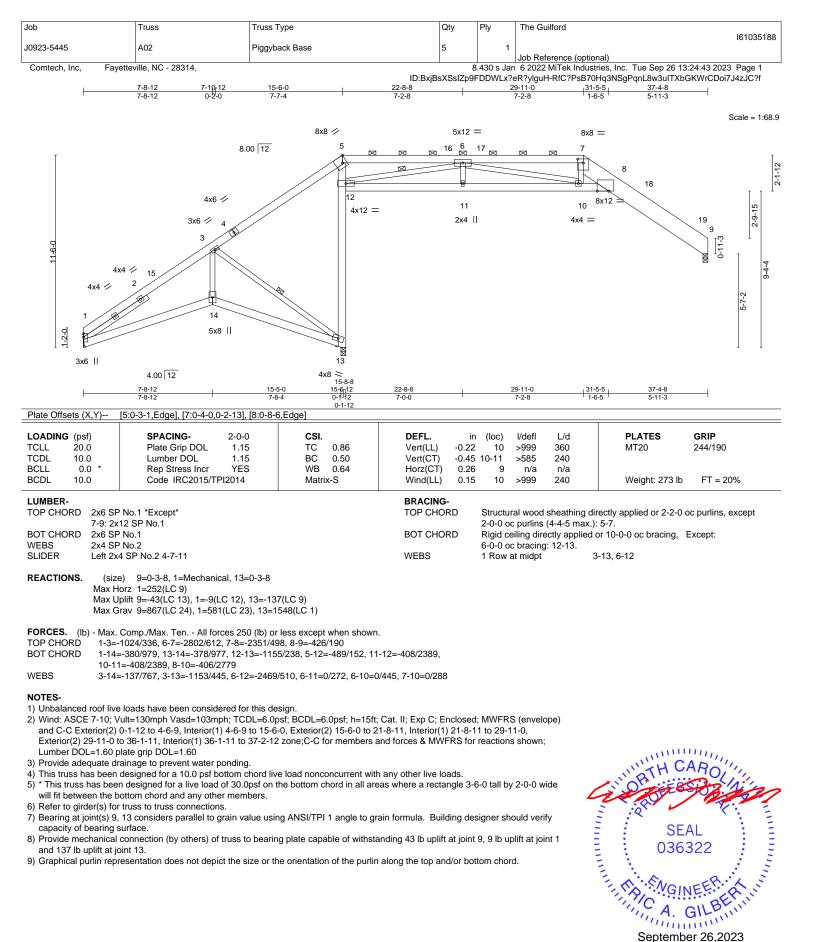
September 26,2023

## Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

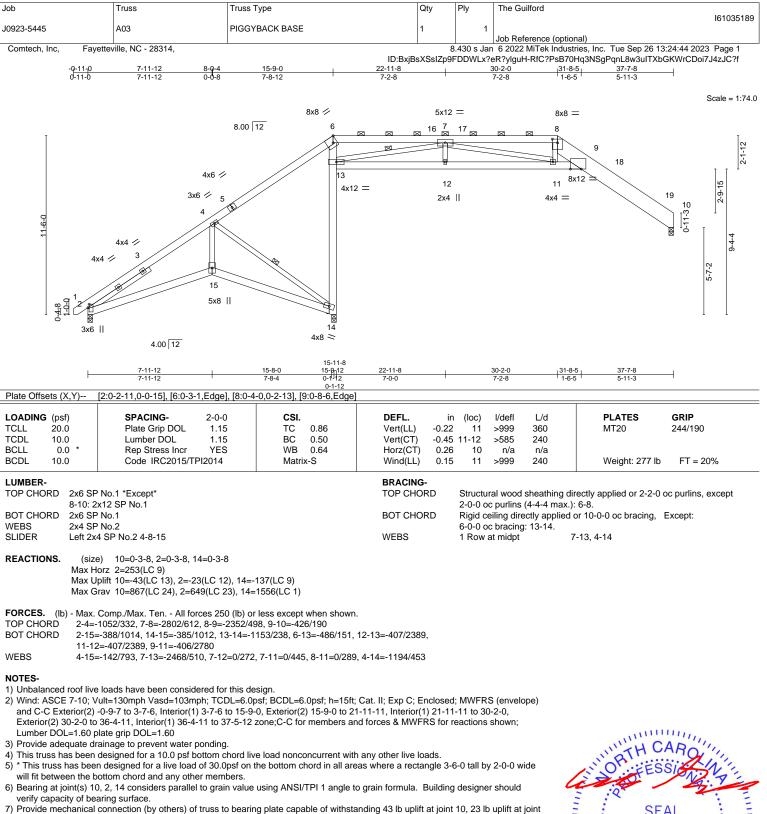


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



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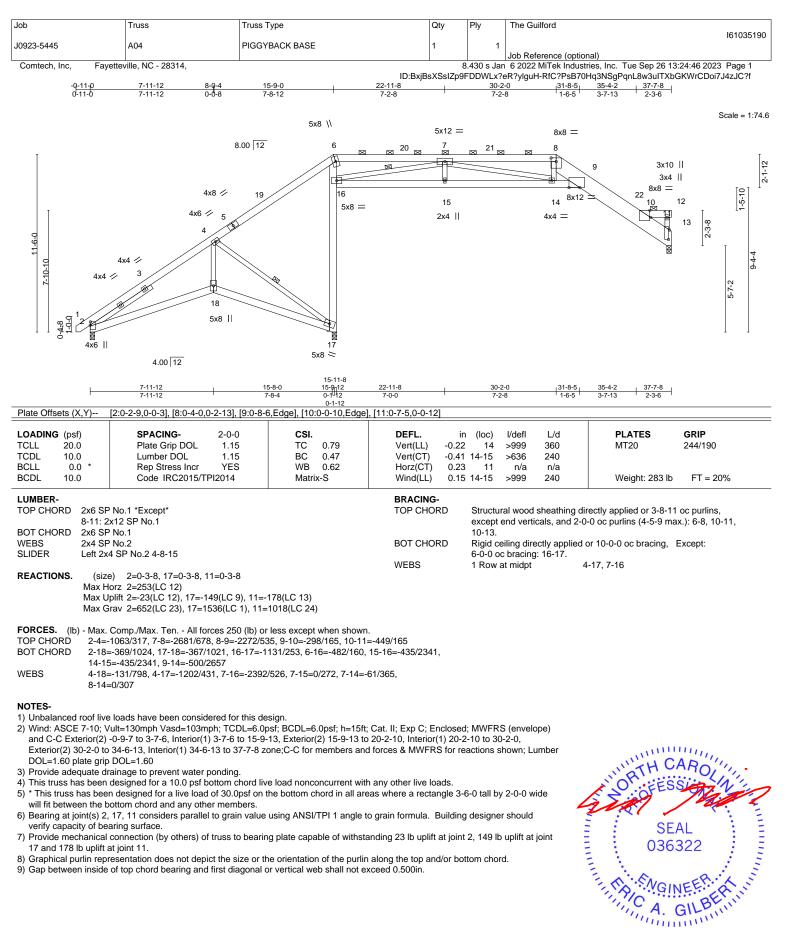


7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 10, 23 lb uplift at joint 2 and 137 lb uplift at joint 14.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

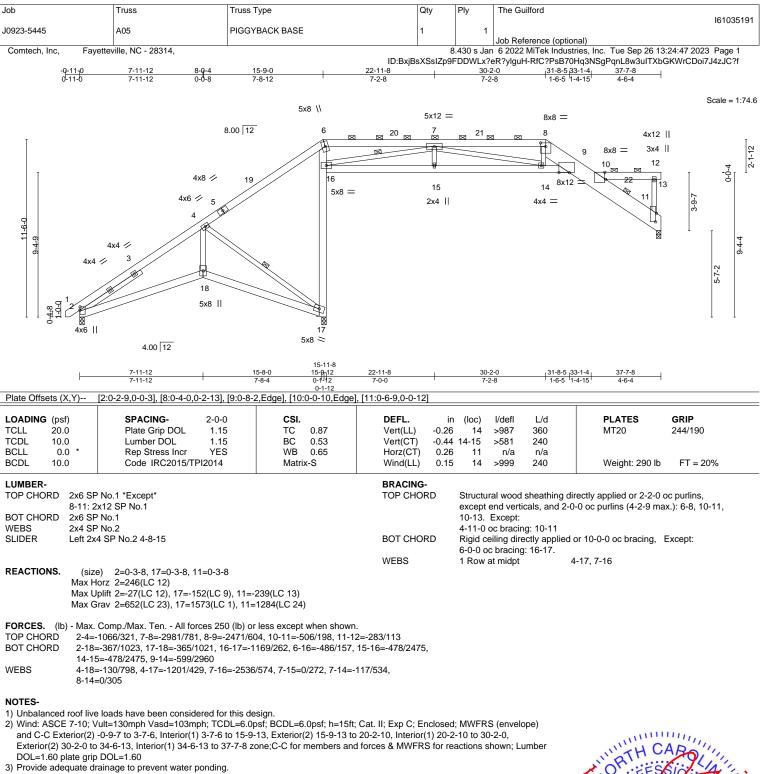


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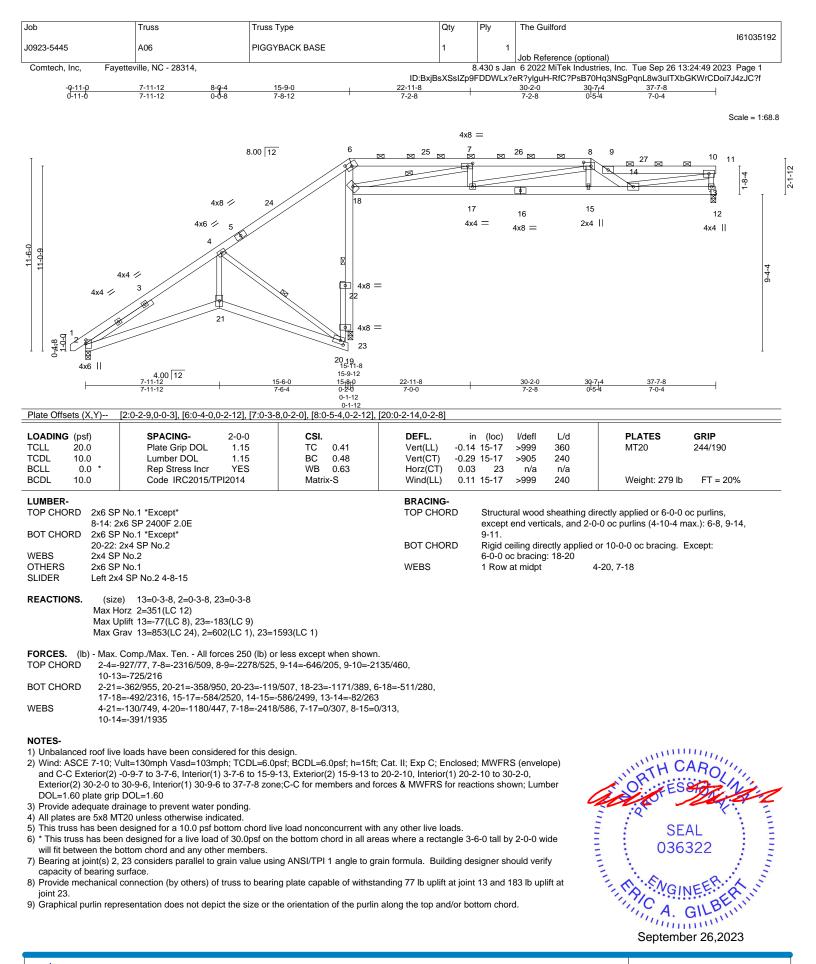
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- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 17, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 2, 152 lb uplift at joint 17 and 239 lb uplift at joint 11.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

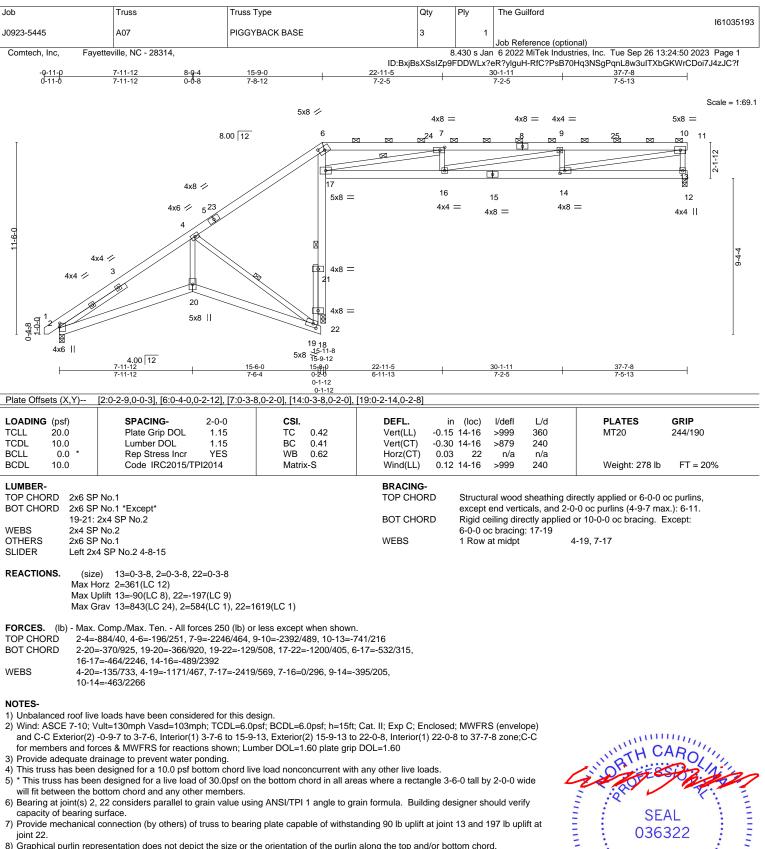


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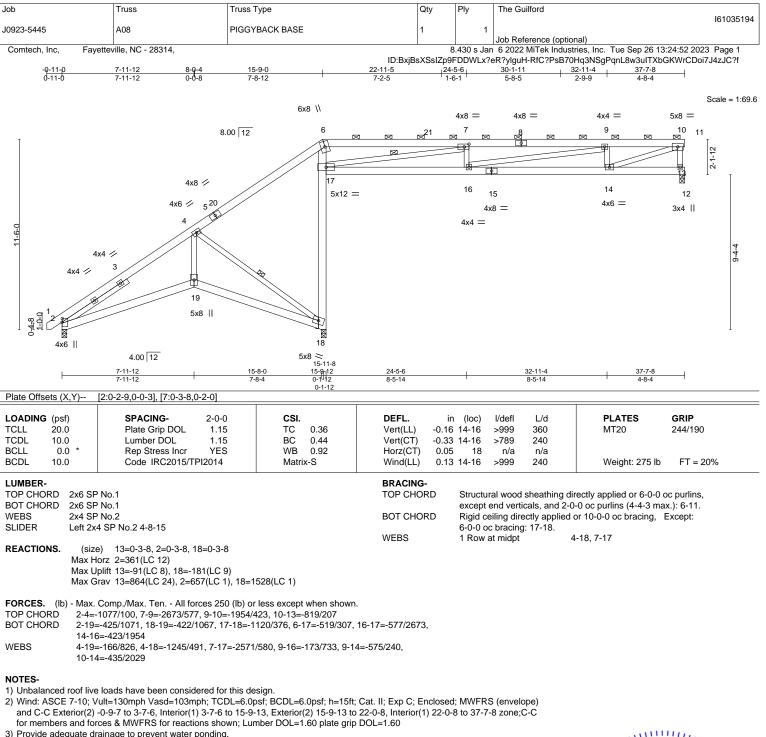
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A MiTek Af 818 Soundside Road



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- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5) will fit between the bottom chord and any other members.

6) Bearing at joint(s) 2, 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 13 and 181 lb uplift at ioint 18.

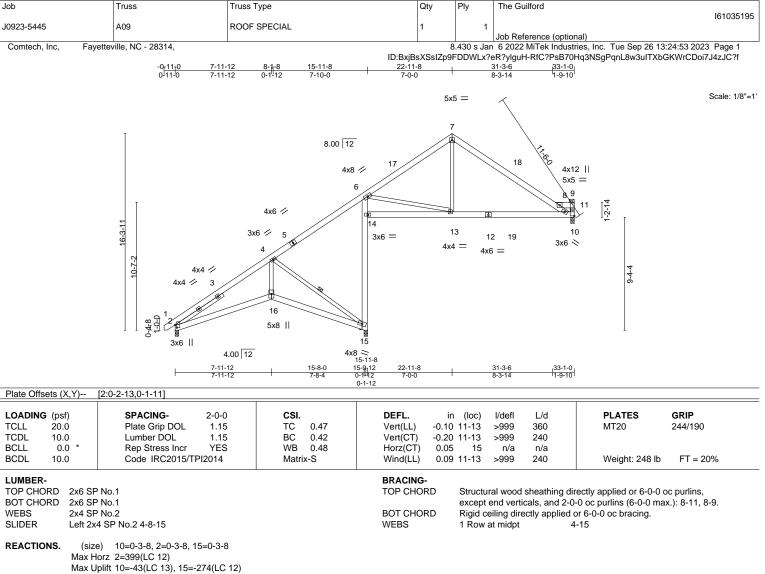
8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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818 Soundside Road



Max Grav 10=43(LC 13), 15=-274(LC 12)Max Grav 10=695(LC 20), 2=659(LC 1), 15=1492(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-1075/142, 4-6=-241/254, 6-7=-828/185, 7-8=-744/153, 8-11=-784/234

- BOT CHORD 2-16=-376/1042, 15-16=-372/1037, 14-15=-1058/294, 6-14=-1033/338, 11-13=-6/573
- WEBS 4-16=-137/807, 4-15=-1197/428, 6-13=-30/595, 7-13=0/343

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-7 to 3-7-6, Interior(1) 3-7-6 to 22-11-8, Exterior(2) 22-11-8 to 27-4-5, Interior(1) 27-4-5 to 32-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

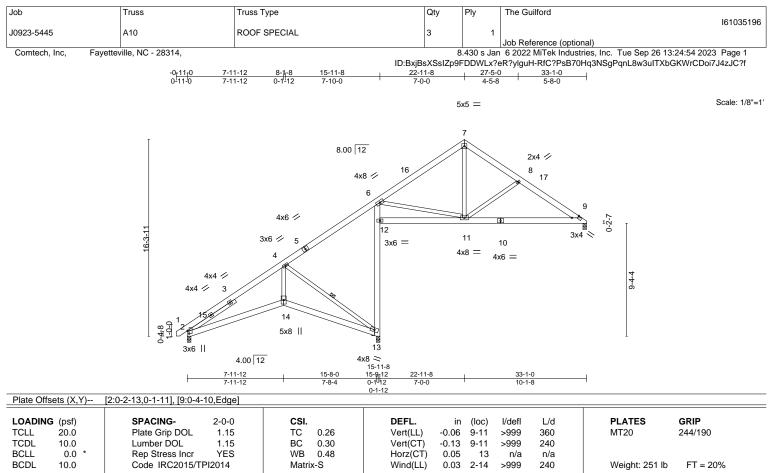
6) Bearing at joint(s) 2, 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 10 and 274 lb uplift at joint 15.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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LUMBER-		BRACING-		
TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood sheathing c	lirectly applied or 6-0-0 oc purlins.
BOT CHORD	2x6 SP No.1	BOT CHORD	Rigid ceiling directly applied	or 10-0-0 oc bracing, Except:
WEBS	2x4 SP No.2		6-0-0 oc bracing: 12-13.	
SLIDER	Left 2x4 SP No.2 4-8-15	WEBS	1 Row at midpt	4-13

REACTIONS. (size) 9=0-3-8, 2=0-3-8, 13=0-3-8 Max Horz 2=383(LC 12) Max Uplift 9=-44(LC 13), 13=-263(LC 12) Max Grav 9=668(LC 1), 2=658(LC 1), 13=1420(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-4=-1071/168, 4-6=-249/262, 6-7=-689/166, 7-8=-635/190, 8-9=-906/238
- BOT CHORD 2-14=-365/1033, 13-14=-361/1029, 12-13=-986/266, 6-12=-932/297, 9-11=-117/724
- WEBS 4-14=-130/802, 4-13=-1188/416, 6-11=0/439, 7-11=-19/413, 8-11=-390/220

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-7 to 3-7-6, Interior(1) 3-7-6 to 22-11-8, Exterior(2) 22-11-8 to 27-7-7, Interior(1) 27-7-7 to 32-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 2, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 9 and 263 lb uplift at joint 13.



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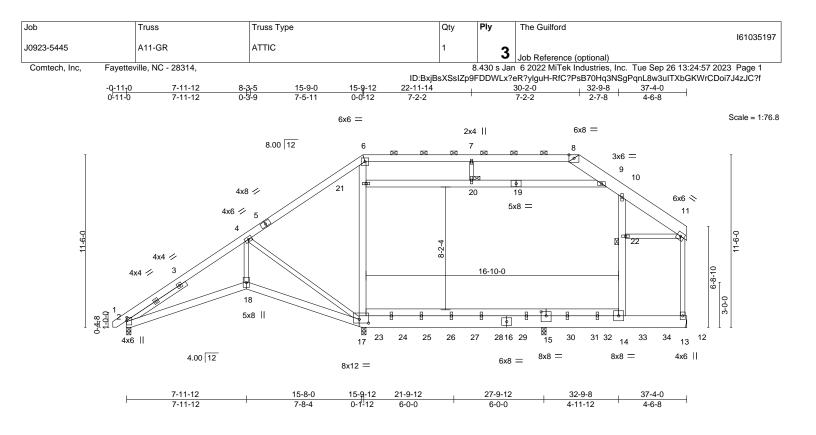


Plate Offsets (X,Y)	[2:0-2-9,0-0-3], [15:0-4-0,0-3-4], [17:0-7	'-8,0-3-0]					
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.46 BC 0.64 WB 0.43 Matrix-S	Vert(LL) -0.15		31 360 43 240 n/a n/a	PLATES MT20 Weight: 1207 lb	<b>GRIP</b> 244/190 FT = 20%
BOT CHORD 2x6 SF 16-17: WEBS 2x4 SF 6-17,10	2x10 SP No.1	.0E	BRACING- TOP CHORD BOT CHORD JOINTS	except end	verticals, and 2-0 directly applied of	rectly applied or 6-0-0 oc -0 oc purlins (6-0-0 max or 10-0-0 oc bracing.	
(lb) - Max H Max U	earings 0-3-8 except (jt=length) 13=Mec lorz 2=260(LC 8) Iplift All uplift 100 lb or less at joint(s) 2 Grav All reactions 250 lb or less at joint 22), 13=3923(LC 14), 15=8796(LC	except 13=-278(LC 9) (s) except 2=917(LC 1), 1	17=2713(LC				
TOP CHORD         2-4=-           9-10:	Comp./Max. Ten All forces 250 (lb) of -1785/218, 4-6=-596/198, 6-7=-838/177, =-716/123, 10-11=-1062/150, 11-13=-86 =-354/1504, 17-18=-353/1500, 15-17=-3 =-130/1084, 4-17=-1285/388, 17-21=-76 2=-306/301, 20-21=-45/473, 9-20=-45/4	, 7-8=-838/177, 8-9=-982/ 66/143 55/375, 14-15=-33/366 60/120, 6-21=-522/143, 14	/187,			ORTH CA	NROUNING
Top chords connect Bottom chords conn Webs connected as 2) All loads are considu ply connections hav 3) Unbalanced roof live 4) Wind: ASCE 7-10; V Lumber DOL=1.60 p 5) Provide adequate di	anected together with 10d (0.131"x3") na ed as follows: 2x6 - 2 rows staggered at lected as follows: 2x6 - 2 rows staggered follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 ered equally applied to all plies, except i e been provided to distribute only loads e loads have been considered for this de /ult=130mph Vasd=103mph; TCDL=6.0p olate grip DOL=1.60 rainage to prevent water ponding. T20 unless otherwise indicated.	0-9-0 oc, 2x10 - 2 rows s d at 0-9-0 oc, 2x10 - 5 row rows staggered at 0-9-0 ú f noted as front (F) or bac noted as (F) or (B), unles esign.	vs staggered at 0-4-0 oc oc. ck (B) face in the LOAD ( ss otherwise indicated.	:. CASE(S) secti	-9-0 oc. on. Ply to nvelope);	SEA 0363 WGIN A. C Septembe	EER.K.
Design valid for use a truss system. Befor building design. Brad	design parameters and READ NOTES ON THIS AN only with MiTek® connectors. This design is based re use, the building designer must verify the applica cing indicated is to prevent buckling of individual trr stability and to prevent collapse with possible per- stability and to prevent collapse with possible per-	only upon parameters shown, a ability of design parameters and uss web and/or chord members of	and is for an individual building properly incorporate this desig only. Additional temporary and	component, not n into the overall I permanent bracii	ng		NING BY NCCO A Mittek Affiliate

is always required for stability and to prevent collaring of introductances were and/or of memory damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	The Guilford
					l61035197
J0923-5445	A11-GR	ATTIC	1	3	
				<b>J</b>	Job Reference (optional)
Comtech, Inc, Fayettev	/ille, NC - 28314,			.430 s Jan	6 2022 MiTek Industries, Inc. Tue Sep 26 13:24:57 2023 Page 2

#### NOTES-

ID:BxjBsXSsIZp9FDDWLx?eR?ylguH-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

- 7) Concentrated loads from layout are not present in Load Case(s): #3 Dead + Uninhabitable Attic Without Storage; #4 Dead + 0.6 MWFRS Wind (Pos. Internal) Left; #5 Dead + 0.6 MWFRS Wind (Pos. Internal) Right; #6 Dead + 0.6 MWFRS Wind (Neg. Internal) Left; #7 Dead + 0.6 MWFRS Wind (Neg. Internal) Right; #8 Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel; #9 Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel; #10 Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel; #11 Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel; #12 Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel; #13 Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel; #20 Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left); #21 Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right); #22 Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel); #23 Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel); #28 Reversal: Dead + 0.6 MWFRS Wind (Pos. Internal) Left; #29 Reversal: Dead + 0.6 MWFRS Wind (Pos. Internal) Right; #30 Reversal: Dead + 0.6 MWFRS Wind (Neg. Internal) Left; #31 Reversal: Dead + 0.6 MWFRS Wind (Neg. Internal) Right; #32 Reversal: Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel; #33 Reversal: Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel; #34 Reversal: Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel; #35 Reversal: Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel; #36 Reversal: Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel; #37 Reversal: Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel; #38 Reversal: Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left); #39 Reversal: Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right); #40 Reversal: Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel); #41 Reversal: Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel).
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and 9) any other members, with BCDL = 10.0psf.
- 10) Ceiling dead load (10.0 psf) on member(s). 20-21, 9-20, 11-22; Wall dead load (5.0psf) on member(s). 17-21, 14-22
- 11) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 15-17, 14-15
- 12) Refer to girder(s) for truss to truss connections.
- 13) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (it=lb) 13=278.
- 15) Magnitude of user added load(s) on this truss have been applied uniformly across all gravity load cases with no adjustments.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 17) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 4554 lb down and 483 lb up at 32-11-4, 590 lb down at 16-8-5, 590 lb down at 18-3-8, 590 lb down at 19-10-11, 590 lb down at 21-5-14, 590 lb down at 23-1-1, 590 lb down at 24-8-4, 533 lb down at 26-3-7, 533 lb down at 27-10-10, 533 Ib down at 29-5-13, 533 lb down at 31-1-0, 533 lb down at 31-11-4, and 533 lb down at 34-3-6, and 533 lb down at 35-10-9 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 18) Attic room checked for L/360 deflection.

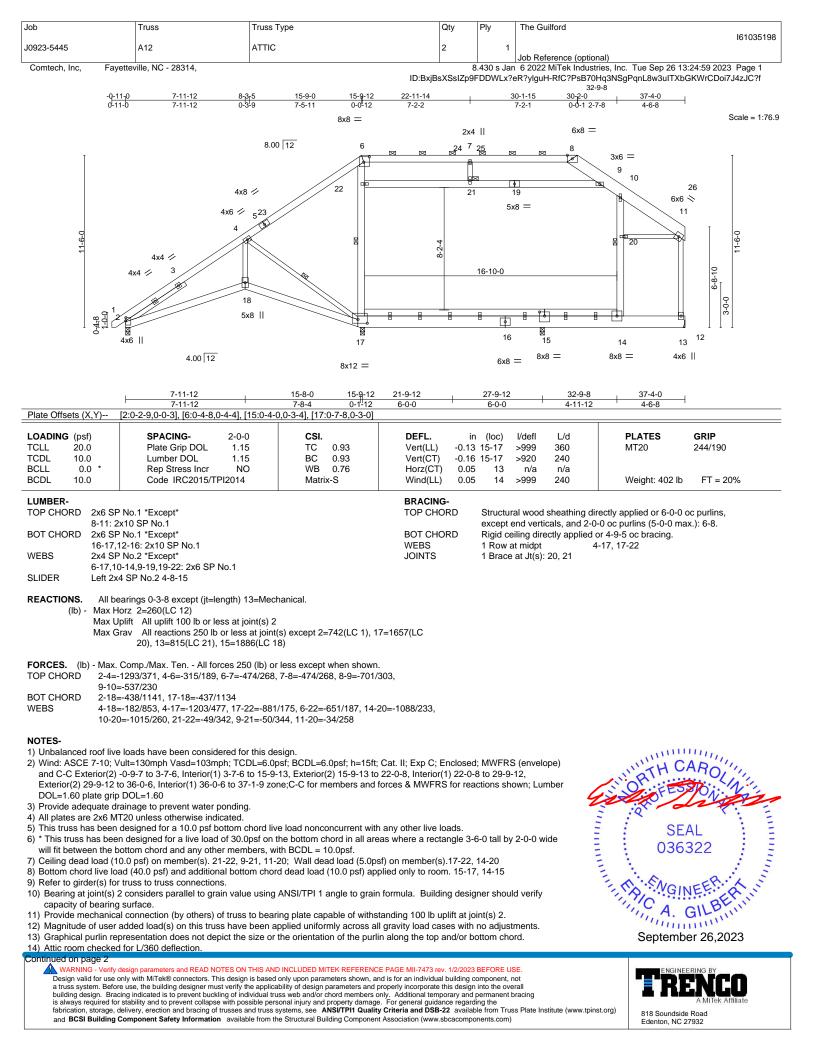
#### LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
  - Vert: 1-6=-60, 6-8=-60, 8-11=-60, 2-18=-20, 17-18=-20, 13-17=-40, 12-13=-20, 9-21=-20, 11-22=-20
- Drag: 17-21=-10, 14-22=-10 Concentrated Loads (lb)

Vert: 14=-4500(F) 15=-109(F) 23=-149(F) 24=-149(F) 25=-149(F) 26=-149(F) 27=-149(F) 28=-149(F) 29=-109(F) 30=-109(F) 31=-109(F) 32=-109(F) 33=-109(F) 33=-34=-109(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





Truss	Truss Type	Qty	Ply	The Guilford
				l61035198
A12	ATTIC	2	1	
				Job Reference (optional)
lle, NC - 28314,		8	.430 s Jan	6 2022 MiTek Industries, Inc. Tue Sep 26 13:24:59 2023 Page 2
	A12	A12 ATTIC	A12 ATTIC 2	A12 ATTIC 2 1

ID:BxjBsXSsIZp9FDDWLx?eR?ylguH-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

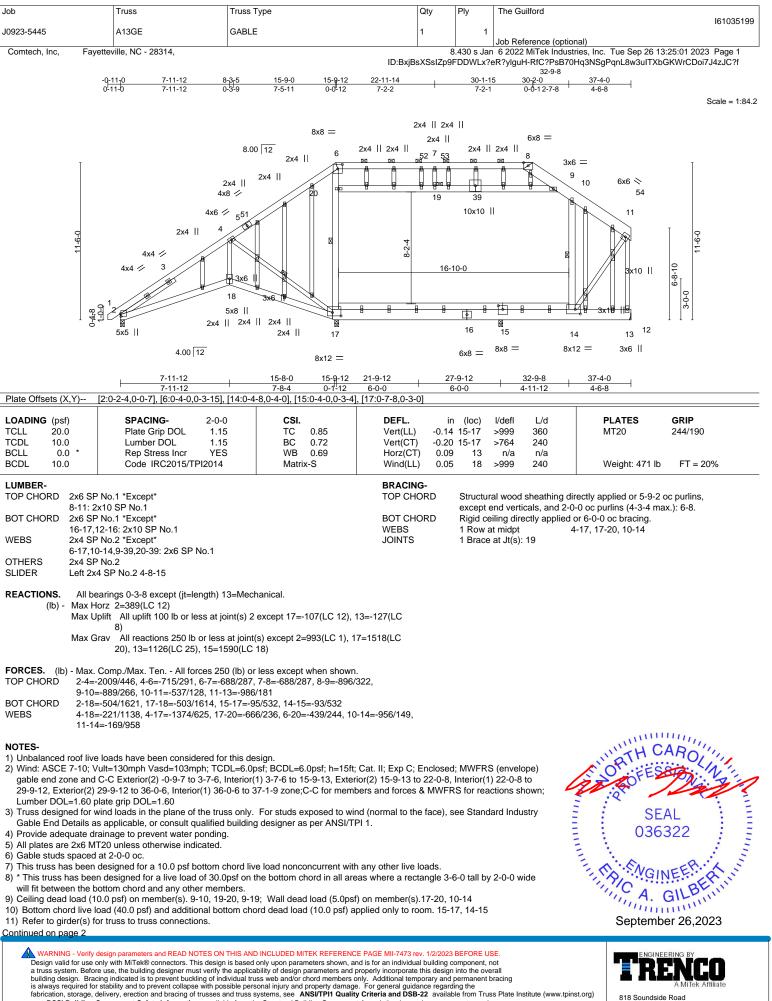
## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-60, 6-8=-60, 8-11=-60, 2-18=-20, 17-18=-20, 13-17=-40, 12-13=-20, 9-22=-20, 11-20=-20 Drag: 17-22=-10, 14-20=-10

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	The Guilford
					I61035199
J0923-5445	A13GE	GABLE	1	1	
					Job Reference (optional)
Comtech, Inc,	Fayetteville, NC - 28314,		6	.430 s Jan	6 2022 MiTek Industries, Inc. Tue Sep 26 13:25:01 2023 Page 2

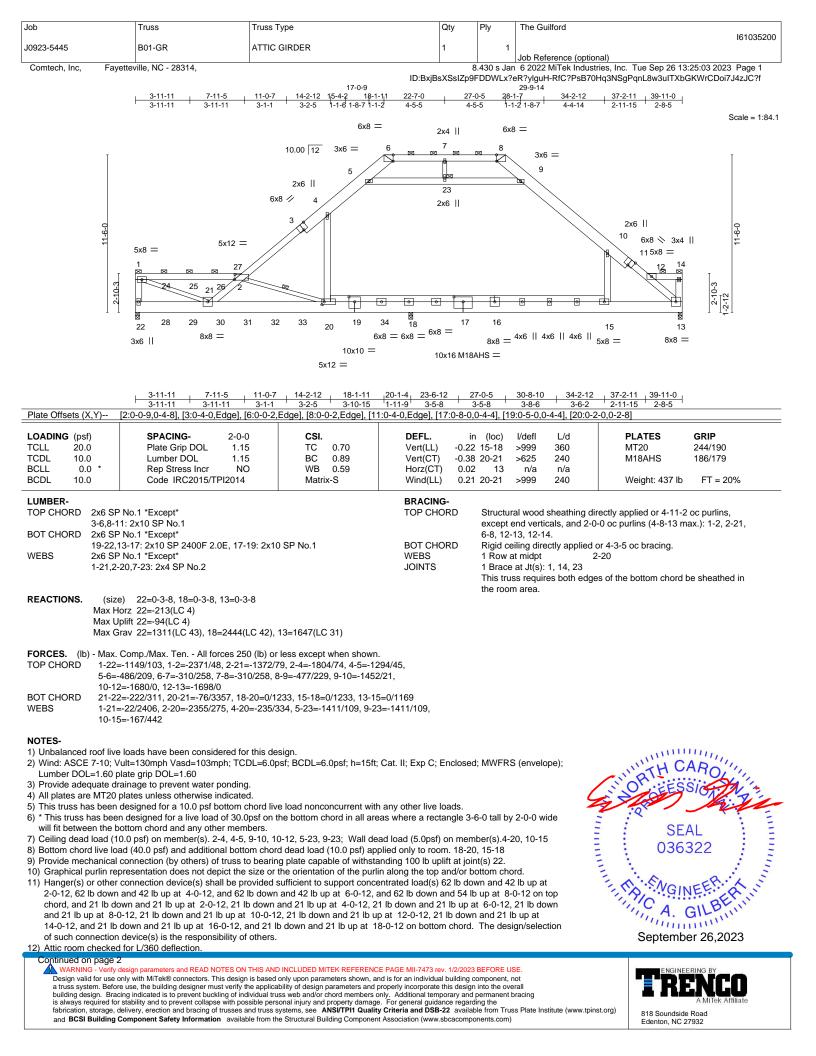
ID:BxjBsXSsIZp9FDDWLx?eR?ylguH-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

NOTES-

12) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 17=107, 13=127.
14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
15) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





Job		Truss	Truss Type	Qty	Ply	The Guilford
						I61035200
JOS	23-5445	B01-GR	ATTIC GIRDER	1	1	
						Job Reference (optional)
C	omtech, Inc, Fayettev	ille, NC - 28314,		8	.430 s Jan	6 2022 MiTek Industries, Inc. Tue Sep 26 13:25:04 2023 Page 2

NOTES-

ID:BxjBsXSsIZp9FDDWLx?eR?ylguH-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

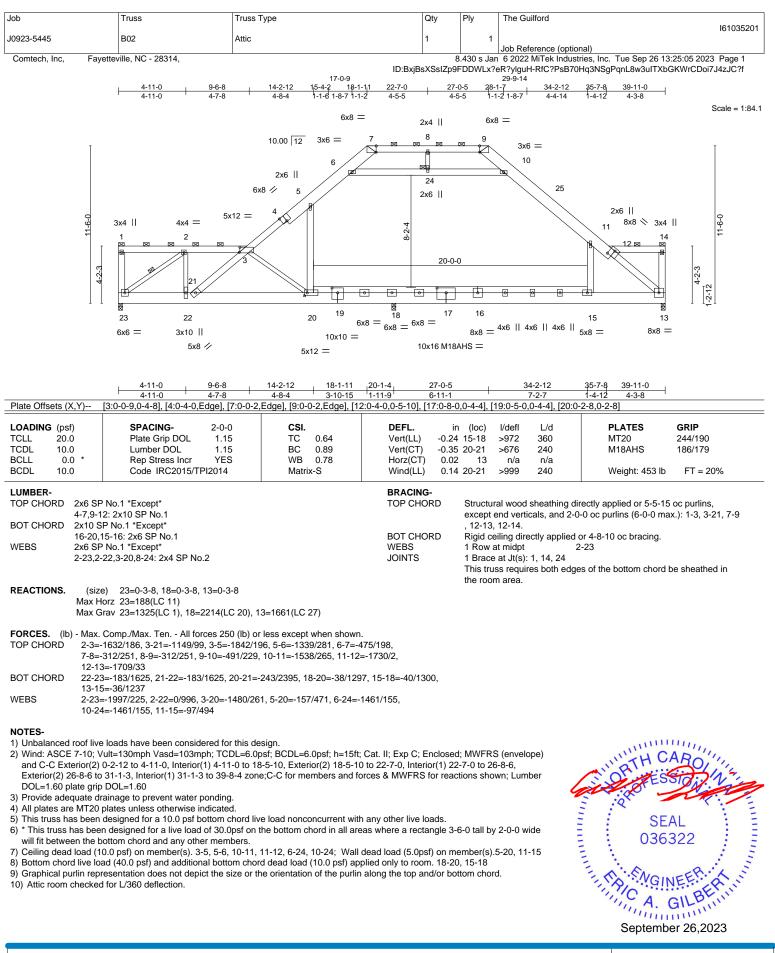
Vert: 1-2-60, 2-5=-80, 5-6=-60, 6-8=-60, 8-9=-60, 9-12=-80, 12-14=-60, 20-22=-20, 19-20=-40, 17-19=-40, 16-17=-40, 15-16=-40, 13-15=-20, 5-9=-20 Drag: 4-20=-10, 10-15=-10

Concentrated Loads (lb)

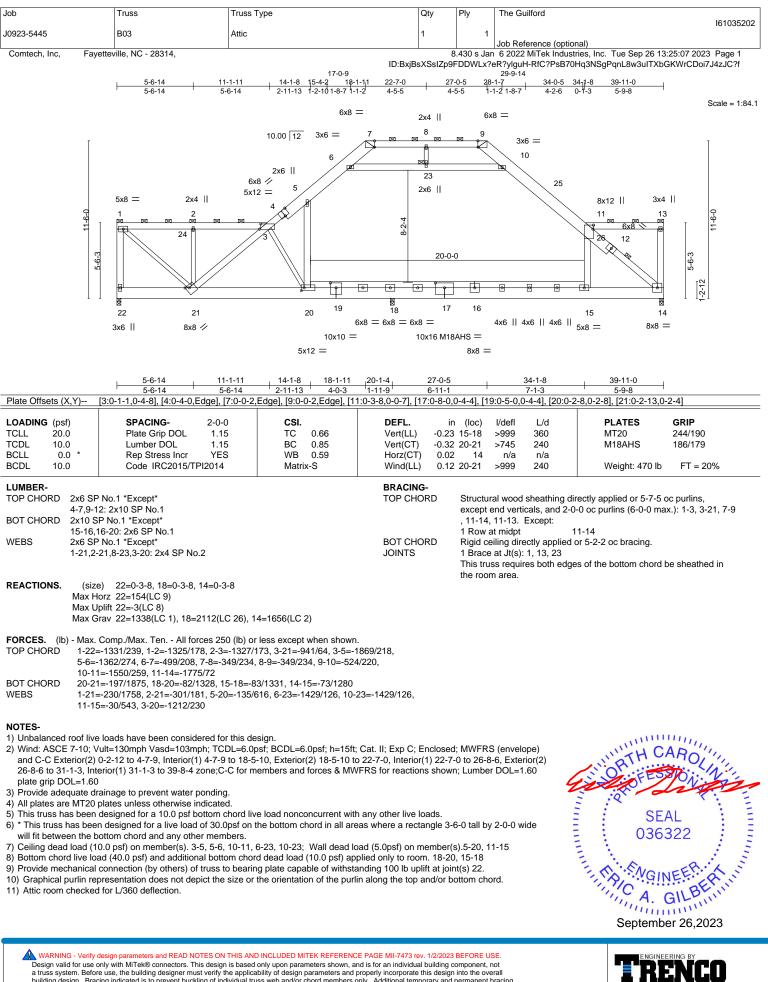
Vert: 19=1(F) 20=1(F) 28=1(F) 29=1(F) 30=1(F) 31=1(F) 32=1(F) 33=1(F) 34=1(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

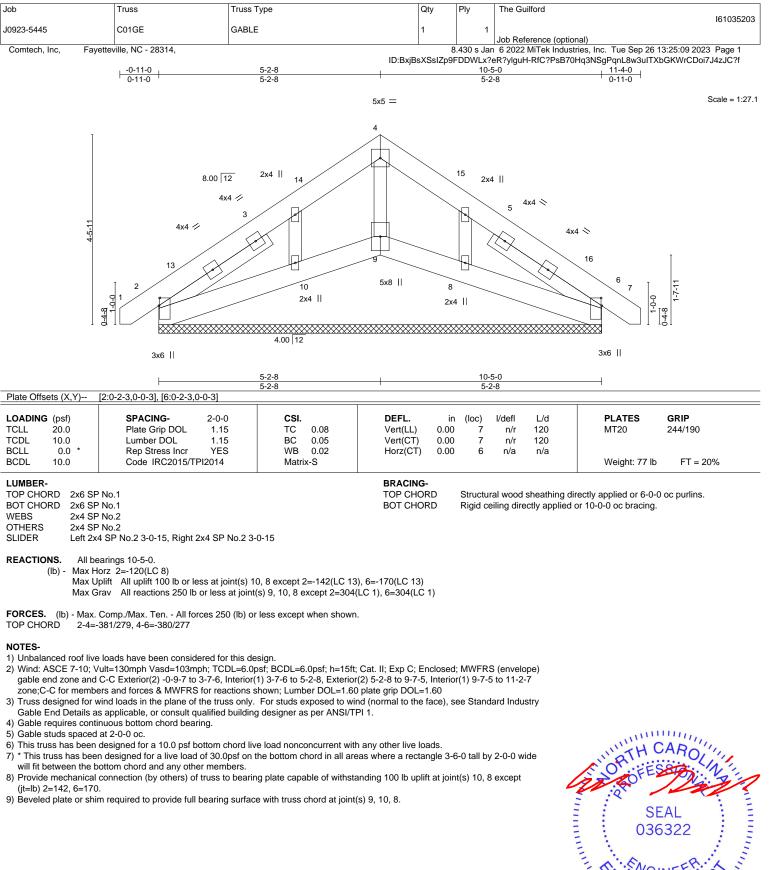




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and trus systems, see **ANSUTPH1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



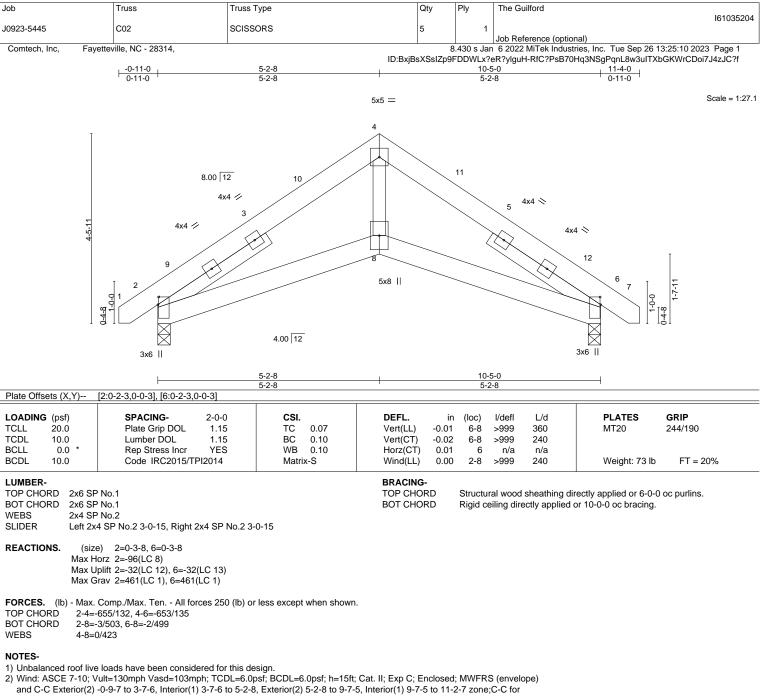
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

818 Soundside Road



members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

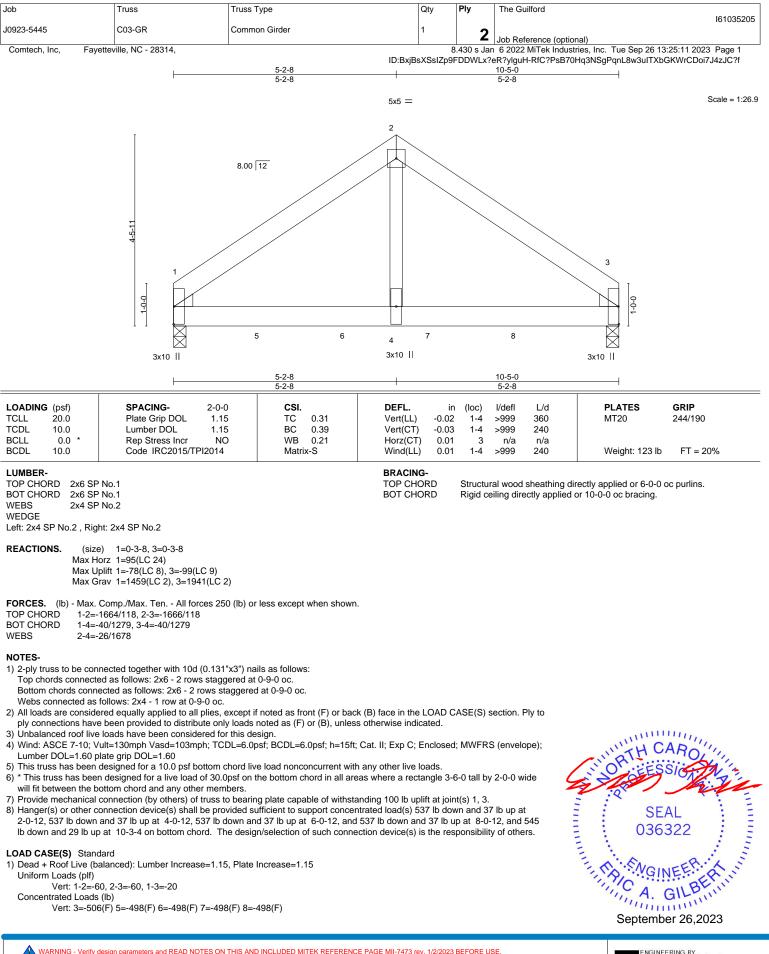
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

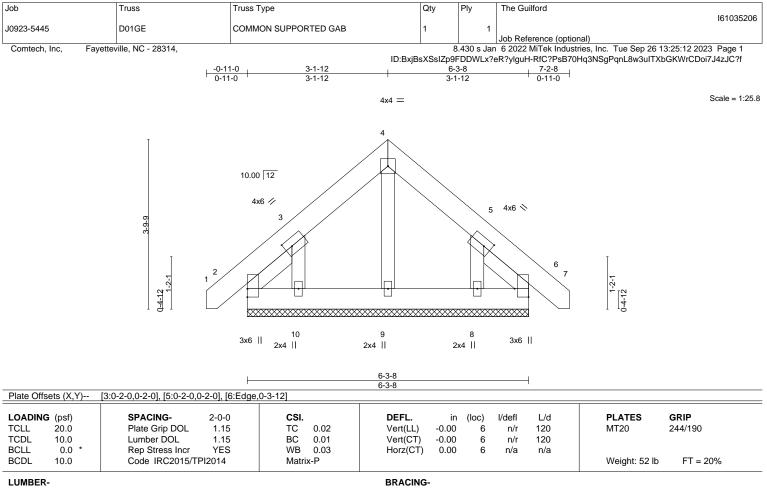
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



# TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1OTHERS2x4 SP No.2

SLIDER Left 2x4 SP No.2 1-4-9, Right 2x4 SP No.2 1-4-9

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

### REACTIONS. All bearings 6-3-8.

(lb) - Max Horz 2=-98(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-139(LC 12), 8=-132(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

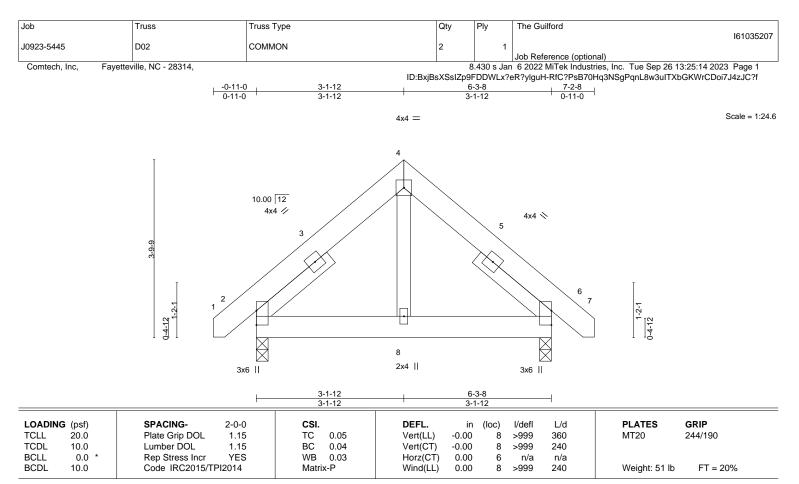
Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=139, 8=132.

9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)





#### LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2SLIDERLeft 2x4 SP I

SLIDER Left 2x4 SP No.2 2-0-5, Right 2x4 SP No.2 2-0-5

REACTIONS.

(size) 2=0-3-0, 6=0-3-0 Max Horz 2=-79(LC 10)

Max Uplift 2=-32(LC 8), 6=-32(LC 9)

Max Grav 2=299(LC 1), 6=299(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-254/240, 4-6=-254/241

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.

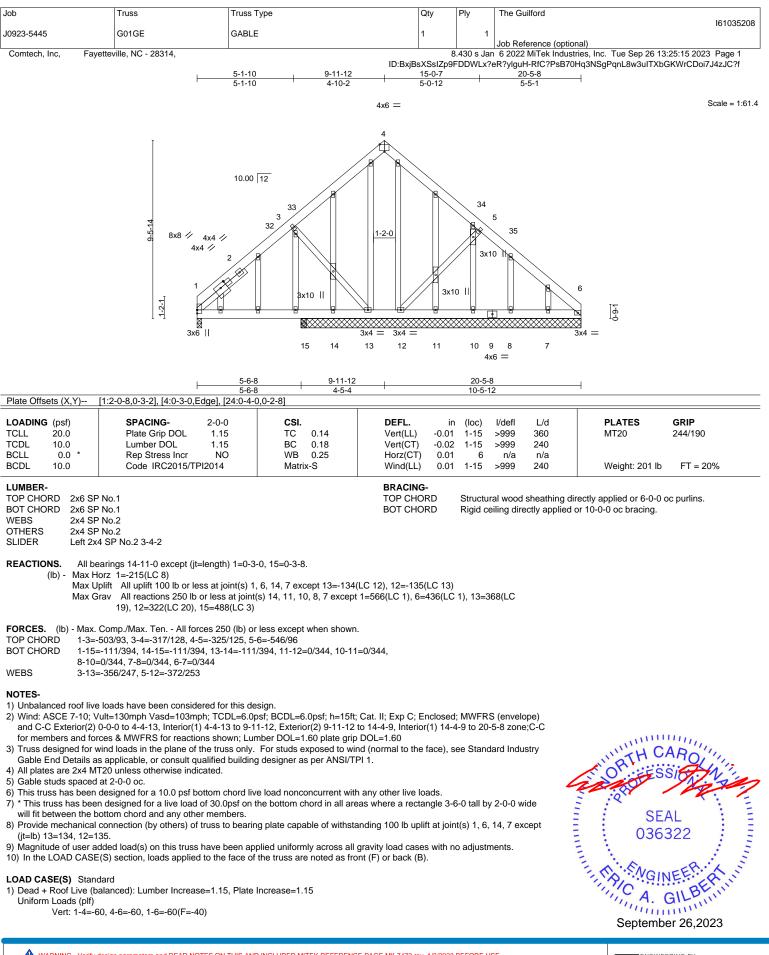


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

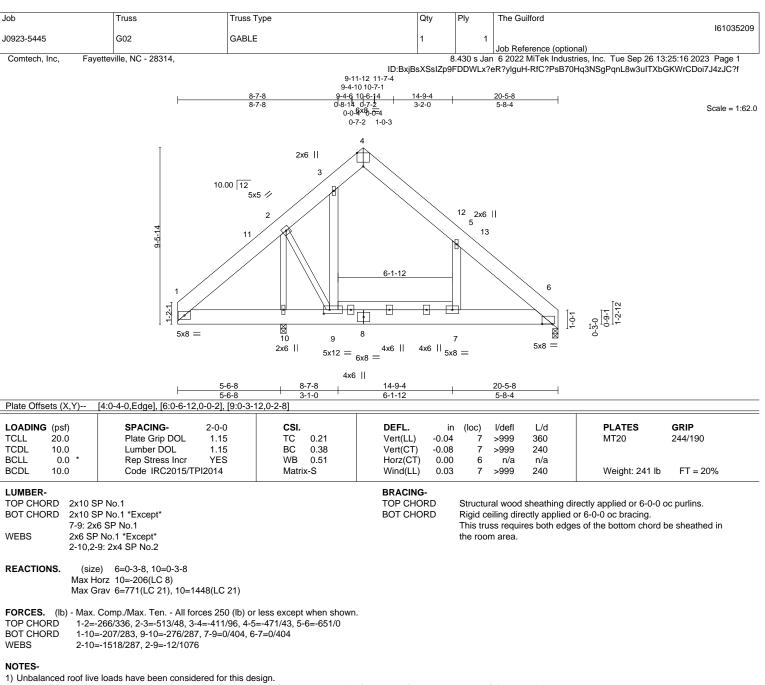
818 Soundside Road Edenton, NC 27932



TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0 to 4-4-13, Interior(1) 4-4-13 to 9-11-12, Exterior(2) 9-11-12 to 14-4-9, Interior(1) 14-4-9 to 20-0-3 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 3-4, 4-5; Wall dead load (5.0psf) on member(s).3-9, 5-7

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 7-9

7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.

8) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

9) Attic room checked for L/360 deflection.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

Comtech, Inc, Fag	vetteville, NC - 28314,			8.430 s Ja	Job Reference (optional) n 6 2022 MiTek Industries, Inc. Tue Sep 26	13:25:17 2023 Page 1
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			3			
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		5	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	4		
				x4		

LOADIN	· · ·	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	0.00	1	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI	2014	Matri	x-R						Weight: 19 lb	FT = 20%

```
LUMBER-
```

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1 *Except*
	3-4: 2x4 SP No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=2-0-0, 4=2-0-0 Max Horz 5=75(LC 9) Max Uplift 4=-75(LC 12) Max Grav 5=145(LC 1), 4=81(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.



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Job	Truss	Truss Type			Qty	Ply	The Gui	lford		161035211
J0923-5445	M02	JACK-OPEN			Э	1				101035211
Comtech, Inc, Fay	vetteville, NC - 28314,		0.44.0	ID:BxjBs 2-0-0			n 6 2022 I		ial) ies, Inc. Tue Sep 26 1 Iq3NSgPqnL8w3uITXI	
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		3-0-0 ,0-4-8, 1-8-0				—M	2-4-6 3-0-0			
			3x4			4				
				2-0-0 2-0-0						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15 YES	CSI. TC 0.05 BC 0.05 WB 0.00	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	-0.00 -0.00	) 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2015/TPI2		Matrix-R	- ()		-			Weight: 16 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x6 SF				BRACING TOP CHC		Structu except	ral wood s end vertic	sheathing dire	ectly applied or 2-0-0	oc purlins,

WEBS 2x6 SP No.1 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 5=0-3-8, 3=Mechanical, 4=Mechanical (size)

Max Horz 5=65(LC 9)

Max Uplift 3=-41(LC 12), 4=-11(LC 9) Max Grav 5=149(LC 1), 3=53(LC 19), 4=33(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

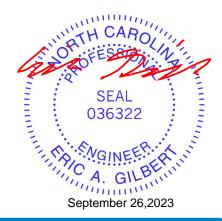
1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

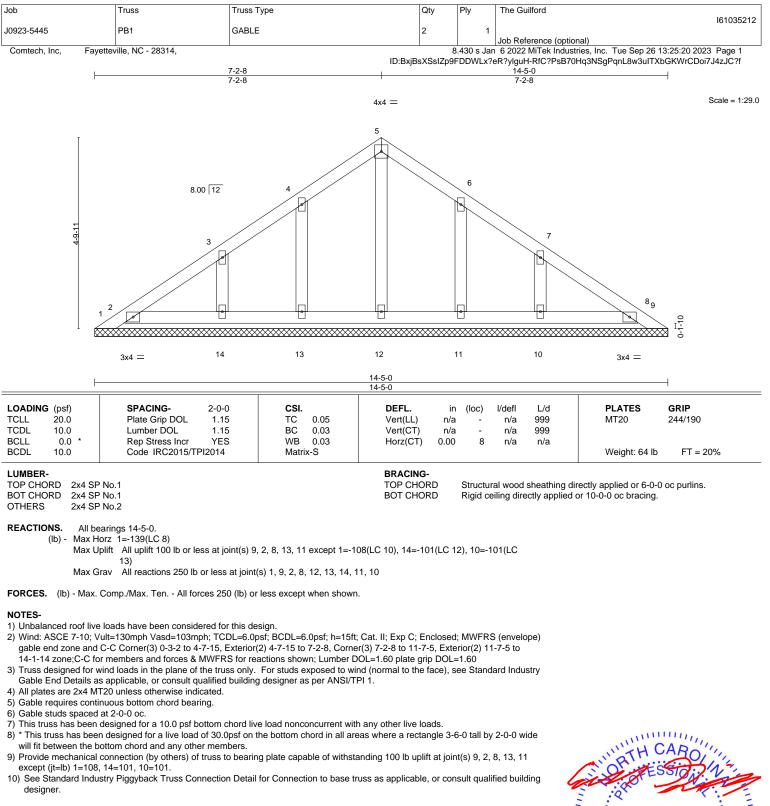
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.



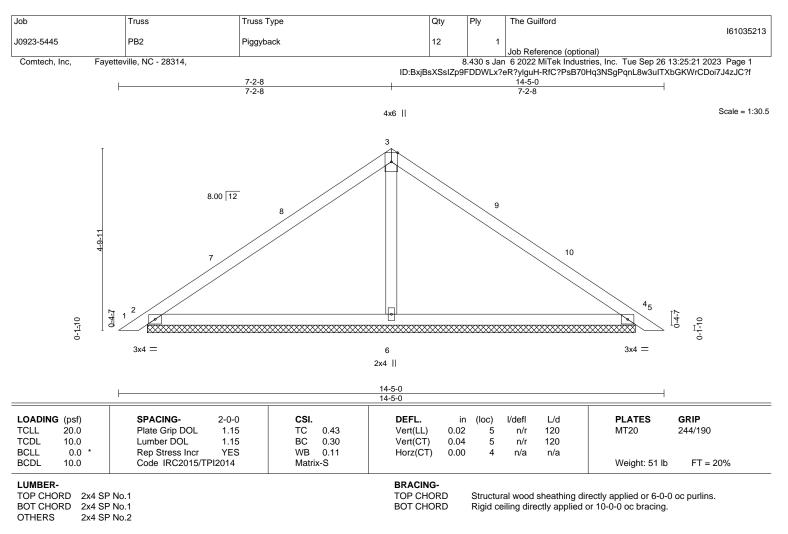
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REACTIONS. (size) 2=12-10-12, 4=12-10-12, 6=12-10-12 Max Horz 2=-111(LC 10) Max Uplift 2=-38(LC 12), 4=-49(LC 13) Max Grav 2=286(LC 1), 4=286(LC 1), 6=520(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 3-6=-321/123

#### NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-2 to 4-7-15, Interior(1) 4-7-15 to 7-2-8, Exterior(2) 7-2-8 to 11-7-5, Interior(1) 11-7-5 to 14-1-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

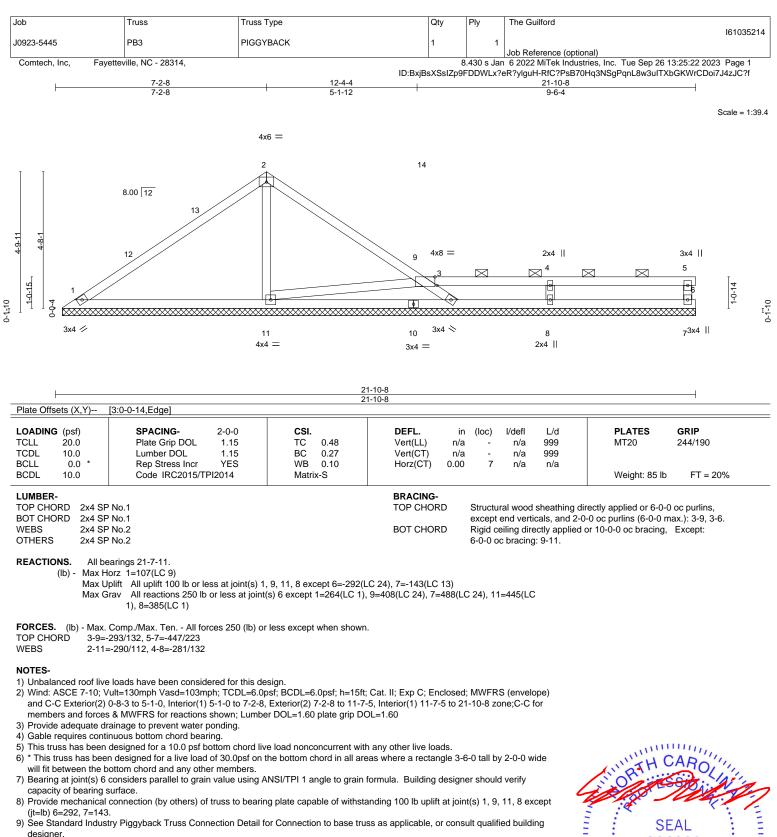
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



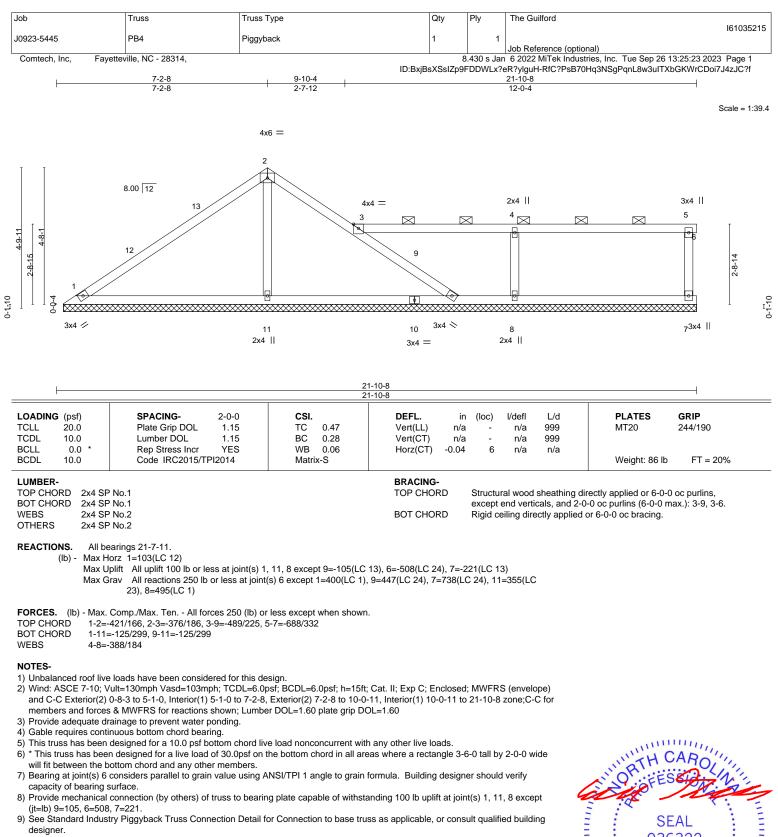
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<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



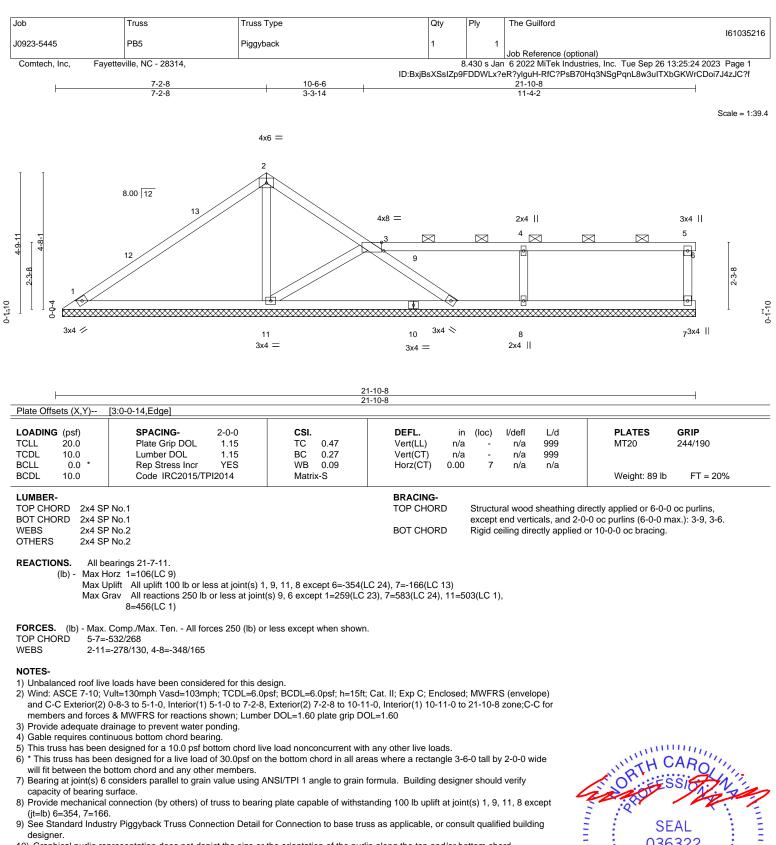


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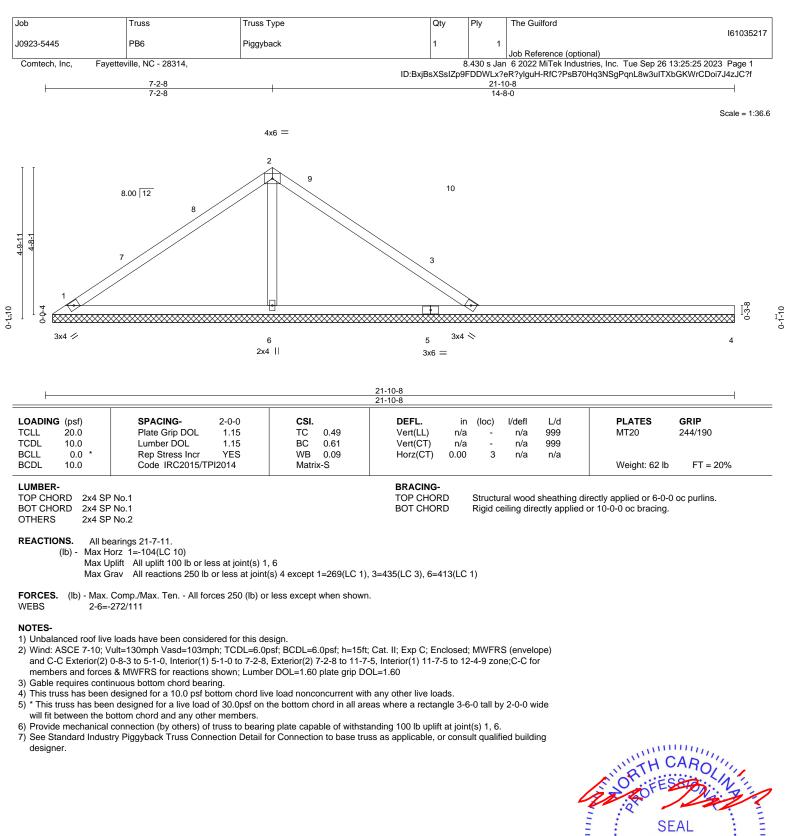
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A MiTek 818 Soundside Road Edenton, NC 27932

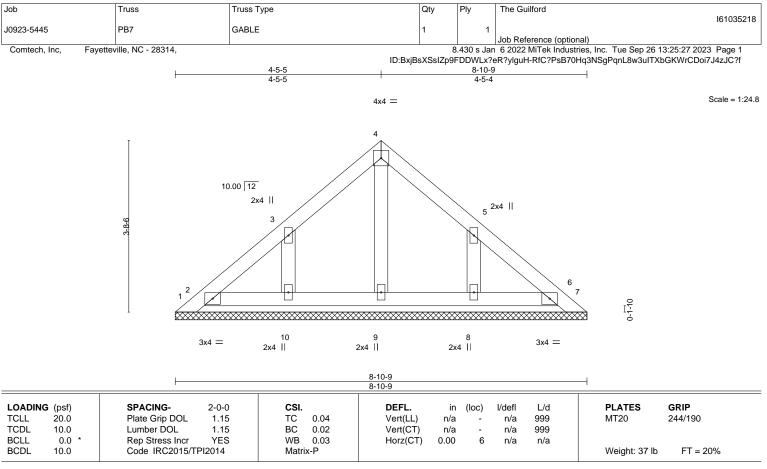




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818 Soundside Road



LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 8-10-9. (Ib) - Max Horz 1=-105(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 2, 6 except 10=-125(LC 12), 8=-124(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 7, 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

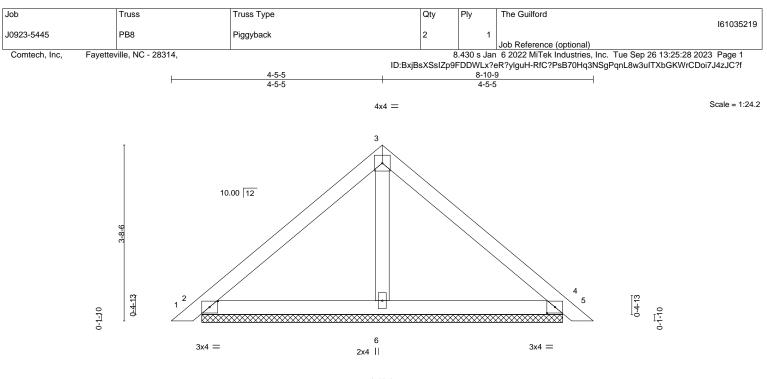
7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 2, 6 except (jt=lb) 10=125, 8=124.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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8-10-9 8-10-9

OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.22	Vert(LL) 0.01	5	n/r	120	MT20 244/190
CDL 10.0	Lumber DOL 1.15	BC 0.11	Vert(CT) 0.01	5	n/r	120	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.03	Horz(CT) 0.00	4	n/a	n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 33 lb FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. (size) 2=7-7-2, 4=7-7-2, 6=7-7-2

Max Horz 2=-84(LC 10) Max Uplift 2=-33(LC 12), 4=-41(LC 13)

Max Grav 2=202(LC 1), 4=202(LC 1), 6=253(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

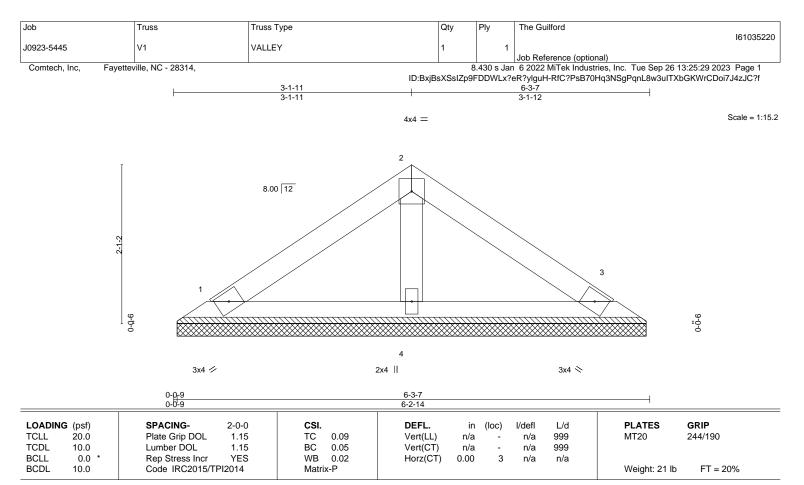


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

lo.1

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=6-2-5, 3=6-2-5, 4=6-2-5 Max Horz 1=-43(LC 8) Max Uplift 1=-18(LC 12), 3=-22(LC 13) Max Grav 1=115(LC 1), 3=115(LC 1), 4=193(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

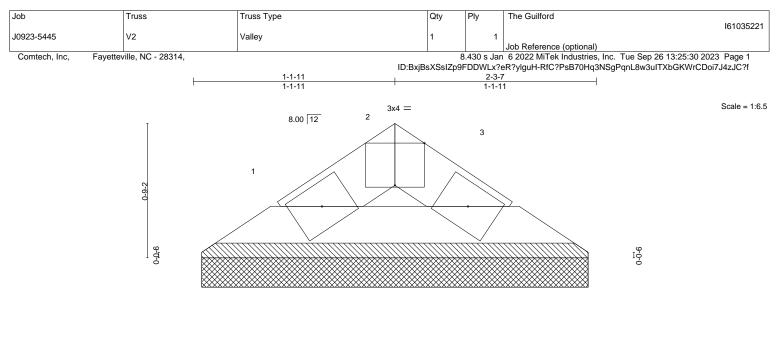
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 6) N/A



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3x4 🥢

3x4 📎

Plate Offsets (X,Y)	0 <u>1</u> 0 <u>19</u> 0 <sup>1</sup> 0 <sup>1</sup> 9 [2:0-2-0,Edge]		2-3-7 2-2-14			
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.01 BC 0.01 WB 0.00 Matrix-P	DEFL. i Vert(LL) n/: Vert(CT) n/: Horz(CT) 0.00	a -	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 6 lb         FT = 20%

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1

REACTIONS. (size) 1=2-2-5, 3=2-2-5 Max Horz 1=-11(LC 8) Max Uplift 1=-3(LC 12), 3=-3(LC 13) Max Grav 1=52(LC 1), 3=52(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 2-3-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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