

RE: J0923-5111 Weaver/Lot 30 West Preserve/Harnett

Site Information:

Customer: Project Name: J0923-5111 Lot/Block: Address: City: Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 21 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	156316675	A1	1/26/2023	21	156316695	PBGE	1/26/2023
2	156316676	A1SE	1/26/2023				
3	156316677	A2	1/26/2023				
4	156316678	A3	1/26/2023				
5	156316679	A4	1/26/2023				
6	156316680	A5	1/26/2023				
7	156316681	A6	1/26/2023				
8	156316682	A7	1/26/2023				
9	156316683	A8	1/26/2023				
10	156316684	A9	1/26/2023				
11	156316685	A9GE	1/26/2023				
12	156316686	B1	1/26/2023				
13	156316687	B1GE	1/26/2023				
14	156316688	C1	1/26/2023				
15	156316689	C2	1/26/2023				
16	156316690	C3	1/26/2023				
17	156316691	G1	1/26/2023				
18	156316692	G1GE	1/26/2023				
19	156316693	G2	1/26/2023				
20	156316694	PB	1/26/2023				

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2023

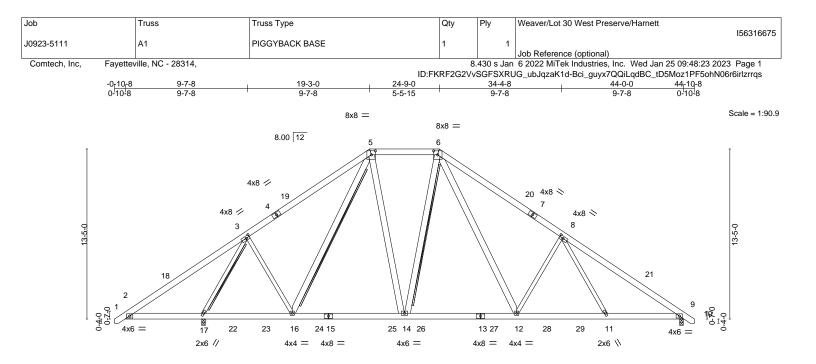
North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

Trenco 818 Soundside Rd Edenton, NC 27932



	6-1-12	13-1-12	22-0-0	1 30	0-10-4	37-10-4	44-0-0	
	6-1-12	7-0-0	8-10-4	8	-10-4	7-0-0	6-1-12	
Plate Offsets (X,Y)	[3:0-3-6,0-2-1], [5:0-3-1	2,0-3-12], [6:0-3	-12,0-3-12], [8:0-3-6,0	-2-0]				
OADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.15	TC 0.48	Vert(LL)	-0.10 12-14	>999 360	MT20	244/190
CDL 10.0	Lumber DOL	1.15	BC 0.45	Vert(CT)	-0.17 12-14	>999 240		
CLL 0.0 *	Rep Stress Incr	YES	WB 0.68	Horz(CT)	0.05 9	n/a n/a		
SCDL 10.0	Code IRC2015/	TPI2014	Matrix-S	Wind(LL)	0.04 12	>999 240	Weight: 395 lb	FT = 20%
JMBER-				BRACING-	_		·	
OP CHORD 2x6 SF				TOP CHOR			directly applied or 4-7-3	oc purlins, except
OT CHORD 2x6 SF						oc purlins (6-0-0 max	/	
	P No.1 *Except*			BOT CHOR		0 7 11	d or 10-0-0 oc bracing,	Except:
3-17,3	-16,8-12,8-11: 2x4 SP N	lo.2			6-0-0 0	oc bracing: 2-17.		
				WEBS	T-Brac	e:	2x4 SPF No.2 - 3-17	
							2x6 SPF No.2 - 5-16, 6-	-14
					Faster	(2X) T and I braces	s to narrow edge of web	with 10d
					(0.131	"x3") nails, 6in o.c.,w	rith 3in minimum end dist	ance.

Brace must cover 90% of web length.

CTIONS.	(size)	17=0-3-8, 9=0-3-8
	Max Horz	17=-320(LC 10)
	Max Uplift	17=-108(LC 12), 9=-94(LC 13)
	Max Grav	17=2255(LC 2), 9=1725(LC 20)

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-405/688, 3-5=-1444/385, 5-6=-1256/435, 6-8=-2202/565, 8-9=-2628/426
- BOT CHORD 2-17=-458/455, 16-17=-181/968, 14-16=-6/1238, 12-14=0/1314, 11-12=-218/1927, 9-11=-190/2042
- WEBS 3-17=-2295/704, 3-16=-19/655, 8-12=-744/366, 8-11=0/310, 5-16=-278/131, 6-14=-283/186, 5-14=-75/684, 6-12=-236/1034

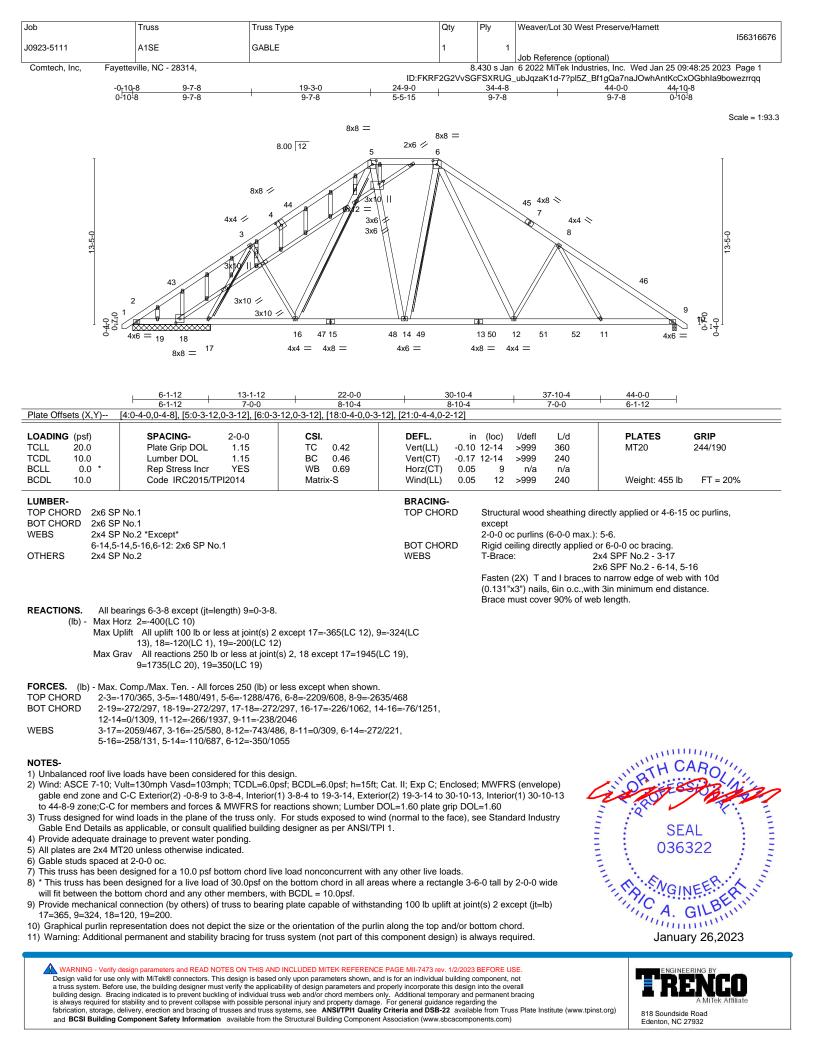
NOTES-

REA

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-9 to 3-8-4, Interior(1) 3-8-4 to 19-3-14, Exterior(2) 19-3-14 to 30-10-13, Interior(1) 30-10-13 to 44-8-9 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 17=108.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



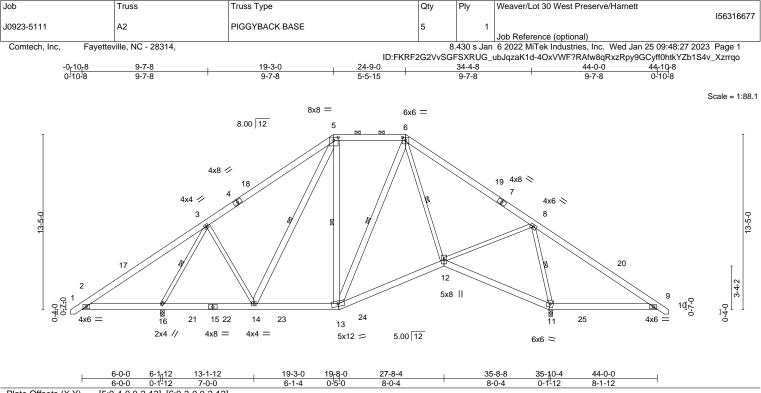


Plate Offsets (X,Y)	[5:0-4-0,0-2-13], [6:0-3-0,	0-2-12]							1	
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.51	Vert(LL)	-0.04 1	13-14	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.21	Vert(CT)	-0.09 1	12-13	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.41	Horz(CT)	0.03	11	n/a	n/a		
BCDL 10.0	Code IRC2015/TF	PI2014	Matrix-S	Wind(LL)	0.02 1	13-14	>999	240	Weight: 381 lb	FT = 20%
LUMBER-		ŀ		BRACING-						
TOP CHORD 2x6 SF BOT CHORD 2x6 SF				TOP CHOR				sheathing di (6-0-0 max.)	rectly applied or 6-0-0 c : 5-6.	oc purlins, except
WEBS 2x4 SP No.2 *Except* 5-13.5-14.6-13: 2x6 SP No.1			BOT CHORI WEBS	D F	Rigid c		ectly applied	or 6-0-0 oc bracing. 3-16, 5-13, 6-12, 8-11, 5	5-14 6-13	

REACTIONS. (size) 11=0-3-8, 16=0-3-8 Max Horz 16=-320(LC 10) Max Uplift 11=-118(LC 13), 16=-112(LC 12) Max Grav 11=1938(LC 1), 16=1673(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD
- 2-3=-408/688, 3-5=-889/255, 5-6=-703/289, 6-8=-853/102, 8-9=-484/769 2-16=-459/457, 14-16=-211/657, 13-14=-60/673, 12-13=-29/726, 11-12=-306/483, BOT CHORD 9-11=-511/519
- WEBS 3-16=-1650/574, 8-12=-6/825, 6-12=-53/432, 3-14=-37/358, 8-11=-1687/540

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-9 to 3-8-4, Interior(1) 3-8-4 to 19-3-0, Exterior(2) 19-3-0 to 30-11-10, Interior(1) 30-11-10 to 44-8-9 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

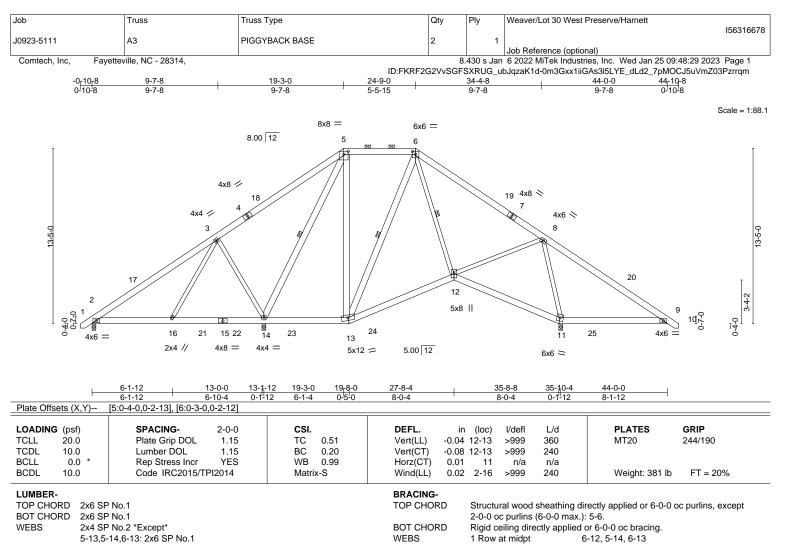
* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5) will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=118, 16=112.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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REACTIONS. (size) 2=0-3-8, 11=0-3-8, 14=0-3-8 Max Horz 14=-320(LC 10) Max Uplift 2=-56(LC 9), 11=-121(LC 13), 14=-141(LC 9) Max Grav 2=483(LC 23), 11=1674(LC 1), 14=1546(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-455/240, 3-5=-59/369, 5-6=-384/210, 6-8=-493/69, 8-9=-484/769

- BOT CHORD 2-16=-71/274, 13-14=-121/314, 12-13=-125/434, 11-12=-361/496, 9-11=-511/519
- WEBS 3-16=-292/330, 8-12=0/547, 5-13=-30/367, 6-12=-87/438, 3-14=-745/533,

8-11=-1400/466, 5-14=-859/143, 6-13=-255/25

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-9 to 3-8-4, Interior(1) 3-8-4 to 19-3-0, Exterior(2) 19-3-0 to 30-11-10, Interior(1) 30-11-10 to 44-8-9 zone; cantilever right exposed ; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

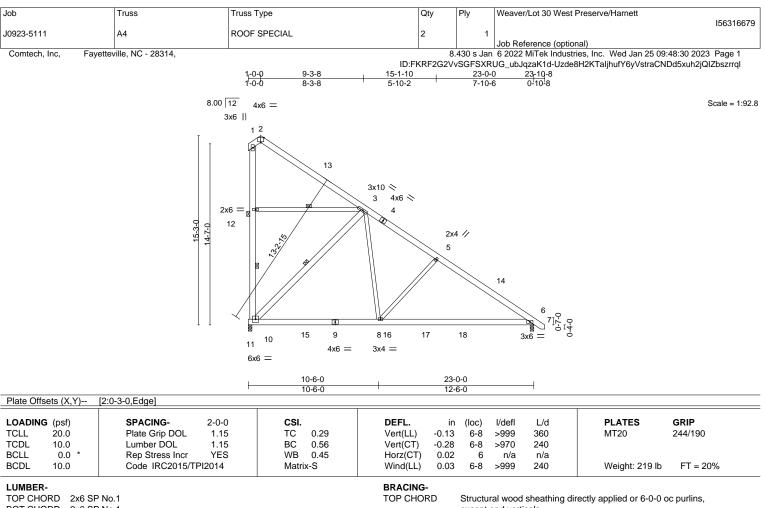
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 11=121, 14=141.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x6 SP No.1		except end verticals.
WEBS	2x4 SP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
	1-10,3-10: 2x6 SP No.1	WEBS	1 Row at midpt 10-12, 3-10, 3-12
		JOINTS	1 Brace at Jt(s): 12
REACTIONS.	(size) 10=0-3-8, 6=0-3-8		

Max Uplift 10=-3-6, 6=0-3-6 Max Uplift 10=-482(LC 13) Max Uplift 10=-236(LC 13) Max Grav 10=1177(LC 20), 6=1071(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-5=-1052/0, 5-6=-1298/0, 10-12=-259/166, 1-12=-259/166

BOT CHORD 8-10=0/743, 6-8=0/987

WEBS 5-8=-424/234, 3-10=-1116/335, 3-8=-26/850

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 5-4-13, Interior(1) 5-4-13 to 23-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

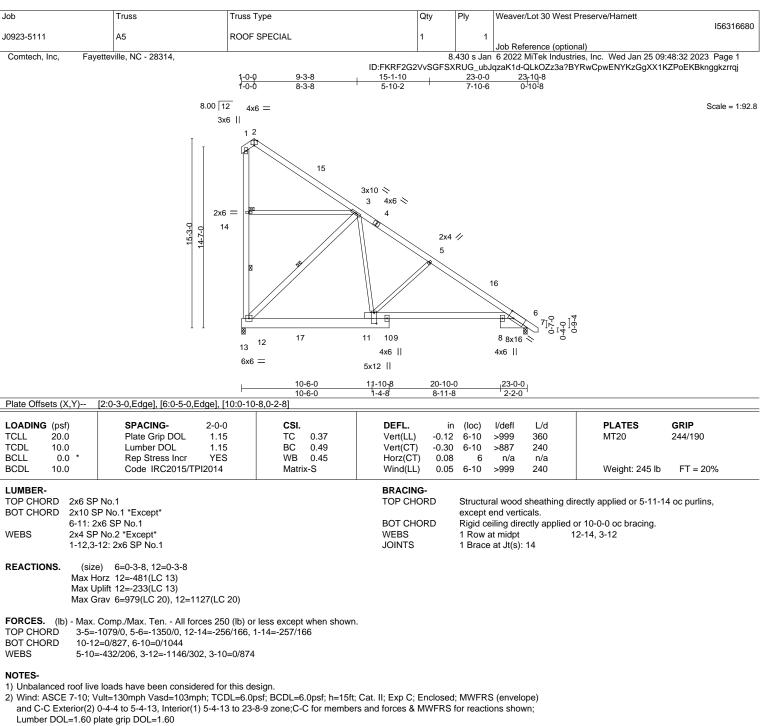
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=236.



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A MiTek Affil 818 Soundside Road Edenton, NC 27932



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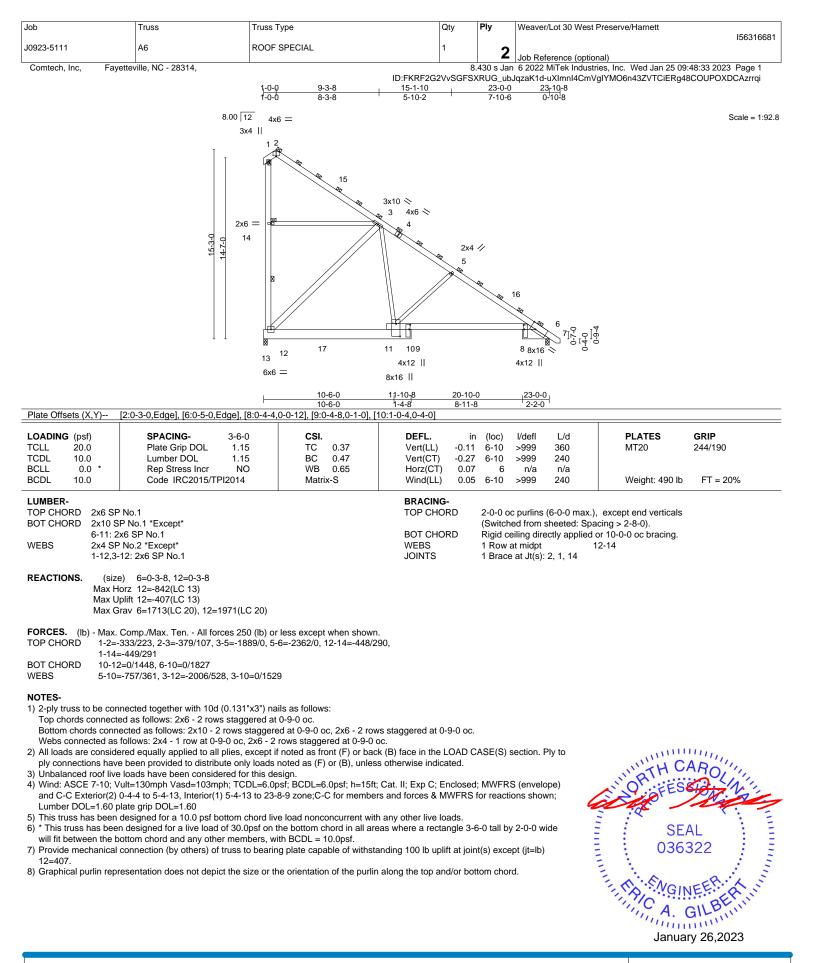
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=233.



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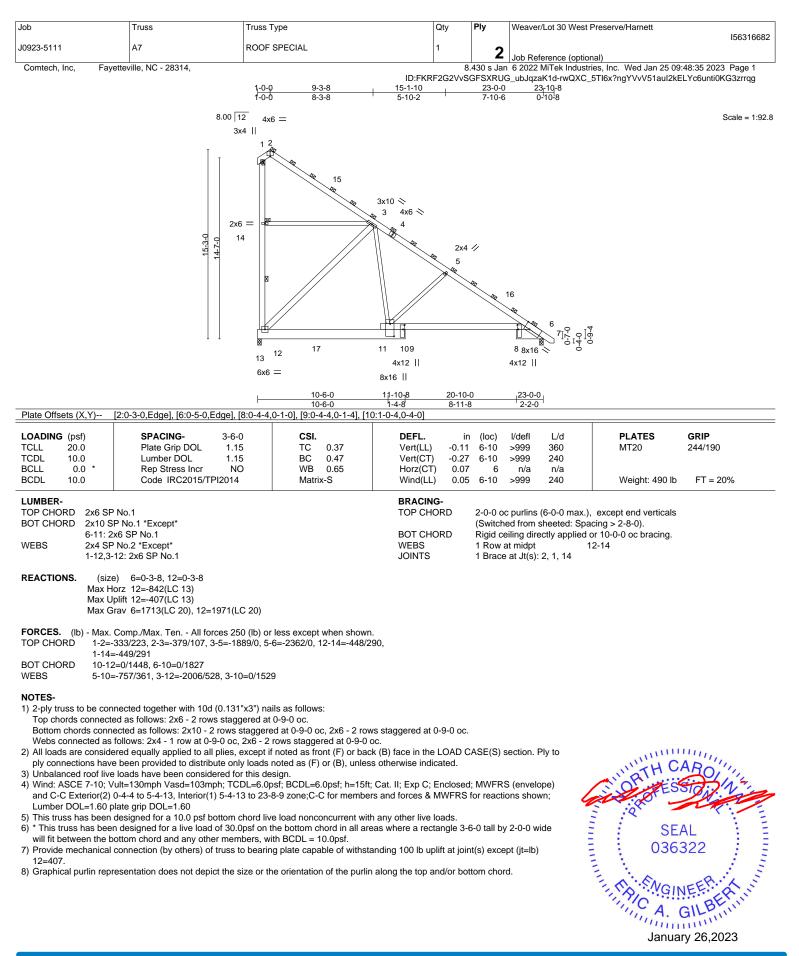


818 Soundside Road



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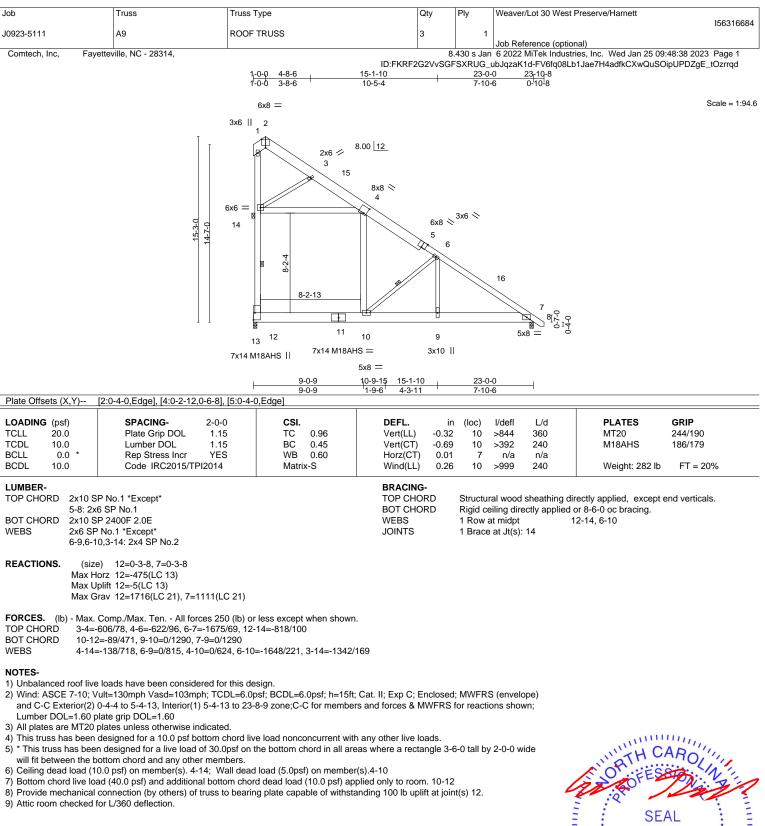
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Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 30 West Preserve/Ha	arnett 156316683
J0923-5111	A8	ROOF TRUSS	1	2	lob Poforones (anties = 1)	156316683
Comtech, Inc, Fay	etteville, NC - 28314,		ID:FKRF2G2VvSG	430 s Jan		
		6x8 =	0-5-3	7-10	<i>-</i> 6 0-10-8	Scale = 1:89.8
		8x6 2				
		$46 = \begin{bmatrix} 2x6 \\ 315 \\ 315 \\ \hline \\ 8 \\ \hline \\ 8 \\ 2 \\ 13 \\ 12 \\ 13 \\ 12 \\ 2x6 \\ \hline \\ 8 \\ 2 \\ 11 \\ 10 \\ 2x6 \\ \hline \\ 8 \\ 8 \\ 2x6 \\ \hline \\ 8 \\ 8 \\ 8 \\ 8 \\ 8 \\ 8 \\ 8 \\ 8 \\ 8$	6	6 *	$ \begin{array}{c} 16 \\ 7 \\ 8 \\ 6 \\ 7 \\ 8 \\ 6 \\ 7 \\ 6 \\ 7 \\ 6 \\ 7 \\ 6 \\ 7 \\ 7 \\ 7 \\ 7 \\ 7 \\ 7 \\ 7 \\ 7 \\ 7 \\ 7$	
Plate Offsets (X,Y)	[2:0-4-0,Edge], [4:0-6-0,0-2-8],	9-0-9 1	-9-15 15-1-10 -9-6 4-3-11	<u>23-(</u> 7-1(
LOADING (psf)	SPACING- 2-8-		DEFL. in	(loc)	l/defl L/d PLA	ATES GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.1 Lumber DOL 1.1	5 TC 0.73	Vert(LL) -0.21 Vert(CT) -0.46	10	>999 360 MT2	
BCLL 0.0 * BCDL 10.0	Rep Stress Incr No Code IRC2015/TPI2014		Horz(CT) 0.01 Wind(LL) 0.17	7	n/a n/a	ght: 563 lb FT = 20%
BOT CHORD 2x10 S WEBS 2x6 SP 6-9,6-1 REACTIONS. (size Max H Max U	P No.1 *Except* 6 SP No.1 P 2400F 2.0E No.1 *Except* 0,3-14: 2x4 SP No.2 e) 12=0-3-8, 7=0-3-8 orz 12=-633(LC 13) plift 12=-7(LC 13) plift 12=-7(LC 13) rav 12=2288(LC 21), 7=1482(L	2 21)	BRACING- TOP CHORD BOT CHORD WEBS JOINTS	except e Rigid ce 1 Row a	al wood sheathing directly applie nd verticals. ling directly applied or 10-0-0 oc t midpt 12-14 at Jt(s): 14	-
FORCES. (Ib) - Max. TOP CHORD 3-4=- BOT CHORD 10-12	Comp./Max. Ten All forces 25 808/104, 4-6=-829/128, 6-7=-22 !=-119/629, 9-10=0/1720, 7-9=0	0 (lb) or less except when shown. 33/92, 12-14=-1091/133	25			
Top chords connecte Bottom chords connecte Bottom chords connected as 2) All loads are conside ply connections have 3) Unbalanced roof live 4) Wind: ASCE 7-10; V and C-C Exterior(2) Lumber DOL=1.60 p 5) All plates are MT20 6) This truss has been will fit between the b 8) Ceiling dead load (1 9) Bottom chord live load	ected as follows: 2x10 - 2 rows follows: 2x6 - 2 rows staggered red equally applied to all plies, a been provided to distribute on loads have been considered fo ult=130mph Vasd=103mph; TC 0-4-4 to 5-4-13, Interior(1) 5-4-1 late grip DOL=1.60 olates unless otherwise indicate designed for a 10.0 psf bottom in designed for a live load of 30.0 ottom chord and any other men 0.0 psf) on member(s). 4-14; W ad (40.0 psf) and additional bott I connection (by others) of truss	Igered at 0-9-0 oc, 2x6 - 2 rows stag staggered at 0-9-0 oc. at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc. except if noted as front (F) or back (F y loads noted as (F) or (B), unless o r this design. DL=6.0psf; BCDL=6.0psf; h=15ft; Ca 3 to 23-8-9 zone;C-C for members a d. shord live load nonconcurrent with an opsf on the bottom chord in all areas	B) face in the LOAD C. therwise indicated. at. II; Exp C; Enclosed and forces & MWFRS f ny other live loads. where a rectangle 3-6 .4-10 ed only to room. 10-12	MWFRS or reactions 6-0 tall by	(envelope) ns shown; 2-0-0 wide	SEAL 036322

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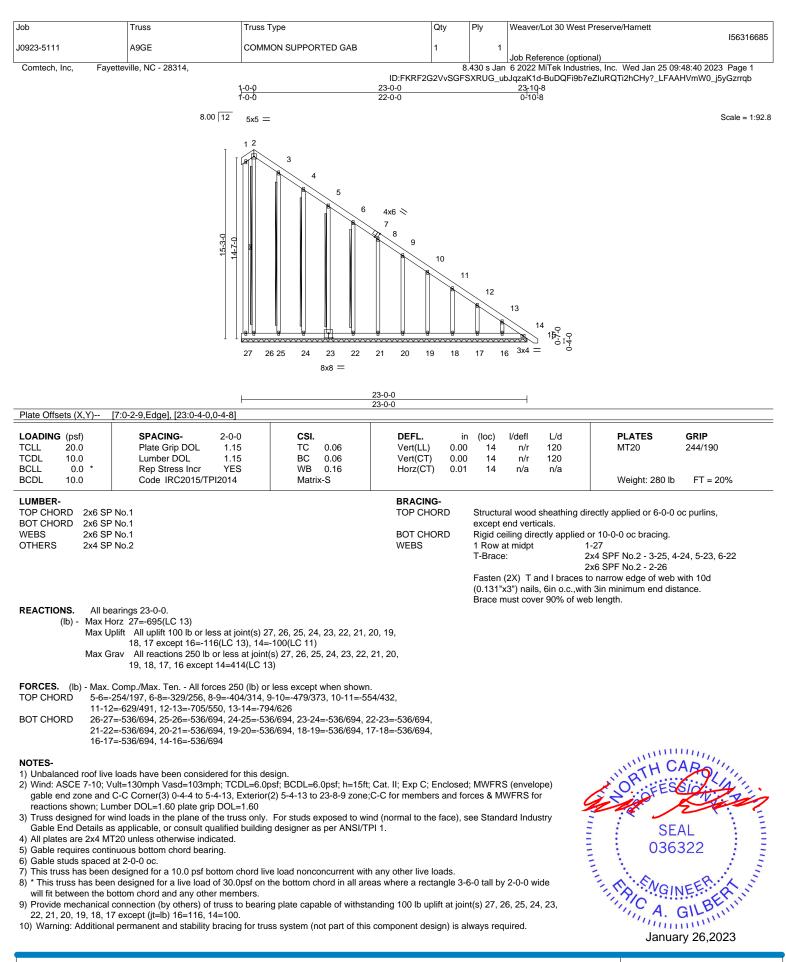
818 Soundside Road Edenton, NC 27932

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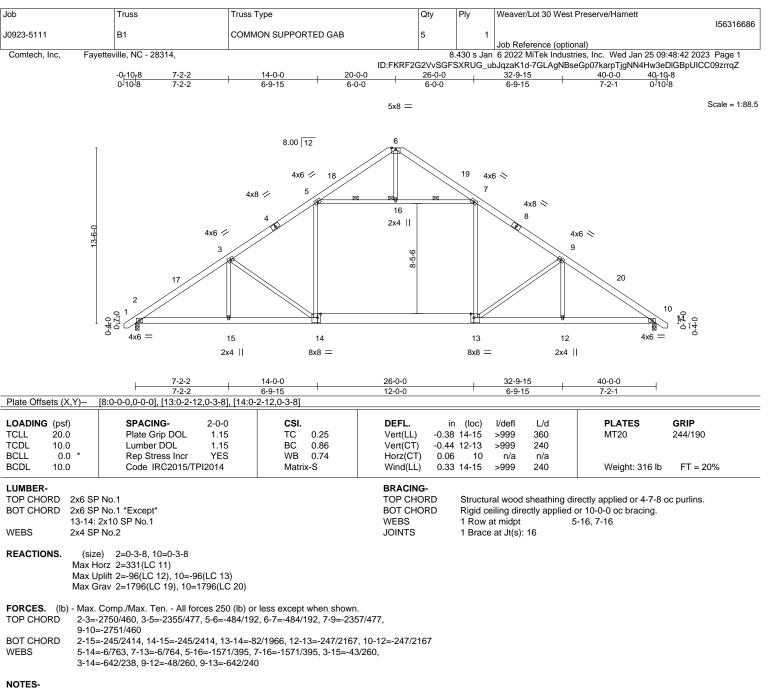


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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-9 to 3-8-4, Interior(1) 3-8-4 to 20-0-0, Exterior(2) 20-0-0 to 24-4-13, Interior(1) 24-4-13 to 40-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

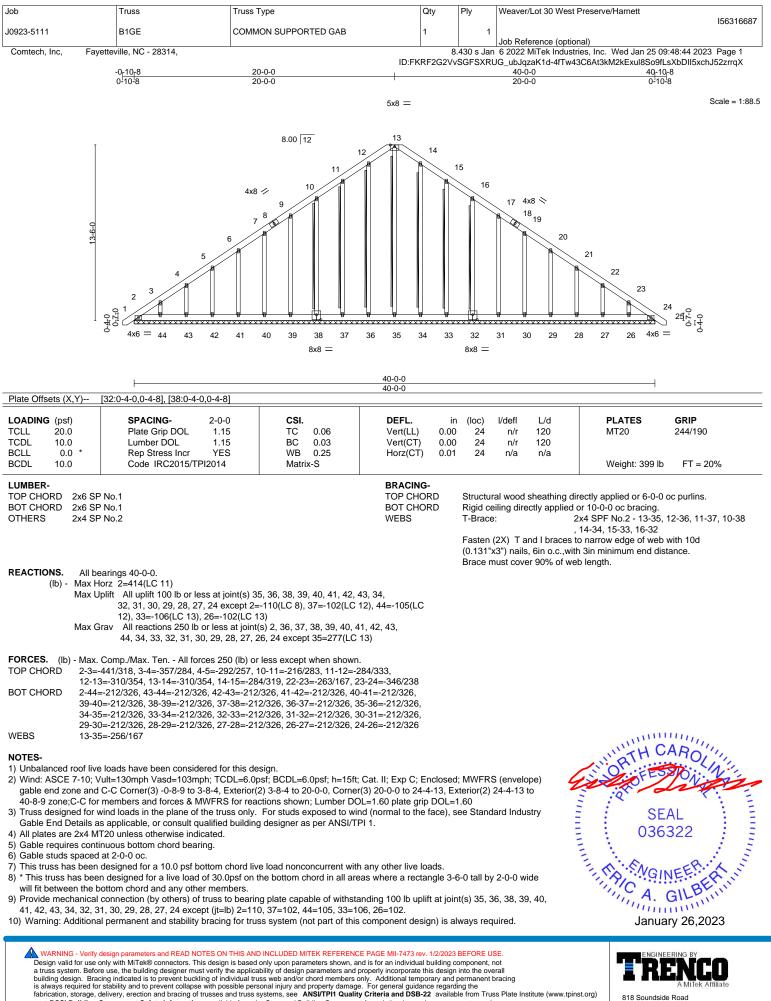
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

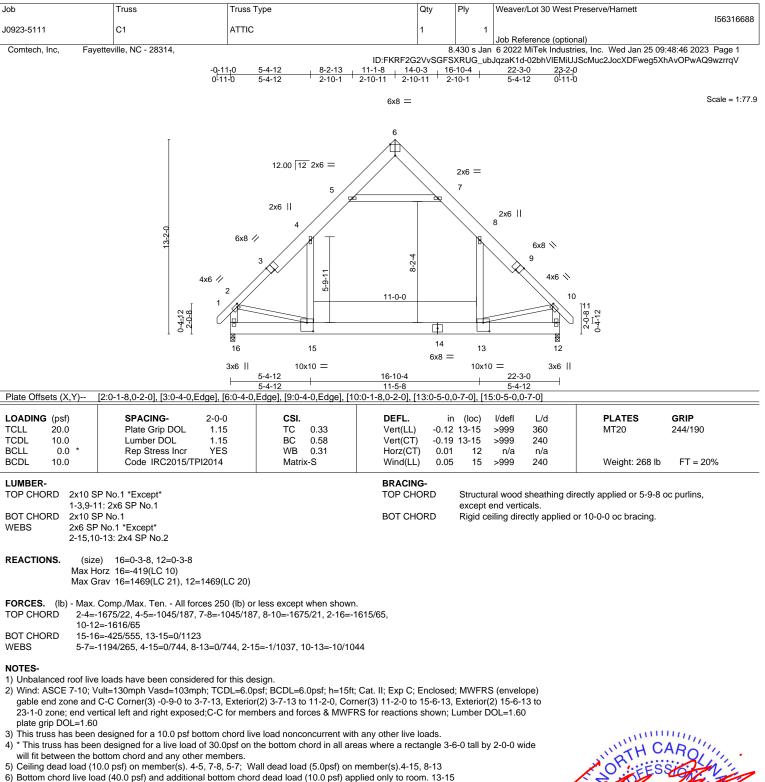


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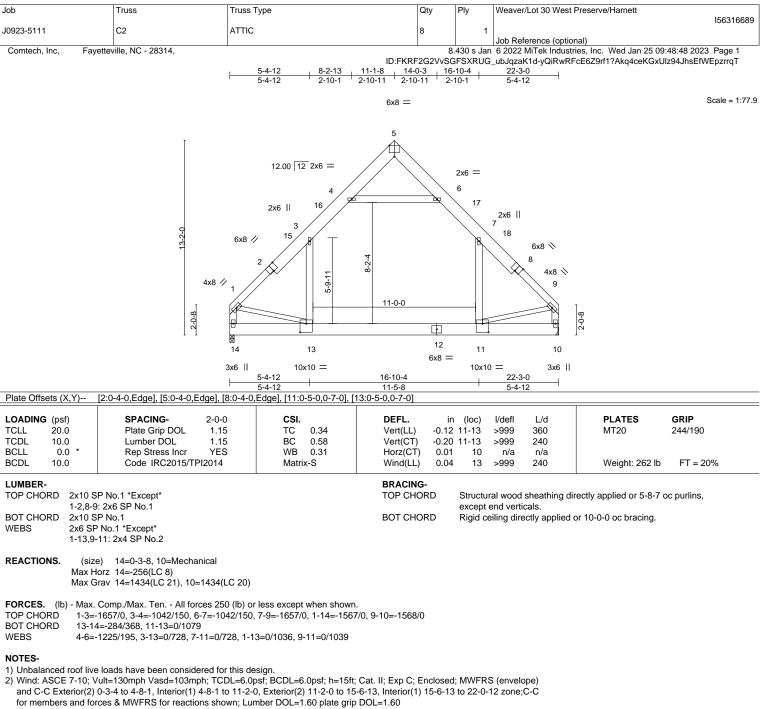
7) Attic room checked for L/360 deflection.

7) Allic room checked for L/360 denection



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3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 3-4, 6-7, 4-6; Wall dead load (5.0psf) on member(s).3-13, 7-11

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

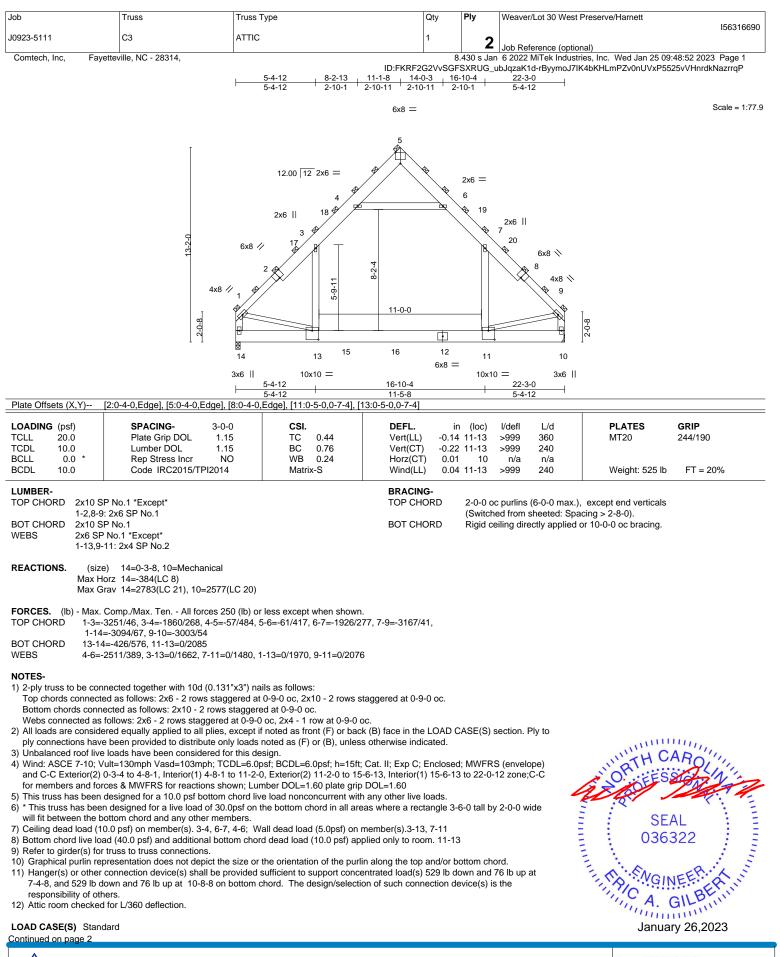
7) Refer to girder(s) for truss to truss connections.

8) Attic room checked for L/360 deflection.



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ſ	Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 30 West Preserve/Harnett
						156316690
	J0923-5111	C3	ATTIC	1	2	
					2	Job Reference (optional)
	Comtech, Inc, Fayettev	ille, NC - 28314,		8	.430 s Jan	6 2022 MiTek Industries, Inc. Wed Jan 25 09:48:52 2023 Page 2

ID:FKRF2G2VvSGFSXRUG_ubJqzaK1d-rByymoJ7IK4bKHLmPZv0nUVxP5525vVHnrdkNazrrqP

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

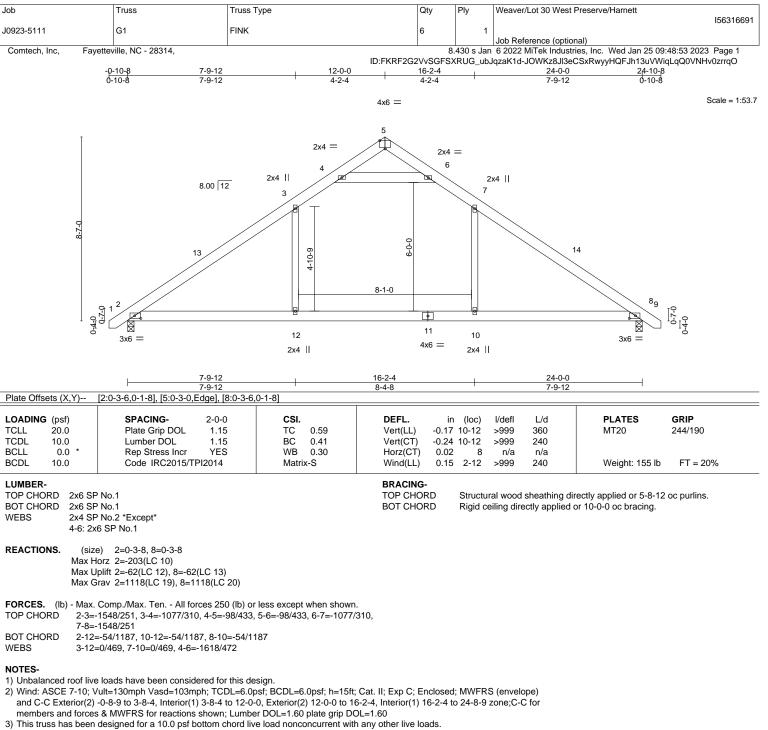
Vert: 13-14=-30, 11-13=-60, 10-11=-30, 1-3=-90, 3-4=-120, 4-5=-90, 5-6=-90, 6-7=-120, 7-9=-90, 4-6=-30 Drag: 3-13=-15, 7-11=-15

Concentrated Loads (lb)

Vert: 15=-300(B) 16=-300(B)

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4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

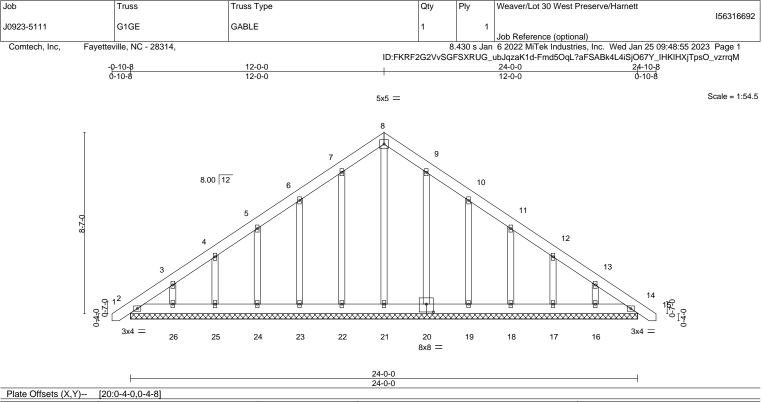
will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



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LOADING (psf)	SPACING- 2-0-0 CSI.		DEFL.	in	(loc)	l/defl		PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.03	Vert(LL)	0.00	14	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.02	Vert(CT)	0.00	14	n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.16	Horz(CT)	0.00	14	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S						Weight: 195 lb	FT = 20%

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 24-0-0.

(lb) - Max Horz 2=-254(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 22, 23, 24, 25, 26, 20, 19, 18, 17, 16

Max Grav All reactions 250 lb or less at joint(s) 2, 14, 21, 22, 23, 24, 25, 26, 20, 19, 18, 17, 16

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-9 to 3-8-4, Exterior(2) 3-8-4 to 12-0-0, Corner(3) 12-0-0 to 16-4-13, Exterior(2) 16-4-13 to 24-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

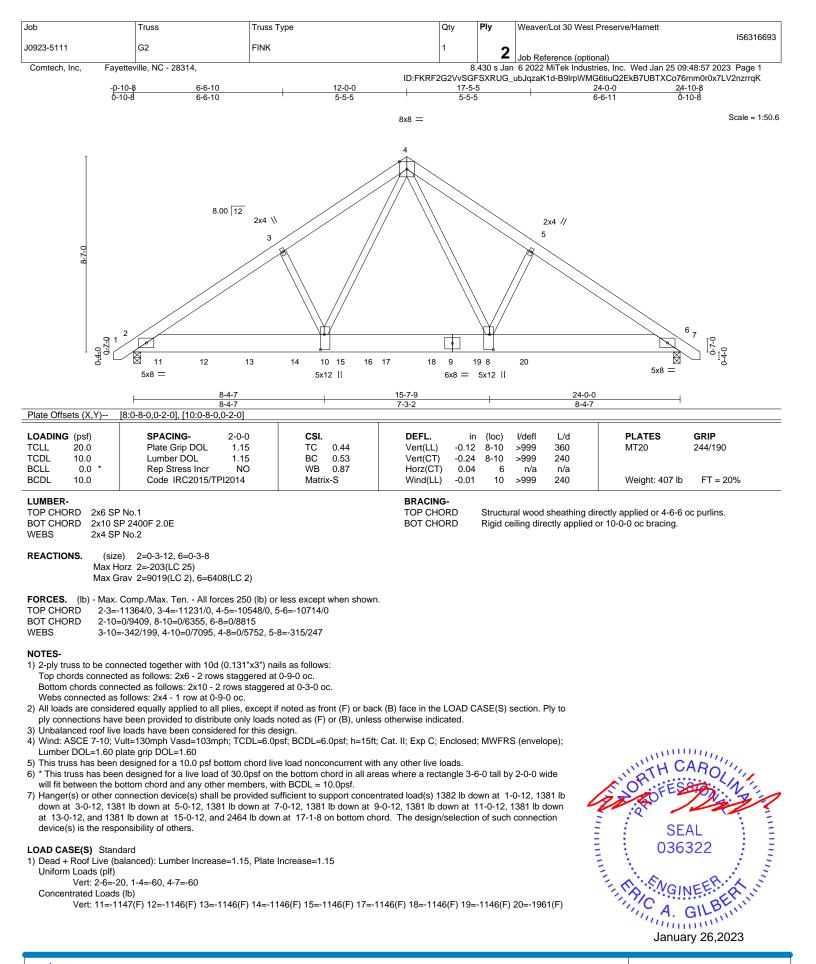
8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 22, 23, 24, 25, 26, 20, 19, 18, 17, 16.

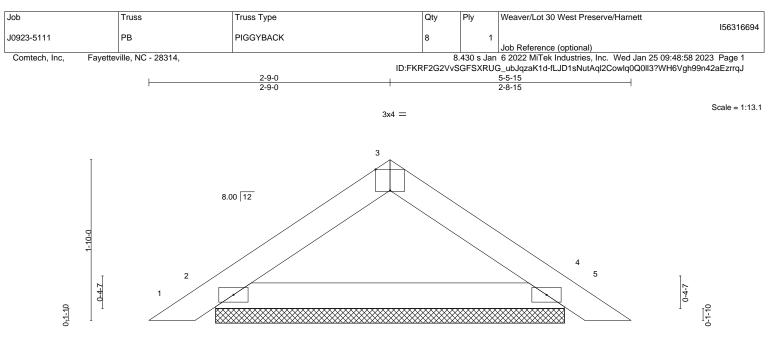


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2x4 =

2x4 =

Rigid ceiling directly applied or 10-0-0 oc bracing.

OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) 0.00 4 n/r 120	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.14	Vert(CT) 0.00 5 n/r 120	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 4 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 16 lb FT = 20%

BOT CHORD

<u>5-5-</u>15

BOT CHORD 2x4 SP No.1

REACTIONS. 2=3-11-11, 4=3-11-11 (size) Max Horz 2=-40(LC 10) Max Uplift 2=-16(LC 12), 4=-16(LC 13) Max Grav 2=189(LC 1), 4=189(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

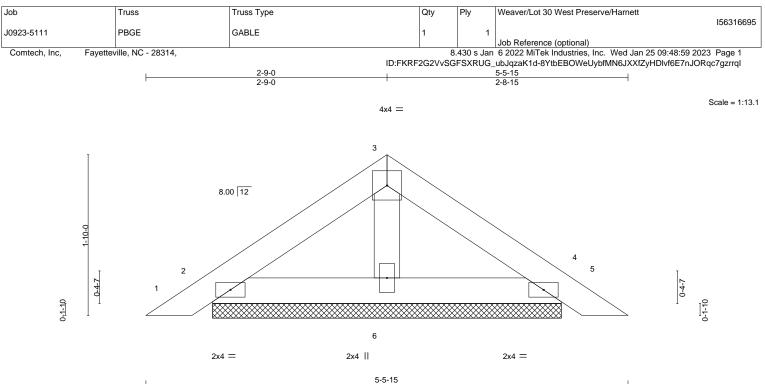
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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	5-5-15												
LOADIN	G (psf)	SPAC	CING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate	Grip DOL	1.15	TC	0.05	Vert(LL)	0.00	4	n/r	120	MT20	244/190
TCDL	10.0	Lumb	er DOL	1.15	BC	0.03	Vert(CT)	0.00	5	n/r	120		
BCLL	0.0 *	Rep S	Stress Incr	YES	WB	0.01	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code	IRC2015/TF	912014	Matri	x-P						Weight: 17 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.2

REACTIONS. 2=3-11-11, 4=3-11-11, 6=3-11-11 (size) Max Horz 2=50(LC 11) Max Uplift 2=-45(LC 12), 4=-52(LC 13) Max Grav 2=119(LC 1), 4=119(LC 1), 6=139(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

Structural wood sheathing directly applied or 5-5-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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Voumment 1111111111 SEAL 036322 G mmm January 26,2023

