

RE: MP18

DRHORTON/WILMINGTON; LOT 18 MCKAY PLACE

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: MP18 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.6 Wind Speed: 120 mph Floor Load: N/A psf

This package includes 12 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	159163116	A01AG	6/26/2023
2	159163117	A02	6/26/2023
3	159163118	A02A	6/26/2023
4	159163119	A03	6/26/2023
5	159163120	A04V	6/26/2023
6	159163121	A05AV	6/26/2023
7	159163122	A05V	6/26/2023
8	159163123	A06AVG	6/26/2023
9	159163124	B01G	6/26/2023
10	159163125	B02GR	6/26/2023
11	159163126	P01G	6/26/2023
12	159163127	P02	6/26/2023

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

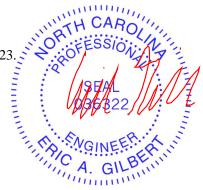
based on the parameters provided by Builders FirstSource-Apex,NC.

Truss Design Engineer's Name: Gilbert, Eric

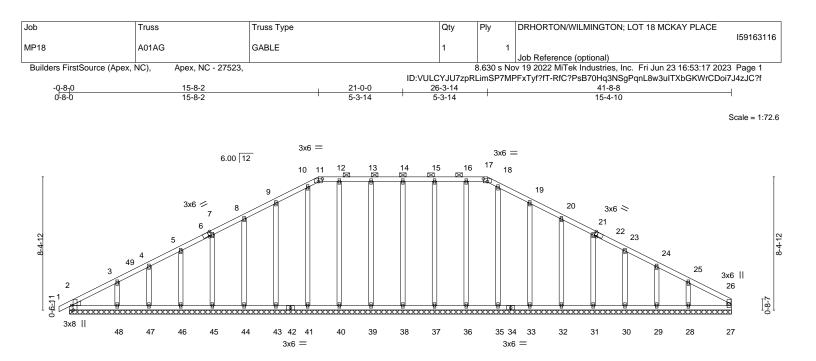
My license renewal date for the state of North Carolina is December 31, 2023

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



L			41-8-8		
			41-8-8		
Plate Offsets (X,Y)	[2:0-3-8,Edge], [6:0-1-13,Edge], [11:0-3	-0,0-2-0], [17:0-3-0,0-2-0],	, [22:0-1-13,Edge]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.10 BC 0.07 WB 0.16 Matrix-S	DEFL. ir Vert(LL) -0.00 Vert(CT) 0.00 Horz(CT) 0.01	1 n/r 120 1 n/r 120	MT20 244/190
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP OTHERS 2x4 SP WEDGE	2 No.2 2 No.3		BRACING- TOP CHORD BOT CHORD	except end verticals, a	hing directly applied or 6-0-0 oc purlins, and 2-0-0 oc purlins (6-0-0 max.): 11-17. pplied or 10-0-0 oc bracing.

Left: 2x4 SP No.3

- REACTIONS. All bearings 41-8-8. (Ib) - Max Horz 2=121(LC 16)
 - Max Uplift All uplift 100 lb or less at joint(s) 2, 38, 39, 40, 43, 44, 45, 46, 47, 48, 37, 36, 33, 32, 31, 30, 29, 28
 - Max Grav All reactions 250 lb or less at joint(s) 27, 2, 38, 39, 40, 41, 43, 44, 45, 46, 47, 48, 37, 36, 35, 33, 32, 31, 30, 29, 28

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

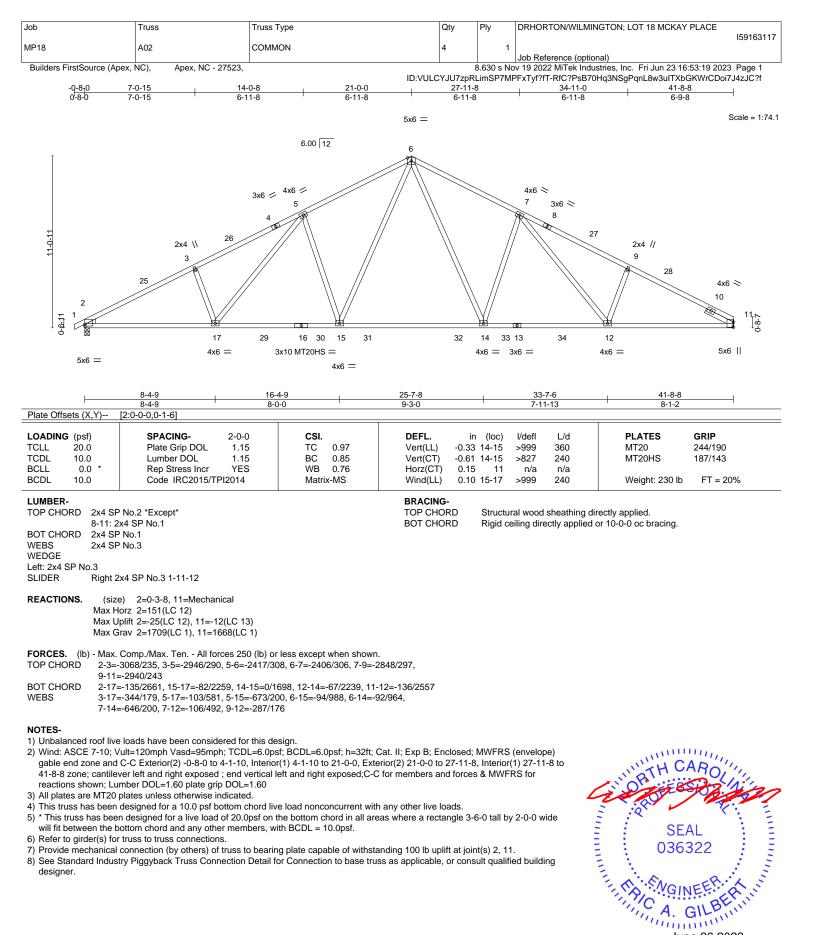
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-0 to 4-1-10, Exterior(2) 4-1-10 to 15-8-2, Corner(3) 15-8-2 to 20-5-12, Exterior(2) 20-5-12 to 26-3-14, Corner(3) 26-3-14 to 31-0-0, Exterior(2) 31-0-0 to 41-6-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 38, 39, 40, 43, 44, 45, 46, 47, 48, 37, 36, 33, 32, 31, 30, 29, 28.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



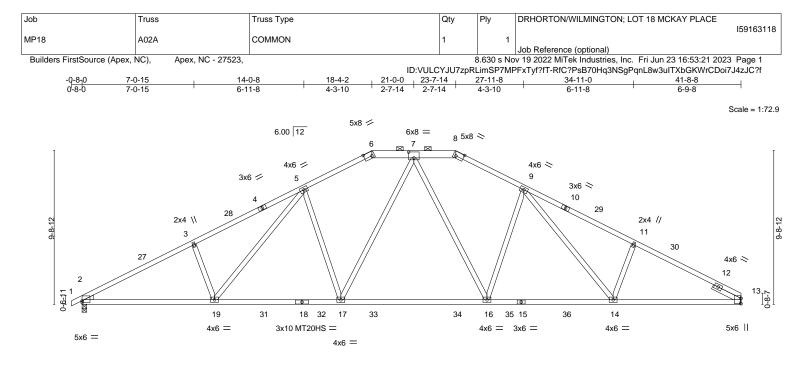


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



June 26,2023

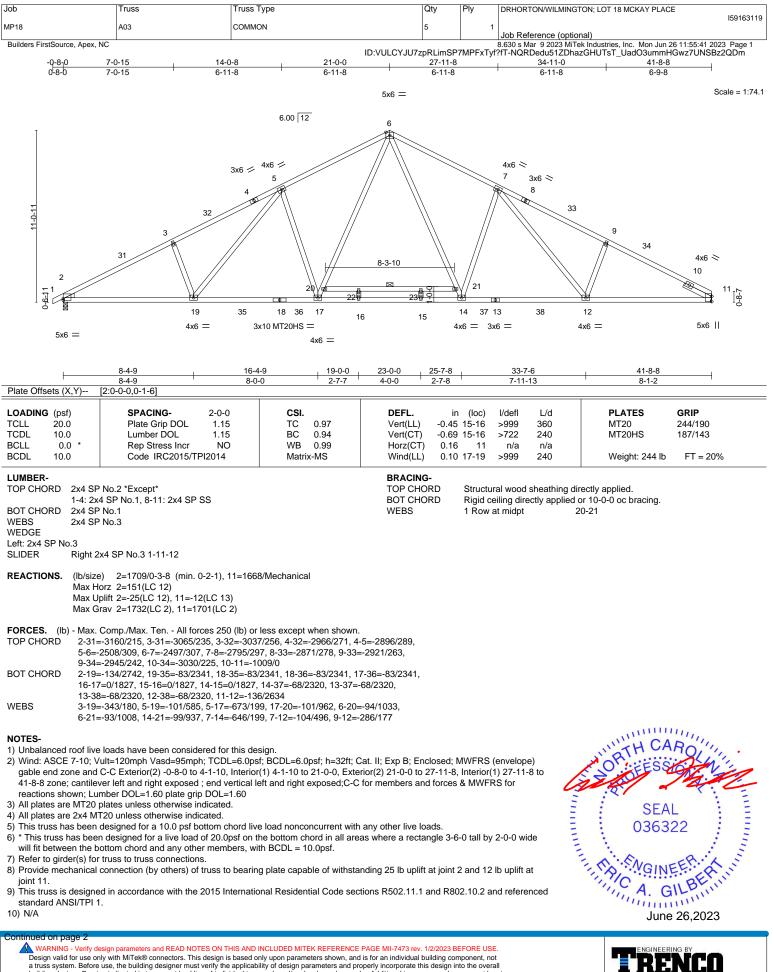
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F	8-4-9	16-4-			25-7-8		_		9-7-6	41-8-8	I
<u> </u>	8-4-9	8-0-0	-	5 4 4 9 9 41	9-3-0			7-1	1-13	8-1-2	
Plate Offsets (X	(,Y) [2:0-0-0,0-1-6], [6:0-5-14	1,0-3-4], [7:0-4-0,0	0-4-8], [8:0	-5-14,0-3-4]	1						
LOADING (psf TCLL 20.0 TCDL 10.0	Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI. TC BC	0.97 0.87	DEFL. Vert(LL) Vert(CT)	-0.40 -0.77	16-17	l/defl >999 >649	L/d 360 240	PLATES MT20 MT20HS	GRIP 244/190 187/143
BCLL 0.0 BCDL 10.0	0 * Rep Stress Incr 0 Code IRC2015/T	YES PI2014	WB Matrix	0.60 ĸ-MS	Horz(CT) Wind(LL)	0.15 0.14	13 17	n/a >999	n/a 240	Weight: 230 lb	FT = 20%
	2x4 SP No.1 *Except* 6-8: 2x6 SP DSS 2x4 SP No.1 2x4 SP No.3 0.3 Right 2x4 SP No.3 1-11-12				BRACING- TOP CHOR BOT CHOR		2-0-0 o	c purlins	(3-2-2 max.)	lirectly applied, except): 6-8. or 10-0-0 oc bracing.	
REACTIONS.	(size) 2=0-3-8, 13=Mecha Max Horz 2=139(LC 12) Max Uplift 2=-16(LC 12), 13=-3 Max Grav 2=1709(LC 1), 13=1	B(LC 13)									
FORCES. (Ib) TOP CHORD	- Max. Comp./Max. Ten All fc 2-3=-3064/269, 3-5=-2945/32 11-13=-2935/276, 6-7=-2066/	4, 5-6=-2355/339	9, 8-9=-234								
BOT CHORD WEBS	2-19=-190/2660, 17-19=-122/ 3-19=-377/186, 5-19=-110/61 9-16=-509/186, 9-14=-113/52	7, 5-17=-534/187	7, 7-17=-82			5					
NoT-0											
 Wind: ASCE gable end zc 41-8-8 zone; reactions shi Provide aded All plates are This truss ha * This truss ha * T	roof live loads have been consic 7-10; Vult=120mph Vasd=95mp one and C-C Exterior(2) -0-8-0 to cantilever left and right exposed own; Lumber DOL=1.60 plate gr quate drainage to prevent water a MT20 plates unless otherwise is been designed for a 10.0 psf thas been designed for a live load en the bottom chord and any oth er(s) for truss to truss connection hanical connection (by others) o d Industry Piggyback Truss Con	bh; TCDL=6.0psf; 4-1-10, Interior(' 1; end vertical lef ip DOL=1.60 ponding. indicated. bottom chord live d of 20.0psf on the er members, with ns. f truss to bearing	; BCDL=6.0 1) 4-1-10 to ft and right load nonco e bottom cl h BCDL = 1 g plate capa	b 18-4-2, Externation exposed;C-C exposed;	erior(2) 18-4-2 to 3 C for members and h any other live loa eas where a rectan anding 100 lb uplift	0-5-5, l forces ds. gle 3-6 at joint	nterior(& MWF -0 tall by	1) 30-5-5 RS for y 2-0-0 w 3.	to	SE/ 0363	• –
designer.	ourlin representation does not de									THE RANGIN	VEEP ALLIN



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Job	Truss	Truss Type	Qty	Ply	DRHORTON/WILMINGTON; LOT 18 MCKAY PLACE	
MP18	A03	COMMON	5	1		159163119
Builders FirstSource, Apex, I	٧C				Job Reference (optional) 8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Jun 26 11:55	:41 2023 Page 2
NOTES-		IC):VULCYJU7zpRLimSP	7MPFxTyf	?fT-NQRDedu51ZDhazGHUTsT_UadO3ummHGwz7L	JNSBz2QDm
11) See Standard Indu	,	on Detail for Connection to base truss e face of the truss are noted as front		sult qualif	ed building designer.	
Uniform Loads (plf)	alanced): Lumber Increase=1.7	5, Plate Increase=1.15				
2) Dead + 0.75 Roof Liv Uniform Loads (plf)	· · · ·	Attic Storage: Lumber Increase=1.15	,			
 Dead + Uninhabitabl Uniform Loads (plf) 		=-50, 36-37=-20, 37-38=-50, 27-38=-2 er Increase=1.25, Plate Increase=1.25 40/F)				
4) Dead + 0.6 C-C Win Uniform Loads (plf)		er Increase=1.60, Plate Increase=1.6	0			
Horz: 1-2=-5 5) Dead + 0.6 C-C Win Uniform Loads (plf)	59, 2-31=-37, 6-31=-26, 6-7=37 d (Pos. Internal) Case 2: Lumb	r, 7-11=26 er Increase=1.60, Plate Increase=1.6	0			
Horz: 1-2=-2	2-5=14, 5-6=25, 6-34=14, 11- 21, 2-5=-26, 5-6=-37, 6-34=26, d (Neg. Internal) Case 1: Lumb		60			
Vert: 1-2=-1 Horz: 1-2=-8	2, 2-6=-33, 6-11=-33, 24-27=-2 3, 2-6=13, 6-11=-13 d (Neg. Internal) Case 2: Lumb	20 er Increase=1.60, Plate Increase=1.6	60			
Horz: 1-2=8	8, 2-6=-33, 6-11=-33, 24-27=-2 , 2-6=13, 6-11=-13					
Uniform Loads (plf) Vert: 1-2=9,	Wind (Pos. Internal) Left: Lum 2-6=-2, 6-11=9, 24-27=-12 21, 2-6=-10, 6-11=21	ber Increase=1.60, Plate Increase=1.	60			
9) Dead + 0.6 MWFRS Uniform Loads (plf) Vert: 1-2=4,	Wind (Pos. Internal) Right: Lui 2-6=9, 6-11=-2, 24-27=-12	nber Increase=1.60, Plate Increase=	1.60			
10) Dead + 0.6 MWFR Uniform Loads (plf)		mber Increase=1.60, Plate Increase= 20	1.60			
Horz: 1-2= 11) Dead + 0.6 MWFR: Uniform Loads (plf)	-5, 2-6=-0, 6-11=11 S Wind (Neg. Internal) Right: L	umber Increase=1.60, Plate Increase	=1.60			
Horz: 1-2=	(/) Ilel: Lumber Increase=1.60, Plate Inc	rease=1.60			
Vert: 1-2= Horz: 1-2=	17, 2-32=22, 6-32=11, 6-11=3, -29, 2-32=-34, 6-32=-23, 6-11=		crease=1.60			
Horz: 1-2=	2, 2-6=3, 6-33=11, 11-33=22, -10, 2-6=-15, 6-33=23, 11-33=	34				
Uniform Loads (plf) Vert: 1-2=7	7, 2-6=11, 6-11=3, 24-27=-12 -19, 2-6=-23, 6-11=15	Illel: Lumber Increase=1.60, Plate Inc	Tease=1.00			
15) Dead + 0.6 MWFR Uniform Loads (plf) Vert: 1-2=-	S Wind (Pos. Internal) 4th Para 2, 2-6=3, 6-11=11, 24-27=-12	Illel: Lumber Increase=1.60, Plate Inc	rease=1.60			
16) Dead + 0.6 MWFR Uniform Loads (plf)		allel: Lumber Increase=1.60, Plate Inc	rease=1.60			
Horz: 1-2=	-29, 2-32=-24, 6-32=-14, 6-11= S Wind (Neg. Internal) 2nd Par		crease=1.60			
Horz: 1-2= 18) Dead + Uninhabital						
	20, 6-11=-20, 24-35=-20, 35-3	6=-60, 36-37=-20, 37-38=-60, 27-38= : Storage + 0.75(0.6 MWFRS Wind (N		Increase=	=1.60, Plate	
Uniform Loads (plf) Vert: 1-2=-		-20, 35-36=-50, 36-37=-20, 37-38=-50	0, 27-38=-20, 20-21=-	30(F)		

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MP18 A03 COMMON 5 1		RHORTON/WILMINGTON; LOT 18 MCKAY PLACE	Ply	Qty	Truss Type	Truss Type	Truss	lob	Job
	I59163119		1	5	COMMON	COMMON	403	/P18	MD
Job Reference (optional)		ob Reference (optional)	'	5		COMMON	705		

Builders FirstSource, Apex, NC

8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Jun 26 11:55:41 2023 Page 3 ID:VULCYJU7zpRLimSP7MPFxTyf?fT-NQRDedu51ZDhazGHUTsT_UadO3ummHGwz7UNSBz2QDm

LOAD CASE(S)

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf) Vert: 1-2=-38, 2-6=-42, 6-11=-50, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

Horz: 1-2=-12, 2-6=-8, 6-11=0

21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-28, 2-32=-32, 6-32=-40, 6-11=-46, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

Horz: 1-2=-22, 2-32=-18, 6-32=-10, 6-11=4

22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-43, 2-6=-46, 6-33=-40, 11-33=-32, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

Horz: 1-2=-7, 2-6=-4, 6-33=10, 11-33=18

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

- Vert: 1-6=-60, 6-11=-20, 24-27=-20
- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-20, 6-11=-60, 24-27=-20

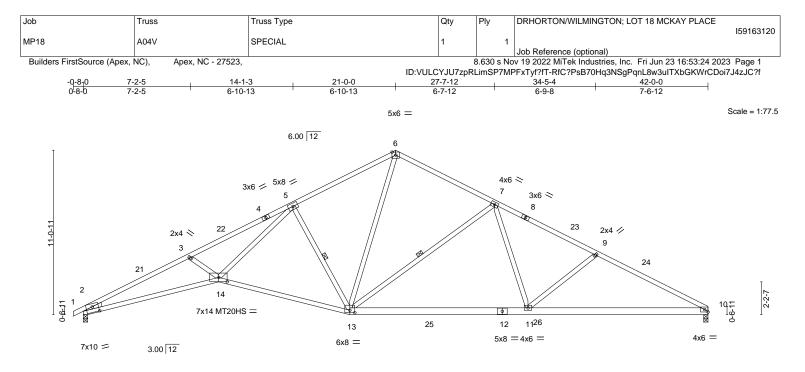
- 25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
- Vert: 1-6=-50, 6-11=-20, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)
- 26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-20, 6-11=-50, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

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Edenton, NC 27932



F	<u>9-1-4</u> 9-1-4		7-11-0 -9-12	<u>29-10-12</u> 11-11-12			42-0-0 12-1-4	
Plate Offsets (X	• • •			11-11-12			12-1-4	
LOADING (psf TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	0 Plate Grip DOL 0 Lumber DOL 0 * Rep Stress Incr	2-0-0 1.15 1.15 YES TPI2014	CSI. TC 0.93 BC 0.86 WB 0.70 Matrix-MS	Vert(LL) -0.44 Vert(CT) -0.94 Horz(CT) 0.33	n (loc) l/defl 4 11-13 >999 4 13-14 >537 5 10 n/a 3 13-14 >999	L/d 360 240 n/a 240	PLATES MT20 MT20HS Weight: 236 lb	GRIP 244/190 187/143 FT = 20%
BOT CHORD	2x4 SP SS *Except* 8-10: 2x4 SP No.2 2x6 SP No.2 *Except* 2-14: 2x4 SP SS, 13-14: 2x4 S 2x4 SP No.3 *Except* 5-14: 2x4 SP No.2	SP No.1		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood Rigid ceiling dir 1 Row at midpt	ectly applied	irectly applied. or 10-0-0 oc bracing. 5-13, 7-13	
REACTIONS.	(size) 2=0-3-8, 10=0-3-8 Max Horz 2=149(LC 16) Max Uplift 2=-24(LC 12), 10= Max Grav 2=1720(LC 1), 10=							
FORCES. (Ib) TOP CHORD BOT CHORD WEBS) - Max. Comp./Max. Ten All 1 2-3=-5379/348, 3-5=-5101/3 9-10=-3079/265 2-14=-262/4846, 13-14=-118 3-14=-264/184, 5-14=-77/28 7-11=0/671, 9-11=-410/186	27, 5-6=-2199/28 5/2602, 11-13=-5	37, 6-7=-1766/270, 7-9=- 5/2193, 10-11=-148/267	2728/239, 1				
2) Wind: ASCE gable end zo 42-0-0 zone;	roof live loads have been cons 7-10; Vult=120mph Vasd=95m one and C-C Exterior(2) -0-8-0 ; cantilever left and right expose own: Lumber DOI = 160 plate c	ph; TCDL=6.0p o 4-1-10, Interio d ; end vertical	sf; BCDL=6.0psf; h=32ft; r(1) 4-1-10 to 21-0-0, Ext	terior(2) 21-0-0 to 27-7-1	2, Interior(1) 27-7-		PRTH C	AD

reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

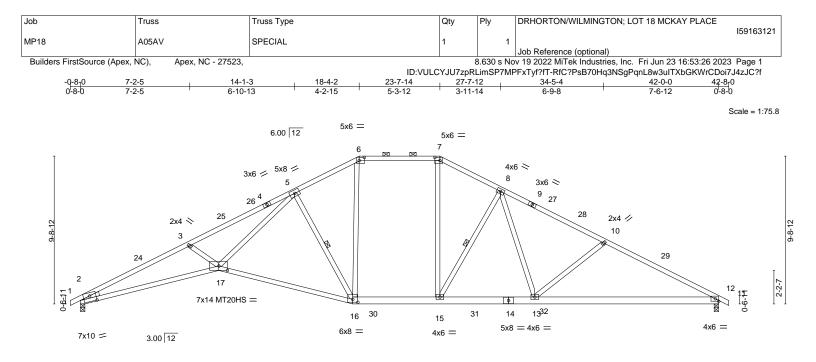
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com) ----



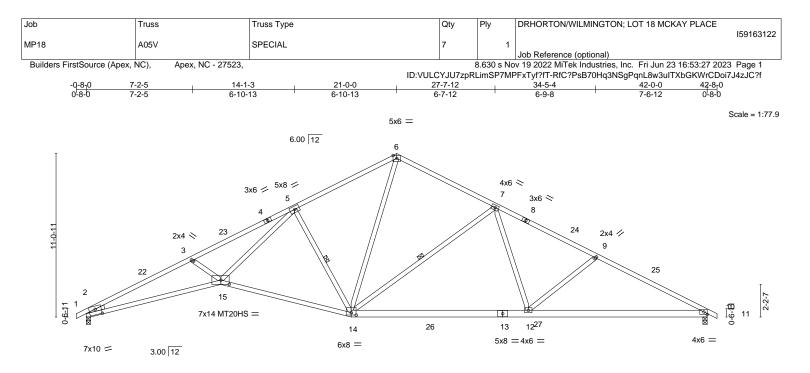


Base Offsets (XY)- 20-60-48[[60-40.0-28] [70-40.0-28] [70-40.0-28] [70-40.0-28] 16-8-14 16-214 121-4 Low Daws (ps) TCOL SPACINC- 20-00-48[[60-40.0-28] [70-40.0-28] [70-40.0-28] CSL DEFL in (fcc) likeli Lid Low Daws (ps) TCOL SPACINC- 20-00-48[[60-40.0-28] [70-40.0-28] CSL DEFL in (fcc) likeli Lid PLATES GRP MT20 Low Daws (ps) TCOL Space (ps) Lumber DO. Space (ps) Lumber DO. CSL DEC Lid Wind(LL) O.58 16.7 >45.9 24.0 Weight: 24.1 lib FT = 20% LUMBER- TOP CHORD 24.8 SP No.1*Except* Space 20 Structural wood sheathing directly applied, except 2-0.0 or putins (6-64 max); 6-7. Structural wood sheathing directly applied, except 2-0.0 or putins (6-64 max); 6-7. COT CHORD Structural wood sheathing directly applied of 10-0-0 oc tracing. Except: Structural wood sheathing directly applied of 10-0-0 oc tracing. Except: 2-17, 244 SP No.2 SPAC WEBS To 2-16 (Cl 2).11=-1760(Cl 2).11=-1700(Cl 2).11 WEBS 1-10-3808292.11 WEBS Structural wood sheathing directly applied of 10-0-0 oc tracing. Except: 10-11-3060205.5 1-12-205.5 <th< th=""><th></th><th>H</th><th>9-1-4</th><th></th><th>7-11-0</th><th>23-7-14</th><th></th><th>9-10-12</th><th></th><th></th><th>42-0-0</th><th></th></th<>		H	9-1-4		7-11-0	23-7-14		9-10-12			42-0-0	
LOADING (pdf) TCLL 20.0 TCLL 20.0 TCLL 20.0 Lumber DOL 1.15 BCLL 0.0 * SPACING- Plate Gip DOL 1.15 Rep Stress lind: YES CSI. TCL 1.00 Wer(LL) -0.58 14:17 - 8975 360 Wer(LL) -0.58 14:17 - 8999 240 PLATES MIT20K 244/190 Wreight: 2411b FT = 20% LUMBER. TOP CHORD 24 SP No.1 "Except" 14.912 244 SP SS, 6-7 244 SP No.1 5-17: 244 SP SS, 16:17: 244 SP No.2 Structural wood sheathing directly applied, except 5-17: 244 SP No.3 BRACING- TOP CHORD 24 SP No.1 "Except" 1-4.912 244 SP SS, 5-124 SP No.1 Structural wood sheathing directly applied, except 5-17: 244 SP No.3 BRACING- TOP CHORD 24 SP No.1 "Except" 5-17: 244 SP No.3 REACTIONS. (size) 2-0-3-8, 114-0-3-8 Max Horz Z-143(LC 12) Max Lipit Z-15](LC 12), 114-15(LC 13) Max Grav Z-172202(25), 174-17202(C), 114-15(LC 13) Max Grav Z-172202(25), 154-17202(25), 154-25 103/263, 5-45-130/351, 7-8-21333(19, 8-10-2709/279, 10-11-3969/209, 6-7-8187702 BOT CHORD 2-17-326/456, 164-7-40/2023(15, 7-8-2133/319, 8-10-2709/279, 10-11-3969/209, 6-7-8187702 BOT CHORD 2-17-326/456, 5-164-714/108, 7-15-5-17/24 BOT CHORD 2-17-326/456, 5-164-7-130/2220, 6-16-50/712, 8-15=802/163, 8-13-41/031, 10-13-431/168, 7-15-517/24 NOTES- 1) Unbalanced rol five loads have been considered for this design. 2) Winch ASCE - CE Xeteric(2) -0-40 to 4 this design. 2) Winch ASCE - 10-100 the toads have been considered for this design. 2) Winch ASCE - 10-100 the toads have been considered for this design. 2) Winch ASCE - 10-100 the toads have been considered for this design. 2) Winch ASCE - 10-100 the toads have been considered for this design. 2) Winch ASCE - 10-100 the toads have been considered for this design. 2) Winch ASCE - 10-100 the toads have been considered for this design. 2) Winch A	Plate Offs	sets (X,Y)						6-2-14			12-1-4	
TCLL 20.0 Plate Grip DOL 1.15 TC 1.00 Vert(L) -0.05 MT20 24/190 BCLL 0.0 Rep Stress Incr YES WB 0.71 Horz(CT) 0.16 HT20 187/143 BCLL 10.0 Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16.17 >2499 240 Weight: 241 Ib FT = 20% LUMBER- TOP CHORD 2x4 SP No.1 "Except" Top CHORD Strespt 16 2-00 op puting (34.54 26.7 20.00 op puting (34.54 26.7 27.7 26.7 26.7				_ 0], [110 1 0,								
TCDL 10.0 Lumber DOL 1.15 BC 0.95 Wert(CT) -1.10 16-17 >458 240 BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16-17 >499 240 LUMBER- TOP CHORD 2x4 SP No.1 Except BRACING- TOP CHORD Structural wood sheathing directly applied, except 2-0-0 oc purling (3-64-max); 6-7. DOT CHORD 2x4 SP No.3 Fiscept Structural wood sheathing directly applied, except 2-2-0 oc bracing; 13-15. Except Structural wood sheathing directly applied, except 2-2-0 oc bracing; 13-15. WEBS 18 w at midpt 5-16.8-15 VEDOE Left: 2x4 SP No.3 WEBS 1 Row at midpt 5-16.8-15 FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. WEBS 1 Row at midpt 5-16.8-15 FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. 10-11-3060(29), 6-7-187/302 1-13-360/202665 BOT CHORD 2-17-2264/985, 6-17-17-140/2587, 15-16-131/2262, 6-16-50/712, 8-11-13-190/2665 1-13-360/2266 1-13-360/2266 1-13-360/2266 BOT CHORD 2-17-2264/485, 6-16-1-191/657, 1-16-191/2162, 1-11-19-160/262											-	•••••
BCLL 0.0 * Rep Stress Indr YES WB 0.71 Matrix-MS Wind(LL) 0.31 16-17 >.999 240 LUMBER- TOP CHORD 2x4 SP No.1 *Except* Top CHORD TOP CHORD Structural wood sheathing directly applied, except -1.4.9-12: 2x4 SP SS, 56-7: 2x4 SP No.2 Structural wood sheathing directly applied, except 2-0:0 courlins (3-64-max), 6-7. BOT CHORD 2x4 SP No.3 *Except* BOT CHORD Structural wood sheathing directly applied or 10-0:0 or bracing, Except _2-0:0 courlins (3-64-max), 6-7. BOT CHORD Structural wood sheathing directly applied or 10-0:0 or bracing, Except _2-17: 2x4 SP SS, 516-17: 2x4 SP No.3 WEBS 1.0 ow at midpt 5-16, 8-15 WEDGE Left: 2x4 SP No.3 WEBS 1.0 ow at midpt 5-16, 8-15 VEDGE Left: 2x4 SP No.3 Structural wood sheathing directly applied or 10-0:0 or bracing, B-xcept 2-2:0 oc tor bracing, B-xcept 2-0:0 oc tracing; B-xcept _0 TO CHORD (3ze) 2-0:0 oc tracing; B-xcept 5-16, 8-15 5-16, 8-15 FORCES. (N) Nax Gorg, MAX, Tan All forces 2:0 (0):0 r less except when shown. 10:10:0 seglege, 6-7:-167/022; 2:0, 6-16:-07/12, 8-15 5-16, 8-15 DOT OHORD 2-17-2:2:0 applic, 5-11:-17:2:0:0				1.15								
BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16-17 >999 240 Weight: 241 lb FT = 20% LUMBER- TOP CHORD 2x4 SP No.1 "Except" BRACING- 14.9-12: 2x4 SP S5, 67: 2x4 SP No.2 BRACING- STUCUTAI wood sheathing directly applied, except DT CHORD 2x6 SP No.2 "Except" 2-0:0 co purifies (3:-64 max,): 6-7. DOT CHORD 5x1: 2x4 SP S5, 67: 2x4 SP No.1 "BRACING- 5-17: 2x4 SP No.2 "BES WEBS 5x4 SP No.3 "Except" 5-16, 8-15 REACTIONS. (Size) 2-0-3-8, 11=0-3-8 (Max Horz 2-134(LC 12) Max Upit 2a-15(LC 12), 11=-15(LC 13) Max Grav 2=1720(LC 1), 11=1720(LC 1) Max Grav 2=172(LC 1), 11=1720(LC 1) "Max Grav 2=170(LC 1), 11=-15(LC 13) Max Grav 2=170(LC 1), 11=1720(LC 1) PFORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 2-17-224/929, 6-718077302 BOT CHORD 2-17-224/929, 6-718077302 BOT CHORD 2-17-224/925, 5-16=-1914257, 1-15=-90/2163, 11-13=-90/2665 BOT CHORD 2-17-224/929, 6-71807742 POTEB- 1) 1) ubalanced root live loads have been considered for this design. 2) Wind: ASCE 7-10: Vult=120mph Vasd=95mph; TODL=6.0pst; BCDL=6.0pst; B-22T; 2-2. Exterior(2) 8-2-6 1-10, 1-110; 0+107(1) 4-110 (1) 1+10 (1) 4-2. 2 10 40-5.5, Interior(1) 30-5-5 (4-2-9 2one; cantilever let and right exposed; end vertical lett and right exposed;	TCDL	10.0		1.15	BC 0.95	Vert(CT)	-1.10	16-17	>458	240	MT20HS	187/143
LUMBER- TOP CHORD 2x4 SP No.1 *Except* 14.9-12: 2x4 SP No.2 *Except* BOT CHORD 2x6 SP No.2 *Except* 2x6 SP No.2 *Except* 2x7: 2x4 SP SS, 67: 2x4 SP No.1 Except: 2x4 SP No.3 *Except* STI: 2x4 SP No.3 *Except: 5-17: 2x4 SP No.3 *Except: 5-17: 2x4 SP No.2 *Except* STI: 2x4 SP No.3 *Except: 5-17: 2x4 SP No.3 *Except: 1 Row at midpt 5-16, 8-15 5-16, 8-15 5-16												
IOP CHORD 2x4 SP No.1 "Except" TOP CHORD Structural wood sheathing directly applied, except 41.4-12: 2x4 SP No.2 "Except" BOT CHORD 2x6 SP No.2 "Except" BOT CHORD Structural wood sheathing directly applied, except 2x17: 2x4 SP No.3 "Except" BOT CHORD X4 SP No.3 "Except" BOT CHORD 2x0 SP No.3 "Except" WEBS 2x17: 2x4 SP No.3 "Except" WEBS 1 Row at midpt 5-16, 8-15 VEDGE Item 2x4 SP No.3 "Except" 5-16, 8-15 5-16, 8-15 REACTIONS. (size) 2-0-38, 11=0-3-8 Max Hortz 2-134(LC 12) Max Uptilt 2-13(LC 12), 11=-13(LC 13) Max Grav 2-1720(LC 1), 11=-1720(LC 1) 5-6-270/279, 10-11-3063(296, 67-1657/302) POP CHORD 2-3-538(427, 3-5-5-1070(429, 7-82133/319, 8-10=-270/279, 10-11-3063(296, 67-1657/302) 5-17-290/178, 11-3-90/2666 90T CHORD 2-17-3224(240287, 1-5-5-6-15-90/712, 8-15=-802/163, 8-17-249/190, 5-17=-117/2875, 5-16=-1312/220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/63, 1, 10-13=-41/163, 1, 1	BCDL	10.0	Code IRC2015/TP	12014	Matrix-MS	Wind(LL)	0.31	16-17	>999	240	Weight: 241 lb	FT = 20%
IOP CHORD 2x4 SP No.1 "Except" TOP CHORD Structural wood sheathing directly applied, except 41.4-12: 2x4 SP No.2 "Except" BOT CHORD 2x6 SP No.2 "Except" BOT CHORD Rigid celling directly applied, except 2x17: 2x4 SP No.3 "Except" BOT CHORD Xes SP No.2 "Except" BOT CHORD 2x0 SP No.3 "Except" WEBS 2x17: 2x4 SP No.3 "Except" WEBS 1 Row at midpt 5-16, 8-15 VEDGE Item SP No.3 "Except" Structural wood sheathing directly applied, except VEDGE Item SP No.3 "Except" Structural wood sheathing directly applied, except VEDGE Structural wood sheathing directly applied, except Structural wood sheathing directly applied, except VEDGE Structural wood sheathing directly applied, except Structural wood sheathing directly applied, except VEDGE Structural wood sheathing directly applied, or 10-0 oc b racing, Except Structural wood sheathing directly applied, except VEDGE Structural wood sheathing directly applied, except Structural wood sheathing directly applied, except VEDGE Structural wood sheathing directly applied, except Structural wood sheathing directly applied, except Structural wood sheathing directly applied, except Structural wood sheathing Strucural wood sheathing d	LUMBER	-				BRACING	-					
BOT CHORD 2x6 SP No.2 "Except" BOT CHORD Rigid ceiling directly applied or 10-0-0 to bracing. Except: 2-24 Oc bracing: 13-15. WEBS 2x4 SP No.3 "Except" WEBS 1 Row at midpt 5-16, 8-15 WEDGE Left: 2x4 SP No.3 Status SP No.2 Status SP No.2 Status SP No.2 WEDGE Left: 2x4 SP No.3 Reactions. (size) 2-0-3-8, 11=0-3-8 Max Horz 2-134(LC 12), 11=-15(LC 13) Max Grav 2-1720(LC 1), 11=1720(LC 1) Status SP No.2 FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 2-36-5388/427, 3-55-5102/409, 5-6e-2190/315, 7-8=213/319, 8-10=-2709/279, 10-11-306/298, 5-7-41877/302 Status SP No.2 BOT CHORD 2.17-2:326/4856, 16-17=-140/2567, 13-15=-86/21483, 11-13=-190/2665 8-13=-41/631, 10-13=-431/186, 7-15=-51/7241 Status SP No.2 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Vimid: ASCEr 7-10; Vulit=120mph Vaad=95mph; TCDL=6, 0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterio(2) -0-8-0 to 4-1-10, Interior(1) 4-1-10 to 18-4-2; Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and nght exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=-1.60 SEAL 0) Trois das dave be provent water pronding. 4) All plates are MT20 plates unless otherwise indicated. SEAL SEAL O36322 SEAL O366322			No.1 *Except*					Structu	Iral wood s	heathing	directly applied, except	
 2-17: 224 SP SS, 16-17: 224 SP No.1 2-2-0 co bracing: 13-16. 5-17: 224 SP No.3 Fixept¹ WEDS 1 Row at midpt 5-16, 8-15 WEDGE Left: 224 SP No.3 REACTIONS (size) 2-0-3-8, 11=-0-3-8 Max Horz 2=134(LC 12) Max Optim 2-15(LC 12), 11=-15(LC 13) Max Grav 2=1720(LC 1), 11=-1720(LC 1) FORCES . (b): Max: Comp.Max. Ten - All forces 520 (b) or less except when shown. TOP CHORD 2-3a=5388/427, 3-5a=5102409, 5-8a=2130/316, 7-8a=2133/319, 8-10a=2709/279, 10-11=-3069/299, 6-7a-1857/302 BOT CHORD 2-17: 226/4856, 16-17a-140/2567, 15-16a=19/1857, 13-15a=86/2183, 11-13a=190/2665 WEBS 3-17: 2269/109, 5-17a=-117/2875, 5-16a-13/12220, 6-16a=-50/712, 8-15a=802/163, 8-13a=41/631, 10-13a=431/186, 7-15a=-51/741 NOTES 1) Uhbalanced rool live loads have been considered for this design. 2) Wind: ASCE 7-10; Vull=120mph Vasd=45mph; TCDL=6, 0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end core and C-C Exterior(2) -0-40 to 14-10, Interior(1) 41-1-10 to 18-42. Exterior(2) 15-4-2 10 30-5-5 to 42-6-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 D 16-1.60 3) Provide adequate drainage for a 10.0 plate grip DOL=-1.60 3) Provide adequate drainage for a 10.0 plate grip DOL=-1.60 3) Provide adequate drainage for a 10 plate grip DOL=-1.60 3) Provide adequate drainage for a 10 plate grip DOL=-1.60 3) Provide adequate drainage for a 10 plate grip DOL=-1.60 3) Provide adequate drainage for a 10 plate grip DOL=-1.60 3) Provide adequate drainage for a 10 plate grip DOL=-1.60 3) Provide adequate drainage for a 10 plate grip DOL=-1.60 3) Provide adequate drainage for a 10 plate grip DOL=-1.60 3) Provide adequate drainage for a 10 plate agrip drain drain areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the b				P No.2								
WEBS 1 Row at midpt 5-16, 8-15 VEDGE 5-17: 2x4 SP No.3 5-16, 8-15 REACTIONS (size) 2=0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Upint 2=-15(LC 12), 11=-15(LC 13) Max Grav 2=172QUC 1), 11=172QUC 1) 5-16, 8-15 FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. 5-16 TOP CHORD 2-3=5388/427, 3-5=-5102/409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279, 10-11=-3006/2929, 6-7-1877/302 5-16 BOT CHORD 2-17=-326/4856, 16-17=-14/02587, 15-16=-19/1857, 13-15=-68/2183, 11-13=-190/2665 5-17 WEBS 3-17=-269/1490, 5-17=-11/2276, 5-16=-131/2220, 6-16==-50/712, 8-15=-60/7163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 5-16 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) 1) Unbalanced in on file vegosed; end vertical 181 and right exposed; C-C for members and forces & MWFRS (envelope) gable end zone and C-C Exterior(2): 0-8-0 to 4-1:10, Interior(1): 41-10 to 18-4.2; Exterior(2): 16-4-2: to 30-5.5; Interior(1): 30-5: to 28-02 cone; cantilever lend on ghi exposed; end vertical 181 and right exposed; C-C for members and forces. WFRS (or velope) gable end zone and C-C Exterior(2): 0-8-0 to 4-1:10, Interior(1): 41-10, Interior(1): 41-1	BOT CHC	ORD 2x6 SF	No.2 *Except*			BOT CHO	RD	Rigid c	eiling direc	tly applied	d or 10-0-0 oc bracing, E	Except:
 5-17: 2x4 SP No.2 WEDGE Left: 2x4 SP No.3 REACTIONS. (size) 2=0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Upit 2=-15(LC 12), 11=-15(LC 13) Max Grav 2=1720(LC 1), 11=1720(LC 1) FORCES. (lb) - Max Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-36-584427, 3-5-610210409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279, 10-11=-3069/299, 6-7=-1857/302 BOT CHORD 2-1732644856, 16-17=-140/2587, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665 WEBS 3-1726914910, 5-17=-117/2875, 5-16=-131/2220, 6-16=-50/712, 8-15=-80/2163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- Unbalanced roof live loads have been considered for this design. Uhrait: ASCE 7-10; Vul=120mph Vasd=95mph; TCDL=6.0psf; h=32f; Cat. II; Exp B: Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-60 to 4-1-10, Interior(1) 4-1-10 to 18-4-2; Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to reactions shown; Lumer DOL=-1.60 pat bottom chord ingl exposed; C-C for members and forces & MWFRS for reactions shown; Lumer DOL=-1.60 pat bottom chord ingl areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. S This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord ine grain poll-e1.00 J Rovide adequate drainage to pravent water ponding. J Rovide adequate drainage to raive load of 10 to 10 ada 2, Extensio(2) -0.0 psf bottom chord ine all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord ine all and shows in mortand. Building designer should verify capacity of bearing auride. Provide mechanical connection toby others) of truss to bearing plate capable of withstanding 100 b uplift at joint(s) 2, 11. See Standard Industry Pigyback Truss Connection Detail for Connection to base truss		2-17: 2x4 SP SS, 16-17: 2x4 SP No.1										
WEDGE Left: 2x4 SP No.3 REACTIONS. (size) 2=0-3-8, 11=0-3-8 Max Horz. 2=134(LC 12) Max Uplit 2=-15(LC 12), 11=-170(LC 13) Max Grav 2=1720(LC 1), 11=1720(LC 1) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD. 2-3=-5388/427, 3-5=-5102/409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279, 10-11=-3069/299, 6-7=-1857/302 BOT CHORD. 2-17=-2264/4856, 16-17=-140/2587, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665 WEBS 3-17=-269/190, 5-17=-117/2875, 5-16=-131/2220, 6-16=-507/12, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- 1) Uhbalanced roof live loads have been considered for this design. 1) Uhbalanced roof live loads have been considered for this design. 2) Wint: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; B2CDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exteric(2) -0-0 to 4-1-10. Interior(1) 4-1-10 to 18-4-2; Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 toget sin DDL=1.60 3) Provide adequate drainage to prevent water ponding. SEAL 4) All plates are MT20 plates unless otherwise indicated. SEAL 5) This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL= 10.0.psf. 1) Bearing at joint(s)	WEBS	2x4 SF	No.3 *Except*			WEBS		1 Row	at midpt		5-16, 8-15	
Left: 2x4 SP No.3 REACTIONS (size) 2=0-34, 11=0-3-8 Max Wolt 2=13(LD 12) Max Ugitt 2=15(LD 12), 11=-15(LC 13) Max Grav 2=1720(LC 1), 11=1720(LC 1) FORCES . (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 2=36-5384/27, 35=6-102/409, 5-6-2130/315, 7-8=-2133/319, 8-10=-2709/279, 10-11=-3069/299, 6-7=-1867/302 BOT CHORD 2-17=-3264/599, 6-71400/2587, 15-16=-13/1252, 0.6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0p5; h=32t; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0.6-10 et-11.0 (Interior(1) 4-1.1 to 10 18-4-2, Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone: cantilever left and right exposed; c-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 pit grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) All plates are MT20 plates unless otherwise indicated. 5) This truss has been designed for a livo [od of 20.0ps on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will flib telvemen the obtain chord and ny other members, with BCDL = 10.0pst. 6) Provide adequate drainage to prevent water ponding. 4) All plates are MT20 plates unless otherwise indicated. 5) This truss has been designed for a livo [od of 20.0ps on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will flib telvemen the obtain chord and any other members, with BCDL = 10.0pst. 7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11. 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer		5-17: 2	x4 SP No.2									
 REACTIONS. (size) 2=0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Uplitt 2=-15(LC 12), 11=-15(LC 13) Max Grav 2=1720(LC 1), 11=-15(LC 13) Max Grav 2=1720(LC 1), 11=-1720(LC 1) FORCES. (b) - Max. Comp./Max. Ten - All forces 250 (b) or less except when shown. TOP CHORD 2-3-5388/427, 35=-5102/409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279, 10-11=-3069/299, 6-71857/302 BOT CHORD 2-17=-326/4866, 18-17=-140/2567, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665 WEBS 3-17=-269/190, 5-17=-117/2875, 5-16=-1312220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 41-10 to 18-4-2, Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 Bids grip DOL=1.60 Provide adequate drainage to prevent water ponding. 4) Al plates unless otherwise indicated. 5) This truss has been designed for a 10.0 psf bottom chord in ela areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord of a 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will tim between the bottom chord of no all stress where a rectangle 3-6-0 tall by 2-0-0 wide will tim between the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will tim between the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will tim between the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will tim between the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will tim between the bottom chord in all stress where a rectangle 3-6-0 tall by 2-0-0	WEDGE											
Max Horz 2=134(LC 12) Max Uplit 2=-15(LC 12), 11=-1720(LC 1) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-5388/427, 3-5=-5102/409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279, 10-11=-3069/299, 6-7=-1857/302 BOT CHORD 2-17=-326/4856, 16-177-140/2587, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665 WEBS 3-17=-269/190, 5-17=-117/2875, 5-16=-1312/220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- 1) Unbalenced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; b=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 4-1-10 to 18-4-2, Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and right exposed; c-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) All plates are MT20 plates unless otherwise indicated. 5) This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Bearing at joint(s) 2 considers parallel to grain value using ANS/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11. 9 See Standard Industry Pigyback Truss Connection beast furs as applicable, or consult qualified building designer.	Left: 2x4 \$	SP No.3										
Max Horz 2=134(LC 12) Max Uplit 2=-15(LC 12), 11=-1720(LC 1) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-5388/427, 3-5=-5102/409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279, 10-11=-3069/299, 6-7=-1857/302 BOT CHORD 2-17=-326/4856, 16-177-140/2587, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665 WEBS 3-17=-269/190, 5-17=-117/2875, 5-16=-1312/220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- 1) Unbalenced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; b=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 4-1-10 to 18-4-2, Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and right exposed; c-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) All plates are MT20 plates unless otherwise indicated. 5) This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Bearing at joint(s) 2 considers parallel to grain value using ANS/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11. 9 See Standard Industry Pigyback Truss Connection beast furs as applicable, or consult qualified building designer.												
Max Uplift 2=-16/LC 12), 11=-16/LC 13) Max Grav 2=1720(LC 1), 11=1720(LC 1) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-5388/427, 35-=5102/409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279, 10-11=-3068/299, 6-7=-1857/302 BOT CHORD 2-17=-326/4856, 16-17=-140/2587, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665 WEBS 3-17=-269/190, 5-17=-117/2875, 5-16=-131/220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 41-10 to 18-4-2, Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 7 areactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) All plates are MT20 plates unless otherwise indicated. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) *This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11. 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.	REACTIO	· · ·										
Max Grav 2=1720(LC 1), 11=1720(LC 1) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-5388/427, 3-5=-5102/409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279, 10:11=-3069/299, 6-77=-1857/302 BOT CHORD 2:17=-326/4856, 16-17=-140/2587, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665 WEBS 3:17=-269/190, 5-17=-117/2875, 5-16=-1312/220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0:8-0 to 4-1:0, Interior(1) 41-10 to 18-4-2; Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=-1.60 plate grip DOL=-1.60 3) Provide adequate drainage to prevent water ponding. 4) All plates are MT20 plates unless otherwise indicated. 5) This truss has been designed for a 10.0 pst bottom chord ine lal areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11. 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.			(/	(1.0.40)								
 FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-5388/427, 3-5=-5102/409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279, 10-11=-3069/299, 6-7=-1857/302 BOT CHORD 2-17=-326/4856, 16-17=-140/2587, 15-16=-191/1857, 13-15=-86/2183, 11-13=-190/2665 WEES 3-17=-269/190, 5-17=-117/2875, 5-16=-1312/220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 41-10 to 18-4-2, Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) All plates are MT20 plates unless otherwise indicated. 5) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Bearing at join(s) 2 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 8) Provide mechanical connection (by others) of truss to beard ing plate capable of withstanding 100 lb uplift at joint(s) 2, 11. 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer. 4) All plates under the dustry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer. 6) Frovide mechanical connection (by others) of truss to bearing plate capable												
 TOP CHORD 2-3=-5388/427, 3-5=-5102/409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279, 10-11=-3069/299, 6-7=-1857/302 BOT CHORD 2-17=-326/4856, 16-17=-140/2587, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665 WEBS 3-17=-269/190, 5-17=-117/2875, 5-16=-1312/220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 4-1-10 to 18-4-2, Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=-1.60 3) Provide adequate drainage to prevent water ponding. 4) All plates are MT20 plates unless otherwise indicated. 5) This truss has been designed for a 10.0 pst bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0pst. 7) Bearing at joint(5) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11. 9) See Standard Industry Piggyback Truss Connection to base truss as applicable, or consult qualified building designer. 4) All plates unemented industry Piggyback Truss Connection to base truss as applicable, or consult qualified building designer. 		wax e	rav = 1720(LC I), II = 17.	20(LC I)								
 TOP CHORD 2-3=-5388/427, 3-5=5102/409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279, 10-11=-3069/299, 6-7=-1857/302 BOT CHORD 2-17=-326/4856, 16-17=-140/2587, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665 WEBS 3-17=-269/190, 5-17=-117/2875, 5-16=-1312/220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 4-1-10 to 18-4-2, Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=-1.60 Provide adequate drainage to prevent water ponding. All plates are MT20 plates unless otherwise indicated. This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11. See Standard Industry Pigyback Truss Connection to base truss as applicable, or consult qualified building designer. 	FORCES	(lb) Max	Comp (Max Top All for	oc 250 (lb) or	loss avaant whan showr							
 10-11=-3069/299, 6-7=-1857/302 BOT CHORD 2-17=-326/4856, 16-17=-140/2587, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665 WEBS 3:17=-526/190, 5-17=-117/2876, 5-16=-1312/220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- Uhbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vull=120mph Vasd=95mph; TCDL=6.0psf; b=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 4-1-10 to 18-4-2, Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; end vertical left and right exposed; c-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Provide adequate drainage to prevent water ponding. All plates are MT20 plates unless otherwise indicated. This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11. See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer. 												
 BOT CHORD 2-17=-326/4856, 16-17=-140/2587, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665 WEBS 3-17=-269/190, 5-17=-117/2875, 5-16=-1312/220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0.8-0 to 4-1-10, Interior(1) 4-1-10 to 18-4-2, Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and right exposed; c-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 Provide adequate drainage to prevent water ponding. All plates are MT20 plates unless otherwise indicated. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to bearing plate capable of bwithstanding 100 lb uplift at joint(s) 2, 11. See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer. 	TOF CITC				15, 7-0=-2155/519, 0-10=	-2109/219,						
 WEBS 3-17=-269/190, 5-17=-117/2875, 5-16=-1312/220, 6-16=-50/712, 8-15=-802/163, 8-13=-41/631, 10-13=-431/186, 7-15=-51/741 NOTES- Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0.9 to 41-10. Interior(1) +1-10 to 18-4-2; Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to 42-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Provide adequate drainage to prevent water ponding. All plates are MT20 plates unless otherwise indicated. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11. See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer. 					0/1857 13-1586/2183	11-13-100/2665						
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Schut Information, purplication component of component development properties. and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



June 26,2023

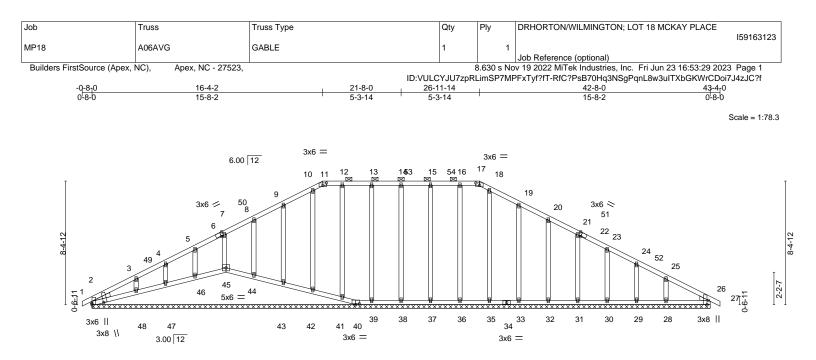


	9-1-4	17-11-0	29-10-12		42-0-0	
Plate Offsets (X,Y)	<u>9-1-4</u> [2:0-5-0,0-4-8], [14:0-4-0,0-2-0],	8-9-12 [15:0-6-12.0-3-8]	11-11-12		12-1-4	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0- Plate Grip DOL 1.1 Lumber DOL 1.1 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. 5 TC 0.93 5 BC 0.86 S WB 0.70	Vert(LL) -0.43 1 Vert(CT) -0.93 1	4-15 >542 240 10 n/a n/a	PLATES MT20 MT20HS Weight: 237 lb	GRIP 244/190 187/143 FT = 20%
WEBS 2x4 S			BOT CHORD F	Structural wood sheathing Rigid ceiling directly applie Row at midpt		
Max I Max I	ze) 2=0-3-8, 10=0-3-8 Horz 2=145(LC 16) Jplift 2=-24(LC 12), 10=-24(LC 1 Grav 2=1720(LC 1), 10=1720(LC					
TOP CHORD 2-3= 9-10 BOT CHORD 2-15 WEBS 3-15	. Comp./Max. Ten All forces 25 5378/342, 3-5=-5100/318, 5-6=- i=-3077/255 i=-228/4845, 14-15=-93/2601, 12 i=-264/185, 5-15=-69/2861, 5-14= i=0/671, 9-12=-412/186	2199/287, 6-7=-1766/268, 7-9=- -14=-47/2191, 10-12=-122/2670	2726/231,			
 Wind: ASCE 7-10; ' gable end zone and 42-8-0 zone; cantile reactions shown; Lu All plates are MT20 	e loads have been considered fo Vult=120mph Vasd=95mph; TCD d C-C Exterior(2) -0-8-0 to 4-1-10 ever left and right exposed ; end v umber DOL=1.60 plate grip DOL= plates unless otherwise indicate	L=6.0psf; BCDL=6.0psf; h=32ft; , Interior(1) 4-1-10 to 21-0-0, Ext vertical left and right exposed;C-0 =1.60 d.	erior(2) 21-0-0 to 27-7-12, I	nterior(1) 27-7-12 to	UPTH C	AROLIN

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5) will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.
 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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[2:0-1-8,0-9-8], [2:0-0-14,Edge], [6:0-1-13,Edge], [11:0-3-0,0-2-0], [17:0-3-0,0-2-0], [22:0-1-13,Edge], [26:0-3-8,Edge], [40:0-3-0,0-0-12]

DEFL

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

in (loc)

27

27

26

0.00

0.00

0.01



43-4_r0

0-8-0

GRIP

244/190

FT = 20%

PLATES

Weight: 275 lb

MT20

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

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818 Soundside Road Edenton, NC 27932

42-8-0

24-1-0

L/d

120

120

n/a

2-0-0 oc purlins (6-0-0 max.): 11-17

Rigid ceiling directly applied or 10-0-0 oc bracing.

l/defl

n/r

n/r

n/a

30, 29, 28 Max Grav

All bearings 42-0-0. Max Horz 2=123(LC 12)

2x4 SP No.2

2x4 SP No.3

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

-0<u>-8-</u>0 0-8-0

20.0

10.0

10.0

TOP CHORD 2x4 SP No.2

(lb) -

0.0

Plate Offsets (X,Y)-

LOADING (psf)

TCLL

TCDL

BCLL

BCDL

LUMBER-

OTHERS

WEDGE

BOT CHORD

REACTIONS.

All reactions 250 lb or less at joint(s) 45, 40, 2, 26, 38, 39, 41, 42, 43, 44, 46, 47, 48, 37, 36 35, 33, 32, 31, 30, 29, 28

Max Uplift All uplift 100 lb or less at joint(s) 45, 40, 2, 38, 39, 41, 43, 44, 46, 47, 48, 37, 36, 33, 32, 31,

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

9-9-4 9-1-4

SPACING-

Plate Grip DOL

Rep Stress Incr

Code IRC2015/TPI2014

Lumber DOL

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 4-1-10 to 15-8-2, Exterior(2) 15-8-2 to 22-5-9, Interior(1) 22-5-9 to 26-3-14, Exterior(2) 26-3-14 to 33-0-0, Interior(1) 33-0-0 to 42-8-0 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

18-7-0

8-9-12

CSI.

0.09

0.07

0.15

тс

BC

WB

Matrix-S

2-0-0

1.15

1.15

YES

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

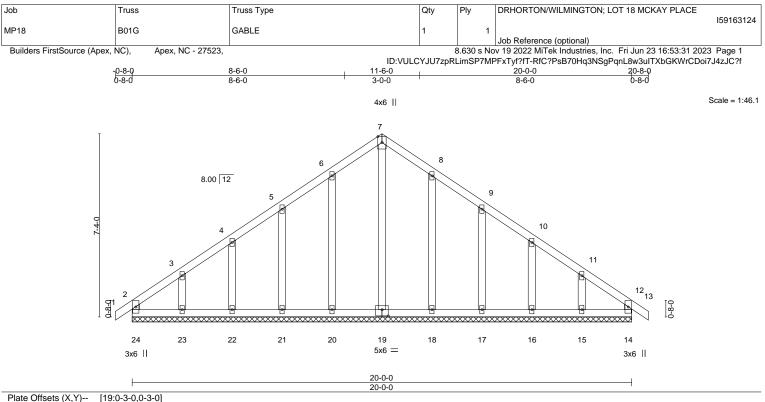
7) Gable studs spaced at 2-0-0 oc.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 9) will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 45, 40, 2, 38, 39, 41, 43, 44, 46, 47, 48, 37, 36, 33, 32, 31, 30, 29, 28,

11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2015/TPI2014	CSI. TC 0.06 BC 0.05 WB 0.13 Matrix-R	Vert(CT) -	in 0.00 0.00 0.00	(loc) 12 12 14	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 120 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP	No.2		BRACING- TOP CHORD) S	Structu	ral wood	sheathing di	rectly applied or 6-0-0 o	oc purlins,

LOWIDEN-		DIVACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3		

REACTIONS. All bearings 20-0-0.

(lb) - Max Horz 24=167(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 24, 14, 20, 21, 22, 23, 18, 17, 16, 15 Max Grav All reactions 250 lb or less at joint(s) 24, 14, 19, 20, 21, 22, 23, 18, 17, 16, 15

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

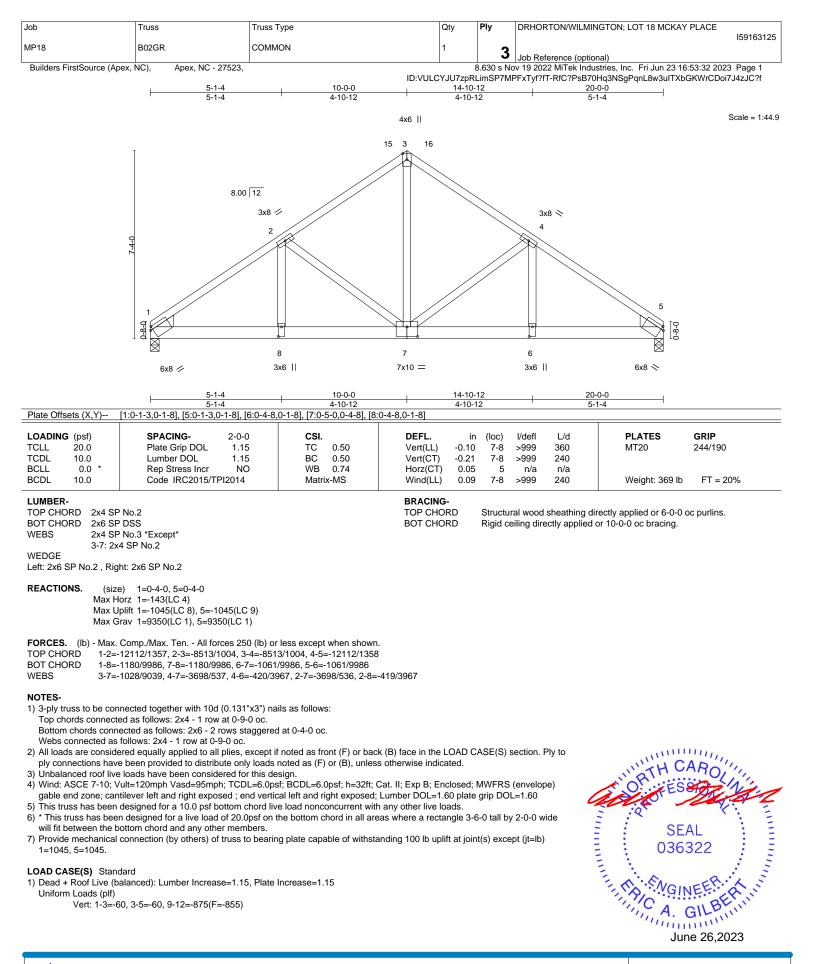
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-0 to 4-0-0, Exterior(2) 4-0-0 to 10-0-0, Corner(3) 10-0-0 to 14-9-10, Exterior(2) 14-9-10 to 20-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 14, 20, 21, 22, 23, 18, 17, 16, 15.



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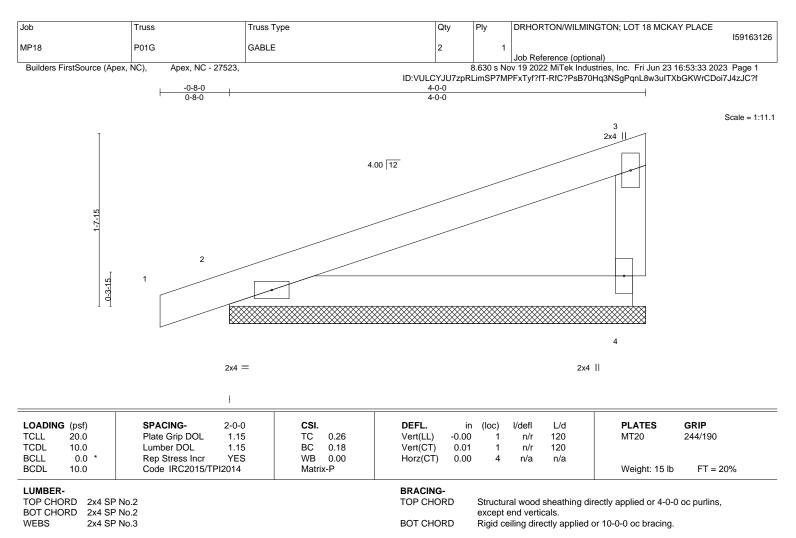
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A MiTek Affi



REACTIONS. (size) 2=4-0-0, 4=4-0-0 Max Horz 2=52(LC 9) Max Uplift 2=-39(LC 8), 4=-21(LC 12) Max Grav 2=198(LC 1), 4=151(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

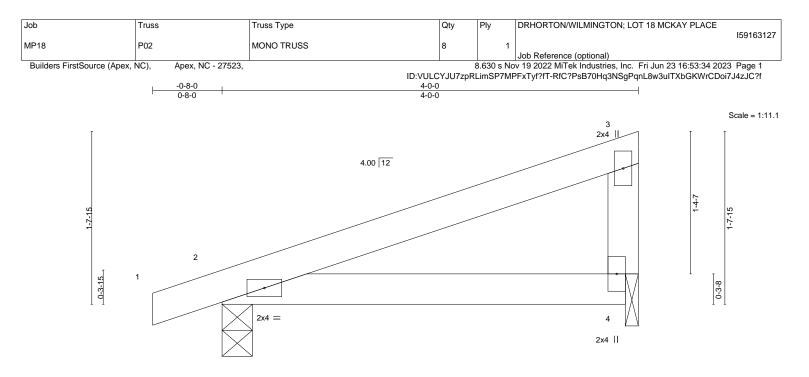
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



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4-0-0 4-0-0								
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.19	DEFL. Vert(LL) -0.0	in (loc) 1 4-7	l/defl >999	L/d 360	PLATES GRIP MT20 244/190	
TCDL 10.0	Lumber DOL 1.15	BC 0.17	Vert(CT) -0.0	2 4-7	>999	240	M120 244/190	
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-MP	Horz(CT) 0.0 Wind(LL) 0.0		n/a >999	n/a 240	Weight: 15 lb FT = 20%	

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=52(LC 11) Max Uplift 2=-39(LC 8), 4=-21(LC 12) Max Grav 2=198(LC 1), 4=151(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



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