

# EXPRESS HOMES HAYEN 4 BR

McKay Place Lot 23 93 Finsbury Court Lillington, NC 27546



40' Series

PLAN CHANGES 02.22.21 INITIAL PLAN RELEASE CLIENT REVISIONS CLIENT REVISIONS 04.14.21 CLIENT REVISIONS 12.03.21 CLIENT REVISIONS CLIENT REVISIONS ADDED LIGHT OVER TUB/SHOWER IN BATH 2 04.25.22 CONSULTANTS:

FOR CONSTRUCTION

## GENERAL NOTES DESIGNER NORTH CAROLINA:

THESE DOCUMENTS ARE THE PROPERTY OF THE DESIGNER AND SHALL NOT BE COPIED. PROVIDE BLOCKING AND/OR BACKING AT ALL TOMEL BAR. TOMEL RING AND/OR WRITTEN APPROVAL OF THE DESIGNER

CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE AND ALL INCONSISTENCES SHALL BE BROUGHT TO THE ATTENTION OF THE DEVELOPER AND THE DESIGNER BEFORE PROCEEDING WITH WORK.

ANY ERRORS OR OMISSIONS FOUND IN THESE DRAWINGS SHALL BE BROUGHT TO

DO NOT SCALE DRAWINGS. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED

ALL DIMENSIONS ARE TO FACE OF STUD OR TO FACE OF FRAMING UNLESS

ALL TRUSS DRAMINGS TO BE REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO ISSUANCE OF BUILDING PERMIT. ALL OR EQUAL SUBSTITUTIONS MUST BE SUBMITTED TO AND APPROVED BY CITY BUILDING OFFICIAL PRIOR TO INSTALLATION.

ALL ANGLED PARTITIONS ARE 45 DEGREES UNLESS OTHERWISE NOTED.

PROVIDE FIREBLOCKING. (PER LOCAL CODES.) ALL ELECTRICAL AND MECHANICAL EQUIPMENT AND METERS ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS, CONTRACTOR TO VERIFY. BATHROOMS AND POWDER ROOMS, VERIEY LOCATIONS AT FRAMING WALK

ELASTOMERIC SHEET WATERPROOFING: FURNISH AND INSTALL ALL WATERPROOFING COMPLETE. A OM ILL. SELF-ADHERING MEMBRANE OF RUBBERIZED
ASPHALT INTEGRALLY BONDED TO POLYETHYLENE SHEETING, OR EQUAL.
INSTALL PER MANIFACTURES AND TRADE ASSOCIATIONS PRINTED
INSTALL LATION INSTRUCTIONS. 6" MINIMUM LAP AT ALL ADJACENT WALL SURFACES.

TO THE BEST OF THE DESIGNER'S KNOWLEDGE THESE DOCUMENTS ARE IN CONFORMANCE WITH THE REQUIREMENTS OF THE BUILDING AUTHORITIES HAVING JURISDICTION OVER THIS TYPE OF CONSTRUCTION AND OCCUPANCY.

SHOP DRAMING REVIEW AND DISTRIBUSTION, ALONG WITH PRODUCT SUBMITTALS, REQUESTED IN THE CONSTRUCTION DOCUMENTS, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR, UNLESS DIRECTED OTHERWISE UNDER A SEPARATE AGREEMENT.

DEVIATIONS FROM THESE DOCUMENTS IN THE CONSTRUCTION PHASE SHALL BE REVIEWED BY THE DESIGNER AND THE OWNER PRIOR TO THE START OF WORK IN QUESTION. ANY DEVIATIONS FROM THESE DOCUMENTS WITHOUT PRIOR REVIEW, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL WORK AND MATERIALS REPRESENTED ON THESE DOCUMENTS INCLUDING MATERIALS FURNISHED BY SUBCONTRACTORS AND VENDORS.

SEOTECHNICAL ENGINEER (SOILS REPORT), ON THE STUDY OF THE PROPOSED SITE, TO THE DESIGNER, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR. IN THE EVENT THE GEOTECHNICAL REPORTS DO NOT EXIST. THE SOILS CONDITION SHALL BE ASSUMED TO BE A MINIMUM DESIGN SOIL PRESSURE STATED BY THE STRUCTURAL ENGINEER OF RECORD FOR THE PURPOSE OF STRUCTURAL DESIGN GENERAL CONTRACTOR SHALL ASSURE THE SOIL CONDITIONS MEET OR EXCEED

ALL WORK PERFORMED BY THE GENERAL CONTRACTOR SHALL COMPLY AND CONFORM WITH LOCAL AND STATE BUILDING CODES, ORDINANCES AND REGULATIONS, ALONG WITH ALL OTHER AUTHORITIES HAVING JURISDICTION. THE GENERAL CONTROLTOR IS RESPONSIBLE TO BE AWARE OF THESE REGUIREMENTS AND GOVERNING REGILATIONS

PROVIDE AN APPROVED WASHER DRAIN PAN AT SECOND FLOOR ONLY

WINDOW SUPPLIER TO VERIFY AT LEAST ONE WINDOW IN ALL BEDROOMS TO HAVE A CLEAR OPENABLE AREA OF 4,0 SQ FT. THE MINIMUM NET CLEAR OPENING HEIGHT SHALL BE 22° AND THE MINIMUM NET CLEAR OPENING MIDTH SHALL BE 20°, GLAZING TOTAL AREA OF NOT LESS THAN 5.0 SQ FT IN THE CASE OF A GROUND WINDOW AND NOT LESS THAN 5.7 SQ FT IN THE CASE OF AN UPPER STORY WINDOW. (PER NCRC SECTION R3IO.1.!)

ALL HANDRAIL BALLUSTERS TO BE SPACED SUCH THAT A 4" SPHERE CANNOT PASS BETWEEN BALLUSTERS, (PER LOCAL CODES.) PROVIDE STAIR HANDRAILS AND GUARDRAILS PER LOCAL CODES.

### BUILDER SET:

THE SCOPE OF THIS SET OF PLANS IS TO PROVIDE A "BUILDER'S SET" OF CONSTRUCTION DOCUMENTS AND GENERAL NOTES HEREINAFTER REFERRED TO AS "PLANS" THIS SET OF PLANS IS SUFFICIENT TO OBTAIN A BUILDING PERMIT; HOWEVER, ALL MATERIALS THIS SET OF PLANS IS SUFFICIENT TO OBTAIN A BUILDING PERMIT; HOMEVER, ALL MATERIALS AND METHODS OF CONSTRUCTION NECESSARY TO COMPETER THE PROJECT ARE NOT NECESSARILY DESCRIBED. THE PLANS DELINEATE AND DESCRIBE ONLY LOCATIONS, DIMENSIONS, TYPES OF MATERIALS, AND GENERAL METHODS OF ASSEMBLING OR FASTENING. THEY ARE NOT INTENDED TO SPECIFY PARTICULAR PRODUCTS OR OTHER METHODS OF ANY SPECIFIC MATERIALS, PRODUCT OR METHOD. THE IMPLEMENTATION OF THE PLANS REQUIRES A CLIENT / CONTRACTOR THOROUGHLY KNOWLEDGEABLE WITH THE APPLICABLE BUILDING CODES AND METHODS OF CONSTRUCTION SPECIFIC TO THIS PRODUCT TYPE AND TYPE OF CONSTRUCTION.

CONSTRUCTION REQUIREMENTS AND QUALITY: PROVIDE WORK OF THE SPECIFIC QUALITY WHERE QUALITY LEVEL IS NOT INDICATED, PROVIDE WORK OF QUALITY CUSTOMARY IN SIMILAR TYPES OF WORK. WHERE THE PLANS AND SPECIFICATIONS, CODES, LAWS, REGULATIONS, MANUFACTURER'S RECOMMENDATIONS OR INDUSTRY STANDARDS REQUIRE WORK OF HIGHER QUALITY OR PERFORMANCE, PROVIDE WORK COMPLYING WITH THOSE REQUIREMENTS AND QUALITY WITH THE MOST STRINGENT REQUIREMENT; WHERE REQUIREMENTS ARE DIFFERENT BUT APPARENTL' EQUAL, AND WHERE IT IS UNCERTAIN WHICH REQUIREMENT IS MOST STRINGENT, OBTAIN CLARIFICATION FROM THE 6MD DESIGN GROUP BEFORE PROCEEDING

## AREA CALCULATIONS:

MODEL 'HAYDEN' SQUARE FOOTAGES

\*\*BASEMENT AREA IS TAKEN TO INSIDE OF CONCRETE WALL\*\*

AREA

OPT. COVERED PORCH

2nd FLOOR

GARAGE

TOTAL LIVING



1445 SF

2511 SF

422 SF

109 SF

80 SF

1006 SF

PROJECT NO: GMD17049

TITLE SHEET

January 22, 2021









Front Elevation 'P'
scale: 1/4\*=1'-0" at 22"x34" LAYOUT 1/8\*=1'-0" AT 11"X17" LAYOUT

NO: DATE: REVISION:

PROFESSIONAL SEAL:

PROJECT

40' Series

FOR CONSTRUCTION

CLIENTS NAME:



PROJECT NO: GMD17049

SHEET TITLE:

QUICK VIEW

PRINT DATE:

January 22, 2021

IEET NO:

0.2

#### AVAILABLE WITH OPTIONAL 9'-1" FIRST FLOOR PLATE

#### NOTES AT OPT 9'-1" PLT:

- WDW HT SET AT 7'-6"
- INTERIOR SOFFITS AT 8'-0"
- EXTERIOR SOFFITS AT 8'-0"

#### NOTES:

- GRADE CONDITIONS MAY VARY FOR INDIVIDUAL SITE FROM THAT SHOWN.
  BILLIDER SHALL VERIFY AND COORDINANE PER ACTUAL SITE CONDITIONS.
  HINDOW HEAD HEIGHTS.
  IST FLOOR = 6-6" UNJO. ON ELEVATIONS.
  210 FLOOR = 7-0" UNJO. ON ELEVATIONS.
   ROOPING: PITCHED SHINGLES FER DEVELOPER.

- WINDOWS: MANUFACTURER PER DEVELOPER, DIVIDED LITES AS SHOWN ON THE EXTERIOR ELEVATIONS ENTRY DOOR: AS SELECTED BY DEVELOPER.
- GARAGE DOORS: AS SELECTED BY DEVELOPER, RAISED PANEL AS SHOWN.
- ALL EXTERIOR MATERIALS TO BE INSTALLED PER MANUFACTURER'S WRITTEN INSTRUCTIONS.

- ALL EXTERIOR MATERIALS TO BE INSTALLED PER MANUFACTUREN'S WRITTEN INSTRUCTIC
  PROTECTION AGAINST DECAY.
  (ALL PORTIONS OF A PORCH, SCREEN PORCH OR DECK, FROM THE BOTTOM OF
  THE HEADER DOWN, INCLUDING POST, RAILS, PICKETS, STEPS AND FLOOR STRUCTURE.)
  INSULATION, PER TABLE NILOZI.2.
  EXTERIOR MALLS.
  CELIUS WITH ATTIC ABOVE.
  FLOOR OVER GARAGE:
  R-19 BATTS WINNMM, VERIFY
  CRANL SPACE FLOORING;
  R-19 BATTS WINNMM, VERIFY
  R-19 BATTS MINIMM, VERIFY
  R-19 BATTS MINIMM, VERIFY
  R-19 BATTS MINIMM, VERIFY

#### KEY NOTES:

#### MASONRY:

- ADHERED STONE VENEER AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.
- 2 MASONRY FULL BRICK AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.
- 3 MASONRY FULL STONE AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.
- 4 8" SOLDIER COURSE.
- 5 ROWLOCK COURSE
- NA
  TYPICALS:
  CORROSION RESISTANT SCREEN LOWERED VENTS, SIZE AS NOTED.
- (a) CODE APPROVED TERMINATION CHIMNEY CAP.
- $\boxed{\textbf{9}}$  CORROSION RESISTANT ROOF TO WALL FLASHING. CODE COMPLIANT FLASHING PER NCRC R905.2.8.3
- IO STANDING SEAM METAL ROOF, INSTALL PER MANUFCATURER'S WRITTEN INSTRUCTIONS. II DECORATIVE WROUGHT IRON, SEE DETAILS.
- SIDING.

  | VINTIL SHAKE SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER.
  (AT SPECIFIED LOCATIONS.
  HIBER CEMENT SHAKE SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.)
- VINYL LAP SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER.
   (AT SPECIFIED LOCATIONS:
   FIBER CEMENT LAP SIDING PER DEVELOPER W IX4 CORNER TRIM BOARD.)

- AT SPECIFIED LOCATIONS.
  FIBER CHENT FALL SIDING WI I/3 BATTS AT I/2" O.C. PER DEVELOPER W I/4 CORNER TRIM BOARD

  (AT SPECIFIC LOCATIONS.
  IX FIBER CHENT TRIM OR EQUIA., UN.D. SIZE AS NOTED

- TYPON SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED.

  (AT SPECIFIC LOCATIONS: FALSE VINYL SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED.)
- ALL MINDOMS WHOSE OPENING IS LESS THAN 24" ABOVE THE FINISH FLOOR AND WHOSE OPENING IS GREATER THAN 12" ABOVE THE OUTSIDE WALKING SURFACE MAST HAVE MINDOW OPENING LIMITING DEVICES COMPLYING MITH THE NCRC SECTION R312.21 AND R312.22.



NO: DATE: REVISION: 04.25.22 PROFESSIONAL SEAL:

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40' Series

FOR CONSTRUCTION

CLIENTS NAME:



PROJECT NO: GMD17049

'HAYDEN' **EXTERIOR** 

**ELEVATIONS** '4EPF-K'

PRINT DATE: January 22, 2021

1K

#### ATTIC VENT CALCULATION FOR PLAN 'HAYDEN': 1:150 RATIO.

THE NET FREE VENTILATING AREA SHALL NOT BE LESS THAN THE ART AND VINITATION AND SPACE VENTILATED, PROVIDED THAT AT LEAST 50 PERCENT AND NOT MORE THAN 80 PERCENT OF THE REQUIRED VENTILATING AREA IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE O BE VENTILATED AT LEAST 3 FEET ABOVE THE EAVE OR CORNICE VENTS WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY EAVE OR CORNICE VENTS.

- EXCEPTIONS:

  EXCLOSED ATTIC/RAFTER SPACES REQUIRING LESS THAN

  I SQ FT OF VENTILATION MAY BE VENTED WITH CONTINUOUS

  SOFFIT VENTILATION ONLY.
- 2. ENCLOSED ATTIC/RAFTER SPACES OVER UNCONDITIONED SPACE MAY BE VENTED WITH CONTINUOUS SOFFIT VENT ONLY

GENERAL CONTRACTOR SHALL VERIFY THE NET FREE VENTILATION OF THE VENT PRODUCT SELECTED BY OWNER. VENTILATION OF THE VENT PRODUCT SELECTED BY OWNER. VERIFY WITH MANUFACTURER OF HIGH AND LOW VENTS TO BE USED FOR MINIMUM CALCULATED VENTS REGUIRED. THE REGUIRED VENTILATION SHALL BE MAINTAINED. PROVIDE INSULATION STOP SUCH THAT INSULATION DOES NOT BESTRUCT RESEARCH AND MOVEMENT AS REGUIRED. BY THE BUILDING OFFICIAL

BY THE BUILDING OFFICIAL.
ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE
OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF
SHEATHING KA ALLOWED BY THE STRICKURAL ENGINEER)
TO ALLOW PASSAGE AND ATTIC VENTILATION
BETWEEN THE TWO OR ISOLATED ATTIC SPACES SHALL
BE VENTED INDEPENDENTLY TO CBC REQUIREMENTS.

PER DEVELOPER, AT ALL CANTILEVERED FLOORS,
CANTILEVERED ARCHITECTURAL POP-OUTS, AND ANY DOUBLE CANTILEVERED ARCHITECTURAL POP-COLIS, AND ANT LOUB FRAMING PROJECTIONS THAT ARE SEPARATED FROM THE VENTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2" CORROSION RESISTANT SOFFIT VENT AT UNDERSIDE OF FRAMED ELEMENT.

#### (PER SECTION RAGE 2)

I SQUARE INCH VENT FOR EVERY 150 SQUARE INCHES OF CEILING

\*144 SQ. IN. = 1 SQ. FT. BLDG. CEILING (SF) X 144 = BLDG (SQ. IN.)

BLDG. (5Q. IN.) / 150 = SQ. IN. OF VENT REQUIRED

**ROOF AREA I:=** 1488 SF 1488 SQ. FT. X 144 = 214272 SQ. IN. 214272 SQ. IN. / I50 = 1428.48 SQ. IN. *O*F VENT REQ'D

**ROOF AREA 2:=** 34 SF 34 SQ. FT. X 144 = 5616 SQ. IN. 5616 SQ. IN. / ISO = 37.44 SQ. IN. *O*F VENT REQ'D

**ROOF AREA 3::** 180 SF 180 SQ, FT. X 144 = 25920 SQ, IN, 25920 SQ, IN, / 150 = 172.80 SQ, IN, *O*F VENT REQ'D

- ALL ROOF DRAINAGE SHALL BE PIPED TO STREET OR APPROVED DRAINAGE FACILITY.
- DASHED LINES INDICATE WALL BELOW. - LOCATE GUTTER AND DOWNSPOUTS PER BUILDER.
- PITCHED ROOFS AS NOTED.
- TRUSS MANUFACTURER SHALL SUBMIT STRUCTURAL CALCS AND SHOP DRAWINGS TO THE BUILDER'S GENERAL CONTRACTOR AND BUILDING DEPARTMENT FOR REVIEW FRIOR TO FABRICATIONS.
- ALL PLUMBING VENTS SHALL BE COMBINED INTO A MINIMUM AMOUNT OF ROOF PENETRATIONS, ALL ROOF PENETRATIONS SHALL OCCUR TO THE REAR OF THE MAIN RIDGE.

#### ATTIC VENT CALCULATION FOR PLAN 'HAYDEN': 1:300 RATIO.

AS AN ALTERNATE TO THE 1/150 RATIO LISTED ABOVE. AS AN ALTERRALE TO THE INDER ANTO ESTED ADVE,
THE NET FREE CROSS-VENTILATION AREA MAY BE REDUCED
TO 1/300 WHEN A CLASS I OR II VAPOR RETARDER IS INSTALLED
ON THE WARM - IN - WINTER SIDE OF THE CEILING.

GENERAL CONTRACTOR SHALL VERIFY THE NET FREE VENTILATION OF THE VENT PRODUCT SELECTED BY OWNER. VERIFY WITH MANUFACTIVER OF HIGH AND LOW VENTS TO BE USED FOR MINIMA CALULALTED VENTS REQUIRED. THE REQUIRED VENTILATION SHALL BE MAINTAINED. PROVIDE INSULATION STOP SUCH THAT INSULATION DOES NOT OBSTRUCT FREE AIR MOVEMENT AS REQUIRED BY THE BUILDING OFFICIAL.

ALL OVERLIAD OF INGLE.

ALL OVERLIAD FRAMED ROOF AREAS SHALL HAVE OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF SHEATHING KAS ALLOWED BY THE STRUCURAL ENGINEER)

TO ALLOW PASSAGE AND ATTIC VENTILATION
BETWEEN THE TWO OR ISOLATED ATTIC SPACES SHALL BE VENTED INDEPENDENTLY TO CBC REQUIREMENTS.

PER DEVELOPER, AT ALL CANTILEVERED FLOORS, CANTILEVERED ARCHITECTURAL POP-OUTS, AND ANY DOUBLE FRAMING PRO-ECTIONS THAT ARE SEPARATED FROM THE VENTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2\* CORROSION RESISTANT SOFFIT VENT AT UNDERSIDE OF FRAMED LEMENT.

I SQUARE INCH VENT FOR EVERY 300 SQUARE INCHES OF CEILING

\*144 SQ IN = 1 SQ FT BLDG. CEILING (SF) X 144 = BLDG (SQ. IN.) BLDG. (SQ. IN.) / 300 = SQ. IN. OF VENT REQUIRED

SQ. IN. OF VENT REQUIRED / 2 = 50% AT HIGH & 50% AT LOW.

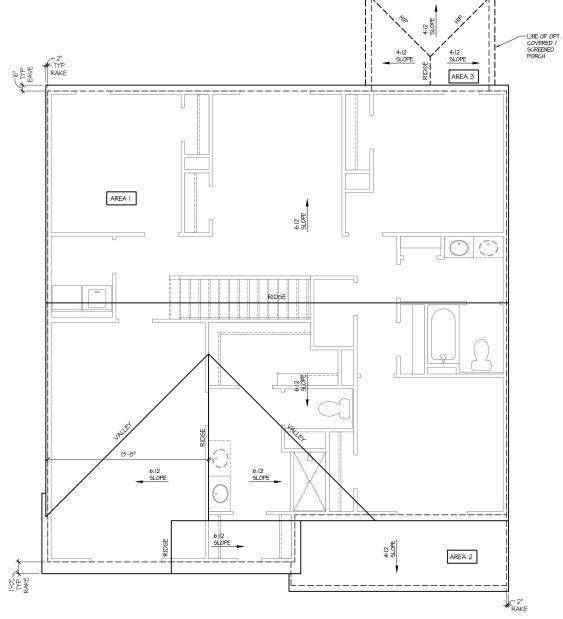
 ROOF AREA 2: = 34 SF.

 36
 50. FT. X 144 = 5616
 5616
 50. FT. X 144 = 1612
 50. IN. OF VENT REA'D

 16:12
 50. IN. / 2 = 436
 436 50. IN. OF VENT AT HIGH 1: 436
 436
 50. IN. OF VENT AT LOW REQUIRED.

BUILDER TO PROVIDE (2) LAYERS OF UNDERLAYMENT AT ANY ROOF W/ A SLOPE FROM 2:12 TO LESS THAN 4:12

> AT SINGLE FAMILY DETACHED PLANS: PREFINISHED VENTED SOFFIT AT EAVE PER MANUFACTURER. (VERIFY FIRE SEPARATION DISTANCE FOR SOFFIT PROTECTION PER NCRC SECTION R302.1.1 AND TABLE R302.1)





NO: DATE: REVISION: 04.25.22 PROFESSIONAL SEAL:

PROJECT TITLE:

40' Series

FOR CONSTRUCTION

CLIENTS NAME:

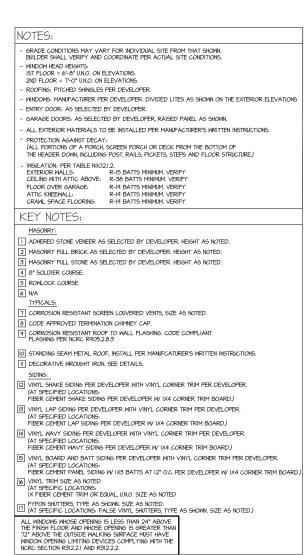


PROJECT NO: GMD17049

'HAYDEN' **ROOF PLAN** '4EPF-K'

PRINT DATE: January 22, 2021

1.1 K



AVAILABLE WITH OPTIONAL

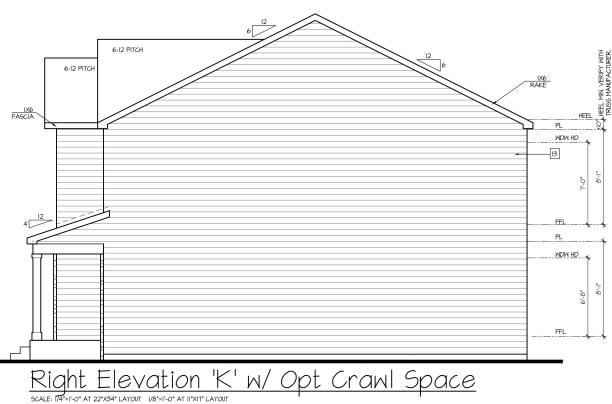
9'-I" FIRST FLOOR PLATE

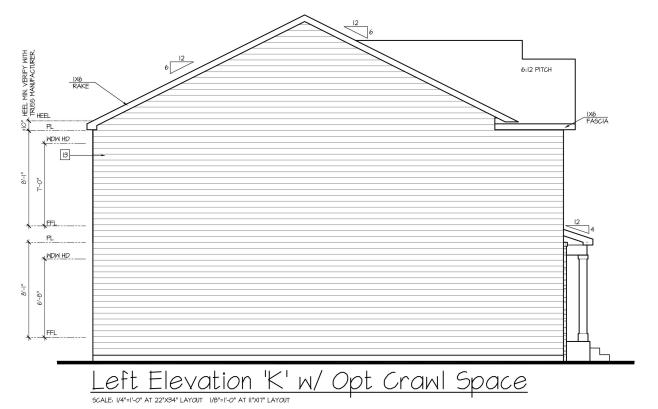
NOTES AT OPT 9'-I" PLT:

- WDW HT SET AT 7'-6"

- INTERIOR SOFFITS AT 8'-0"

- EXTERIOR SOFFITS AT 8'-0"









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40' Series

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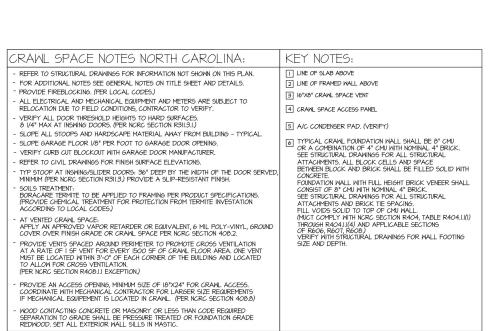
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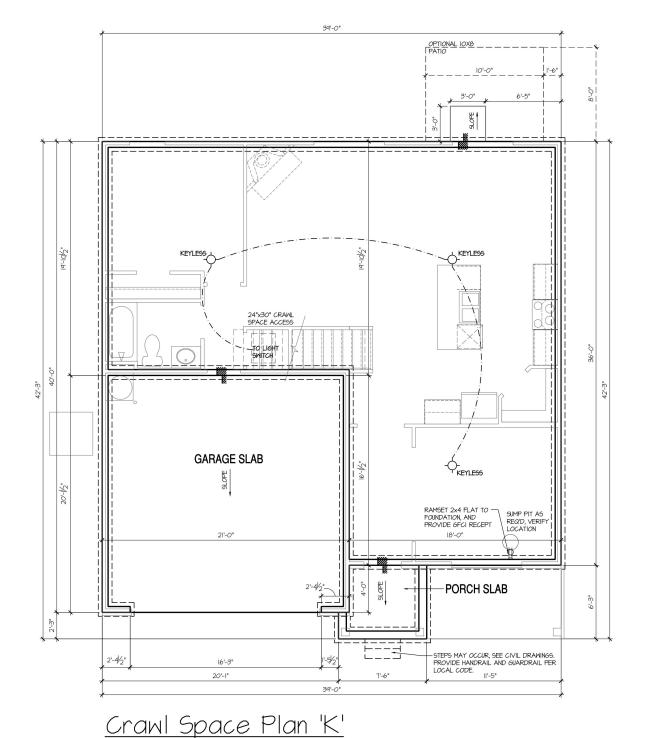
'HAYDEN' EXTERIOR ELEVATIONS '4EPF-K'

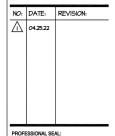
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PRINT DATE:

2.1 K







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40' Series

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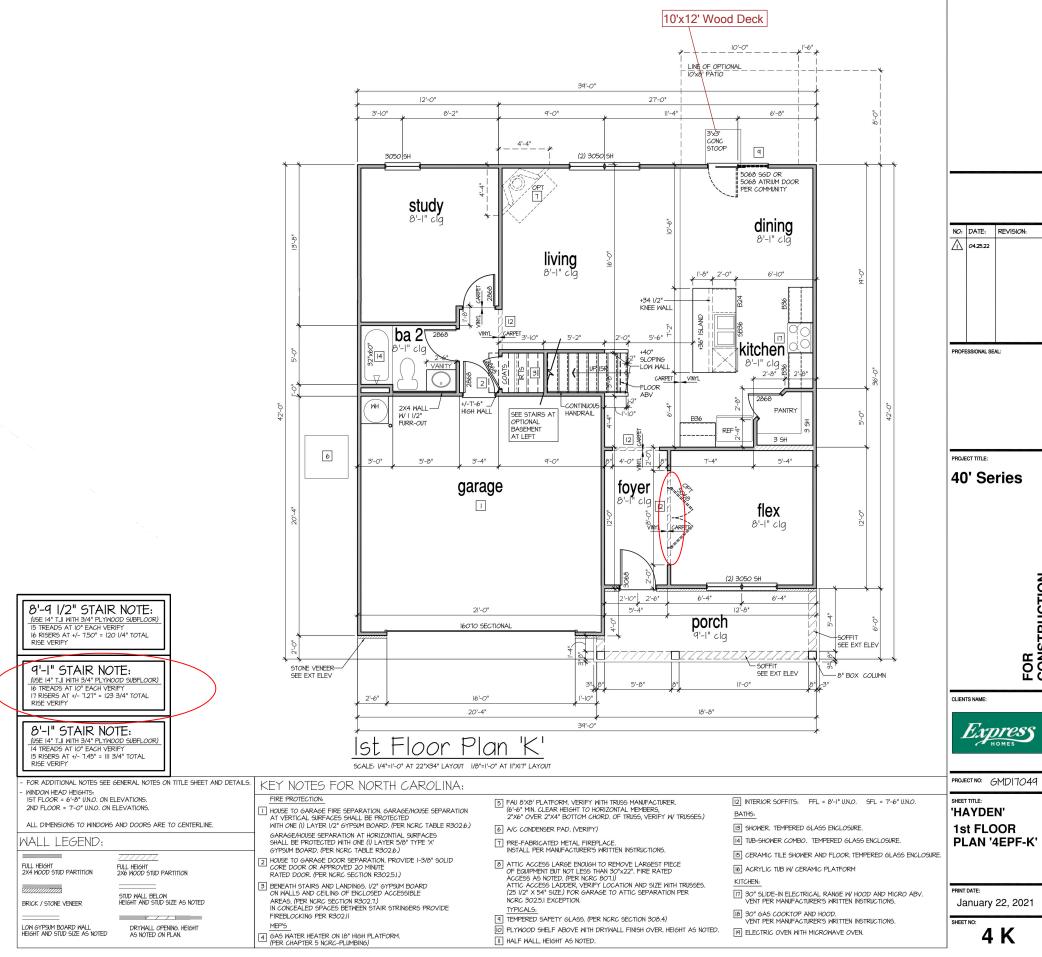
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SHEET TITLE:
'HAYDEN'

CRAWL SPACE
PLAN '4EPF-K'

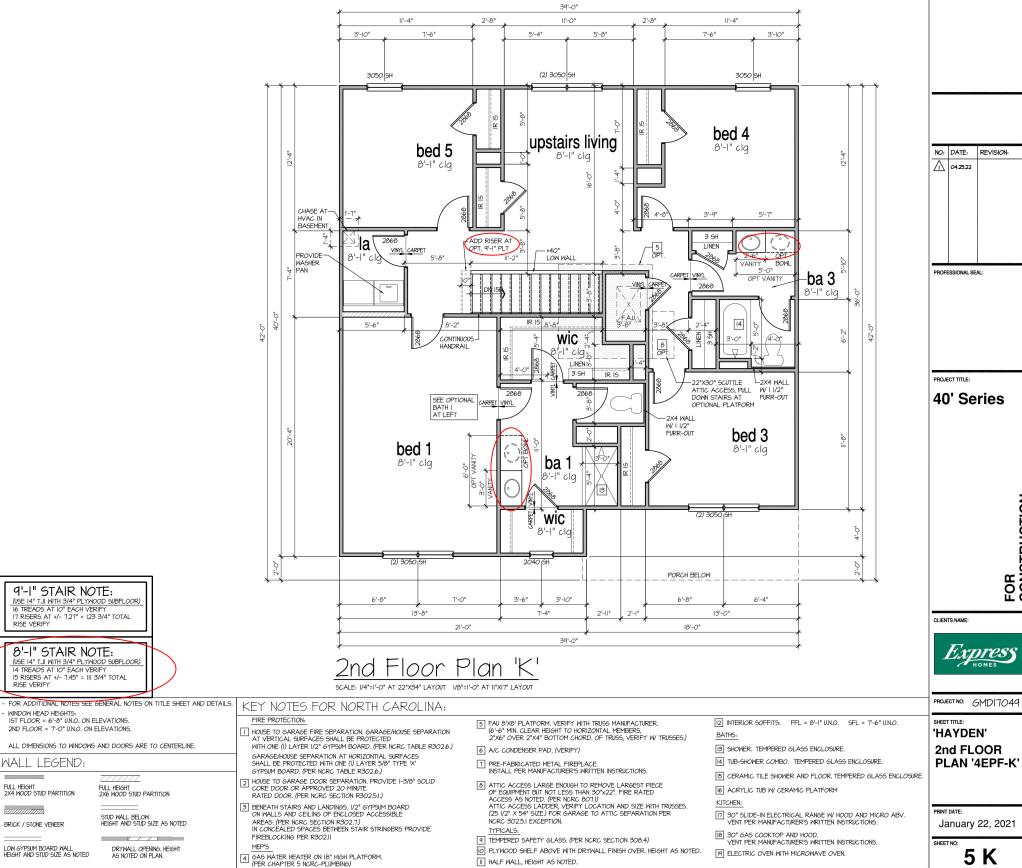
PRINT DATE:
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3 CS K



FOR CONSTRUCTION

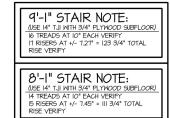




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FOR CONSTRUCTION





#### NOTES:

REFER TO FLOOR PLAN NOTES FOR TYPICAL FIRE PROTECTION NOTES AND LOCATIONS.

THESE BUILDING SECTIONS MAY VARY AT ALTERNATE ELEVATION STYLES AND AT "PLAN OPTION"
CONDITIONS, REFER TO MAIN FLOOR PLAN AND ALTERNATE FLOOR PLANS FOR INFORMATION NOT SHOWN HERE.

BUILDING SECTIONS SHOWN HERE DEPICT VOLUMN SPACES WITHIN THE STRUCTURE, REFER TO STRUCTURAL
DRAWINGS, TRUSS DRAWINGS, STRUCTURAL DETAILS AND CALCULATIONS BY OTHER FOR ALL STRUCTURAL INFO.

ROOFING: PITCHED SHINGLE ROOF, REFER TO ROOF PLAN FOR TYPICALS.

PER STATE RESIDENTIAL CODE COMPLIANCE METHOD TO BE DETERMINED BY BUILDER.

- WOOD FLOORS: FLOOR SHEATHING OVER FLOOR JOIST.
REFER TO STRUCTURAL AND TRUSS DRAWINGS BY OTHERS.

VERIFY STAIRS MINIMUM AND MAXIMUM REQUIREMENTS FOR CONSTRUCTION CLEARANCES WITH LOCAL CODES.

INSULATION:

- INSULATION:
EXTERIOR WALLS ZONE 3:
R-13 BATTS MINIMUM. VERIFY
EXTERIOR WALLS ZONE 4:
R-15 BATTS MINIMUM. VERIFY
CEILING WITH ATTIC ABOVE COMPRESSED INSULATION.
R-30 BATTS MINIMUM. VERIFY
CEILING WITH ATTIC ABOVE INCOMPRESSED INSULATION (HEELS IN TRUSSES):
R-30 BATTS MINIMUM. VERIFY

R-30 BATTS MINIMUM. VERIFY

R-30 BATTS MINIMUM. VERIFY

R-31 BATTS MINIMUM. VERIFY

R-32 BATTS MINIMUM. VERIFY

R-33 BATTS MINIMUM. VERIFY

R-34 BATTS MINIMUM. VERIFY

R-35 BATTS MINIMUM. VERIFY

R-36 BATTS MINIMUM. VERIFY

R-37 BATTS MINIMUM. VERIFY

R-37 BATTS MINIMUM. VERIFY

R-38 BATTS MINIMUM. VERIFY

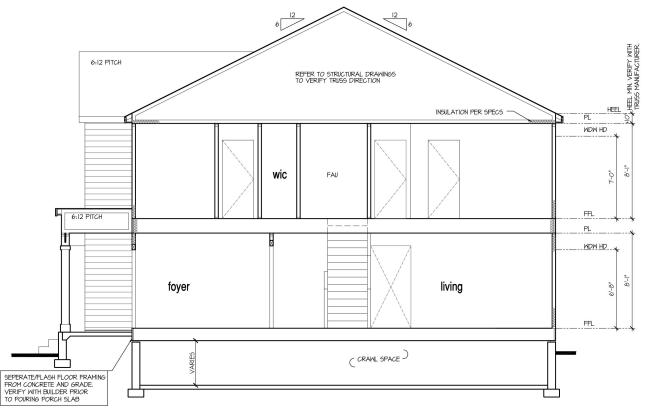
R-38 BATTS MINIMUM. VERIFY

R-38 BATTS MINIMUM. VERIFY

R-39 BATTS MINIMUM. VERI

R-19 BATTS MINIMUM, VERIFY R-19 BATTS MINIMUM, VERIFY R-19 BATTS MINIMUM, VERIFY FLOOR OVER GARAGE: ATTIC KNEEWALL: CRAWL SPACE FLOORING:

WINDOW GLAZING "U" FACTOR: 0.35



Building Section Lat Crawl Space Scale: 1/4\*=1-0\* AT 22\*\$34\* LAYOUT 1/8\*=1-0\* AT 11\*XIT\* LAYOUT

INSULATION PER SPECS-MDW HD ba 3 WDW HD ba 2 kitchen ( CRAWL SPACE ) Building Section 2 at Crawl Space Scale: 1/4"=1"-0" at 227934" LAYOUT 1/9"=1"-0" at 11"71" LAYOUT

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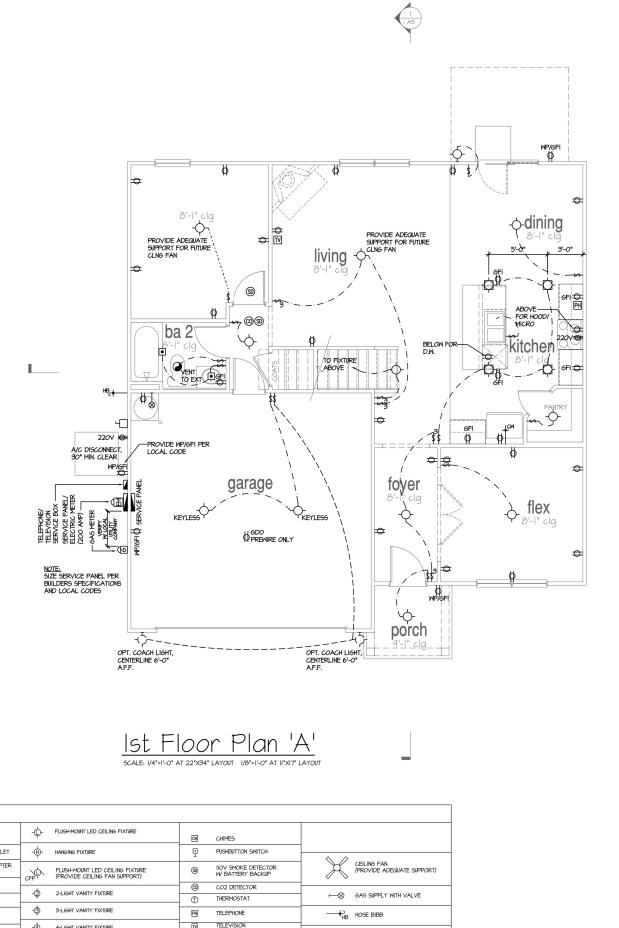


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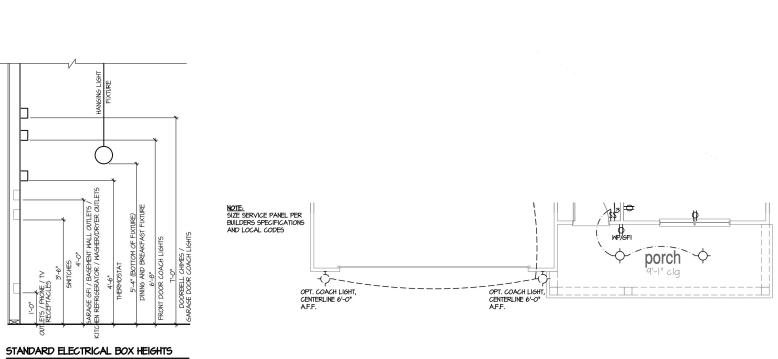
'HAYDEN' **BUILDING SECTIONS** 

PRINT DATE: January 22, 2021

1.1.2A S



→ WALL SCONCE



1st Floor Plan 'B' SCALE: I/4"=I'-0" AT 22"X34" LAYOUT | I/8"=I'-0" AT II"XIT" LAYOU NOTES: LEGEND: PROVIDE GROUNDING ELECTRICAL ROD PER LOCAL CODES. PROVIDE AND INSTALL ARC FAULT CIRCUIT-INTERRIPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. DUPLEX OUTLET ALL EXHAUST FANS SHALL HAVE BACKDRAFT DAMPERS. MP/GFI WEATHERPROOF GFI DUPLEX OUTLET FAN/LIGHTS IN WET/DAMP LOCATIONS SHALL BE LABLED "SUITABLE FOR WET OR DAMP LOCATIONS." - ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE ENGINEERED BY OTHERS, THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION AND PLACEMENT.
- PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRIPTERS (6FI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES. 30" HIGH VANITY BASE CABINET HALF-SWITCHED DUPLEX OUTLET \$220V 220 VOLT OUTLET REINFORCED JUNCTION BOX ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS. HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS. 4-LIGHT VANITY FIXTURE WALL SWITCH ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FIRMACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PI DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS. PROVIDE POWER, LIGHT AND SMITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS. ☐ ELECTRIC METER THREE-WAY SWITCH WALL MOUNT FIXTURE SWITCH AND RECEPTACLE BOXES OVER KITCHEN CABINETS SWITCH AND RECEPTACLE BOXES ELECTRIC PANEL FOUR-WAY SWITCH OVER BATH CABINETS EXHAUST FAN (VENT TO EXTERIOR)  NO: DATE: REVISION:

042522

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CLIENTS NAME:



PROJECT NO: GMD17049

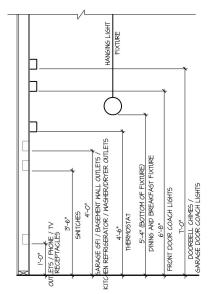
SHEET TITLE:
'HAYDEN'

1st FLOOR UTILITY PLAN

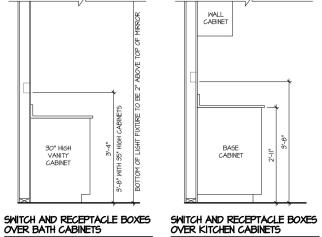
PRINT DATE:
January 22, 2021

ΓNO:

7



#### STANDARD ELECTRICAL BOX HEIGHTS



NOTES:

- PROVIDE GRONDING ELECTRICAL ROD PER LOCAL CODES.
- PROVIDE AND INSTALL ARC FAULT CIRCUIT-INTERRUPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- ALL EXPANSIT FANS PHALL HAVE BACKDRAFT DAMPERS.
- FANALIGHTS IN WET/DAMP LOCATIONS SHALL BE LABLED "SUITABLE FOR WET OR DAMP LOCATIONS."
- ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE REGISTREMERED BY OTHERS, THE CONTRACTOR SHALL BE REGISTREMED ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE REGISTREMERED BY OTHERS, THE CONTRACTOR SHALL BE REGISTREMED ELECTRICAS AND CAZD PETECTORS AS REQUIRED BY NATIONAL FIRE PROVIDE AND INSTALL GROUND FAULT CIRCUIT-INTERRUPTERS (SFI) AS REQUIRED BY NATIONAL FIRE PROVIDE REQUIREMENTS OF ALL GOVERNING CODES.
- PROVIDE AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- PROVIDE AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- PROVIDE AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- PROVIDE TOWER LIGHT AND MECHANICAL EQUIPMENT LOCATIONS.
- LECTRICAL CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.
- ALL ELECTRICAL AND MECHANICAL EQUIPMENT PROVIDES REQUIRED DIVECT HOOK-LAYERS.
- ALL ELECTRICAL AND MECHANICAL EQUIPMENT PROVIDES REQUIRED DIVECT HOOK-LAYERS.
- PROVIDE POWER, LIGHT AND SMITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURERS WITTEN INSTRUCTIONS.

bed 4 PROVIDE ADEQUATE SUPPORT FOR FUTURE bed 5 upstairs living 8'-1" clg PROVIDE ADEQUATE SUPPORT FOR FUTURE CLNG FAN PROVIDE ADEQUATE SUPPORT FOR FUTURE CLNG FAN 0 (SD) PROVIDE 2ND GFI/LIGHT AT OPTIONAL BOWL la ba 3 **®** VENT TO EXT **-**099 VENT TO EXT PROVIDE ADEQUATE SUPPORT FOR FUTURE CLNG FAN -PROVIDE 2ND - SS GFI/LIGHT AT OPTIONAL BOWL 25GFN  $\rightarrow$  bed 3 MENT TO EXT bed 1 PROVIDE ADEQUATE & SUPPORT FOR FUTURE CLNG FAN (E) ba 1 ĭ ♦ V. 0 WIC

## 2nd Floor Plan 'A' scale: 1/4\*=1-0" at 22\*x34" layout 1/8\*=1-0\* at 11\*x17" layout

LEGEND: -- FLUSH-MOUNT LED CEILING FIXTURE DUPLEX OUTLET CHIMES DWP/GFI WEATHERPROOF GFI DUPLEX OUTLET PUSHBUTTON SWITCH HANGING FIXTURE CEILING FAN (PROVIDE ADEQUATE SUPPORT) GFI GROUND-FAULT CIRCUIT-INTERRUPTER DUPLEX OUTLET (9) IIOV SMOKE DETECTOR W BATTERY BACKUP FLUSH-MOUNT LED CEILING FIXTURE (PROVIDE CEILING FAN SUPPORT) HALF-SWITCHED DUPLEX OUTLET CO2 DETECTOR - 2 2-LIGHT VANITY FIXTURE ---- GAS SUPPLY WITH VALVE \$220V 220 VOLT OUTLET THERMOSTAT ① **-**(3) 3-LIGHT VANITY FIXTURE PH TELEPHONE → HB HOSE BIBB REINFORCED JUNCTION BOX TELEVISION 4-LIGHT VANITY FIXTURE WALL SWITCH ☐ ELECTRIC METER \$3 THREE-WAY SWITCH WALL MOUNT FIXTURE ELECTRIC PANEL → WALL SCONCE \$4 FOUR-WAY SMITCH EXHAUST FAN (VENT TO EXTERIOR) 

NO: DATE: REVISION:

O4.25.22

PROFESSIONAL SEAL:

PROJECT TITLE:

40' Series

FOR CONSTRUCTION

CLIENTS NAME:



PROJECT NO: GMD17049

IEET TITLE:

'HAYDEN' 2nd FLOOR UTILITY PLAN

PRINT DATE:
January 22, 2021

IEET NO:

3

Construction Type: Commerical ☐ Residential ☒

Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments

ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

## Design Loads:

1.	KOOI	Live Loads		
	1.1.	Conventional 2x	20	PE
	1.2.	Truss	20	PE
		1.2.1. Attic Truss	60	P
2.	Roof	Dead Loads		
	2.1.	Conventional 2x	10 F	-SF
	2.2.	Truss	20	PS
3.	Snow		15 F	SF
	3.1.	Importance Factor	1.0	
4.	Floor	Live Loads		
	4.1.	Typ. Dwelling	40	P

42. Sleeping Areas . 40 PSF 5. Floor Dead Loads Conventional 2x 10 PSF

6. Ultimate Design Wind Speed (3 sec. gust) ...
6.1. Exposure ..... 6.2. Importance Factor... 6.3. Wind Base Shea

6.3.1. VX = 6.32. VY = 7. Component and Cladding (in PSF)

MEAN ROOF HT.	UP TO 30'	30'1"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.6,-18.9	18.3,-19.7	18.8,-20.2
ZONE 2	16.7,-21.0	17.6,-22.1	18.3,-22.9	18.8,-23.6
ZONE 3	16.7,-21.0	17.6,-22.1	18.3,-22.9	18.8,-23.6
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.8	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.2	20.4,-26.9

8. Seismic

00151111	_	
8.1.	Site Class	D
82.	Design Category	C
8.3.	Importance Factor	1.0
0.1	Catanta Han Canada	1

Spectral Response Acceleration
 S.S.I. Sms = %g
 S.S.2.Sml = %g

86 Seismic Base Shea

8.62.Vu = 8.7. Basic Structural Sustem (check one) □ Bearing Wall
 □ Building Frame
 □ Moment Frame

Dual w/ Special Moment Frame Dual w/ Intermediate R/C or Special Steel

Inverted Pendulum

Wind 🖂 9. Assumed Soil Bearing Capacity 20000sf

### HAYDEN LH

PROJECT ADDRESS

DR Horton, Inc. 8001 Arrowridge Blvd. Charlotte, NC 28273

DESIGNER: GMD Design Group 102 Fountain Brook Circle Suite C Cary, NC 27511

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory 4 Testing, P.C. before construction begins.

#### PLAN ABBREVIATIONS

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	9C	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
ΕW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
oc	ON CENTER	TYP	TYPICAL
P6F	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
P5I	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory 4 Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by D.R. Horton, Inc., Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

#### SHEET LIST:

REVISION LIST:

Sheet No.	Description
C9I	Cover Sheet, Specifications, Revisions
S1.Øm	Monolithic Slab Foundation
S1.Øs	Stem Wall Foundation
51.0c	Crawl Space Foundation
S1.0b	Basement Foundation
52.0	Basement Framing Plan
63.0	First Floor Framing Plan
54.0	Second Floor Framing Plan
S5.Ø	Roof Framing Plan
S6.Ø	Basement Bracing Plan
ST.Ø	First Floor Bracing Plan
58.0	Second Floor Bracing Plan

Revision No.	Date	Project No.	Description
1	4.19.21	TØ177	Updated elevation names
			Added Stem Wall, Crawlspace, and Basement Foundations
2	6.14.21	TØ177	Added OX-15 option and table for framing
3	11.23.21	TØ177	Updated framing in the first floor
		1	

#### DR HORTON PROJECT SIGN-OFF:

Manager	Signature
Operations	
Operations System	
Operations Product Development	

SUMMIT



DR Horton, Inc. 8001 Arrowridge Blv. Charlotte, NC 28213



DATE: IV23/2002 9CALE: 22x34 1/4"+1"-@" lbd1 1/8"+1"-@"

CHECKED BY: CTB

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

STRUCTURAL MEMBERS ONL

PROJECT 4 528-TØITT DRAWN BY: JCEF

GENERAL STRUCTURAL NOTES:

The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, and the periormance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory 4 Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.

The structure is only stable in its completed form. The contractor

shall provide all required temporary bracing during construction to stabilize the structure.

The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents.

should any non-conformities occur.

Any structural elements or details not fully developed on the construction drawings shall be completed under the direction o a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as i relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT. Verification of assumed field conditions is not the responsibility.

of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before

construction begins.

The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically

noted on the structural drawings.

This structure and all construction shall conform to all applicable sections of the international residential code.

of the current local building code.

This structure and all construction shall conform to all applicable sections of local building codes.

All structural assemblies are to meet or exceed to requirements

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding

The bottom of all footings shall extend below the frost line for rise portion of all rootings and extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade. Any fill shall be placed under the direction or recommendation of a licensed professional engineer.

The resulting soil shall be compacted to a minimum of 95% extending the control of the procession of the provider of the procession of the provider o

maximum dry density.

Excavations of footings shall be lined temporarily with a 6 mil polysthylene membrane if placement of concrete does not occur within 24 hours of excavation.

No concrete shall be placed against any subgrade containing water, ice, frost, or loose materia

STRUCTURAL STEEL

Structural steel shall be fabricated and erected in accordance with the American institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design"

Structural steel shall receive one coat of shop applied rust-inhibitive paint. All steel shall have a minimum yield stress  $\langle F_u \rangle$  of 36 ksi unless

otherwise noted.

Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS DIL. Electrodes for shop and field welding shall be class ETØXX. All welding shall be performed by a certified welder per the above

<u>ICREIE:</u>

Concrete shall have a normal weight aggregate and a minimum compressive strength (f'<sub>c</sub>) at 28 days of 3000 psi, unless otherwise noted on the plan.

Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301:

"Specifications for Structural Concrete for Buildings".

Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:

3.1. Footings: 5% 32.Exterior Slabs: 5%

No admixtures shall be added to any structural concrete without written permission of the SER.

Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab

The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pcl and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions

Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished

Reinforcing steel may not extend through a control joint.

Reinforcing steel may extend through a saw cut joint.

All welded wire fabric (W.W.F.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WWF, shall be securely supported during the concrete pour.

CONCRETE REINFORCEMENT:

Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered

aue to strinkage and tremal expansionicontraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
Filoemesh reinforcing to be 100% virgin polygropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
Application of fibermesh per cubic yard of concrete shall equal

a minimum of 0.1% by volume (1.5 pounds per cubic yard)
Fibermesh shall comply with ASTM CIII6, any local building code
requirements, and shall meet or exceed the current industry

Steel reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.

Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"

Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B

tension splice. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonru shall be a minimum of 48 bar diameters.

Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.

O. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted

WOOD FRAMING Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Southern-Yellow-Pine (SYP) #2.

LVL or PSL engineered wood shall have the following minimum

design values: 2.1. E = 1,900,000 psi 22. Fb = 2600 psi 23. Fv = 285 psi

2.4.Fc = 100 psi Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AUPA standard C-I5. All other moisture exposed wood shall be treated in accordance

with AWPA standard C-2 Nails shall be common wire nails unless otherwise noted. Lag screws shall conform to ANSI/ASME standard B182.1-1981. Lead holes for lag screws shall be in accordance with NDS specifications

All beams shall have full bearing on supporting framing members unless otherwise noted.

Exterior and load bearing stud walls are to be 2x4 SYP \*2 @ 16 O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.

Ning studs shall be continuous, individual studs forming a column shall be attached with one lod nail 6 6" OC. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfe Multi-ply beams shall have each ply attached with (3) 10d nails @

Four and five ply beams shall be bolted together with (2) rows of  $12^n$  diameter through bolts staggered e [6" OC. unless noted otherwise.

WOOD TRUSSES:

The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for

the wood trusses.

The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "I'lnimm Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided folloads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to

The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."

The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.

And chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

WOOD STRUCTURAL PANELS:

"Residential and Commercial" and all other applicable APA

Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide

standards.
All structurally required wood sheathing shall bear the mark of

Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards. All structurally required fiberboard sheathing shall bear the mark or the AFA.

Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more interesting the properties.

STRUCTURAL FIBERBOARD PANELS:

Wood wall sheathing shall comply with the requirements of local

building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction

perpendicular to framing, unless noted otherwise.
Roof sheathing shall be APA rated sheathing exposure 1 or 2.
Roof sheathing shall be continuous over two supports and

attached to its supporting roof framing with (1)-8d CC nail at

6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with

over fraining. Apply building paper over the sheathing as required by the state Building Code.

Wood floor sheathing shall be APA rated sheathing exposure 1

or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshark nail at 6"o/c at panel edges and at 12"o/c in panel

field unless otherwise noted on the plans. Sheathing shall be

applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T4G plywood or lumber blocking unless

otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by th

Sheathing shall have a 1/8" gap at panel ends and edges as

the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur

Sheathing shall have a 1/8" gap at panel ends and edges are

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

CRAWL SPACE FOUNDATION PLAN
SCALE: 1/4\*1"-0" ON 22"334" OR 109"1"-0" ON 11"11"

ELEVATION B.F.K





DR Horton, Inc. 8001 Arrowidge Blvd. Charlotte, NC 28213

Hayden LH Crawl Space Foundation



22869 5/5/9 REFER TO COVER 9-EET FOR A COMPLETE LIST OF REVISIONS

S1.1c

#### BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018
- NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE AMENDMENTS. WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS UP TO
- REFER TO ARCHITECTURAL PLAN FOR DOOR/JUNDOULOPENING SIZES
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R602.10.1
- TADLE ROUSIDI ALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED Ø FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.1.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO). FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON
- FOR COMMISSION OFFICIALS (INCLUDING INFILL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS, FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION
- OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.

  10. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A BRACED WALL LINE
- THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- 12. MASONRY OR CONCRETE STEM WALLS W/ A LENGTH OF 48" OR LESS SUPPORTING A TASONRY OR CONCRETE STEM WALLS W/A LENGTH OF 48" OR LESS SUPPORTING A
  BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE
  R602.104.3 OF THE 2010 NCRC.

   BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN
  ACCORDANCE WITH SECTION R602.104.4

   BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN
  ACCORDANCE WITH SECTION R602.104.5

   CONDENSE WITH SECTION R602.104.5

- 15. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R6021046
- PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.1 (UNO)

GB = GYPSUM BOARD

WSP = WOOD STRUCTURAL PANEL GB = GYP5UM BOARD
C3-XXX = CONT. SHEATHED
FF = PORTAL FRAME
FF-ENG = ENG. PORTAL FRAME

#### GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL
- BUILDING CODE WITH ALL LOCAL AND STATE AMENDMENTS.
  CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONTRACTOR SHALL
  COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM
- 3 CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING
- REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS: PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS:
  MICROLLAM (LVL), F<sub>B</sub> = 1600 PSI, F<sub>1</sub> = 285 PSI, E = 1.9x10<sup>6</sup> PSI
  PARALLAM (PSI), F<sub>1</sub> = 2900 PSI, F<sub>2</sub> = 290 PSI, E = 1.25x10<sup>6</sup> PSI
  ALL WOOD MEMBERS SHALL BE '2 SYP UNLESS NOTED ON PLAN, ALL
  STUD COLUMNS AND JOISTS SHALL BE '2 SYP (WO).
  ALL BEAM'S SHALL BE SUPPORTED WITH A (2) 2x4 '2 SYP STUD COLUMN
  AT EACH END UNLESS NOTED OTHERWISE.
  ALL REINFORCING STEEL SHALL BE GRADE 60 BAR'S CONFORMING TO
  ASTM AGE AND SAULK LUGGE A MINIMARY COURSE DE 31<sup>8</sup>

- ASTM AGIS AND SHALL HAVE A MINIMUM COVER OF 3".

  8. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN
- PERPENDICULAR TO RAFTERS.
- PERFENDICULAR 10 RAFIERS.

  FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH I/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL I/D3f. MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.

  10. ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP 12,
- DROPPED. FOR NON-LOAD BEARING HEADERS EXCEEDING 8"-0" IN WIDTH AND/OR WITH MORE THAN 2"-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP 12, DROPPED. (UNLESS NOTED OTHERWISE)

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR HORTON
COMPLETED/REVISED ON 4/15/2]. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING LABORATORY 4 TESTING PC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

#### STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS.

ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO
BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

FIRST FLOOR BRACING (FT)

CONTINUOUS SHEATHING METHOD

11.6

BWL 1-1

BWL 1-2 BWL I-A REQUIRED PROVIDED

24.8

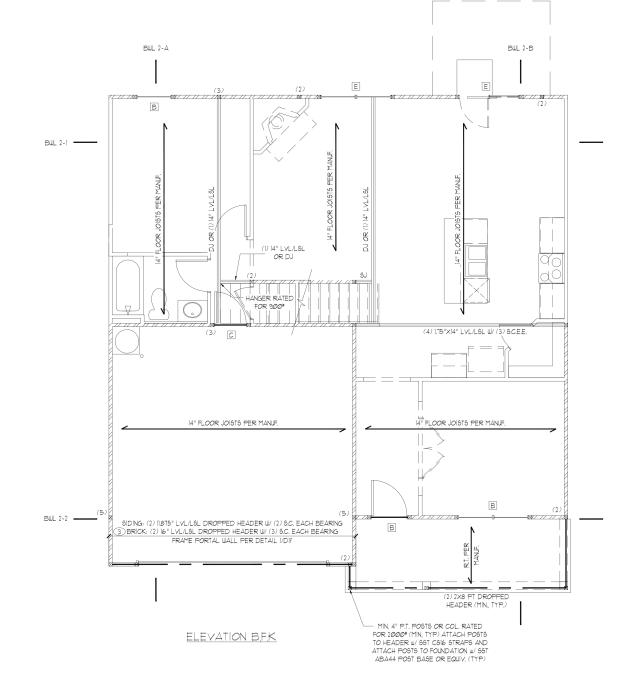
15.0

40.0

STRUCTURAL ANALYSIS BASED ON 2018 NCRC

FIRST FLOOR FRAMING PLAN

SCALE: 1/4"=1'-0" ON 22"x34" OR 1/8"=1'-0" ON 11"x17"



HE,	HEADER SCHEDULE				
TAG	SIZE	JACKS (EACH END)			
Д	(2) 2x6	(1)			
В	(2) 2x8	(2)			
С	(2) 2xlØ	(2)			
D	(2) 2x12	(2)			
E	(2) 9-1/4" LSL/LVL	(3)			
F	(3) 2x6	(1)			
G	(3) 2x8	(2)			
Н	(3) 2xlØ	(2)			
T	(3) 2xl2	(2)			

I. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS, GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION.

2. ALL HEADERS TO BE DROPPED (UN.O.). 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS LISTED ABOVE (UN.O.)

SCHEDULE
MINIMUM KING STUDS E.E.
(1)
(2)
(2)
(3)
(3)
(3)
(4)
(4)

	WALL STUD SCHEDULE (10 FT HEIGHT)						
	STUD SIZE		STUD SPACING (O.C.)				
		ROOF ONLY	ROOF & 1 FLOOR	ROOF & 2 FLOORS	NON-LOAD BEARING		
	2×4	24"	16"	12"	24"		
	2x6	24"	24"	16"	24"		

<u>NOTES:</u>

I. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C.

2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX OF 16" OC

3, TWO STORY WALLS SHALL BE FRAMED W/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED W/ HORIZ. BLOCKING @ 6'-0" O.C. VERTICALLY.

LINTEL SCHEDULE				
TAG	SIZE	OPENING SIZE		
①	L3x3x1/4"	LESS THAN 6'-0"		
2	L5x3x1/4"	6'-0" TO 10'-0"		
3	L5x3-1/2x5/16"	GREATER THAN 10'-0		
4	L5x3-1/2x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS		
SECURE LINTE SCREWS STAG	DIAMETER LAG FOR )			

ALL HEADERS WHERE BRICK IS USED, TO BE:

SHADED WALLS INDICATED LOAD BEARING WALLS

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

JOIST & BEAM SIZES SHOUN ARE MINIMUMS BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDE! THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602.10.8 & FIG. R602.10.7 OF THE 2018 NCRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIET LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC

SUMMIT

SUMMIT

DR Horton, Inc. 8001 Arrowridge E Charlotte, NC 2821

Ω Ø Ĭ.



DATE: 11/23/20/2 PROJECT 5 528-TØTT CHECKED BY: CTB

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S3.1

REQUIRED BRACED WALL PANEL CONNECTIONS							
			REQUIRED CONNECTION				
METHOD	MATERIAL	MIN. THICKNESS	@ PANEL EDGES	@ INTERMEDIATE SUPPORTS			
CS-USP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS # 12" O.C.			
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** ⊕ 1" O.C.	5d COOLER NAILS** @ T" O.C.			
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS	6d COMMON NAILS # 12" O.C.			
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.6.4	PER FIGURE R602.10.6.4			
	"OR EQUIVALENT PER TABLE RT0235						

#### BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE AMENDMENTS
- WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS UP TO
- REFER TO ARCHITECTURAL PLAN FOR DOOR/JUNDOULOPENING SIZES
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R602.10.1
- ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED IØ FEET FOR ISOLATED PANEL METHOD AND IZ FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.1.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO), FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE SURFACES NCLUDING INFILL AREAS BETWEEN BRACED WALL
  PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.

  9. FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION
- OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A BRACED WALL LINE.
- THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT
- 12. MASONRY OR CONCRETE STEM WALLS W/ A LENGTH OF 48" OR LESS SUPPORTING A 12. MASONRY OR CONCRETE STEM WALLS W/A LENGTH OF 48" OR LESS SUPPORTING A
  BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE
  R602.10.4.3 OF THE 2010 NCRC.

  13. BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN
  ACCORDANCE WITH SECTION R602.10.4.4

  14. BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN
  ACCORDANCE WITH SECTION R602.10.4.5

  COORDANCE WITH SECTION R602.10.4.5

- 15. CRIPPLE WALLS AND WALK OUT BASETHENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.104.6
- PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.1 (UNO) 17. ABBREVIATIONS:

GB = GYPSUM BOARD

WSP = WOOD STRUCTURAL PANEL GB = GYP5UM BOARD

C6-XXX = CONT. SHEATHED

FF = PORTAL FRAME

WOT = WOOD VIRULIANAL | AND VIRULIANAL | AND

#### GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL
- BUILDING CODE WITH ALL LOCAL AND STATE AMENDMENTS.

  CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONTRACTOR SHALL

  COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM
- 3 CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING
- REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION. PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS: PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS:
  MICROLLAM (LVL): F<sub>B</sub> = 2600 PSI, Fv = 238 PSI, E = 1.9x10<sup>6</sup> PSI
  PARALLAM (19L): F<sub>B</sub> = 2900 PSI, Fv = 239 PSI, E = 1.25x10<sup>6</sup> PSI
  ALL WOOD MEMBERS SHALL BE 12 SYP UNLESS NOTED ON PLAN, ALL
  STUD COLUMNS AND JOISTS SHALL BE 12 SYP (UND).
  ALL BEAYIS SHALL BE SUPPORTED WITH A (2) 2x4 12 SYP STUD COLUMN
  AT EACH END UNLESS NOTED OTHERWISE.
  ALL RENFORCING STEEL SHALL BE GRADE 60 BARS CONFORMING TO
  ASTMACKER AND SHALL USES AND MISSING COLUMN COLUMN.

- ASTM A615 AND SHALL HAVE A MINIMUM COVER OF 3".

  8. CONTRACTOR TO PROVIDED LOCKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- PERFENDICULAR 10 RAFIERD.
  FLITCH BEAMS, 4-PLY LYLS AND 3-PLY SIDE LOADED LYLS SHALL BE
  BOLTED TOGETHER WITH I/2" DIA. THRU BOLTS SPACED AT 24" O.C.
  (MAX.) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL I/D3f. MIN EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.

  10. ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP 12,
- DROPPED FOR NON-LOAD BEARING HEADERS EXCEEDING 8"-0" IN WIDTH AND/OR WITH MORE THAN 2"-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 6YP 12, DROPPED. (UNLESS NOTED OTHERWISE)

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANS PROVIDED BY DR HORTON
COMPLETED/REVISED ON 4/15/21, IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING LABORATORY 4 TESTING PC. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

#### STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS.

ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO
BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SCALE: 1/4"=1'-Ø" ON 22"x34" OR 1/8"=1'-Ø" ON 11"x17"

SECOND FLOOR FRAMING PLAN

BWL 2-1 6.0 BWL 2-2 6.0 BWL 2-A

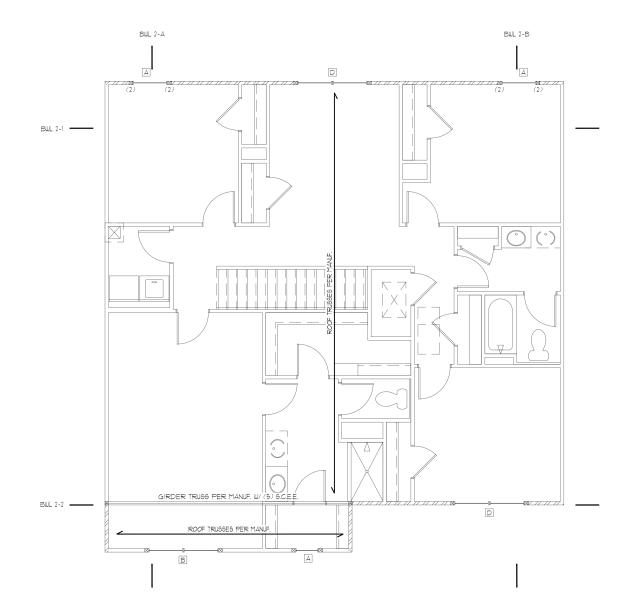
SECOND FLOOR BRACING (FT)

CONTINUOUS SHEATHING METHOD REQUIRED

27.0

25.0

40.0



ELEVATION B.F.K.

HE,	ADER SCHED	JLE
TAG	SIZE	JACKS (EACH END
А	(2) 2x6	(1)
В	(2) 2x8	(2)
С	(2) 2xlØ	(2)
D	(2) 2x12	(2)
E	(2) 9-1/4" LSL/LVL	(3)
F	(3) 2x6	(1)
G	(3)2x8	(2)
Н	(3) 2xlØ	(2)
	(3) 2xl2	(2)

I. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS, GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. 2. ALL HEADERS TO BE DROPPED (UN.O.). 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD

COLUMNS LISTED ABOVE (UN.O.).

KING STUD	SCHEDULE
MAXIMUM HEADER SPAN	MINIMUM KING STUDS E.E.
4'-Ø"	(1)
6'-0"	(2)
8'-0"	(2)
10'-0"	(3)
12'-Ø"	(3)
14'-Ø"	(3)
16'-0"	(4)
18'-0"	(4)

WALL 9	STUD SCI	HEDULE	(10 FT H	HEIGHT)
STUD SIZE		STUD SPA	CING (0.C.)	
	ROOF ONLY	ROOF \$ 1 FLOOR	ROOF \$ 2 FLOORS	NON-LOAD BEARING
2×4	24"	16"	12"	24"
2x6	24"	24"	16"	24"
NOTES				

I. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX OF 16" OC

3. TWO STORY WALLS SHALL BE FRAMED W/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED W/ HORIZ. BLOCKING @ 6'-0" O.C. VERTICALLY.

	LINTEL SCHED	ULE
TAG	SIZE	OPENING SIZE
①	L3x3x1/4"	LESS THAN 6'-0"
2	L5x3x1/4"	6'-0" TO 10'-0"
3	L5x3-1/2x5/16"	GREATER THAN 10'-0'
4	L5x3-1/2x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS
SECURE LINTE	TO HEADER #1/ (2) 1/2"	DIAMETER LAG

9CREWS STAGGERED @ 16" O.C. (TYP FOR 3) ALL HEADERS WHERE BRICK IS USED, TO BE:

SHADED IIIALLS INDICATED LOAD BEARING IIIALLS

JOIST & BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDE THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602:10.8 4 FIG. R602:10.7 OF THE 2018 NCRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLET LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R60235 OF THE 2018 NCRC.



SUMMIT



DR Horton, Inc. 8001 Arrowridge E Charlotte, NC 2821;

QĬ. 00 Ĭ  $\sigma$ Hayden LH Secon



DATE: 11/23/20/2 8CALE: 22x34 |/4"=|'-@" |bd7 |/8"=|'-@" PROJECT 9 528-TØTT DRAWN BY: JCEF CHECKED BY: CTB

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S4.1

TRUS:	TRUSS UPLIFT CONNECTOR SCHEDULE			
MAX. UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FND	
600 LBS	H2.5A	PER WALL SHEATHIN	NG & FASTENERS	
1200 LBS	(2) H2.5A	CSI6 (END = II")	DTT2Z	
145Ø LBS	HT52Ø	CSI6 (END = II")	DTT2Z	
2000 LBS	(2) MTS2Ø	(2) CSI6 (END = II")	DTT2Z	
2900 LBS	(2) HTS2Ø	(2) CSI6 (END = II")	HTT4	
3685 LBS	LGT3-5D62.5	MSTC52	HTT4	

JOBB LBS LEGIS-505/5 MSICS2

1. ALL PRODUCTS LISTED ARE SIMPSON STRONG-TIE. EQUIVALENT PRODUCTS MAY BE USED PER MANUFACTURER'S SPECIFICATIONS.

2. UPLIFT VALUES LISTED ARE FOR STP TO GRADE MEMBERS.

3. REFER TO TRUISS LATOUT PER MANUF, FOR UPLIFT VALUES AND TRUSS TO TRUSS CONNECTIONS. CONNECTOR'S SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE.

4. CONTACT SUMMIT FOR REQUIRED CONNECTOR'S UHEN LOADS EXCEED THOSE LISTED ABOVE.

NOTE: 19T PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, WNO)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

REFER TO DETAIL 5/D3F FOR EYEBROW, RETURN OR SHED ROOF FRAMING REQUIREMENTS, (TYP FOR ROOFS PROTRUDING MAXIMUM 24" FROM STRUCTURE)

NOTE: TRUSS UPLIFT LOADS SHALL BE DETERMINED PER TRUSS MANUFACTURER IN ACCORDANCE WITH SECTION REOZIILI. WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION REOZI35 OF THE 2018 NCRC. REFER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER REQUIREMENTS.

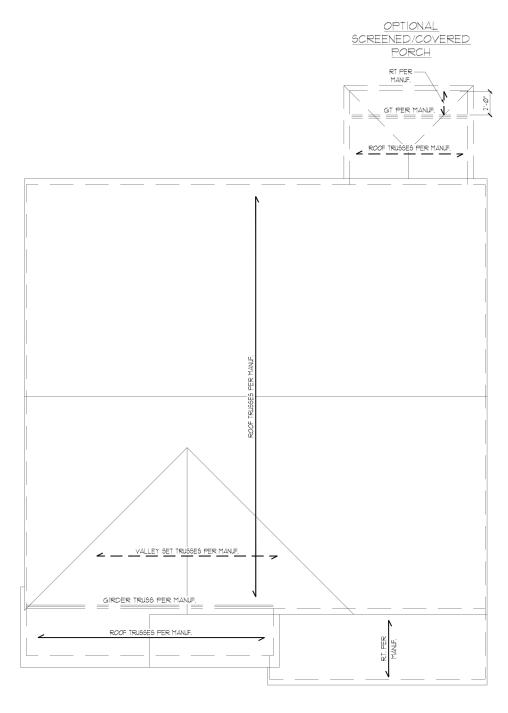
THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANS PROVIDED BY DR. HORTON COMPLETED/REVISED ON A/B/2/. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY & TESTING, P.C., IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

#### STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN SCALE: 1/4"=1'-@" ON 22"x34" OR 1/8"=1'-@" ON 11"x17"



ELEVATION B.F.K.





Framing



DATE: 1/23/2021 8CALE: 22x34 1/4"=1"-@" lkd1 1/8"=1"-@" PROJECT 9 528-TØITT DRAWN BY: JOEF CHECKED BY: CTB

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S5.1

### Applicable Building Codes:

• 2018 North Carolina Residential Building Code with All Local Amendments ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

## Design Loads:

sign Loa	105:	
1. ₹	Roof Live Loads	
	I.I. Conventional 2x	20 PSF
	1.2. Truss	20 PSF
	1.2.1. Attic Truss	60 PSF
2. ₹	Roof Dead Loads	
	2.l. Conventional 2x1	Ø PSF
	2.2. Truss2	20 PSF
3. S	now1	5 PSF
	3.l. Importance Factor1	.Ø
4. F	loor Live Loads	
	4.1. Typ. Dwelling	4Ø PSF

4.2. Sleeping Areas ... 40 PSF 4.3. Decks . 4.4. Passenger Garage .. .50 PSF 5. Floor Dead Loads 5.1. Conventional 2x 5.2. I-Joist .... . 15 PSF

MEAN ROOF HT.	UP TO 30'	3Ø'1"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

#### 8 Seismic

Seismi	C	
8.1.	Site Class	D
8.2.	Design Category	C
	Importance Factor	1.0
8.4.	Seismic Use Group	1
8.5.	Spectral Response Acceleration	

8.5.1. Sms = %a 8.5.2. Sml = %a 8.6. Seismic Base Shear

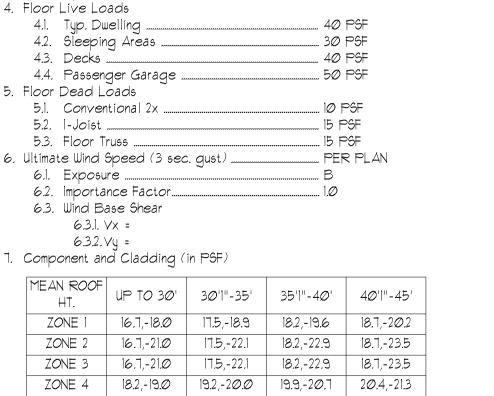
8.6.1. Vx = 8.7. Basic Structural System (check one)

9. Assumed Soil Bearing Capacity .....

 Bearing Wall
 ■ ☐ Building Frame

☐ Moment Frame ☐ Dual w/ Special Moment Frame ☐ Dual w/ Intermediate R/C or Special Steel

□ Inverted Pendulum 8.8. Arch/Mech Components Anchored. 8.9. Lateral Design Control: Seismic 🗆 Wind 🖂





STRUCTURAL PLANS PREPARED FOR:

## STANDARD DETAILS

PROJECT ADDRESS:

DR Horton Carolinas Division 8001 Arrowridge Blvd Charlotte, NC 28273

ARCHITECT/DESIGNER: GMD Design Group 1845 Satellite Blvd. Duluth, GA 30097

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory & Testing, P.C. before construction begins.

#### PLAN ABBREVIATIONS:

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
ΕW	EACH WAY	ŤJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
<i>0</i> C	ON CENTER	TYP	TYPICAL
PSF	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton</u>, <u>Inc.</u> Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

### SHEET LIST:

Sheet No.	Description
CS1	Cover Sheet, Specifications, Revisions
Dlm	Monolithic Slab Foundation Details
Dls	Stem Wall Foundation Details
Dic	Crawl Space Foundation Details
Dlb	Basement Foundation Details
DIf	Framing Details

## DR HORTON PROJECT SIGN-OFF:

Manager	Signature
Operations	
Operations System	
Operations Product Development	

REVISION LIST:

Revision No.	Date	Project No.	Description
1	5.11.17		Added box bay detail (2/D2f). Added deck options with basement. Revised deck options with stem wall and crawl space foundations
2	T.12.1T		Revised stem wall insulation note.
3	2.15.18		Revised garage door detail, NC only
4	2.28.18		Added high-wind foundation details
5	12.19.18		Revised per 2018 NCRC
6	2.19.19		Revised per Mecklenburg County Comments
٦	3.1.19		Revised stem wall deck attachment and roof sheathing on wall sections.
8	3.6.19		Corrected dimensions at perimeter footings
9	3.2.20		Added tall turndown detail
100	3.18.20		Added balloon framing detail
11	10.20.20		Added alternate two-pour detail for slab and added note for crawl girder above grade
12	3.1.21		Added OX-16 Standard Details
13	5.18.21		Updated OX-19 Standard Details
14	Ø2.14.23		Added 4/D2m - Tall Slab Detail w/ Siding

Revision No.	Date	Project No.	Description
1	5.11.17		Added box bay detail (2/D2f). Added deck options with basement. Revised deck options with
			stem wall and crawl space foundations
2	T.12.1T		Revised stem wall insulation note.
3	2.15.18		Revised garage door detail, NC only
4	2.28.18		Added high-wind foundation details
5	12.19.18		Revised per 2018 NCRC
6	2.19.19		Revised per Mecklenburg County Comments
7	3.1.19		Revised stem wall deck attachment and roof sheathing on wall sections.
8	3.6.19		Corrected dimensions at perimeter footings
9	3.2.2Ø		Added tall turndown detail
10	3.18.20		Added balloon framing detail
11	10.20.20		Added alternate two-pour detail for slab and added note for crawl girder above grade
12	3.1.21		Added OX-15 Standard Details
13	5.18.21		Updated OX-IS Standard Details
14	Ø2.14.23		Added 4/D2m - Tall Slab Detail w/ Siding
			-

- GENERAL STRUCTURAL NOTES: The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT
- shall be considered the same entity. 2. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction
- to stabilize the structure. 3. The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- 5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- 7. This structure and all construction shall conform to all applicable sections of the international residential code. 8. This structure and all construction shall conform to all
- applicable sections of local building codes. 9. All structural assemblies are to meet or exceed to requirements of the current local building code.

## FOUNDATIONS:

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.

- 2. The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- 3. Any fill shall be placed under the direction or recommendation of a licensed professional engineer. 4. The resulting soil shall be compacted to a minimum of 95%
- maximum dry density. 5. Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- 6. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

- STRUCTURAL STEEL: Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- Structural steel shall receive one coat of shop applied rust-inhibitive paint.
- 3. All steel shall have a minimum yield stress  $(F_{ij})$  of 36 ksi unless otherwise noted. 4. Welding shall conform to the latest edition of the American
- Welding Society's Structural Welding Code AWS D.I. Electrodes for shop and field welding shall be class ETØXX. All welding shall be performed by a certified welder per the above standards.

- Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- 2. Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- 3. Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows: 3.1. Footings: 5%

## 3.2.Exterior Slabs: 5%

4. No admixtures shall be added to any structural concrete without written permission of the SER.

- 5. Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab Construction".
- 6. The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- 7. Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
- 8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- 9. Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint. 10. All welded wire fabric (W.W.F.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The W.W.F. shall be securely

supported during the concrete pour.

## CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- 2. Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- 3. Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard) 4. Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry
- 5. Steel reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- 6. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures" Horizontal footing and wall reinforcement shall be continuous

and shall have 90° bends, or corner bars with the same

size/spacing as the horizontal reinforcement with a class B tension splice. 8. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

- 9. Where reinforcing dowels are required , they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters
- 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.
- WOOD FRAMING: 1. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Spruce-Yellow-Pine (SYP) #2.
- 2. LVL or PSL engineered wood shall have the following minimum design values:
  - 2.1. E = 1,9*00,000* psi  $2.2.\,F_{\rm b} = 2600\,$  psi  $2.3.F_{V} = 285 \text{ psi}$
- 2.4.Fc = 700 psi 3. Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- 4. Nails shall be common wire nails unless otherwise noted. 5. Lag screws shall conform to ANSI/ASME standard B18.2.1-1981. Lead holes for lag screws shall be in accordance with NDS
- specifications. 6. All beams shall have full bearing on supporting framing members
- 7. Exterior and load bearing stud walls are to be 2x4 SYP #2 @ 16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- 8. Individual studs forming a column shall be attached with one 10d nail @ 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer.

9. Multi-ply beams shall have each ply attached with (3) 10d nails a

10. Flitch beams, 4-ply beams and 3-ply side loaded beams shall be bolted together with (2) rows of 1/2" diameter through bolts staggered @ 16" O.C. unless noted otherwise. Min. edge distance shall be 2" and (2) bolts shall be located a min. 6" from each end of the beam.

## WOOD TRUSSES:

- l. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-05), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to
- 3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- 5. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

## EXTERIOR WOOD FRAMED DECKS:

 Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

## WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA
- 2. All structurally required wood sheathing shall bear the mark of the APA.

- 3. Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Wood floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- 6. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

## STRUCTURAL FIBERBOARD PANELS:

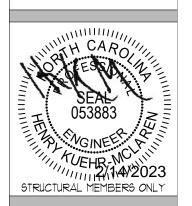
Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards. 2. All structurally required fiberboard sheathing shall bear the

- mark of the AFA. 3. Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more
- 4. Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

SUMMIT RALEIGH, NC 27603 OFFICE: 919.380.9991

120 PENMARC DR., SUITE 108 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM





DATE: 02/14/2023 SCALE: 22x34 |/4"=1'-0" ||x|T |/8"=1'-0" PROJECT \*: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

> PRIGINAL INFORMATION PROJECT \*

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

UNDISTURBED SOIL

6A COVERED PATIO DETAIL

STANDARD - BRICK

CHARTS

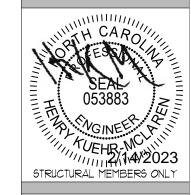
STANDARD - SIDING

6 PATIO SLAB DETAIL

SUMMIT 120 PENMARC DR., SUITE 108 RALEIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM

& Testing, Inc.

CLIENT: DR Horton Carolina Divis 8001 Arrowridge Blvd. Charlotte, NC 28273



DRAWING DATE: Ø2/14/2Ø23 PROJECT \*: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

ORIGINAL INFORMATION

4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR

5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL

AMENDMENTS AND REQUIREMENTS NOT SHOWN

CONNECTIONS

BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND

6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE

ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

Dm

PER PLAN CONTINUOUS

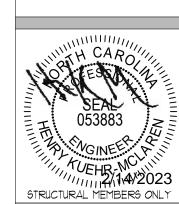
4 TALL SLAB DETAIL W/ SIDING

1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET

- FOR ADDITIONAL INFORMATION. 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE. 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
- 4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS
- 5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
- 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

SUMMIT 120 PENMARC DR., SUITE 108 RALEIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993

WWW.SUMMIT-COMPANIES.COM & Testing, Inc.

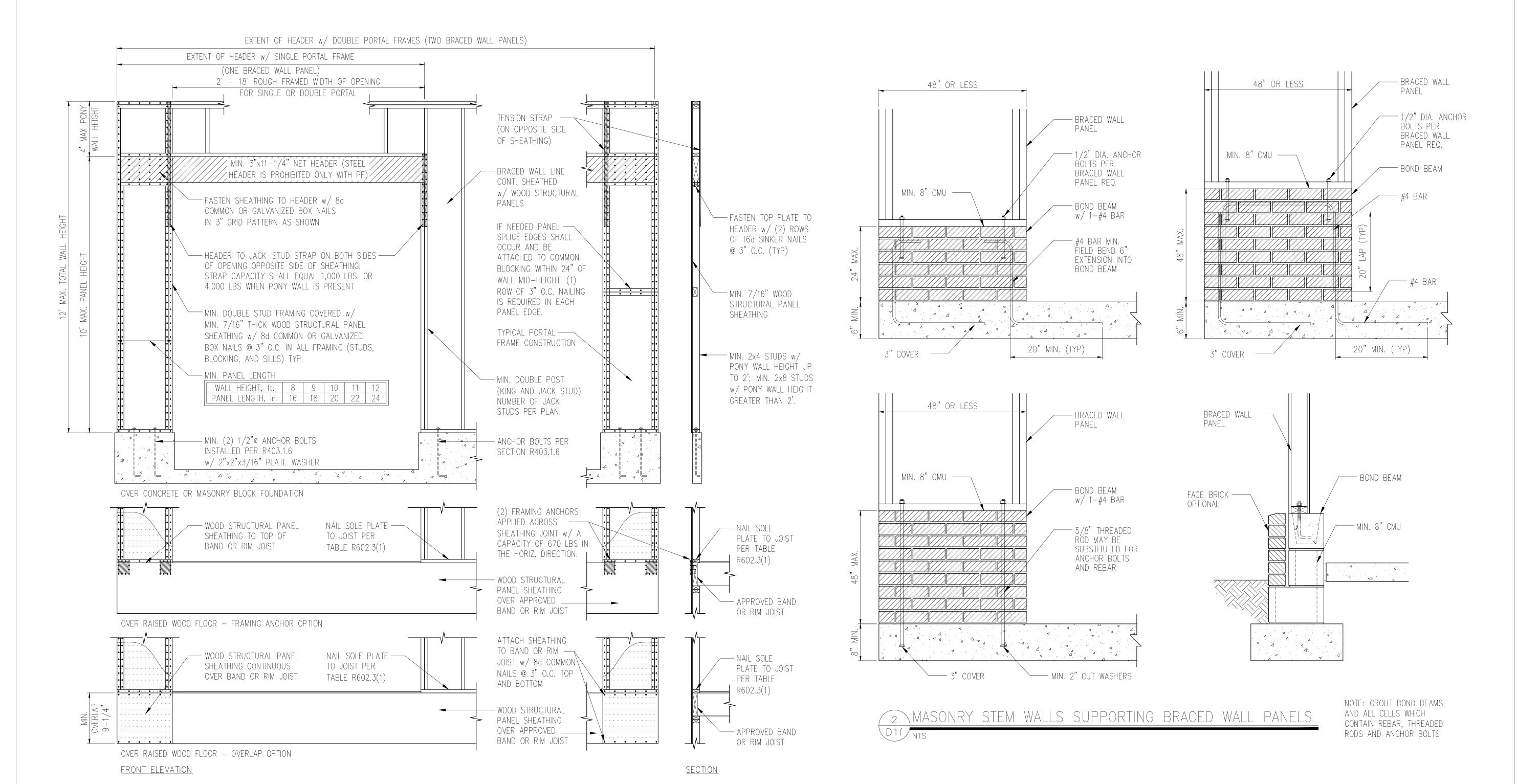


DRAWING DATE: Ø2/14/2Ø23 PROJECT \*: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

> ORIGINAL INFORMATION PROJECT • DATE 1/31/2017

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D2m







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& Testing, Inc. No. F-1454

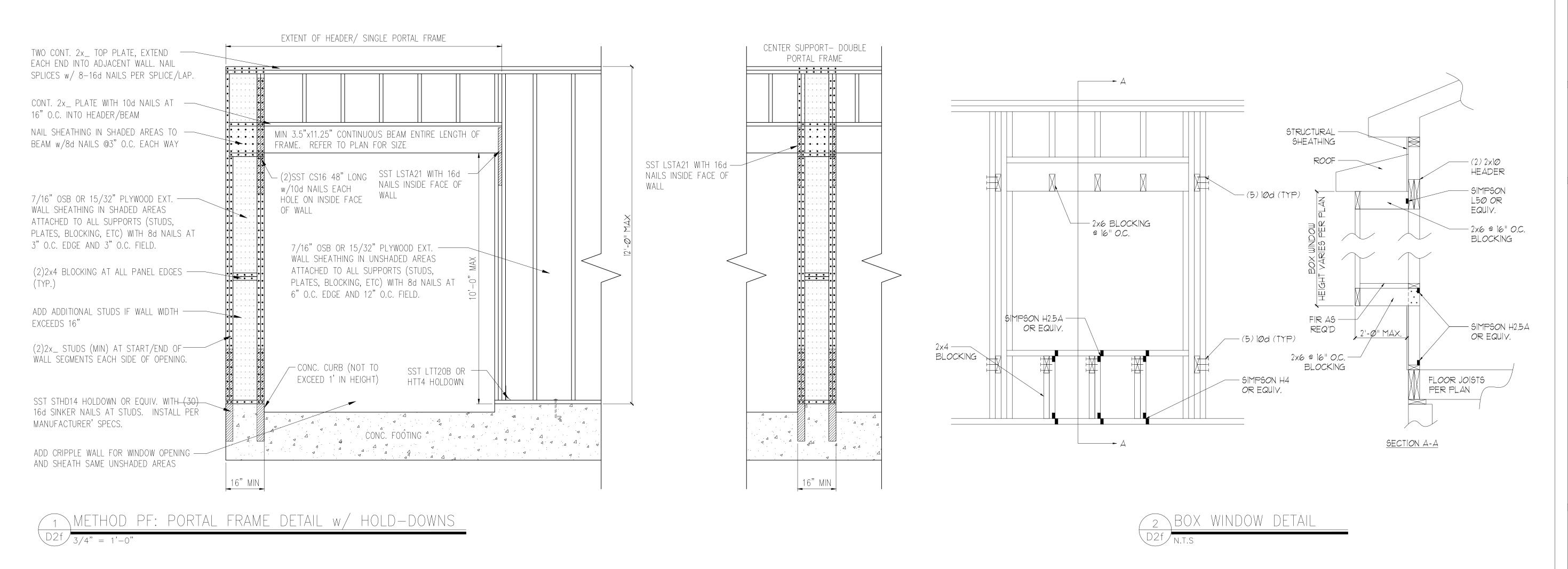


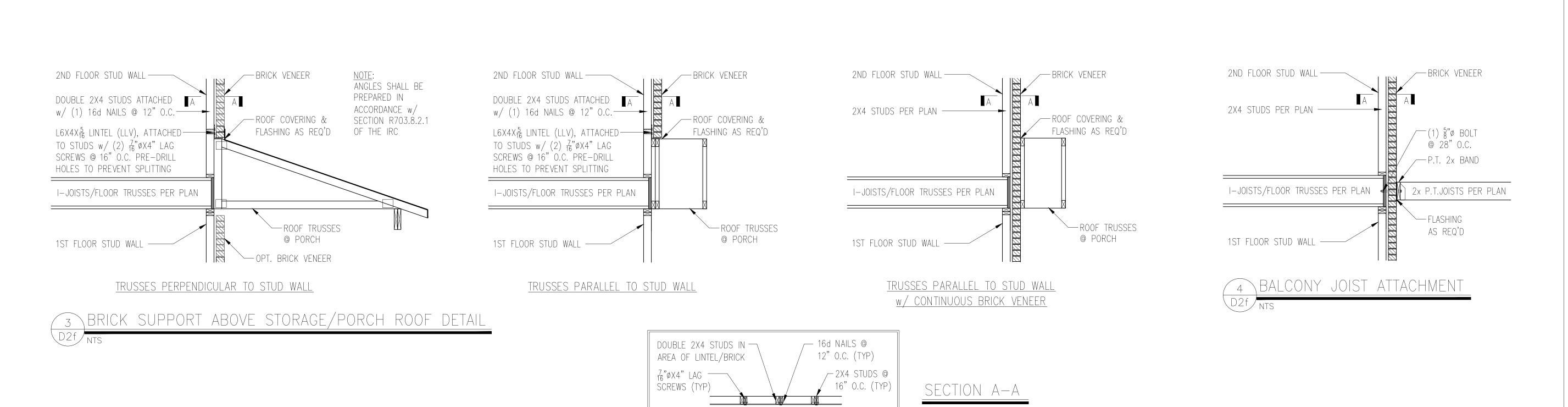
DRAWING DATE: Ø2/14/2Ø23 SCALE: 22x34 1/4"=1'-0" ||x|7 ||/8"=1'-0" PROJECT \*: 528-06R DRAWN BY: JCEF CHECKED BY: BCP

ORIGINAL INFORMATION

PROJECT • DATE 1/31/2017

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

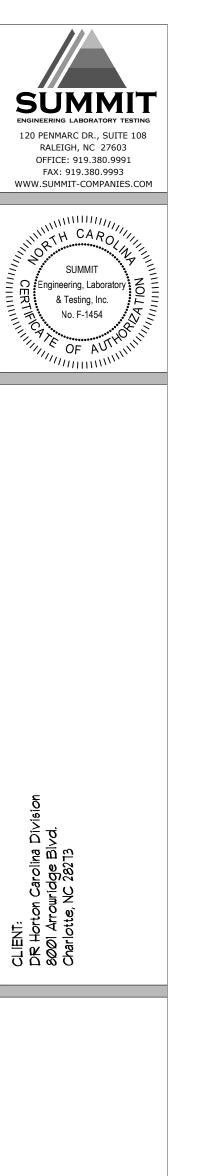




 $L6X4X_{\overline{16}}^{5}$  LINTEL (LLV), ATTACHED

\_ TO STUDS w/ (2)  $\frac{7}{16}$  "øx4" LAG SCREWS @ 16" O.C. PRE-DRILL

HOLES TO PREVENT SPLITTING



ille (0x-15) □@ţâ∐s

 $\bigcirc$ 

STRUCTURAL MEMBERS ONLY

9CALE: 22x34 |/4"=1'-@" ||x|1 |/6"=1'-@" |PROJECT \*: 528-06R

PROJECT • DATE 1/31/2017

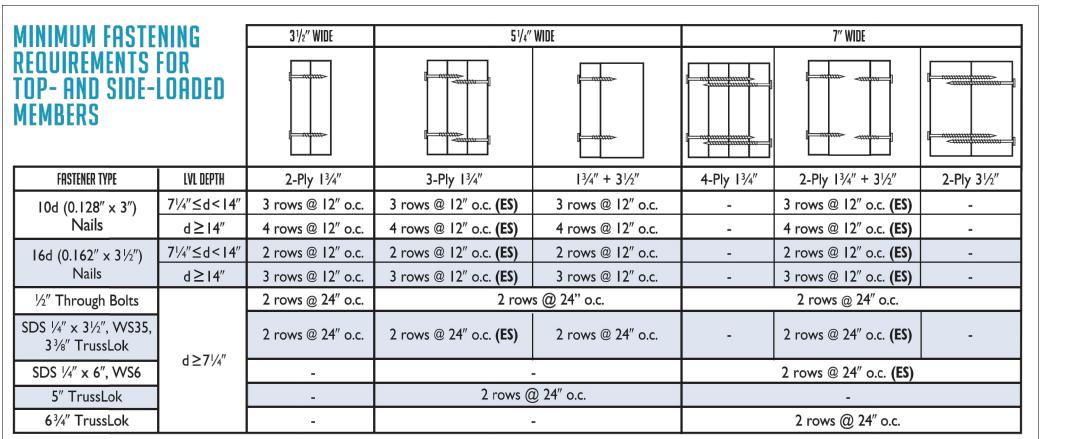
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

DRAWING

DATE: Ø2/14/2Ø23

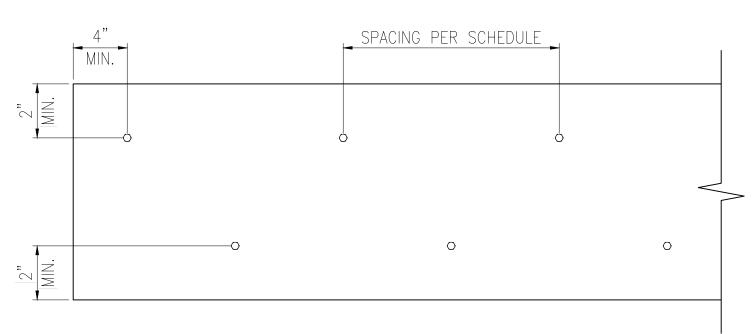
DRAWN BY: JCEF
CHECKED BY: BCP

ORIGINAL INFORMATION



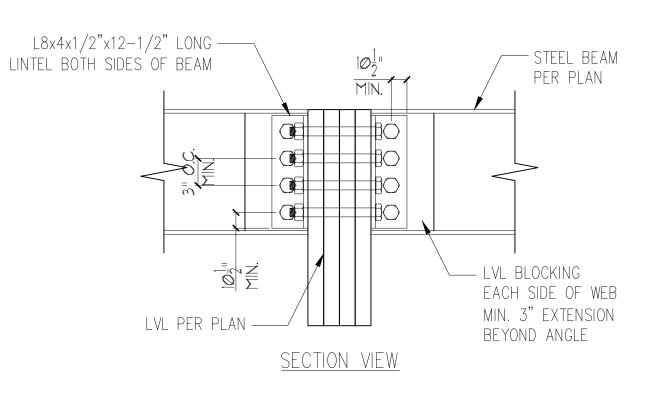
#### NOTES:

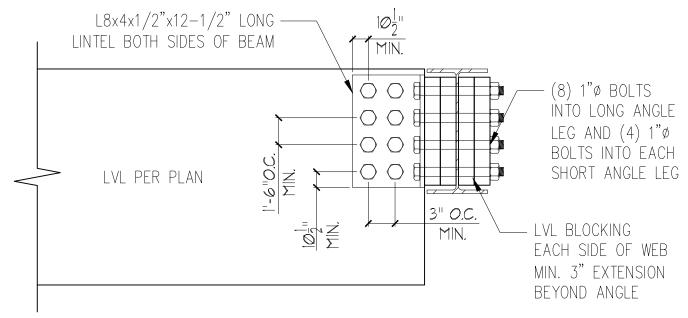
- I.All fasteners must meet the minimum requirements in the table above. Side-loaded multiple-ply members must meet the minimum fastening and side-loading capacity requirements given on page 48.
- 2. Minimum fastening requirements for depths less than  $7\frac{1}{4}$ " require special consideration. Please contact your technical representative.
- 3. Three general rules for staggering or offsetting for a certain fastener schedule:
- (1) if staggering or offsetting is not referenced, then none is required;(2) if staggering is referenced, then fasteners installed in adjacent rows on the front side are to be staggered up to one-half the o.c. spacing, but maintaining the fastener
- clearances above; and
  (3) if "ES" is referenced, then the fastener schedule must be repeated on each side, with the fasteners on the back side offset up to one-half the o.c. spacing of the front side (whether or not it is staggered).



ELEVATION VIEW

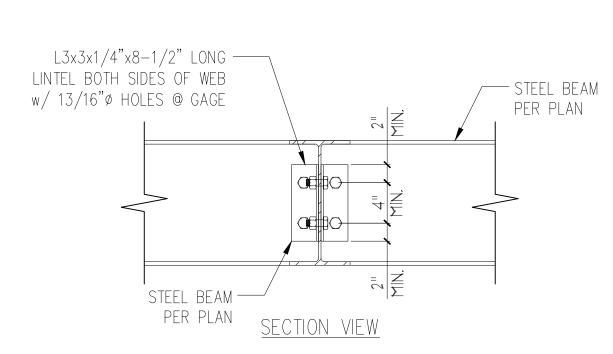
## MULTI-PLY BEAM CONNECTION DETAIL

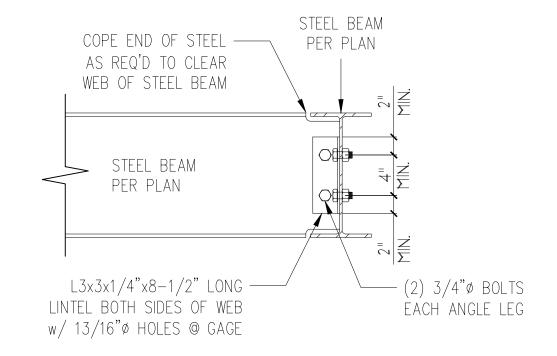




ELEVATION VIEW

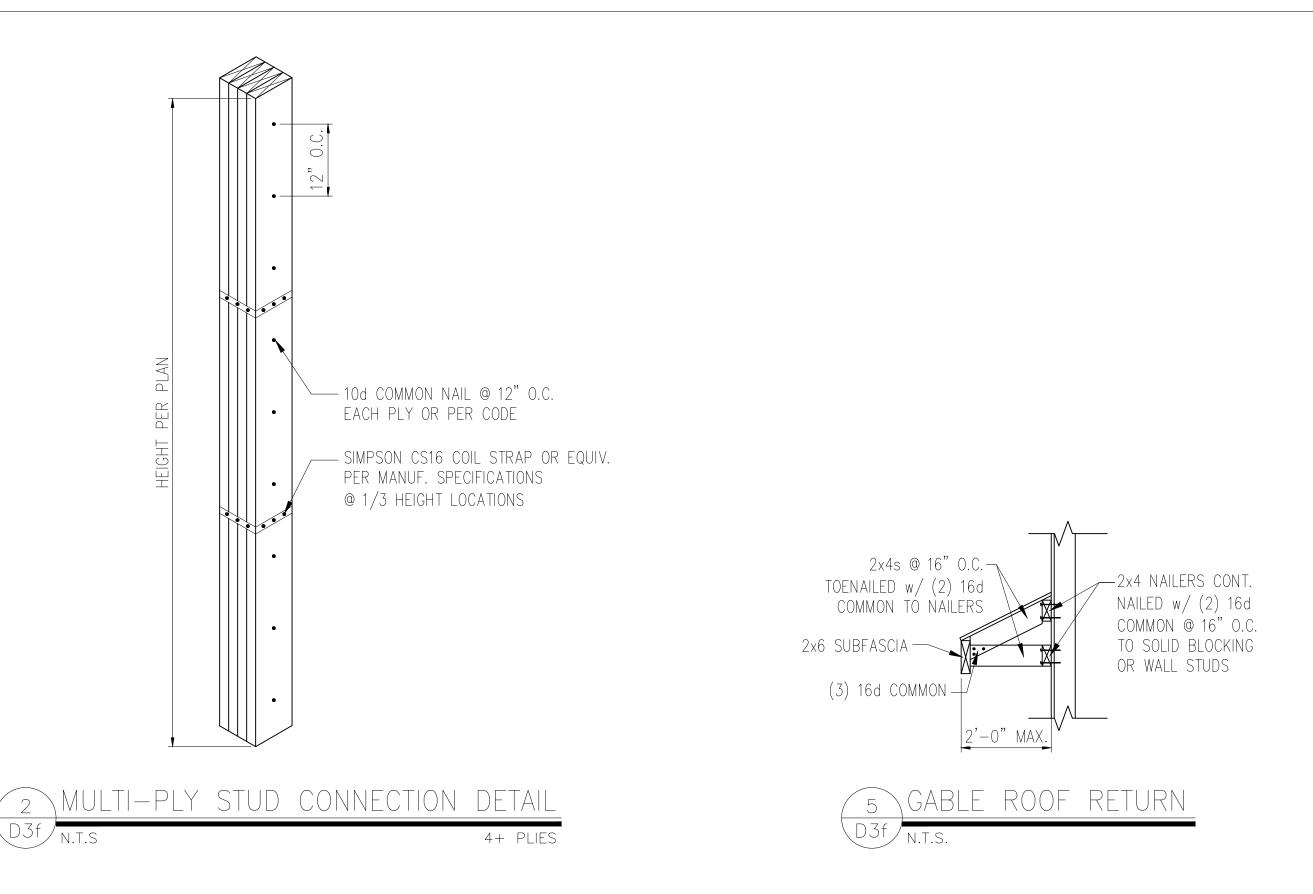


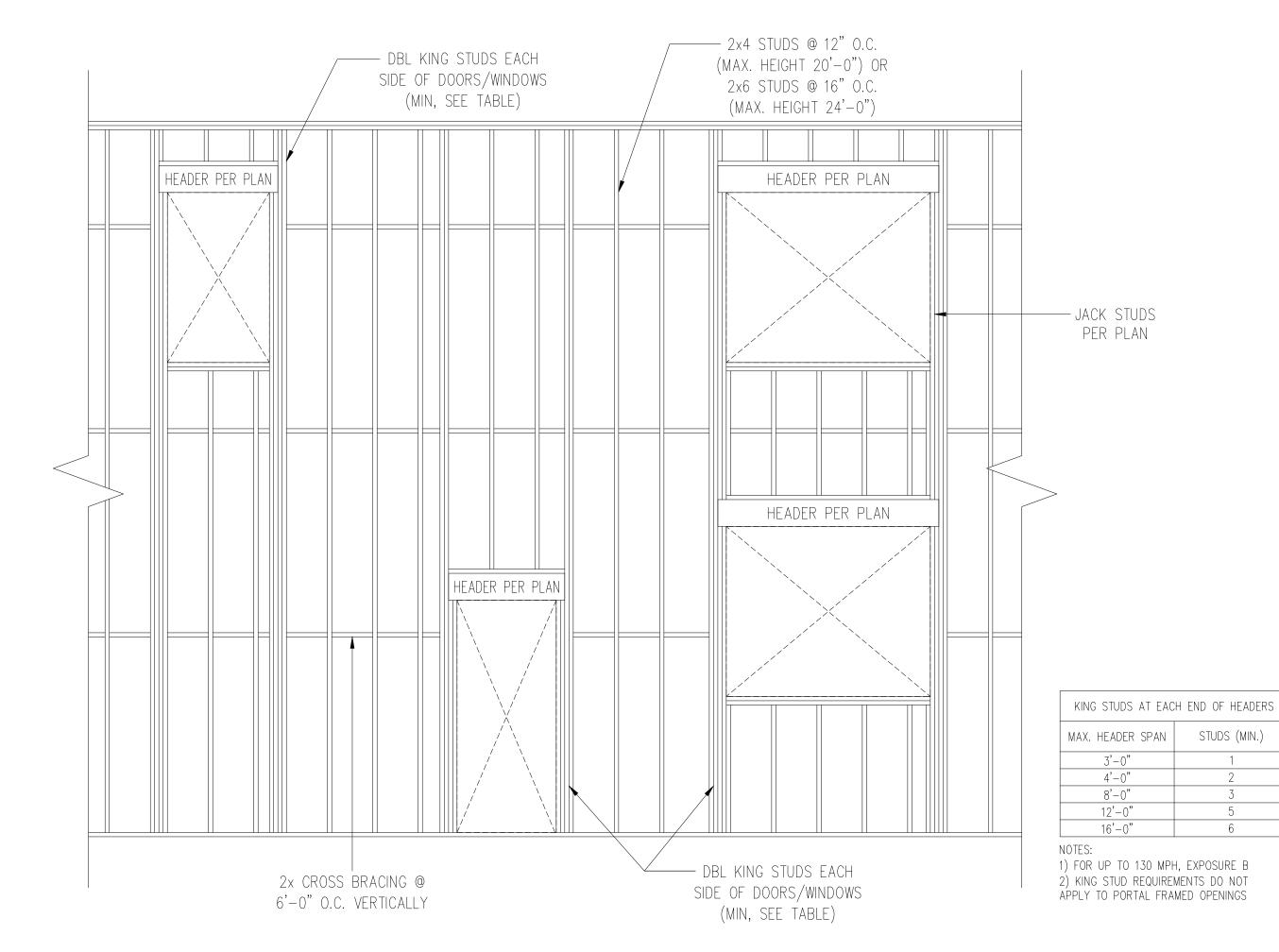




ELEVATION VIEW



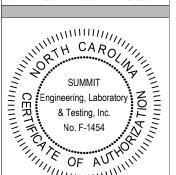




6 TYP. BALLOON FRAMING DETAIL

D3f N.T.S





JEIENI: DR Horton Carolina Division 8001 Arrowridge Blvd. Charlotte, NC 28213



DRAWING

DATE: 02/14/2023

SCALE: 22x34 |/4"=|'-0"
||x|T |/8"=|'-0"

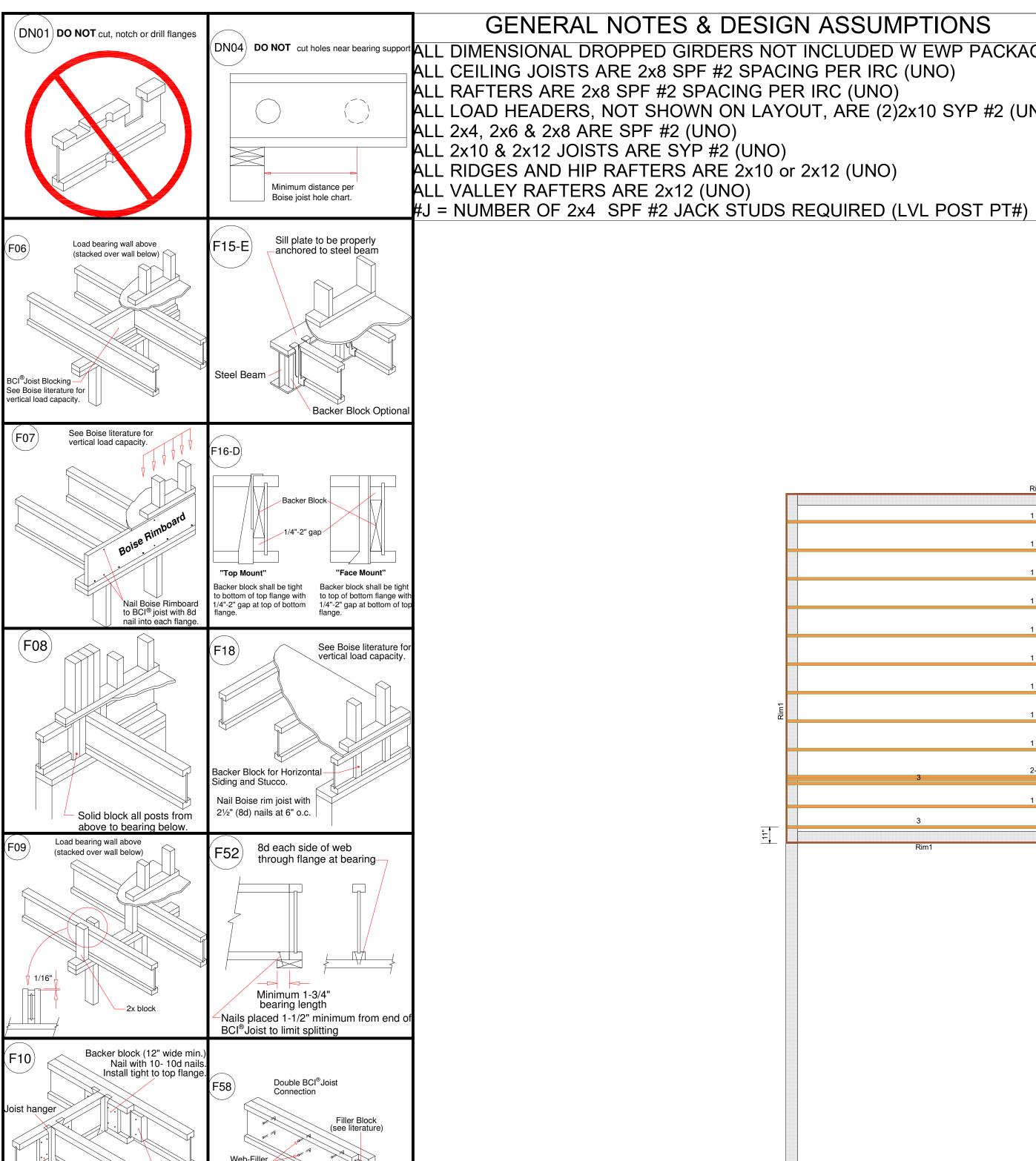
PROJECT \*: 528-06R

DRAWN BY: JCEF

CHECKED BY: BCP

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D3f



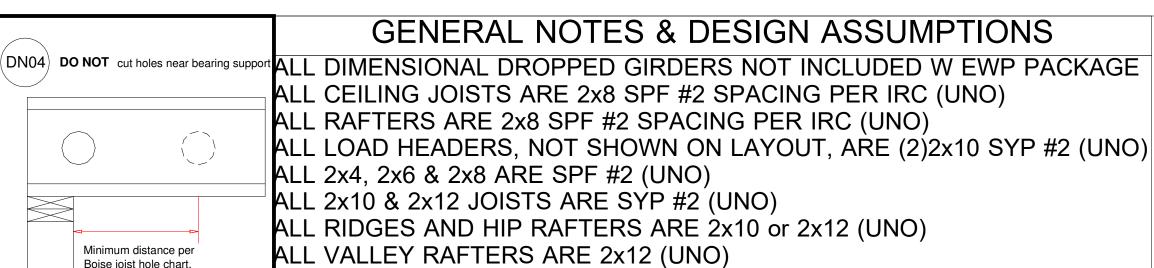
Nail with

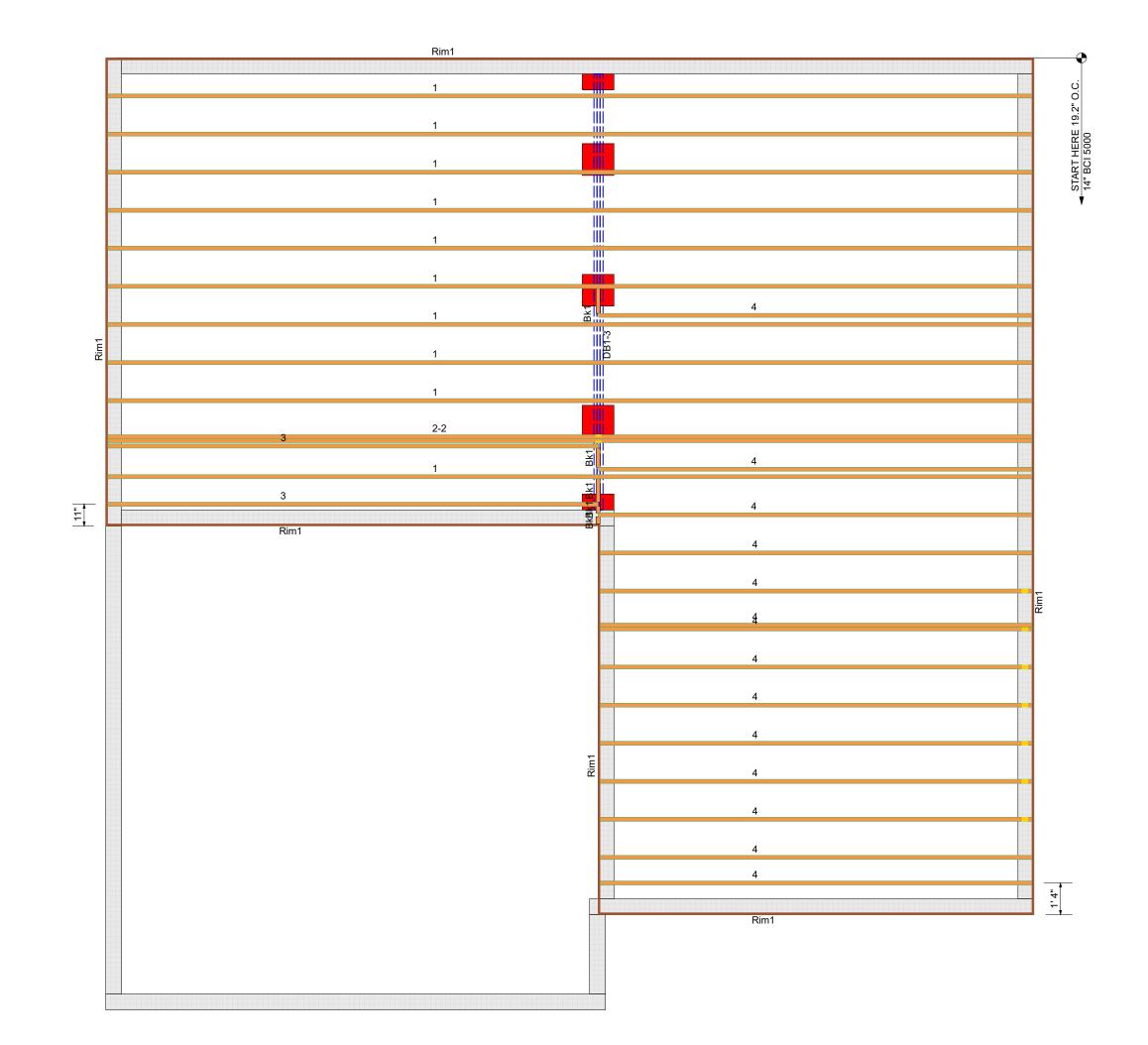
Backer block required where top mount hanger load exceeds 250 lbs.

Install tight to top flange.

Connection valid for all applications.

Contact Boise EWP Engineering for specific condition





	Products					
PlotID	Length	Product	Plies	Net Qty		
1	39' 0"	14" BCI® 5000s-1.8	1	10		
2-2	39' 0"	14" BCI® 5000s-1.8	2	2		
3	21' 0"	14" BCI® 5000s-1.8	1	2		
4	19' 0"	14" BCI® 5000s-1.8	1	14		
DB1-3	20' 0"	2x10 SP No.2	3	3		

1" x 14" BC RIM BOARD OSB

14" BCI® 5000s-1.8

Live Load psf Dead Load psf Joist & Rafter Area Loads Primary Living Secondary & Attic Permanent Stairs 30 Ceiling - Limited Storage 20 10 Ceiling - No Storage 10 Roof - No Ceiling Load 20 Flat Roof or Catherdral w/Drywall Ceiling 20

www.bldr.com **Boise Cascade** 

7601 BOEING DRIVE GREENSBORO, NC 27409 V (336) 884-5454 4575 HAMPTON ROAD CLEMMONS, NC 27409 V (336) 712-9910

1135 ROBESON STREET FAYETTEVILLE, NC 28305 V (910) 485-1111

3189 NC HIGHWAY 5 ABERDEEN NC 28315 V (910) 944-2516

This layout and associated materials list has been prepared based on project plans and/or information provided to Builders FirstSource (BFS). It remains the responsibility of the builder, architect, designer, or other responsible persons to review this information to assure that it is appropriate, accurate, complete and complies with applicable building codes.

2.

HAYDEN MASTER 

MINIMUM DESIGN DATA

LIVE LOAD 40 PSF DEAD LOAD 10 PSF

TOTAL LOAD 50 PSF

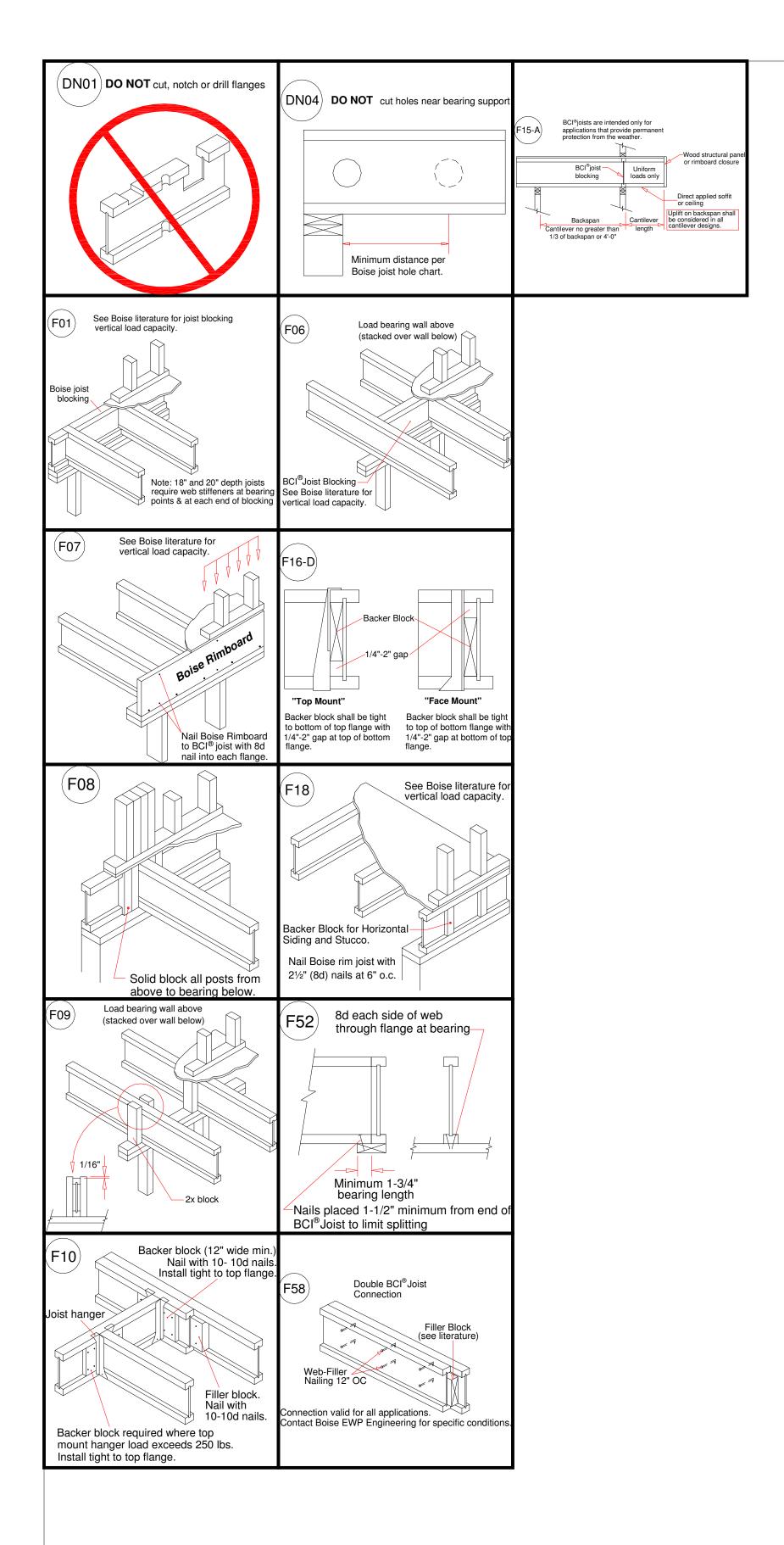
DOL = 100%

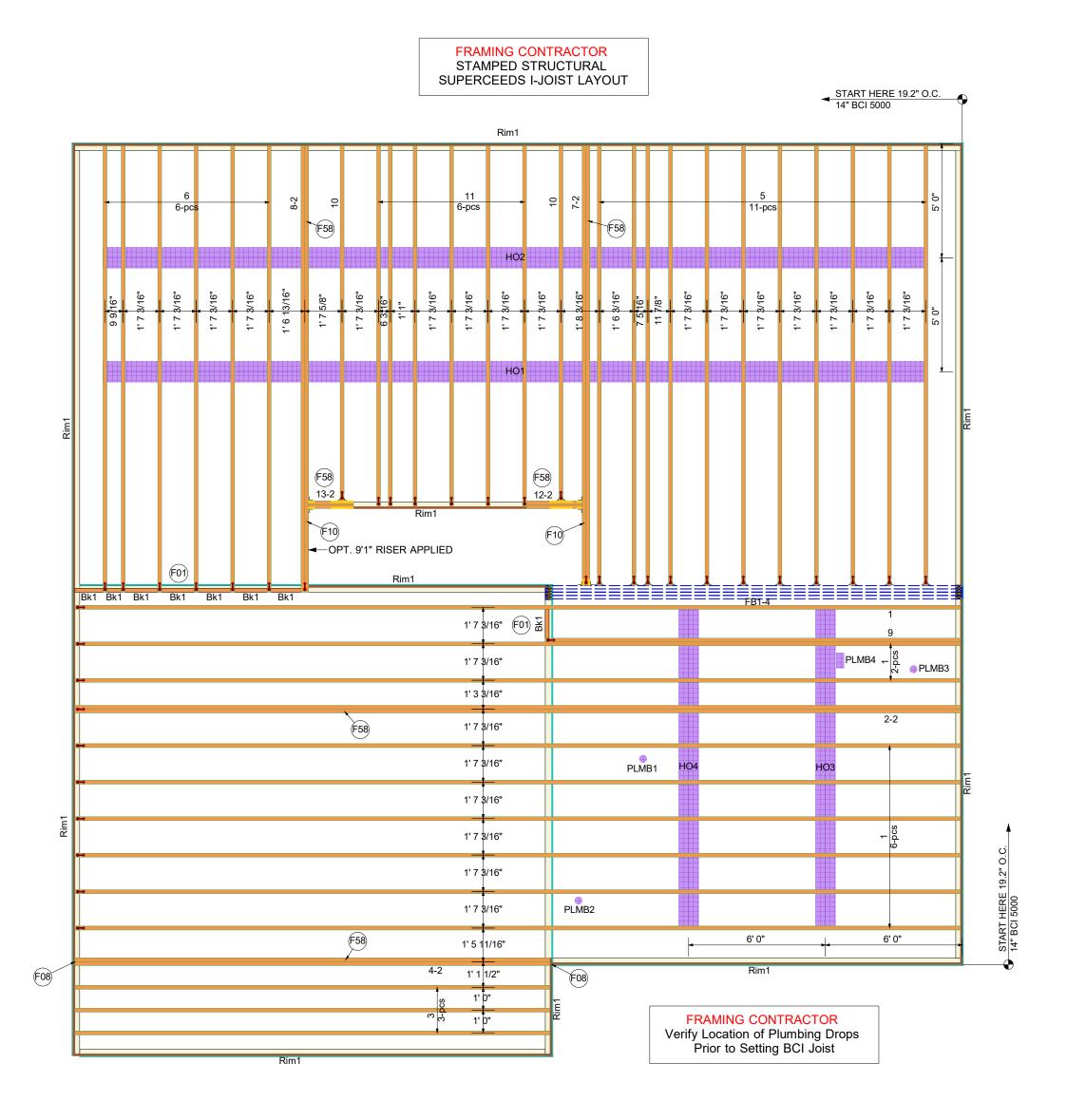
**DEFLECTION CRITERIA** L/480 (MINIMUM)

ARCHITECTUAL PLAN DATE XX-XX-XX REVISED ARCH. PLAN DATE XX-XX-XX

XXXXXXX

Sheet 1 of 2





HAYDEN K

	Products					
PlotID	Length	Product	Plies	Net Qty		
1	39' 0"	14" BCI® 5000s-1.8	1	9		
2-2	39' 0"	14" BCI® 5000s-1.8	2	2		
3	21' 0"	14" BCI® 5000s-1.8	1	3		
4-2	21' 0"	14" BCI® 5000s-1.8	2	2		
5	20' 0"	14" BCI® 5000s-1.8	1	11		
6	20' 0"	14" BCI® 5000s-1.8	1	6		
7-2	20' 0"	14" BCI® 5000s-1.8	2	2		
8-2	20' 0"	14" BCI® 5000s-1.8	2	2		
9	19' 0"	14" BCI® 5000s-1.8	1	1		
10	16' 0"	14" BCI® 5000s-1.8	1	2		
11	16' 0"	14" BCI® 5000s-1.8	1	6		
12-2	3' 0"	14" BCI® 5000s-1.8	2	2		
13-2	2' 0"	14" BCI® 5000s-1.8	2	2		
FB1-4	20' 0"	1-3/4" x 14" VERSA-LAM® LVL 2.1E 3100 SP	4	4		
Rim1	12' 0"	1" x 14" BC RIM BOARD OSB	1	15		
Bk1	11' 0"	14" BCI® 5000s-1.8	1	1		

Hangers				
PlotID Qty		Manuf	Product	
H1	13	Simpson	IUS2.06/14	
H2	3	Simpson	MIU4.12/14	

www.bldr.com Boise Cascade

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1135 ROBESON STREET FAYETTEVILLE, NC 28305 V (910) 485-1111

3189 NC HIGHWAY 5 ABERDEEN NC 28315 V (910) 944-2516

This layout and associated materials list has been prepared based on project plans and/or information provided to Builders FirstSource (BFS). It remains the responsibility of the builder, architect, designer, or other responsible persons to review this information to assure that it is appropriate, accurate, complete and complies with applicable building codes.

ORE MAKING ANY RE TO DO SO MAN

2.

DR HORTON HAYDEN MASTER

MINIMUM DESIGN DATA

LIVE LOAD 40 PSF DEAD LOAD 10 PSF

TOTAL LOAD 50 PSF

DOL = 100%

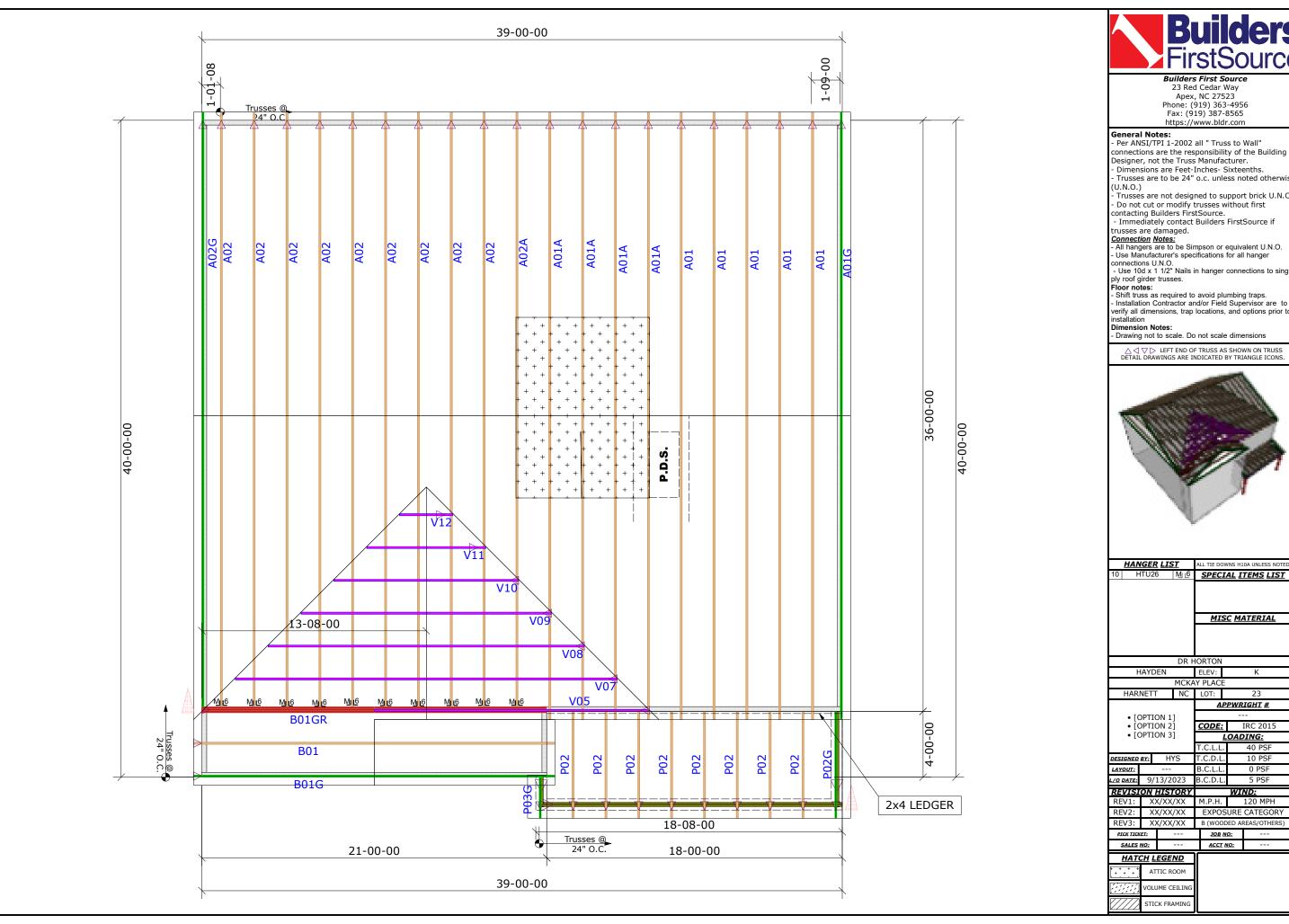
DEFLECTION CRITERIA

L/480 (MINIMUM) ARCHITECTUAL PLAN DATE

XX-XX-XX REVISED ARCH. PLAN DATE XX-XX-XX

Wall Legend Bearing Wall

XXXXXX Sheet 1 of 1





Builders First Source 23 Red Cedar Way Apex, NC 27523 Phone: (919) 363-4956 Fax: (919) 387-8565 https://www.bldr.com

#### **General Notes:**

- Per ANSI/TPI 1-2002 all " Truss to Wall" connections are the responsibility of the Building Designer, not the Truss Manufacturer.

- Dimensions are Feet-Inches- Sixteenths.
   Trusses are to be 24" o.c. unless noted otherwise
- (U.N.O.)

   Trusses are not designed to support brick U.N.O.

   Do not cut or modify trusses without first contacting Builders FirstSource.

   Immediately contact Builders FirstSource if
- trusses are damaged.

- Connection Notes:

   All hangers are to be Simpson or equivalent U.N.O.

   Use Manufacturer's specifications for all hanger
- connections U.N.O.

   Use 10d x 1 1/2" Nails in hanger connections to single ply roof girder trusses.

Floor notes:
- Shift truss as required to avoid plumbing traps.
- Installation Contractor and/or Field Supervisor are to verify all dimensions, trap locations, and options prior to installation

#### Dimension Notes:

- Drawing not to scale. Do not scale dimensions

 $\triangle$   $\triangleleft$   $\nabla$   $\triangleright$  LEFT END OF TRUSS AS SHOWN ON TRUSS DETAIL DRAWINGS ARE INDICATED BY TRIANGLE ICONS.



HANGER LIST				ALL TIL D	JWNJ III	OA ONEESS NOTED		
10 HTU26 <u>М</u> յ <u>6</u>			SPECIAL ITEMS LIST					
				MI	SC MA	ATERIAL		
			DR F	IORTON				
Н	IAYD			ELEV:		K		
MCKAY PLACE								
HARNETT NC			NC	LOT:		23		
			APPWRIGHT #					
• [0	PTIC	ON 1						
		ON 2]		<b>CODE:</b> IRC 2015				
• [C	PTIC	ON 3]		LOADING:				
				T.C.L.L		40 PSF		
DESIGNED BY: HYS			'S	T.C.D.L		10 PSF		
LAYOUT:				B.C.L.L		0 PSF		
L/O DATE:	9/13/2023		)23	B.C.D.I		5 PSF		
REVISION HISTORY				<u>WIND:</u>				
REV1:	XX/XX/XX		M.P.H.		120 MPH			
REV2:	XX/XX/XX		EXPOSURE CATEGORY					
REV3:	XX/XX/XX		B (WOODED AREAS/OTHERS)					
PICK TICKET:			JOB NO:					
SALES NO:				ACC	NO:			
HAT	EGEI	VD.						
<u>nai</u>								

OLUME CEILIN STICK FRAMING