

RE: J0923-5108 Weaver/Lot 36 West Preserve/Harnett Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Lot/Block:	Project Name:	J0923-5108	
Address:			
Citv:			

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	159224652	A1	6/29/2023	21	159224672	VA7	6/29/2023
2	159224653	A1A	6/29/2023	22	159224673	VA8	6/29/2023
3	159224654	A1GE	6/29/2023	23	159224674	VA9	6/29/2023
4	159224655	A2	6/29/2023	24	159224675	VA10	6/29/2023
5	159224656	A3	6/29/2023	25	159224676	VA11	6/29/2023
6	159224657	A4	6/29/2023	26	159224677	VP1	6/29/2023
7	159224658	A4A	6/29/2023	27	159224678	VP2	6/29/2023
8	159224659	A4GE	6/29/2023	28	159224679	VP3	6/29/2023
9	159224660	G1	6/29/2023	29	159224680	VP4	6/29/2023
10	159224661	G1GE	6/29/2023				
11	159224662	P1	6/29/2023				
12	159224663	P1GE	6/29/2023				
13	159224664	PB1	6/29/2023				
14	159224665	PB1GE	6/29/2023				
15	159224666	VA1	6/29/2023				
16	159224667	VA2	6/29/2023				
17	159224668	VA3	6/29/2023				
18	159224669	VA4	6/29/2023				
19	159224670	VA5	6/29/2023				
20	159224671	VA6	6/29/2023				

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

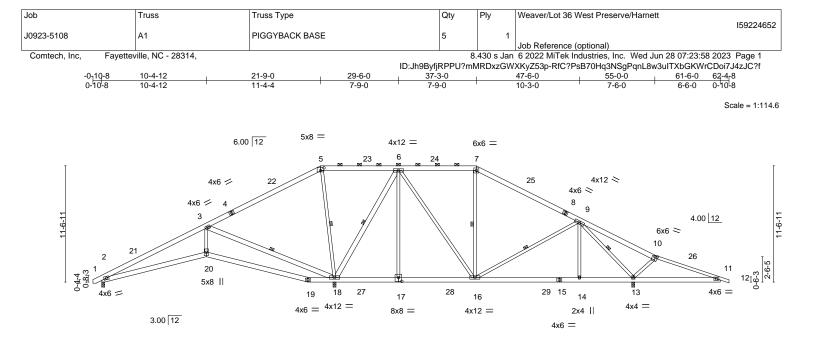
My license renewal date for the state of North Carolina is December 31, 2023

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



—	10-4-12	20-6-0	23-1-12	29-6-0	37-3-0		47-6		52-10-4	61-6-0	
Plate Offsets (X,Y)	10-4-12 [5:0-4-0,0-3-4], [10:0-3-0	10-1-4 ,0-0-12], [17:0-4-	¹ 2-7-12 ¹ ·0,0-4-8]	6-4-4	7-9-0		10-3	5-0	5-4-4	8-7-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TI	2-0-0 1.15 1.15 YES Pl2014	CSI. TC 0.63 BC 0.33 WB 0.6 Matrix-S	5	· · /	in -0.12 1 -0.24 1 0.04 0.05	19-20 >99 18 n	9 360 9 240 ⁄a n/a		PLATES MT20 Weight: 471 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x6 SF WEBS 2x4 SF	2x4 SP No.1				BRACING- TOP CHORI BOT CHORI WEBS	נ ו כ	2-0-0 oc pur	lins (6-0-0 m directly appl	ax.): 5-7. ied or 6-0-0		oc purlins, except 3-18, 9-13
Max H Max U	e) 2=0-3-8, 18=0-3-8, 1 Horz 2=-149(LC 10) Jplift 2=-13(LC 13), 18=-2 Grav 2=495(LC 23), 18=23	37(LC 12), 13=-2									
TOP CHORD 2-3= 9-10 BOT CHORD 2-20 14-1 WEBS 5-18	Comp./Max. Ten All foi -623/0, 3-5=-195/1123, 5- =-854/1241, 10-11=-844/5 =0/602, 19-20=0/601, 18 6=-10/496, 13-14=-10/496 =-971/342, 6-18=-1481/30 =-427/228, 3-18=-1394/28	6=-90/1009, 6-7 958 19=0/530, 17-18 6, 11-13=-846/85 00, 9-14=0/288, 6	=-456/253, 7-9= =-338/345, 16-1 3 6-17=0/365, 6-1	-637/247 17=-338/3 6=-117/77	45,						
 Wind: ASCE 7-10; \ and C-C Exterior(2) Exterior(2) 37-3-0 tc reactions shown; LL WARNING: This loo handling and erectic Trusses ("BCSI"), jc qualified registered permanent individue bracing. Provide adequate d This truss has been will fit between the te This truss has been will fit between the te Bearing at joint(s) 2 capacity of bearing Provide mechanical 18 and 203 lb uplift 	I connection (by others) of	ph; TCDL=6.0ps 1) 3-8-3 to 21-9- 12 to 62-4-8 zor o DOL=1.60 reme care and e Good Practice f and TPI. The buil e design and ins pracing. MiTek a ponding. ottom chord live of 20.0psf on the r members, with n value using AN truss to bearing	f; BCDL=6.0psf 0, Exterior(2) 21 e; cantilever rig experience for p or Handling, Ins Iding owner or the pection of the te assumes no resp load nonconcur e bottom chord n BCDL = 10.0p ISI/TPI 1 angle for plate capable co	I-9-0 to 26 ht expose roper and stalling & I he owner' emporary ponsibility rrent with in all area sf. to grain fo of withstan	S-1-13, Interior(1) ad ;C-C for member Bracing of Metal F s authorized agen installation restrai for truss manufact any other live load s where a rectang rmula. Building d iding 13 lb uplift a	26-1-13 ers and late Co it shall o nt/braci cture, hi ds. gle 3-6-i esigner t joint 2	8 to 37-3-0, forces & M on. For gen onnected Wo contract with ng and the andling, ere 0 tall by 2-0 - should veri , 237 lb upli	WFRS for eral ood ction, or 0 wide fy t at joint		SEA 0365	S22
Design valid for use a truss system. Befo building design. Bra	design parameters and READ NC only with MiTek® connectors. Th ore use, the building designer mu- cing indicated is to prevent buck or stability and to prevent collapse	is design is based or st verify the applicabi ing of individual truss	nly upon parameters lity of design parameters s web and/or chord r	shown, and eters and pro members on rty damage.	is for an individual but operly incorporate this ly. Additional tempora	ilding con design in ry and pe	nponent, not to the overall rmanent bracir	g		TRE	ERING BY ENCO A MITek Affiliate

building design. Bracing indicated is to prevent buckling of individual russ web and/or confor members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 36 West Preserve/Harnett
					159224652
J0923-5108	A1	PIGGYBACK BASE	5	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8	430 s Jan	6 2022 MiTek Industries, Inc. Wed Jun 28 07:23:59 2023 Page 2

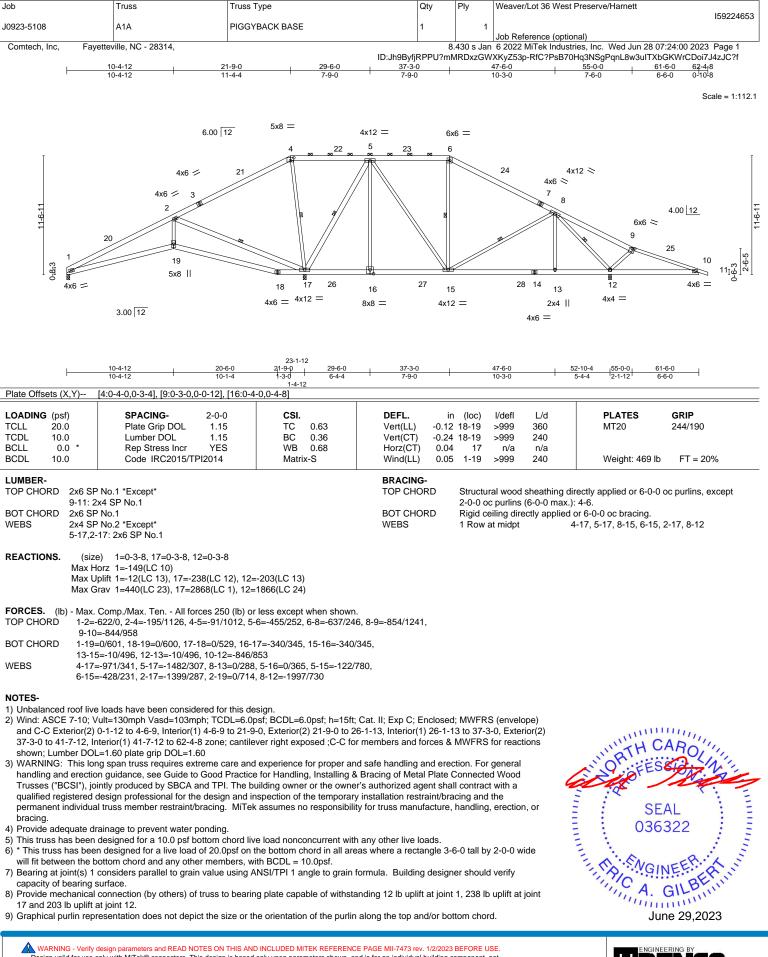
NOTES-

ID:Jh9ByfjRPPU?mMRDxzGWXKyZ53p-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

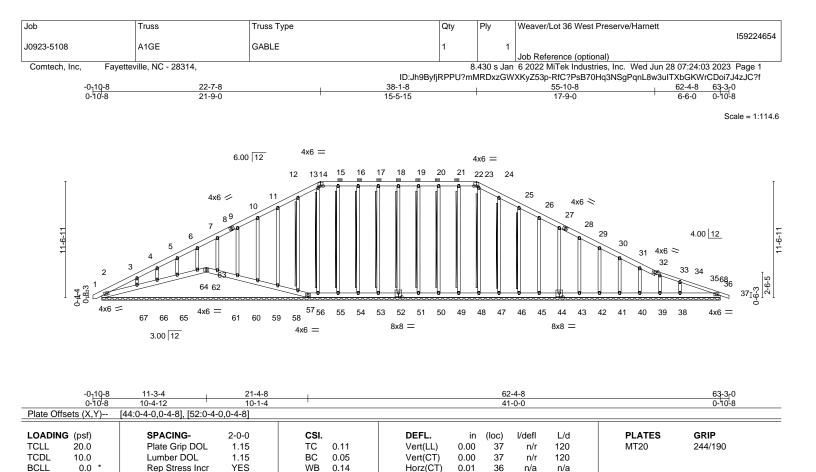
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





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Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

WEBS

0.01

36

T-Brace:

n/a

n/a

2-0-0 oc purlins (6-0-0 max.): 14-22

Brace must cover 90% of web length.

Rigid ceiling directly applied or 10-0-0 oc bracing.

All bearings 61-6-0. (lb) -Max Horz 2=228(LC 12)

2x6 SP No.1 *Except*

33-37: 2x4 SP No.1

2x6 SP No.1

2x4 SP No.2

- Max Uplift All uplift 100 lb or less at joint(s) 2, 57, 52, 53, 54, 55, 58, 59, 60, 61, 62, 64, 65, 66, 51, 50, 49, 47, 46, 45, 44, 43, 42, 41, 40, 39, 36 except 67=-142(LC 12), 38=-117(LC 13) Max Grav
 - All reactions 250 lb or less at joint(s) 2, 63, 57, 52, 53, 54, 55, 56, 58, 59, 60, 61, 62, 64, 65, 66, 51, 50, 49, 48, 47, 46, 45, 44, 43, 42, 41, 40, 39, 36 except 67=274(LC 23), 38=316(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Rep Stress Incr

Code IRC2015/TPI2014

2-3=-308/128, 9-10=-90/270, 10-11=-110/328, 11-12=-131/388, 12-13=-150/441, TOP CHORD 13-14=-141/400, 14-15=-138/425, 15-16=-138/425, 16-17=-138/425, 17-18=-138/425, 18-19=-138/425, 19-20=-138/425, 20-21=-138/425, 21-22=-138/425, 22-23=-141/400, 23-24=-150/429. 24-25=-131/376. 25-26=-110/316. 26-27=-90/258 WEBS 3-67=-194/267

NOTES-

BCLL

BCDL

LUMBER-

OTHERS

TOP CHORD

BOT CHORD

REACTIONS.

0.0

10.0

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-10 to 3-6-0, Exterior(2) 3-6-0 to 21-9-0, Corner(3) 21-9-0 to 26-1-13, Exterior(2) 26-1-13 to 37-3-0, Corner(3) 37-3-0 to 41-6-0, Exterior(2) 41-6-0 to 62-4-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

0.14

Matrix-S

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

5) Provide adequate drainage to prevent water ponding. Contribution are gate MT20 unless otherwise indicated

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FT = 20%

Weight: 574 lb

2x4 SPF No.2 - 18-52, 17-53, 16-54, 15-55

, 13-56, 12-58, 19-51, 20-50, 21-49, 23-48,

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

24-47.25-46 Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails. 6in o.c. with 3in minimum end distance.



818 Soundside Road

Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 36 West Preserve/Harnett
10000 5100	A10F	CARLE		1	159224654
J0923-5108	A1GE	GABLE	1	1	Job Reference (optional)
Comto als la s				100 - 1	
Comtech, Inc,	Fayetteville, NC - 28314,				6 2022 MiTek Industries, Inc. Wed Jun 28 07:24:04 2023 Page 2
			ID:Jh9ByfjRPPU?ml	MRDxzGW	/XKyZ53p-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

NOTES-

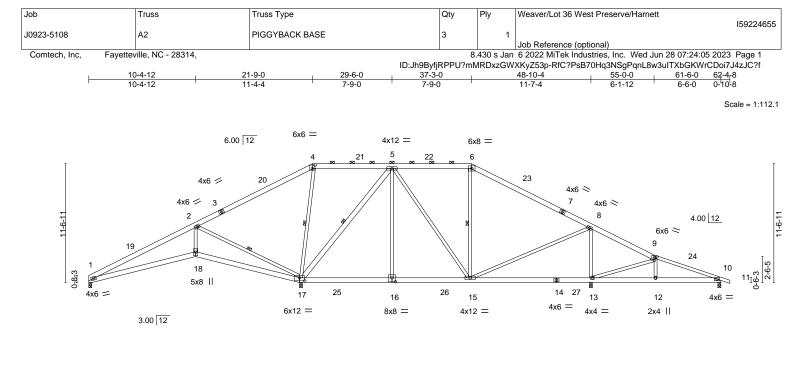
7) Gable requires continuous bottom chord bearing.

8) Gable studs spaced at 2-0-0 oc.

- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 57, 52, 53, 54, 55, 58, 59, 60, 61, 62, 64, 65, 66, 51, 50, 49, 47, 46, 45, 44, 43, 42, 41, 40, 39, 36 except (jt=lb) 67=142, 38=117.
- 12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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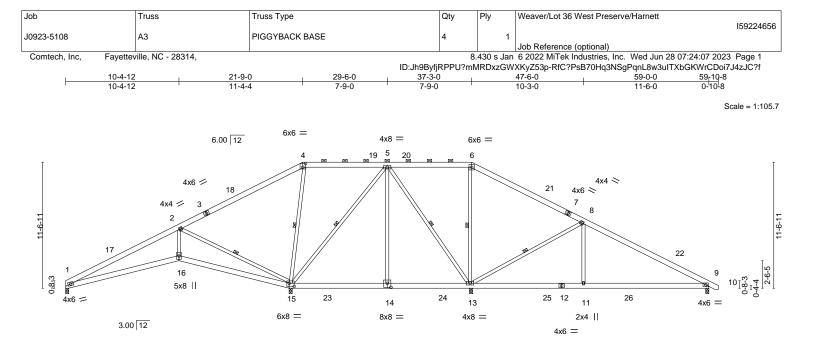




 	10-4-12		20-7-12 0-1-12	29-6-0 8-10-4	37-3-0			48-10-4 11-7-4	<u>55-0-</u>		
Plate Offsets (X,Y)	[4:0-3-0,0-4-0], [9:0-2-8,0										
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES Pl2014	BC	0.63 0.33 0.69 -S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.10 -0.21 0.03	1-18 : 1-18 : 10	l/defl L/d >999 360 >999 240 n/a n/a >999 240	N	PLATES //T20 Veight: 455 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x6 SI WEBS 2x4 SI	2x4 SP No.1				BRACING TOP CHO BOT CHO WEBS	RD	2-0-0 oc	purlins (6-0-0 ma	ix.): 4-6. ed or 6-0-0		oc purlins, except
(Ib) - Max H Max U	earings 0-3-8. Horz 1=-149(LC 10) Jplift All uplift 100 lb or le Grav All reactions 250 lb 24)						(LC 24), 1	10=446(LC			
TOP CHORD 1-2= 9-10 BOT CHORD 16-1 WEBS 4-17	. Comp./Max. Ten All for 261/110, 2-4=-105/1120,)=-515/100 7=-88/387, 15-16=-88/387 '=-971/305, 5-17=-1386/26)=0/485, 6-15=-393/196, 2-	4-5=0/774, 5-6=- 7, 12-13=-13/413, 64, 5-15=-92/461,	-506/333, 6 , 10-12=-10 , 8-15=-6/6	6-8=-710/264 0/421 54, 8-13=-1:	64, 8-9=0/303, 1220/387,						
 Wind: ASCE 7-10; ' and C-C Exterior(2) 37-3-0 to 41-7-12, I plate grip DOL=1.60' WARNING: This lo handling and erectin Trusses ("BCSI"), jo qualified registered permanent individua bracing. Provide adequate d This truss has been will fit between the I Bearing at joint(s) 1 capacity of bearing Provide mechanical (jt=lb) 17=172, 13=" 	ong span truss requires ext on guidance, see Guide to ointly produced by SBCA a design professional for the al truss member restraint/k drainage to prevent water p in designed for a 10.0 psf b en designed for a live load bottom chord and any othe I considers parallel to grair surface. I connection (by others) of	ph; TCDL=6.0psf;) 4-6-9 to 21-9-0, 8 zone;C-C for m reme care and ex Good Practice for and TPI. The build e design and insp pracing. MiTek as ponding. ottom chord live live of 20.0psf on the pr members, with n value using ANS truss to bearing p	F; BCDL=6. , Exterior(2 nembers ar xperience f for Handling ding owner bection of tl ssumes no load nonco b bottom ch BCDL = 11 SI/TPI 1 an plate capal) 21-9-0 to 2 ad forces & I for proper ar join proper ar or the owner the temporar responsibili ncurrent with bord in all ard 0.0psf. gle to grain bole of withst	26-1-13, Interior(1 MWFRS for reacti and safe handling a & Bracing of Meta her's authorized ag ary installation restri ility for truss manuf ith any other live lo reas where a recta n formula. Building standing 100 lb upli	26-1-1: ons sho Plate C ent shal aint/bra acture, ads. ngle 3-6 designe t at join	3 to 37-3- wn; Lumbu tion. For g connected I contract t cing and th handling, t contall by 2 contall by 2 con	D, Exterior(2) er DOL=1.60 wood with a he erection, or 2-0-0 wide verify except	and the second s	in num	S22
	/ design parameters and READ NC					1/2/2022		E			RING BY

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

A MiTek Aff 818 Soundside Road Edenton, NC 27932



L	10-4-12	20-6-0	29-6-0	37-4-0	1	47-6-0		59-0-0	
	10-4-12	10-1-4	9-0-0	7-10-0	1	10-2-0	1	11-6-0	
Plate Offsets	(X,Y) [4:0-3-0,0-4-0],	[14:0-4-0,0-4-8], [15:0-5	-8,0-3-8]						
LOADING (p TCLL 20	osf) SPACINO 0.0 Plate Gri		CSI. TC 0.63	DEFL. Vert(LL) -0.1	in (loc) 0 9-11	l/defl >999	L/d 360	PLATES MT20	GRIP 244/190
BCLL	0.0 Lumber I 0.0 * Rep Stre 0.0 Code IR		BC 0.38 WB 0.77 Matrix-S	Vert(CT) -0.2 Horz(CT) 0.0 Wind(LL) 0.0		n/a	240 n/a 240	Weight: 426 lb	FT = 20%
BCDL N		02013/11/2014	Matrix-0	Wind(LL) 0.0	0 3-11	2000	240	Weight. 420 lb	11 = 2078
LUMBER- TOP CHORD BOT CHORD WEBS				BRACING- TOP CHORD BOT CHORD WEBS	2-0-0 Rigid	oc purlins (1	0-0-0 max.): ily applied or	ectly applied or 6-0-0 c : 4-6. r 6-0-0 oc bracing. 15, 4-15, 5-15, 5-13, 6	
()	 Max Horz 1=-147(LC Max Uplift All uplift 1(Max Grav All reaction 24) (b) - Max. Comp./Max. Ter 1-2=-351/92, 2-4=-10 	00 lb or less at joint(s) 1 ns 250 lb or less at joint n All forces 250 (lb) or 7/1077, 4-5=0/739, 5-6=	except 15=-219(LC 12), 1 (s) except 1=389(LC 23), 7 less except when shown. =0/573, 6-8=0/708, 8-9=-7 51, 13-14=-392/261, 11-13	15=2162(LC 23), 13=1 40/169		6), 9=691(Lt	с		
WEBS	9-11=-17/558 2-16=0/481, 2-15=-12	,	, 5-15=-690/107, 5-14=0/4	,					
 Wind: ASC and C-C E Exterior(2) Lumber D0 Provide ac This truss * This truss will fit betw 6) Bearing at 	ed roof live loads have bee CE 7-10; Vult=130mph Vas Exterior(2) 0-1-12 to 4-6-9, 37-3-0 to 43-5-10, Interio OL=1.60 plate grip DOL=1 dequate drainage to prever has been designed for a 1 s has been designed for a s has been designed for a s ipont (s) 1 considers parallif t bearing surface	sd=103mph; TCDL=6.0p Interior(1) 4-6-9 to 21-9 r(1) 43-5-10 to 59-8-10 .60 nt water ponding. 0.0 psf bottom chord liv live load of 20.0psf on 1 any other members, w	bsf; BCDL=6.0psf; h=15ft; -0, Exterior(2) 21-9-0 to 27 zone;C-C for members an e load nonconcurrent with the bottom chord in all area ith BCDL = 10.0psf.	7-11-11, Interior(1) 27- d forces & MWFRS for any other live loads. as where a rectangle 3	11-11 to reaction -6-0 tall I	37-3-0, s shown; by 2-0-0 wide	,	NORTH C	AROUNING

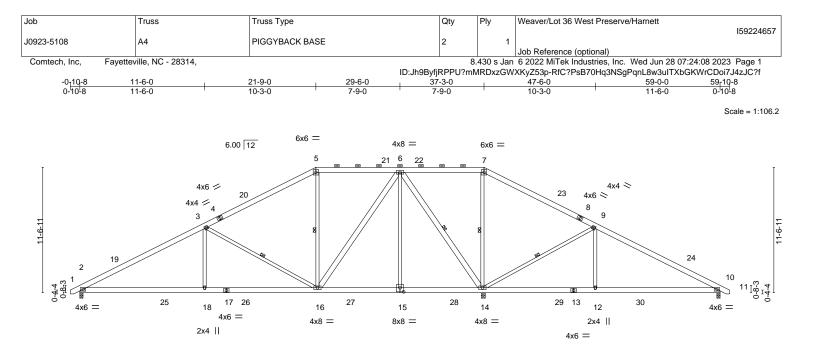
capacity of bearing surface. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 15=219, 13=111, 9=102.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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A MiTek Affilia 818 Soundside Road Edenton, NC 27932



 	11-6-0 11-6-0	<u>21-9-0</u> 10-3-0		<u>29-6-0</u> 7-9-0			7-3-0	47-6-0		<u>59-0-0</u> 11-6-0	
Plate Offsets (X,Y)	[15:0-4-0,0-4-8]										
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI2	2-0-0 1.15 1.15 YES 2014	CSI. TC BC WB Matrix	0.63 0.49 0.88 <-S	DEFL. Vert(LL Vert(CT Horz(C Wind(L	-0.11) -0.26 T) 0.04		l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 452 lb	GRIP 244/190 FT = 20%
					BRACII TOP CH BOT CH WEBS	IORD	except 2-0-0 c Rigid c	oc purlins ((6-0-0 max.) ectly applied	rectly applied or 4-7-14 : 5-7. or 6-0-0 oc bracing. 3-16, 5-16, 6-14, 7-14, §	• •
Max H Max L Max G ORCES. (lb) - Max. OP CHORD 2-3=	 2=0-3-8, 14=0-4-4, 10= Horz 2=146(LC 11) Jplift 2=-106(LC 12), 14=-13 Grav 2=1292(LC 23), 14=33 Comp./Max. Ten All force -2033/440, 3-5=-1002/363, =-412/390 	8(LC 12), 10=- 09(LC 2), 10= es 250 (lb) or l	=543(LC 24) less except	when shown							
BOT CHORD 2-18 WEBS 3-18	=-412/390 =-269/1727, 16-18=-269/17 =0/520, 3-16=-1182/350, 6- =-926/295, 9-14=-1223/359	16=-228/1041	,		53/431,						
 Wind: ASCE 7-10; V and C-C Exterior(2) Exterior(2) 37-3-0 to Lumber DOL=1.60 Provide adequate d This truss has been 	e loads have been consider Vult=130mph Vasd=103mph -0-8-10 to 3-8-3, Interior(1) o 43-5-10, Interior(1) 43-5-10 plate grip DDL=1.60 Irainage to prevent water po designed for a 10.0 psf bot on designed for a lio.0 psf bot	n; TCDL=6.0ps 3-8-3 to 21-9- 0 to 59-8-10 zo nding. tom chord live	sf; BCDL=6 -0, Exterior(one;C-C for	2) 21-9-0 to 2 members ar	27-11-11, Inter nd forces & MV n any other live	or(1) 27-1 /FRS for i loads.	1-11 to a eactions	37-3-0, s shown;		NUMBTH C	AROLA

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

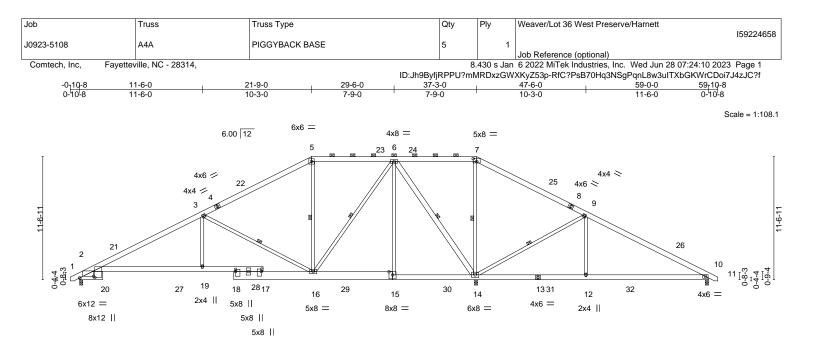
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (jt=lb) 2=106, 10=107.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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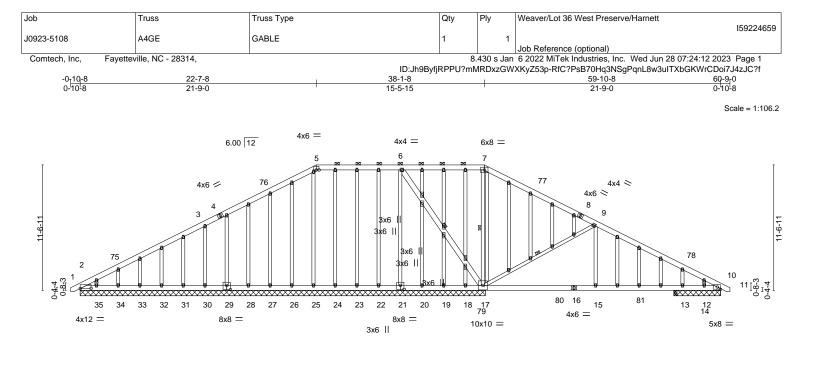


	<u>11-6-0</u> <u>11-6-0</u> <u>10-3-0</u>	29-6-0	37-2-4	37-3-0 0-0-12	47-6-0	<u>59-0-0</u> 11-6-0	
Plate Offsets (X,Y)	[2:0-0-9,1-4-15], [2:1-3-7,0-1-14], [7:0-4						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.69 BC 0.61 WB 0.94 Matrix-S	Vert(CT) - Horz(CT)	in (loc) 0.19 2-19 0.40 2-19 0.10 14 0.15 2-19	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20	GRIP 244/190 FT = 20%
BCDL 10.0	Code IRC2015/1912014	Matrix-5	Wind(LL)	0.15 2-19	>999 240	Weight: 474 lb	F1 = 20%
2-20,1 WEBS 2x4 SF	P No.1 P 2400F 2.0E *Except* I5-18: 2x10 SP No.1 P No.2 *Except* 2x6 SP No.1		BRACING- TOP CHORD BOT CHORD WEBS	except 2-0-0 o Rigid c	c purlins (6-0-0 max eiling directly applied	directly applied or 4-8-13 .): 5-7. d or 6-0-0 oc bracing. 3-16, 5-16, 6-16, 6-14, 7	
Max H Max U	ze) 2=0-3-8, 14=0-3-8, 10=0-3-8 Horz 2=148(LC 11) Jplift 2=-90(LC 12), 14=-57(LC 12), 10=- Grav 2=1131(LC 23), 14=3662(LC 2), 10						
TOP CHORD 2-3= 9-10	. Comp./Max. Ten All forces 250 (lb) o 1905/437, 3-5=-636/300, 5-6=-421/358)≕173/976 ≕-281/1609, 16-19=-263/1609, 15-16=-3	, 6-7=-45/1494, 7-9=-172/	1731,				
10-1 WEBS 3-19	2=-809/202 2=-809/202 9=0/635, 3-16=-1395/403, 5-16=-294/156 8=-2089/461, 7-14=-1175/339, 9-14=-123	, 6-16=-250/1159, 6-15=0	,				
 Wind: ASCE 7-10; and C-C Exterior(2) Exterior(2) 37-3-0 tt Lumber DOL=1.60 3) Provide adequate d 4) This truss has been will fit between the 1 6) Provide mechanical (jt=lb) 10=226. 	re loads have been considered for this de Vult=130mph Vasd=103mph; TCDL=6.0) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 21- o 43-5-10, Interior(1) 43-5-10 to 59-8-10 plate grip DOL=1.60 drainage to prevent water ponding. n designed for a 10.0 psf bottom chord live en designed for a live load of 20.0psf on bottom chord and any other members, w I connection (by others) of truss to bearin stry Piggyback Truss Connection Detail f	posf; BCDL=6.0psf; h=15ft; 9-0, Exterior(2) 21-9-0 to 2 zone;C-C for members an re load nonconcurrent with the bottom chord in all are ith BCDL = 10.0psf. ng plate capable of withsta	27-11-11, Interior(1) ad forces & MWFRS an any other live load as where a rectangl anding 100 lb uplift a	27-11-11 to 3 for reactions s. e 3-6-0 tall by t joint(s) 2, 14	y 2-0-0 wide 4 except	O365	AL

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

In The State C G minim June 29,2023

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-0 <u>-10-8</u> 0-10-8	<u>22-7-8</u> 21-9-0		38-1-8 15-5-15	-	<u>59-10-8</u> 21-9-0	<u> </u>
Plate Offsets (X,Y)	[2:1-0-0,0-0-4], [21:0-4-0,0-4-8], [29:0-4	-0,0-4-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.54 BC 0.50 WB 0.65 Matrix-S	Vert(LL) -0.05		D M D a	LATES GRIP T20 244/190 Veight: 628 lb FT = 20%
	No.1 No.2 *Except* x6 SP No.1		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood shea 2-0-0 oc purlins (6-0- Rigid ceiling directly a 1 Row at midpt	0 max.): 5-7.	0
(lb) - Max H Max U	earings 37-4-0 except (jt=length) 10=4-3 orz 2=228(LC 12) plift All uplift 100 lb or less at joint(s) 2 21=-365(LC 19), 17=-416(LC 13), 3 12=-396(LC 13) rav All reactions 250 lb or less at joint 31, 32, 33, 34, 20, 19, 13 except 2= 10=448(LC 1), 35=521(LC 23), 12=	, 10, 34, 14 except 30=-5 35=-374(LC 12), 18=-271((s) 21, 22, 23, 24, 25, 26, -411(LC 1), 30=797(LC 1)	26(LC 12), LC 3), 13=-453(LC 2), 27, 28, , 17=2054(LC 1),			
TOP CHORD 2-3=- 9-10= BOT CHORD 2-35= 30-31 24-25 19-20 13-14 WEBS 3-30=	Comp./Max. Ten All forces 250 (lb) of 1111/538, 3-5=-1123/904, 5-6=-855/94 1283/682 301/855, 34-35=-301/855, 33-34=-301 [=-301/855, 28-30=-301/855, 27-28=-30 5=-301/855, 18-19=-301/855, 17-18=-30 !=-301/855, 18-19=-301/855, 17-18=-30 !=-361/1003, 12-13=-361/1003, 10-12=- -728/686, 6-21=-304/468, 6-17=-1142/ -0/421	7, 6-7=-304/535, 7-9=-398 /855, 32-33=-301/855, 31 1/855, 26-27=-301/855, 2 1/855, 21-22=-301/855, 2 1/855, 15-17=-361/1003, 361/1003	8/499, -32=-301/855, 5-26=-301/855, 0-21=-301/855, 14-15=-361/1003,			ATH CARO
 Wind: ASCE 7-10; V gable end zone and 37-3-0, Corner(3) 37 shown; Lumber DOL Truss designed for v Gable End Details a: Provide adequate dr All plates are 2x4 M Gable studs spaced This truss has been * This truss has been 	e loads have been considered for this de 'ult=130mph Vasd=103mph; TCDL=6.0 C-C Corner(3) -0-8-10 to 3-8-3, Exterior '-3-0 to 41-7-12, Exterior(2) 41-7-12 to 5 =1.60 plate grip DOL=1.60 vind loads in the plane of the truss only. s applicable, or consult qualified building ainage to prevent water ponding. T20 unless otherwise indicated. at 2-0-0 oc. designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on ottom chord and any other members, w	bsf; BCDL=6.0psf; h=15ft; r(2) 3-8-3 to 21-9-0, Corne i9-8-10 zone;C-C for men For studs exposed to wir g designer as per ANSI/TI re load nonconcurrent with the bottom chord in all are	er(3) 21-9-0 to 26-1-13, I nbers and forces & MWF nd (normal to the face), s Pl 1. n any other live loads.	Exterior(2) 26-1-13 to RS for reactions see Standard Industry	With Marken Barris	SEAL 036322 A. GILBER June 29,2023
Design valid for use of a truss system. Befor building design. Brac is always required for fabrication, storage, of	bening parameters and READ NOTES ON THIS AN only with MiTek® connectors. This design is based e use, the building designer must verify the applica- ing indicated is to prevent buckling of individual tri stability and to prevent collapse with possible per felivery, erection and bracing of trusses and truss Component Safety Information available from the	only upon parameters shown, au ability of design parameters and p uss web and/or chord members of sonal injury and property damage systems, see ANSI/TPI1 Qualit	nd is for an individual building c properly incorporate this design ponly. Additional temporary and a. For general guidance regard y Criteria and DSB-22 availat	omponent, not into the overall permanent bracing ing the ele from Truss Plate Institute	(www.tpinst.org)	ENGINEERING BY AMITek Affiliate 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 36 West Preserve/Harnett
					159224659
J0923-5108	A4GE	GABLE	1	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8	.430 s Jan	6 2022 MiTek Industries, Inc. Wed Jun 28 07:24:13 2023 Page 2
		ID:Jh9Byfj	RPPU?mN	/IRDxzGW	XKyZ53p-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

NOTES-

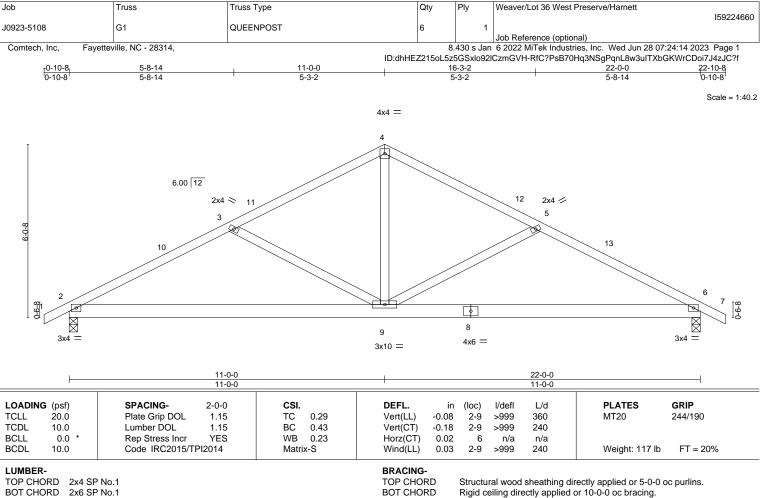
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 34, 14 except (jt=lb) 30=526, 21=365, 17=416, 35=374, 18=271, 13=453, 12=396.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

REACTIONS. 6=0-3-8, 2=0-3-8 (size) Max Horz 2=77(LC 11) Max Uplift 6=-66(LC 13), 2=-66(LC 12) Max Grav 6=930(LC 1), 2=930(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1425/377, 3-4=-1076/286, 4-5=-1076/286, 5-6=-1425/377 TOP CHORD

BOT CHORD 2-9=-249/1194, 6-9=-258/1194

WEBS 3-9=-365/248, 4-9=-76/632, 5-9=-365/248

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 11-0-0, Exterior(2) 11-0-0 to 15-4-13, Interior(1) 15-4-13 to 22-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

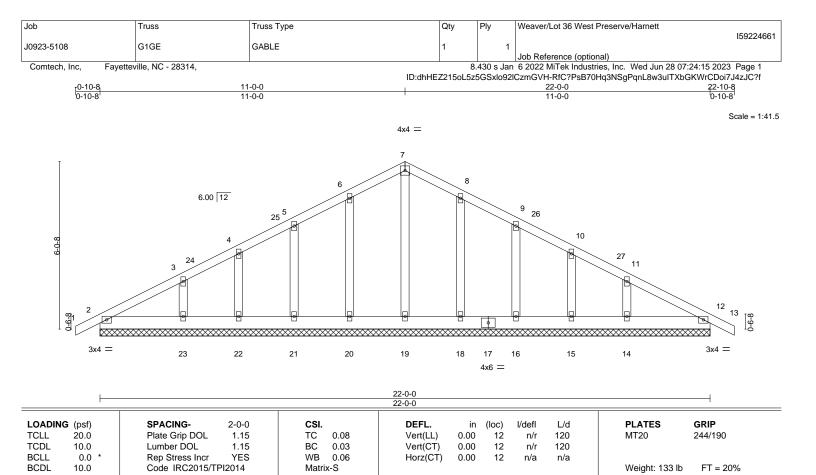
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.



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BRACING-TOP CHORD

BOT CHORD

	JME		
- L (JIVIE	SER	-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 OTHERS 2x4 SP No.2

OTHERS 2x4 SP No.2

REACTIONS. All bearings 22-0-0.

(lb) - Max Horz 2=120(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 12, 2, 20, 21, 22, 18, 16, 15 except 23=-108(LC 12), 14=-106(LC 13) Max Grav All reactions 250 lb or less at joint(s) 12, 2, 19, 20, 21, 22, 23, 18, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 11-0-0, Corner(3) 11-0-0 to 15-4-13, Exterior(2) 15-4-13 to 22-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

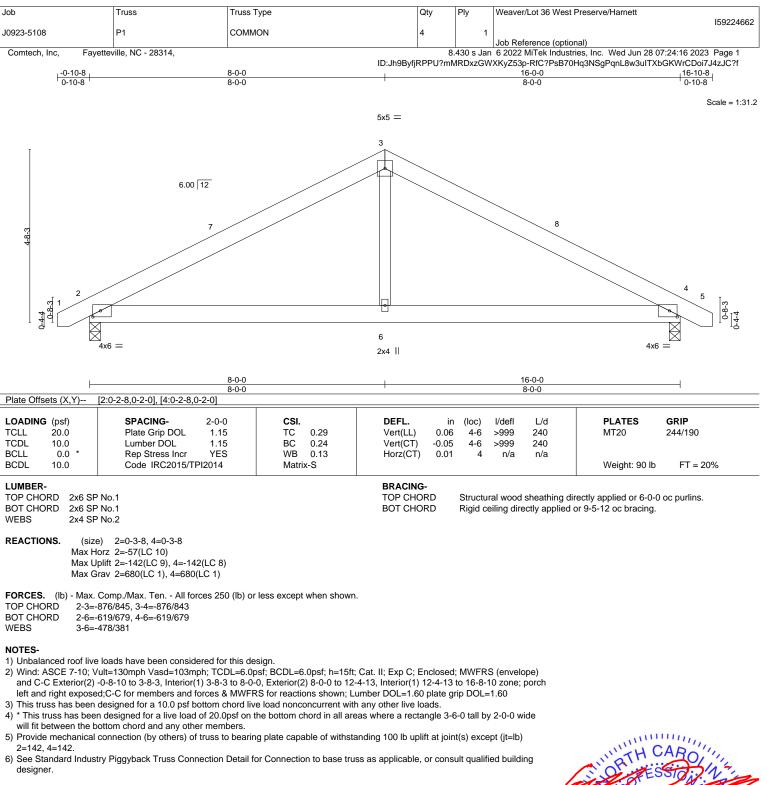
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 2, 20, 21, 22, 18, 16, 15 except (jt=lb) 23=108, 14=106.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

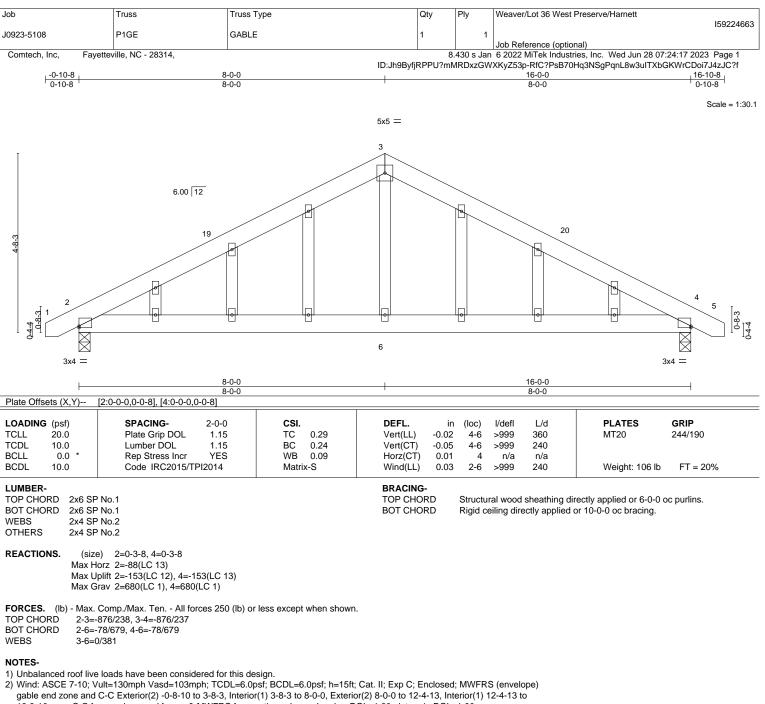
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16-8-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

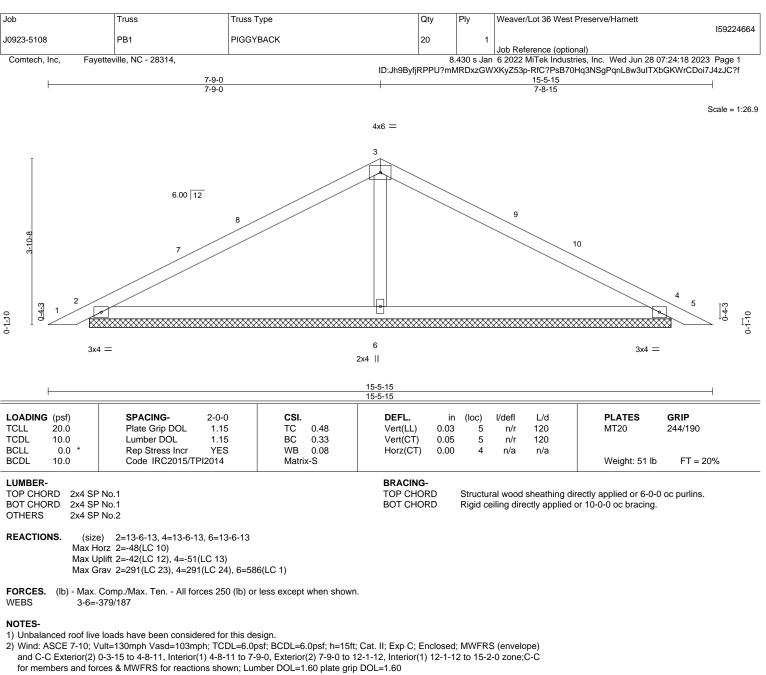
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=153, 4=153.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

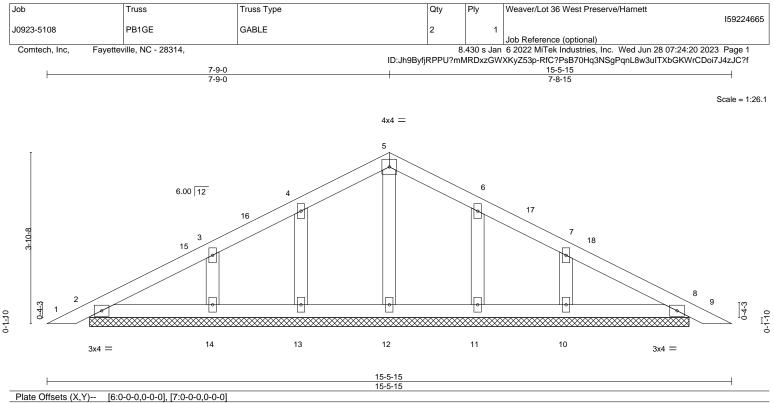
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



LOADING ((psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 2	20.0	Plate Grip DOL	1.15	тс	0.07	Vert(LL)	0.00	` <i>8</i>	n/r	120	MT20	244/190
ICDL 1	10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	0.00	9	n/r	120		
SCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	8	n/a	n/a		
BCDL 1	10.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 61 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 13-6-13.

(lb) - Max Horz 2=75(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 14, 11, 10

Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-3-15 to 4-8-11, Exterior(2) 4-8-11 to 7-9-0, Corner(3) 7-9-0 to 12-1-12, Exterior(2) 12-1-12 to 15-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 14, 11, 10.

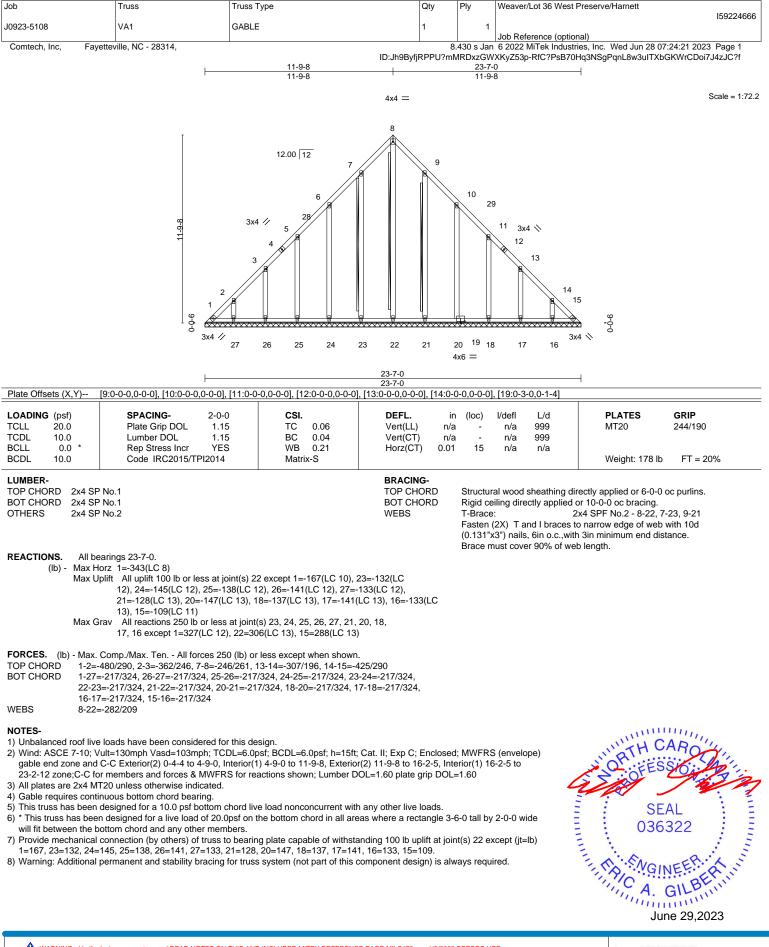
10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



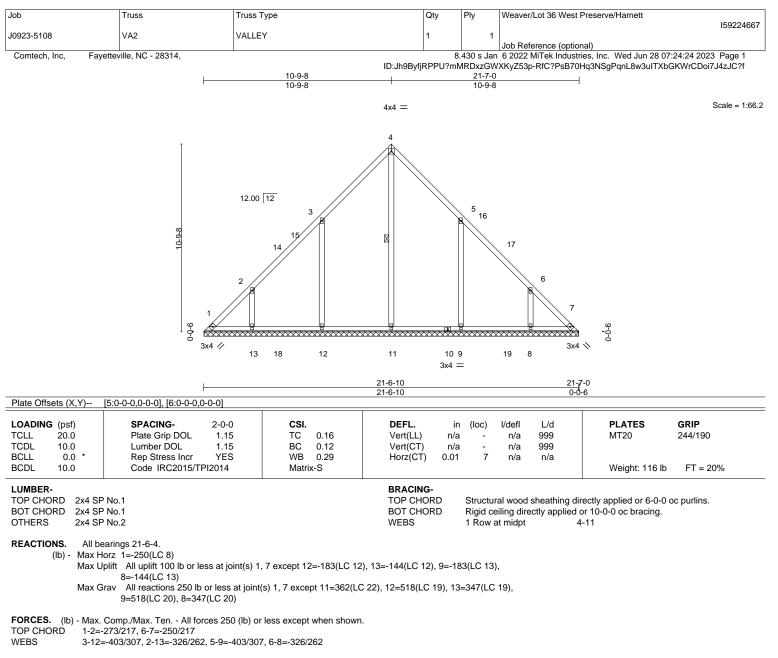
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 10-9-8, Exterior(2) 10-9-8 to 15-2-5, Interior(1) 15-2-5 to 21-2-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

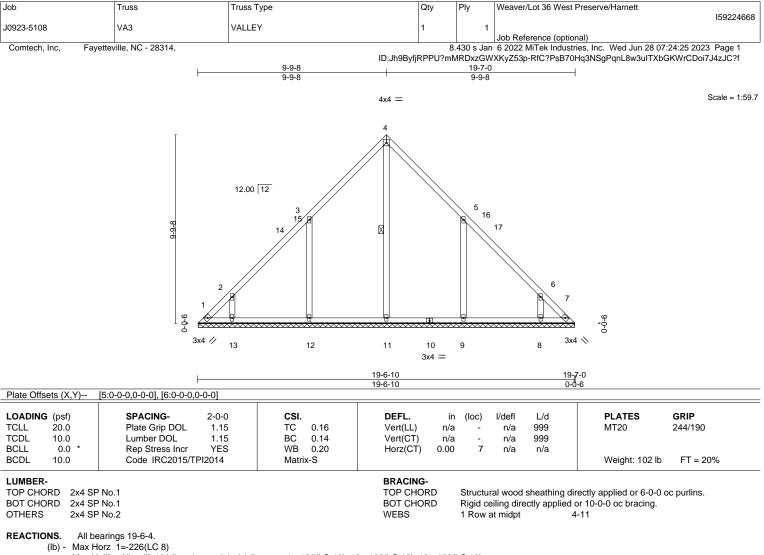
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7 except (jt=lb)

12=183, 13=144, 9=183, 8=144.



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Max Uplift All uplift 100 lb or less at joint(s) 7 except 1=-125(LC 10), 12=-185(LC 12), 13=-132(LC 12), 9=-185(LC 13), 8=-132(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 11=368(LC 22), 12=461(LC 19), 13=286(LC 19), 9=460(LC 20), 8=286(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-267/225, 6-7=-259/225

WEBS 3-12=-406/309, 2-13=-307/258, 5-9=-406/309, 6-8=-307/259

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 9-9-8, Exterior(2) 9-9-8 to 14-2-5, Interior(1) 14-2-5 to 19-2-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

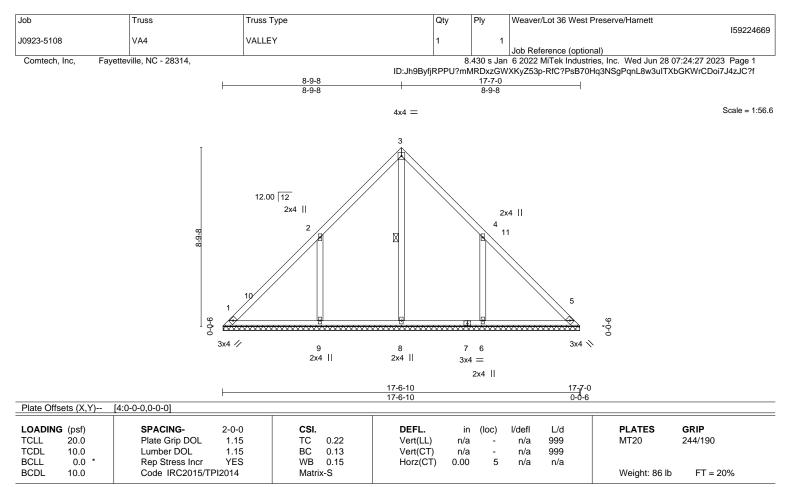
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb)

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 ib uplift at joint(s) 7 except (t=ib) 1=125, 12=185, 13=132, 9=185, 8=132.



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LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 3-8

REACTIONS. All bearings 17-6-4.

(lb) - Max Horz 1=-202(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-213(LC 12), 6=-213(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=345(LC 22), 9=523(LC 19), 6=522(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-9=-457/338, 4-6=-457/338

NOTES-

1) Unbalanced roof live loads have been considered for this design.

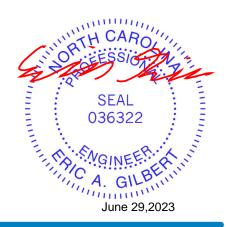
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-8, Interior(1) 4-9-8 to 8-9-8, Exterior(2) 8-9-8 to 13-2-5, Interior(1) 13-2-5 to 17-2-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

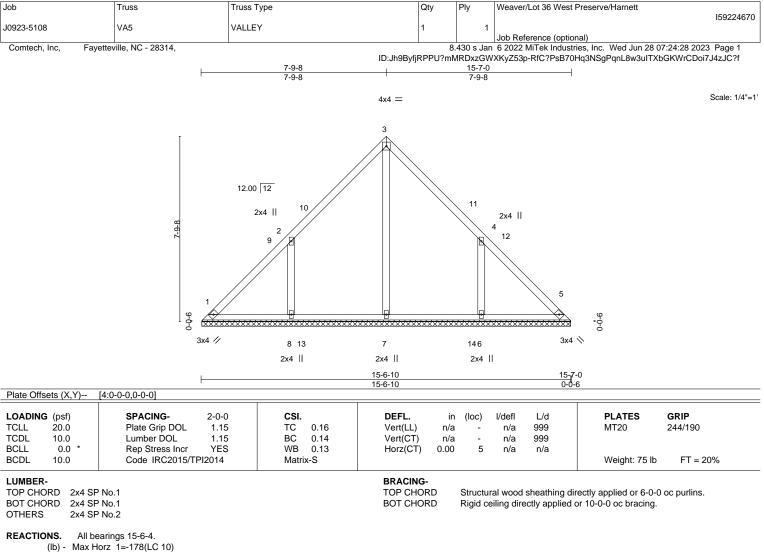
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=213, 6=213.



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Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-186(LC 12), 6=-186(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=348(LC 22), 8=449(LC 19), 6=449(LC 20)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- WEBS 2-8=-403/309, 4-6=-403/309

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 7-9-8, Exterior(2) 7-9-8 to 12-2-5, Interior(1) 12-2-5 to 15-2-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

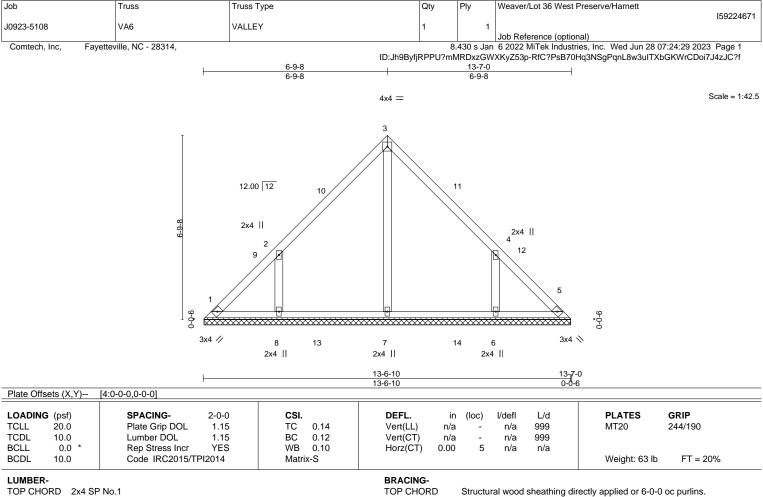
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=186, 6=186.



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818 Soundside Road



BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. All bearings 13-6-4.

(lb) - Max Horz 1=-154(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-166(LC 12), 6=-166(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=335(LC 19), 8=379(LC 19), 6=379(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-364/291, 4-6=-364/292

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-9-8, Exterior(2) 6-9-8 to 11-2-5, Interior(1) 11-2-5 to 13-2-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

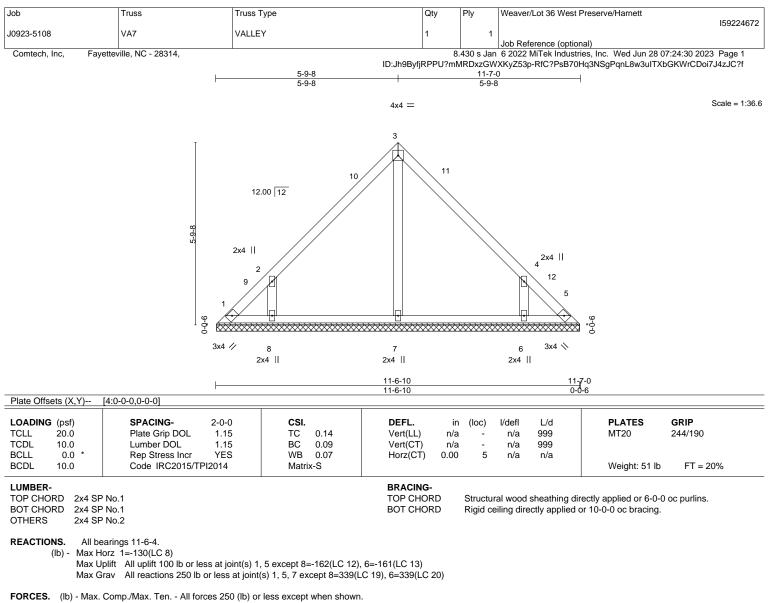
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=166, 6=166.



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WEBS 2-8=-361/303, 4-6=-361/303

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 5-9-8, Exterior(2) 5-9-8 to 10-2-5, Interior(1) 10-2-5 to 11-2-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

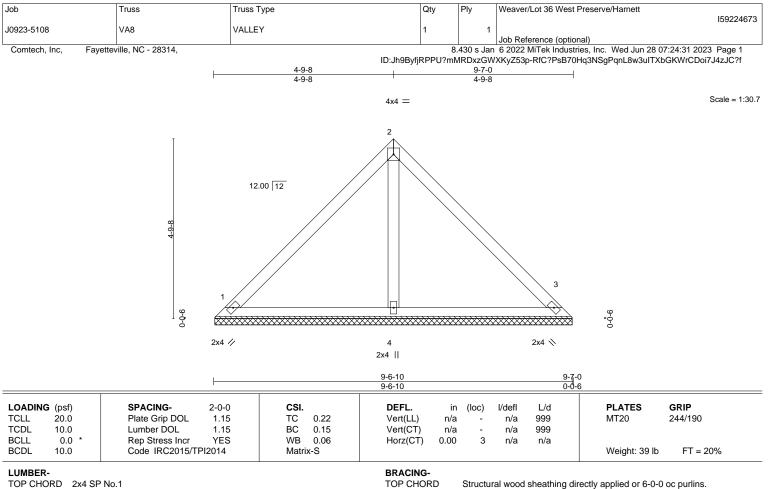
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=162, 6=161.



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A MiTek Affiliate 818 Soundside Road



BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

REACTIONS. (size) 1=9-6-4, 3=9-6-4, 4=9-6-4 Max Horz 1=-106(LC 8)

Max Tiol2 1=-100(EC 0) Max Uplift 1=-26(LC 13), 3=-26(LC 13) Max Grav 1=201(LC 1), 3=201(LC 1), 4=308(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

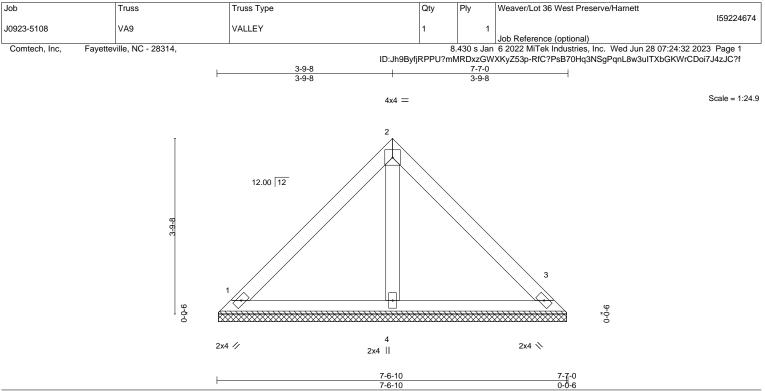
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





			1			7-6-10				0-0-6		
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.09	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2	014	Matrix	κ-P						Weight: 30 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2

OTHERS

REACTIONS. 1=7-6-4, 3=7-6-4, 4=7-6-4 (size) Max Horz 1=82(LC 9) Max Uplift 1=-30(LC 13), 3=-30(LC 13) Max Grav 1=168(LC 1), 3=168(LC 1), 4=215(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

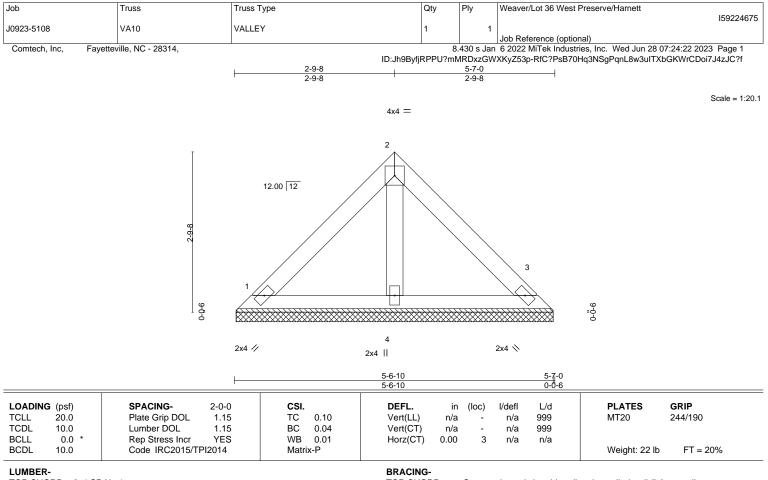


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-7-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=5-6-4, 3=5-6-4, 4=5-6-4 Max Horz 1=-58(LC 8) Max Uplift 1=-21(LC 13), 3=-21(LC 13) Max Grav 1=119(LC 1), 3=119(LC 1), 4=153(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

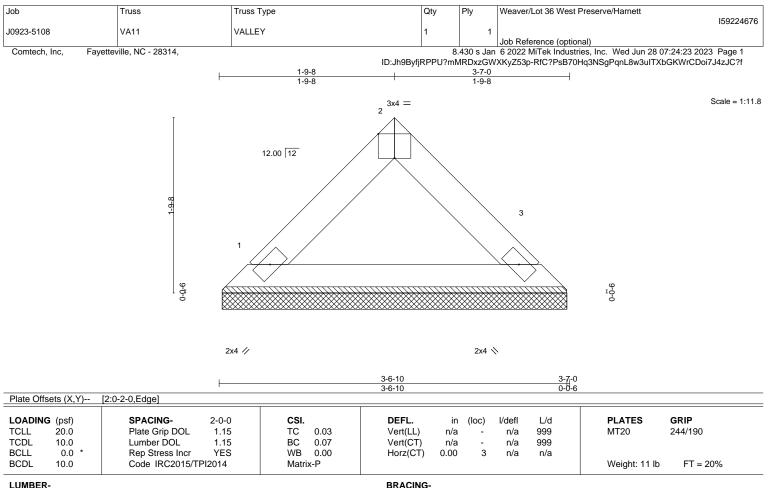
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. 1=3-6-4, 3=3-6-4 (size) Max Horz 1=35(LC 11) Max Uplift 1=-4(LC 12), 3=-4(LC 12) Max Grav 1=115(LC 1), 3=115(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

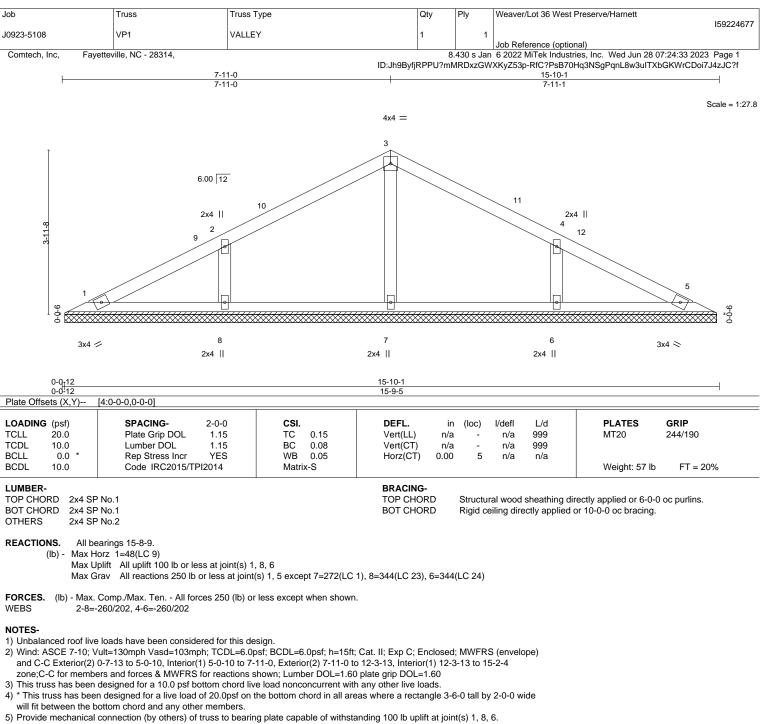


Structural wood sheathing directly applied or 3-7-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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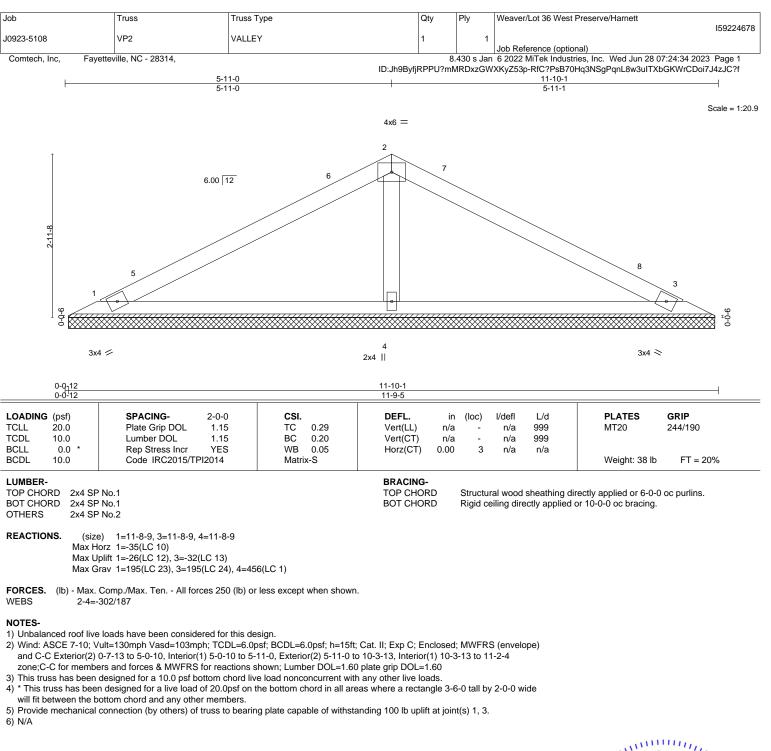


6) N/A



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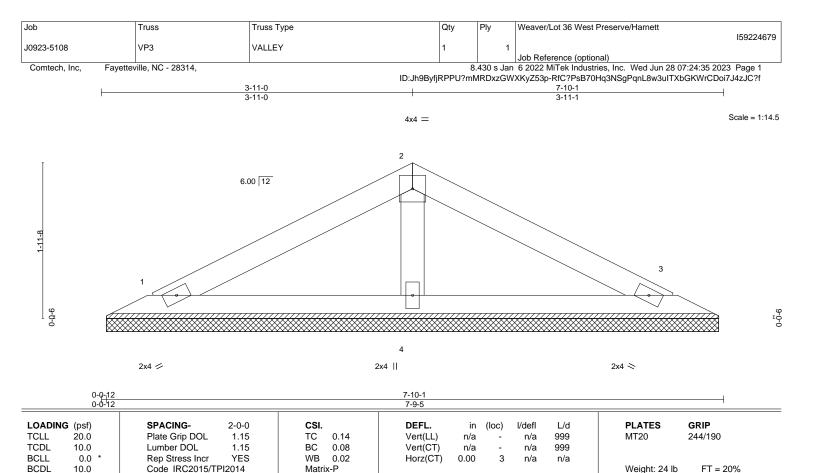




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ENGINEERING BY RENCO A MITek Affiliate

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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.2

REACTIONS. 1=7-8-9, 3=7-8-9, 4=7-8-9 (size) Max Horz 1=-21(LC 8) Max Uplift 1=-21(LC 12), 3=-25(LC 13) Max Grav 1=133(LC 1), 3=133(LC 1), 4=256(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 6) N/A



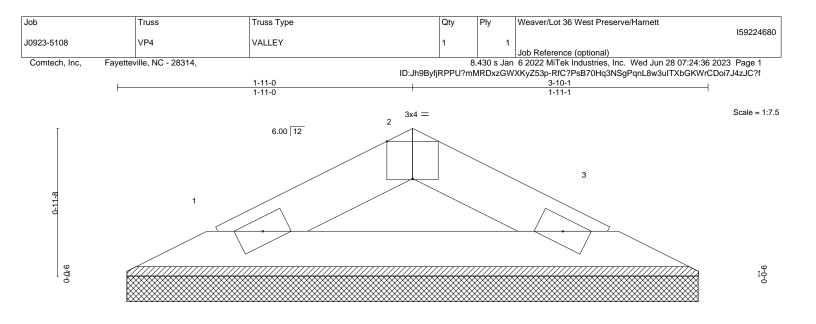
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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2x4 💋

2x4 📚

Rigid ceiling directly applied or 10-0-0 oc bracing.

0-0- Plate Offsets (X,Y) [2:0-2-0,Edge]		3-9-5	-
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.03	Vert(LL) n/a - n/a 999	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.06	Vert(CT) n/a - n/a 999	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a	
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 10 lb FT = 20%

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. 1=3-8-9, 3=3-8-9 (size) Max Horz 1=8(LC 9) Max Uplift 1=-6(LC 12), 3=-6(LC 13) Max Grav 1=101(LC 1), 3=101(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 6) N/A



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