

RE: J0823-4709 Weaver/Lot 21 West Preserve/Harnett Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Lot/Block:	Project Name:	J0823-4709	
Address:			
City:			

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 29 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	156272870	A1	1/23/2023	21	156272890	G2	1/23/2023
2	156272871	A1GE	1/23/2023	22	156272891	VB1	1/23/2023
3	156272872	A10L A2	1/23/2023	22	156272892	VB1 VB2	1/23/2023
4	156272873	A3	1/23/2023	23	156272893	VC1	1/23/2023
4 5	156272874	A3 A4	1/23/2023	24 25	156272894	VC1 VC2	1/23/2023
-				-		-	
6	156272875	A5	1/23/2023	26	156272895	VC3	1/23/2023
7	156272876	A6	1/23/2023	27	156272896	VC4	1/23/2023
8	156272877	A7	1/23/2023	28	156272897	VC5	1/23/2023
9	156272878	A7GE	1/23/2023	29	156272898	VC6	1/23/2023
10	156272879	B1	1/23/2023				
11	156272880	B1GE	1/23/2023				
12	156272881	B2	1/23/2023				
13	156272882	B3	1/23/2023				
14	156272883	B4	1/23/2023				
15	156272884	C1	1/23/2023				
16	156272885	C1GE	1/23/2023				
17	156272886	C2	1/23/2023				
18	156272887	C3	1/23/2023				
19	156272888	G1	1/23/2023				
20	156272889	G1GE	1/23/2023				

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

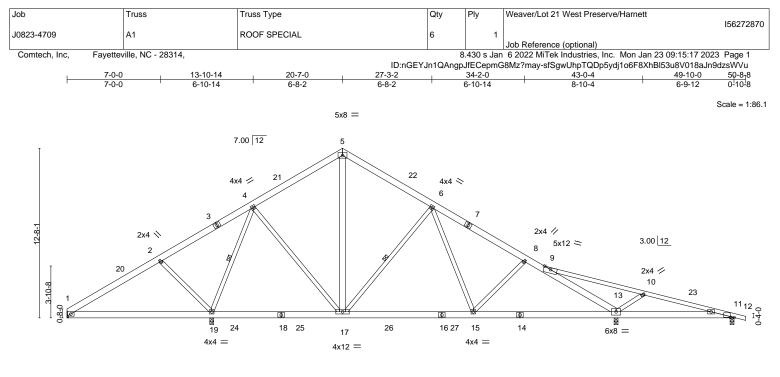
My license renewal date for the state of North Carolina is December 31, 2023

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

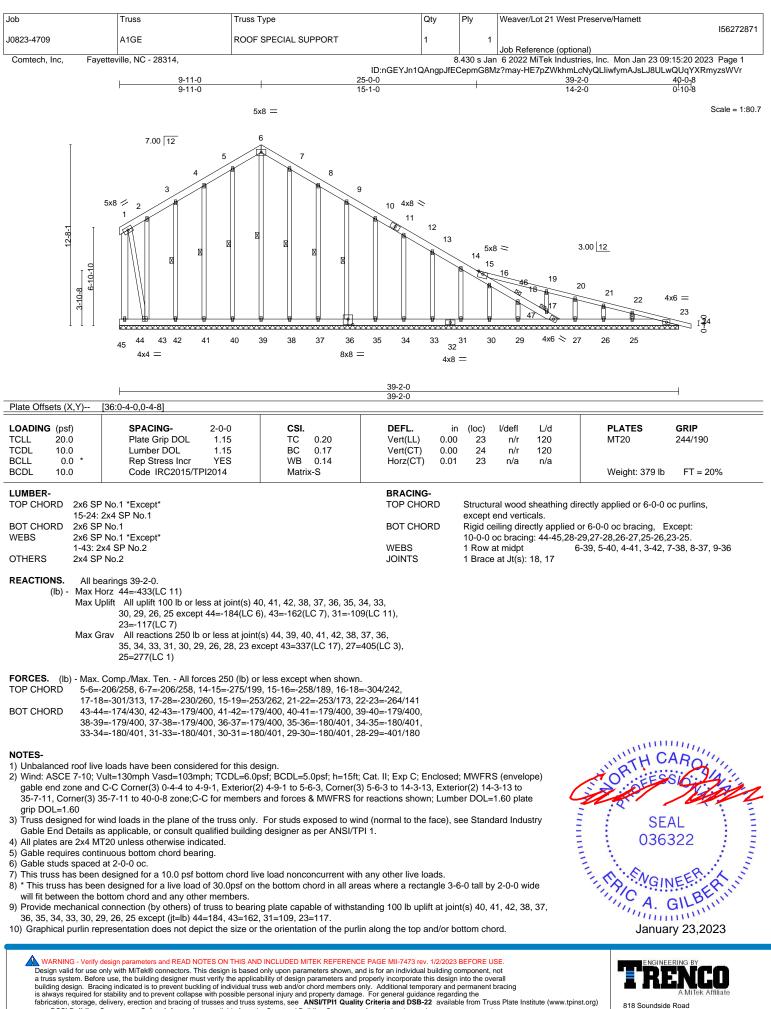


L	10-9-12		20-7-0	30-4-4			41-0-4	49-1	
	10-9-12		9-9-4	9-9-4	1		10-8-0	8-9-	12
Plate Offsets (X,Y)	[11:0-3-4,0-0-3]			1					
LOADING(psf)TCLL20.0TCDL10.0BCLL0.0BCDL10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES I2014	CSI. TC 0.86 BC 0.41 WB 0.51 Matrix-S	Vert(LL) -0.11 Vert(CT) -0.17 Horz(CT) 0.02	(loc) 15-17 15-17 13 13 15-17	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 354 lb	GRIP 244/190 FT = 20%
9-12: 2 BOT CHORD 2x6 SF WEBS 2x4 SF	P No.1 *Except* x4 SP No.1 P No.1 P No.2 *Except* x6 SP No.1			BRACING- TOP CHORD BOT CHORD WEBS	Rigid o		ectly applied	lirectly applied or 6-0-0 c l or 6-0-0 oc bracing. 4-19, 6-17	oc purlins.
(Ib) - Max H Max U	earings 0-3-8 except (jt=ler lorz 1=-297(LC 6) plift All uplift 100 lb or les rav All reactions 250 lb o 22)	s at joint(s) 1,	11 except 13=-189(LC 1		0(LC 17)), 11=279	(LC		
TOP CHORD 2-4=- 9-13: BOT CHORD 17-19 WEBS 2-19:	Comp./Max. Ten All ford -53/432, 4-5=-879/341, 5-6 2019/488, 9-10=-136/53; 9=0/367, 15-17=-27/1101, 484/260, 4-19=-1463/35; 3=-608/280, 6-15=-23/481	=-851/342, 6- 7 13-15=-157/1:	8=-1514/361, 8-9=-1585, 388	/359,					
 NOTES- Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 16-2-3, Exterior(2) 16-2-3 to 24-11-13, Interior(1) 24-11-13 to 46-3-11, Exterior(2) 46-3-11 to 50-8-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 All plates are 4x6 MT20 unless otherwise indicated. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. Refer to girder(s) for truss to truss connections. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11 except 									

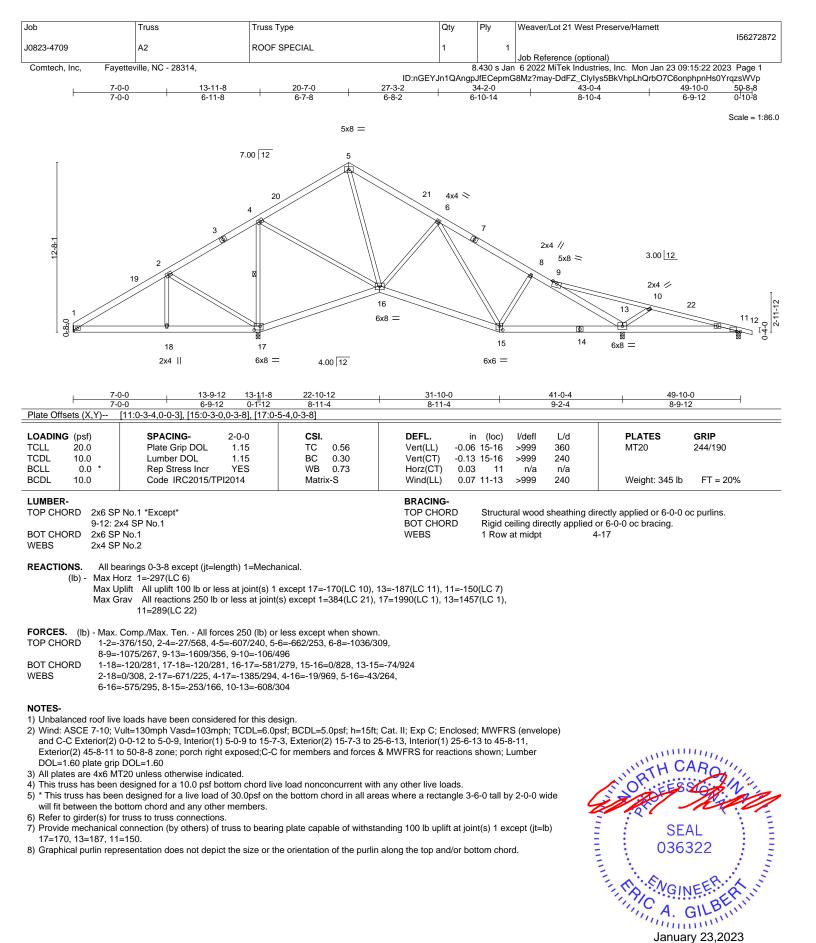
(jt=lb) 13=189, 19=186. 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



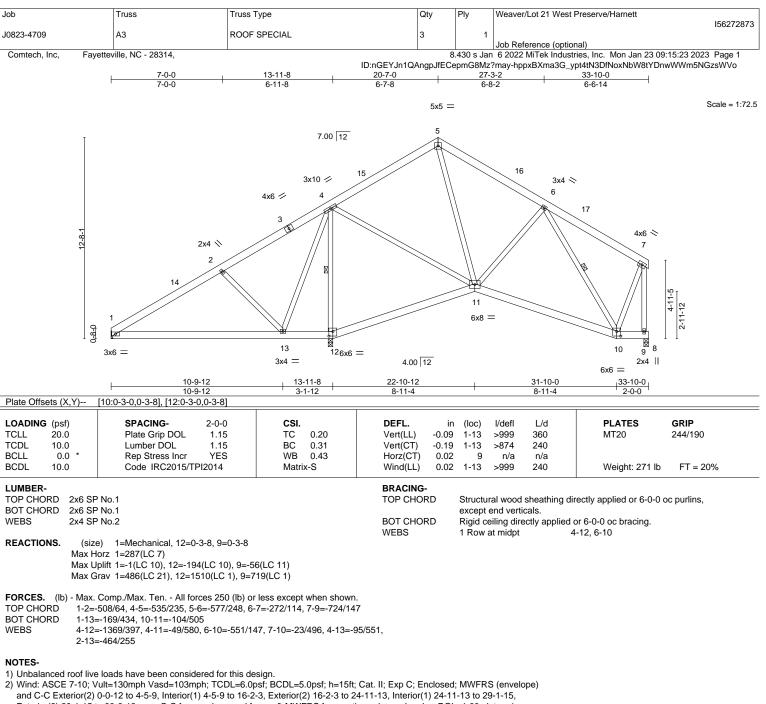
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



A MiTek Afi 818 Soundside Road



Exterior(2) 29-1-15 to 33-6-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

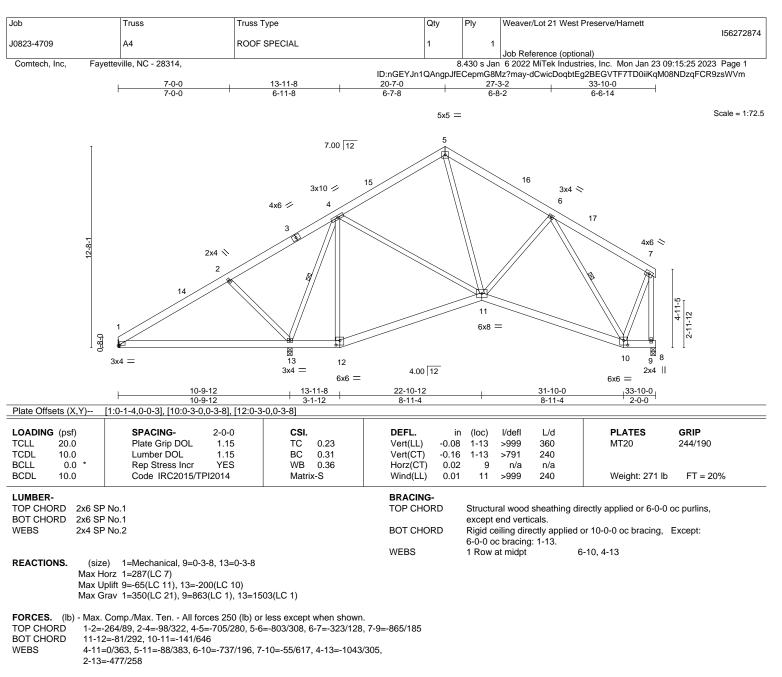
5) Refer to girder(s) for truss to truss connections

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9 except (jt=lb) 12=194.



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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 16-2-3, Exterior(2) 16-2-3 to 24-11-13, Interior(1) 24-11-13 to 29-1-15, Exterior(2) 29-1-15 to 33-6-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

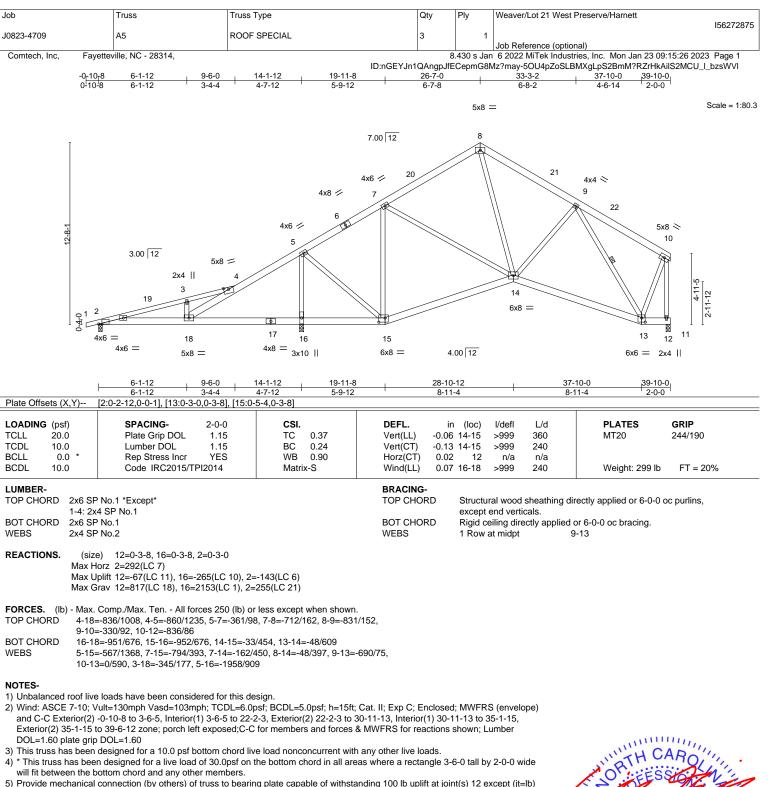
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 13=200.



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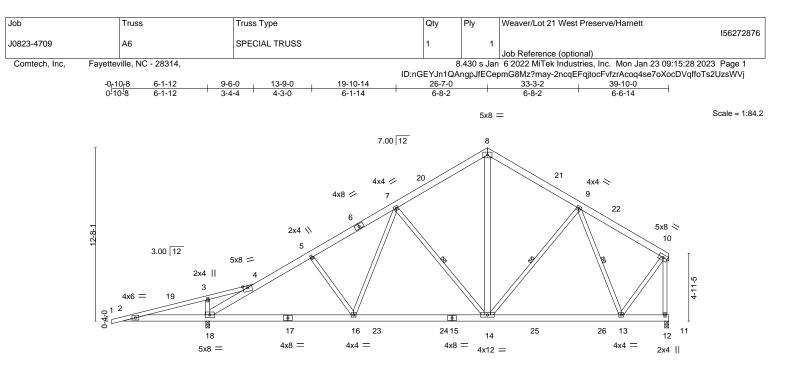


5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 16=265, 2=143.

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	6-1-12 6-1-12	16-9-12 10-8-0	<u>+ 19-10-14</u> <u>+ 26-7-0</u> <u>- 3-1-2</u> <u>6-8-2</u>	33-3-2 6-8-2	<u>36-4-4</u> <u>39-10-0</u> <u>3-1-2</u> <u>3-5-12</u>
DADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.62	Vert(LL) -0.13 14-16	>999 360	MT20 244/190
CDL 10.0	Lumber DOL 1.15	BC 0.47	Vert(CT) -0.20 14-16	>999 240	
CLL 0.0 *	Rep Stress Incr YES	WB 0.37	Horz(CT) 0.03 12	n/a n/a	
CDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) -0.05 16-18	>999 240	Weight: 307 lb FT = 20%

LUMBER-		BRACING-		
TOP CHORD	2x6 SP No.1 *Except*	TOP CHORD	Structural wood sheathing	directly applied or 5-3-4 oc purlins,
	1-4: 2x4 SP No.1		except end verticals.	
BOT CHORD	2x6 SP No.1	BOT CHORD	Rigid ceiling directly applie	d or 10-0-0 oc bracing, Except:
WEBS	2x4 SP No.2 *Except*		6-0-0 oc bracing: 2-18.	
	8-14: 2x6 SP No.1	WEBS	1 Row at midpt	7-14, 9-14, 9-13
			-	

REACTIONS. (size) 12=0-3-8, 18=0-3-8 Max Horz 18=292(LC 7) Max Uplift 12=-84(LC 11), 18=-199(LC 10) Max Grav 12=1444(LC 17), 18=1936(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-982/991, 3-4=-924/958, 4-18=-2613/1034, 4-5=-2011/179, 5-7=-1879/219, TOP CHORD

- 7-8=-1233/368, 8-9=-1263/366, 9-10=-837/187, 10-12=-1490/264
- BOT CHORD 2-18=-920/1004, 16-18=-261/1814, 14-16=-174/1529, 13-14=-150/935 WEBS 7-14=-755/209, 8-14=-169/852, 10-13=-124/1195, 7-16=-16/504, 9-13=-698/207, 3-18=-344/175

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 22-2-3, Exterior(2) 22-2-3 to 30-11-13, Interior(1) 30-11-13 to 35-1-15, Exterior(2) 35-1-15 to 39-6-12 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

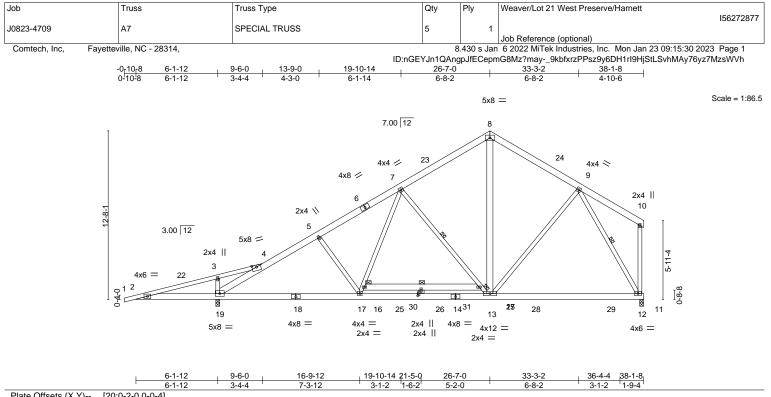
* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 18 = 199

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Flate Offsets (A, f)	[20.0-2-0,0-0-4]				
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. ir	n (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.65	Vert(LL) -0.25	5 12-13 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.61	Vert(CT) -0.37	7 12-13 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.58	Horz(CT) 0.04	4 12 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) -0.05	5 17-19 >999 240	Weight: 314 lb FT = 20%
LUMBER-			BRACING-		
TOP CHORD 2x6 SP	No.1 *Except*		TOP CHORD	Structural wood sheathing d	irectly applied or 5-0-4 oc purlins,
1-4: 2x4	1 SP No.1			except end verticals.	
BOT CHORD 2x6 SP	No.1		BOT CHORD	Rigid ceiling directly applied	or 10-0-0 oc bracing, Except:

	1-4. 224 01 110.1		except enu verticais.	
BOT CHORD	2x6 SP No.1	BOT CHORD	Rigid ceiling directly a	applied or 10-0-0 o
WEBS	2x4 SP No.2 *Except*		6-0-0 oc bracing: 2-19	Э.
	8-13,20-21: 2x6 SP No.1		6-0-0 oc bracing: 15-	16
		WEBS	1 Row at midpt	7-15, 9-12
REACTIONS.	(size) 12=0-3-8, 19=0-3-8			
	Max Horz 19=291(LC 7)			

Max Uplift 12=-40(LC 10), 19=-145(LC 10) Max Grav 12=1591(LC 17), 19=1965(LC 2)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3-989/988, 3-4=-931/955, 4-19=-2783/922, 4-5=-2188/61, 5-7=-2056/100, 7-8=-1270/284, 8-9=-1297/280

 BOT CHORD
 2-19=-916/1010, 17-19=-196/1959, 13-17=-97/1754, 12-13=-109/739

 WEBS
 7-15=-848/184, 13-15=-1063/135, 8-13=-78/880, 9-13=-9/545, 16-17=-26/430,

7-16=0/661, 9-12=-1462/232, 3-19=-343/175

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 22-2-3, Exterior(2) 22-2-3 to 30-11-13, Interior(1) 30-11-13 to 33-3-2, Exterior(2) 33-3-2 to 37-10-4 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

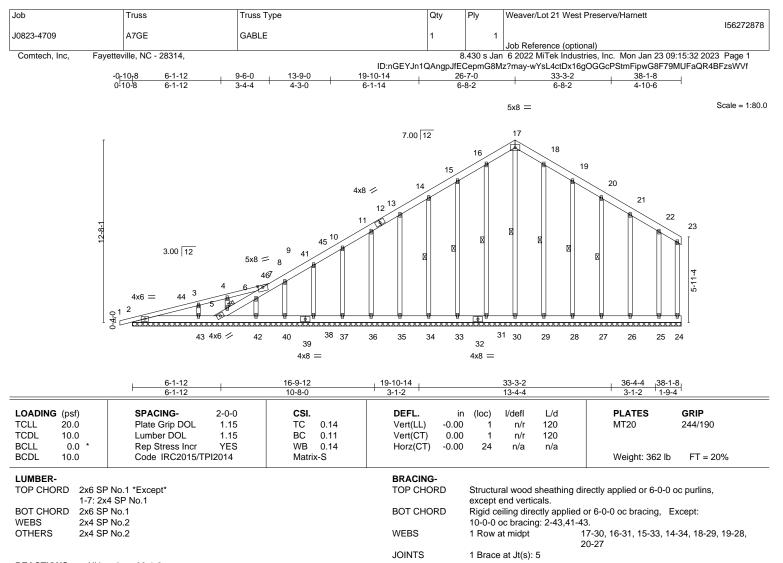
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 19=145.

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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REACTIONS. All bearings 38-1-8.

(Ib) - Max Horz 2=406(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 24, 30, 31, 33, 34, 35, 36, 37, 38, 40, 43, 29, 28, 27, 26, 25

except 2=-137(LC 6), 42=-113(LC 6) Max Grav All reactions 250 lb or less at joint(s) 2, 24, 41, 30, 31, 33, 34, 35, 36, 37, 38, 40, 42, 29, 28, 27, 26, 25 except 43=398(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-257/190, 7-8=-327/280, 8-9=-301/283, 9-10=-264/256, 14-15=-182/272,

15-16=-221/313, 16-17=-250/325, 17-18=-250/312, 18-19=-221/274

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 22-2-3, Corner(3) 22-2-3 to 30-11-13, Exterior(2) 30-11-13 to 33-5-7, Corner(3) 33-5-7 to 37-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 polate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

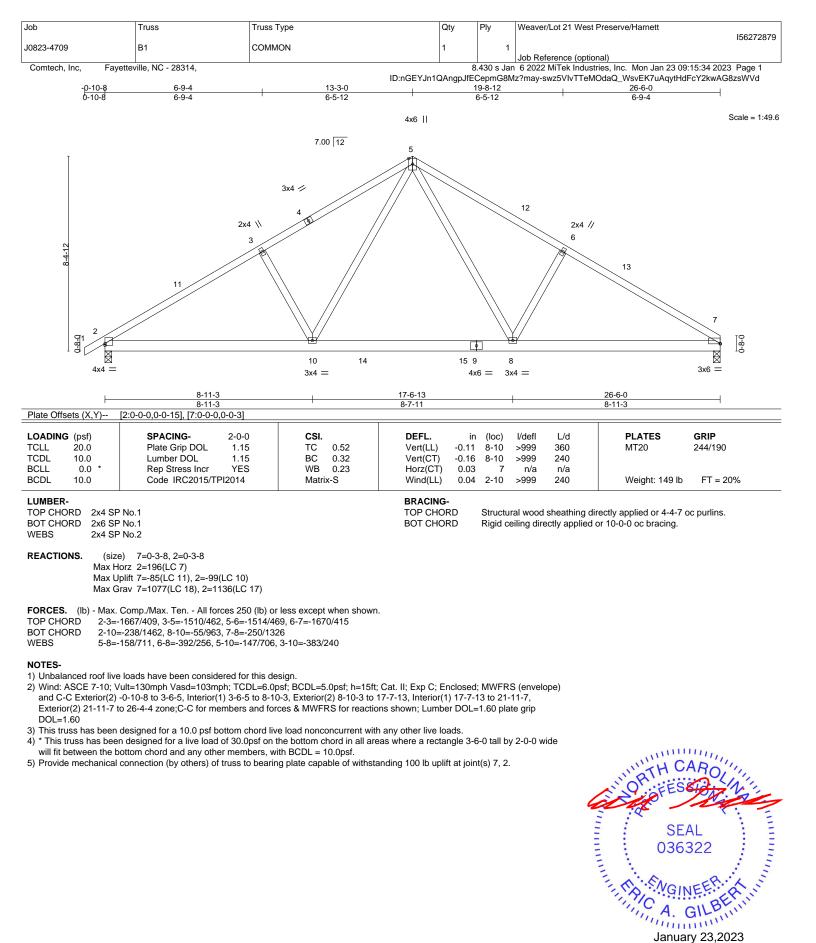
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 30, 31, 33, 34, 35, 36, 37, 38, 40, 43, 29, 28, 27, 26, 25 except (jt=lb) 2=137, 42=113.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



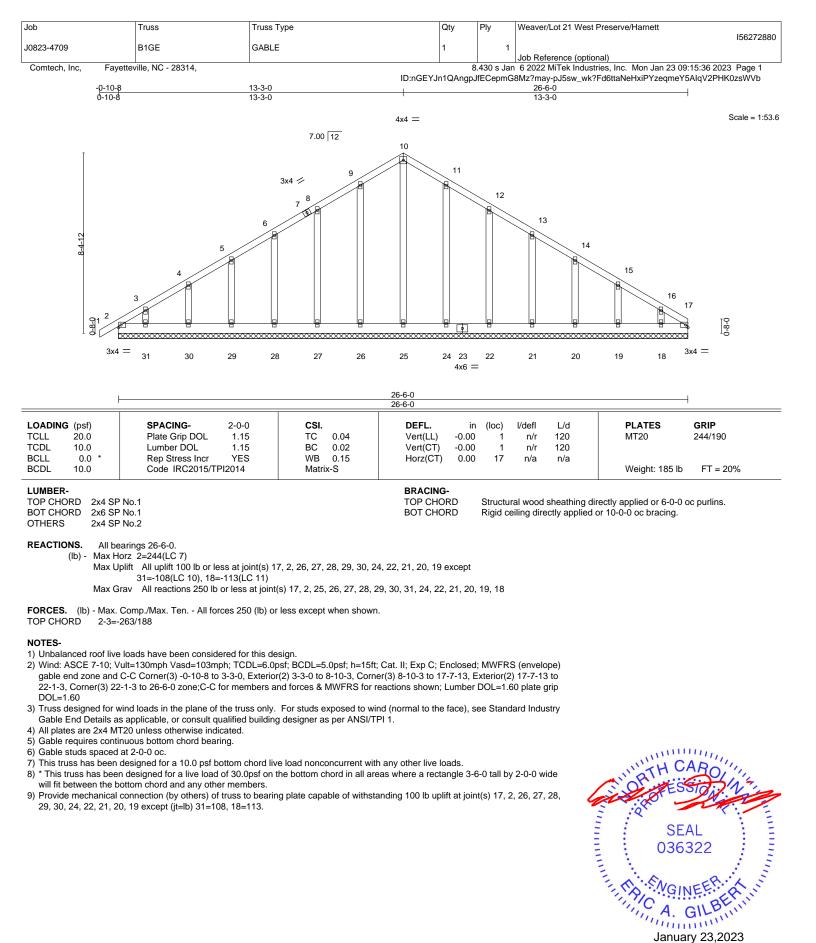
A MITEK ATSIIIate

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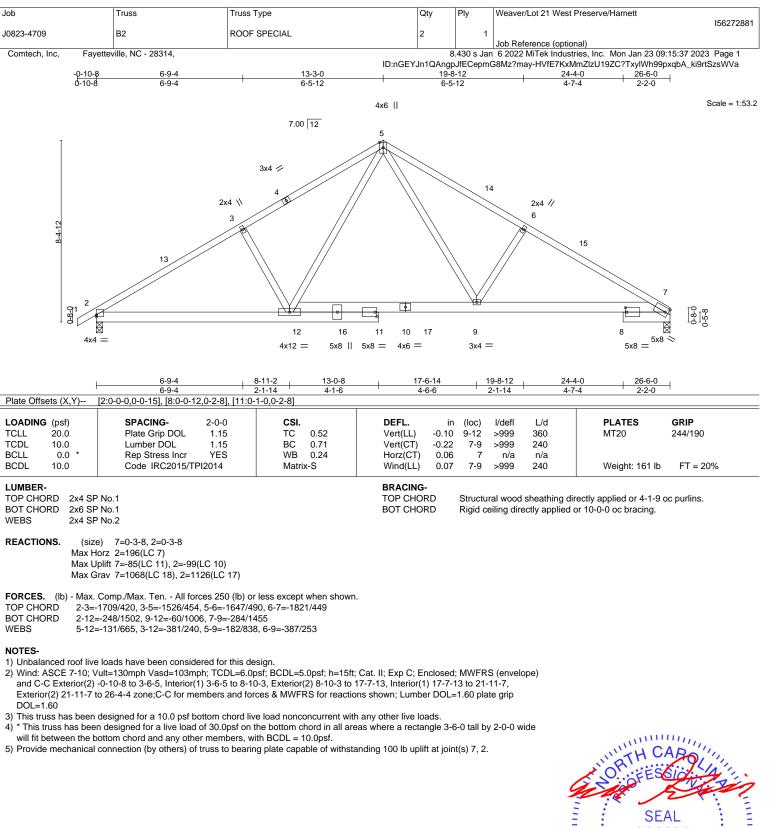


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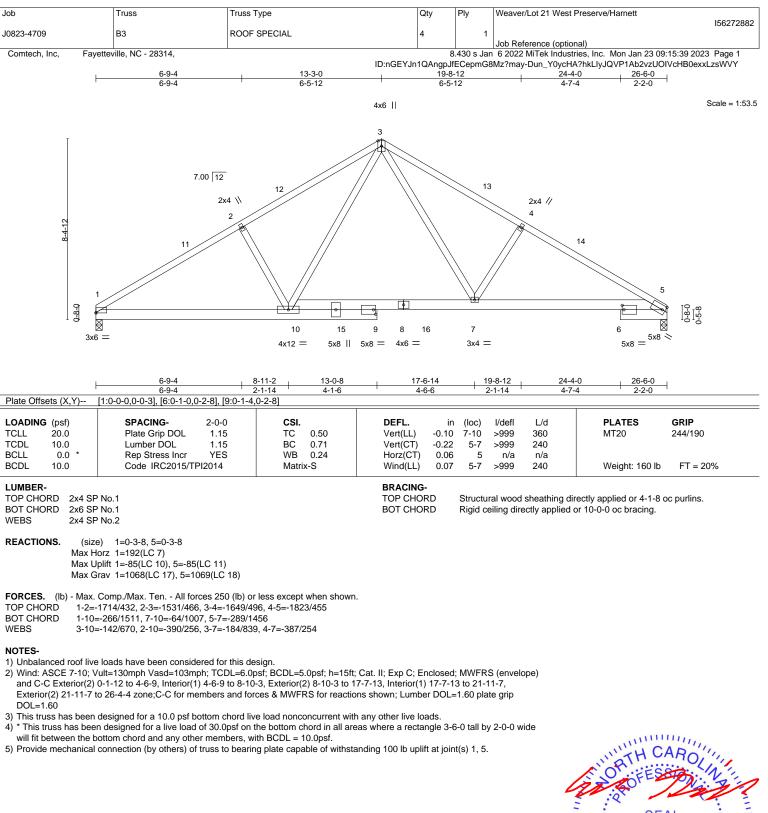
TRENCO AMIak Affiliate





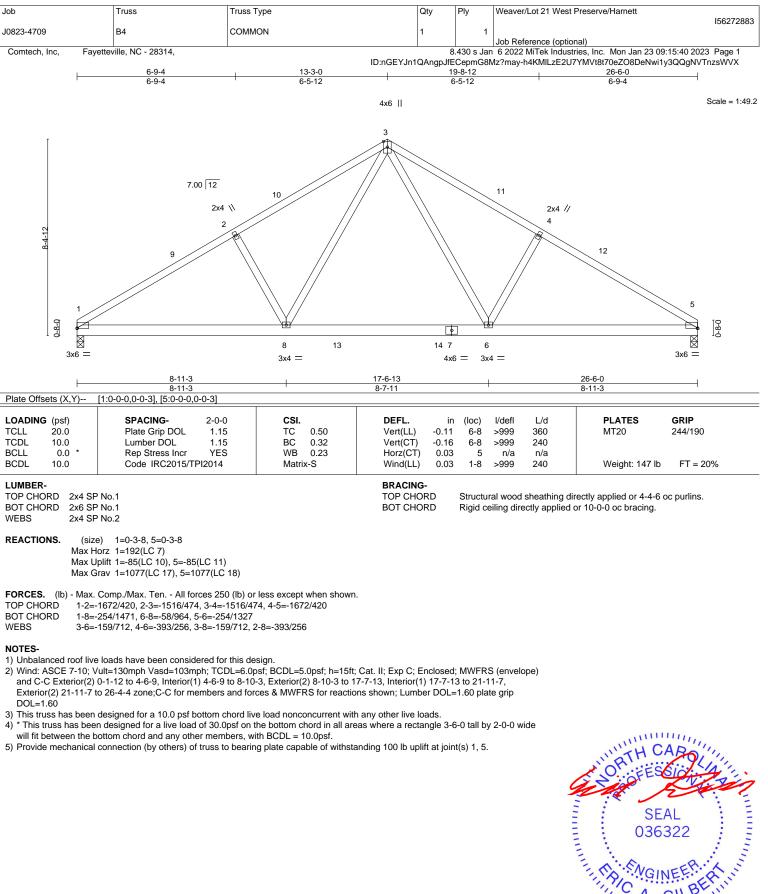


818 Soundside Road

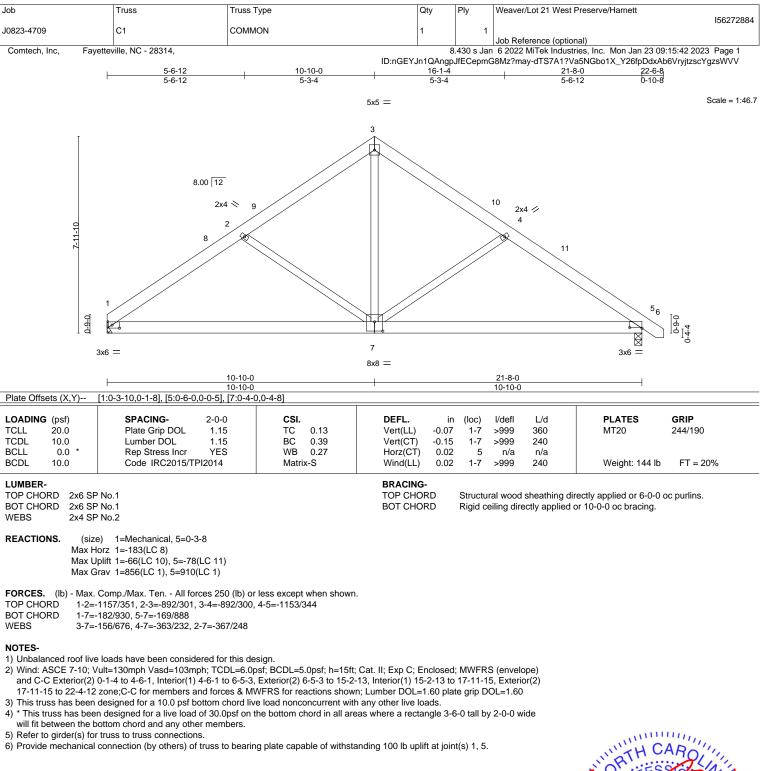




A MiTek Affilia 818 Soundside Road



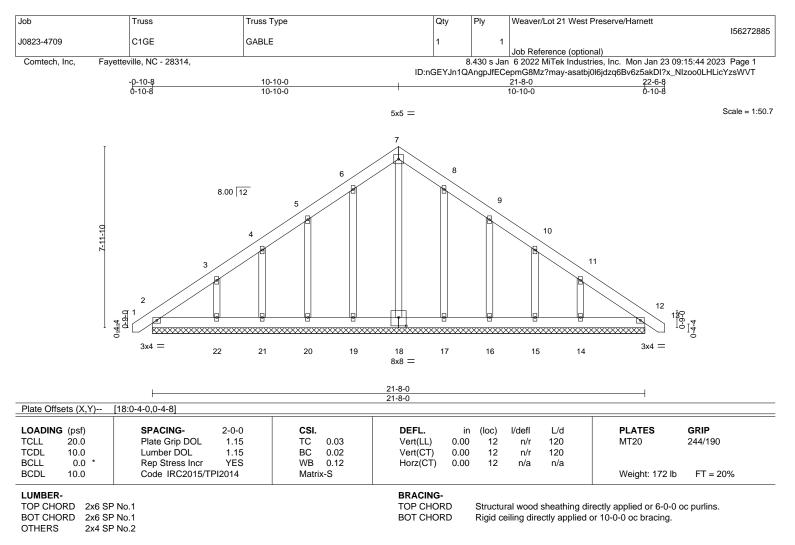






TRENCE

⁸¹⁸ Soundside Road Edenton, NC 27932



REACTIONS. All bearings 21-8-0.

(lb) - Max Horz 2=231(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 21, 17, 15 except 20=-101(LC 10), 22=-146(LC 10), 16=-103(LC 11), 14=-143(LC 11)

Max Grav All reactions 250 lb or less at joint(s) 2, 12, 18, 19, 20, 21, 17, 16, 15 except 22=255(LC 17), 14=251(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-12 to 3-8-1, Exterior(2) 3-8-1 to 6-5-3, Corner(3) 6-5-3 to 15-2-13, Exterior(2) 15-2-13 to 17-11-15, Corner(3) 17-11-15 to 22-4-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

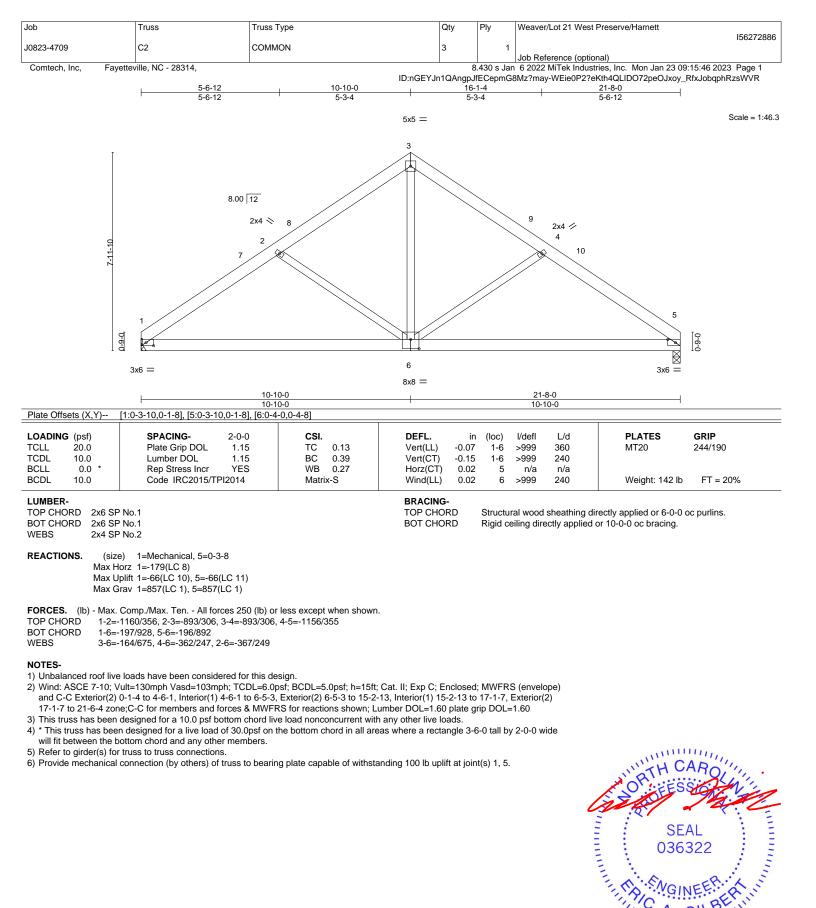
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 21, 17, 15 except (jt=lb) 20=101, 22=146, 16=103, 14=143.



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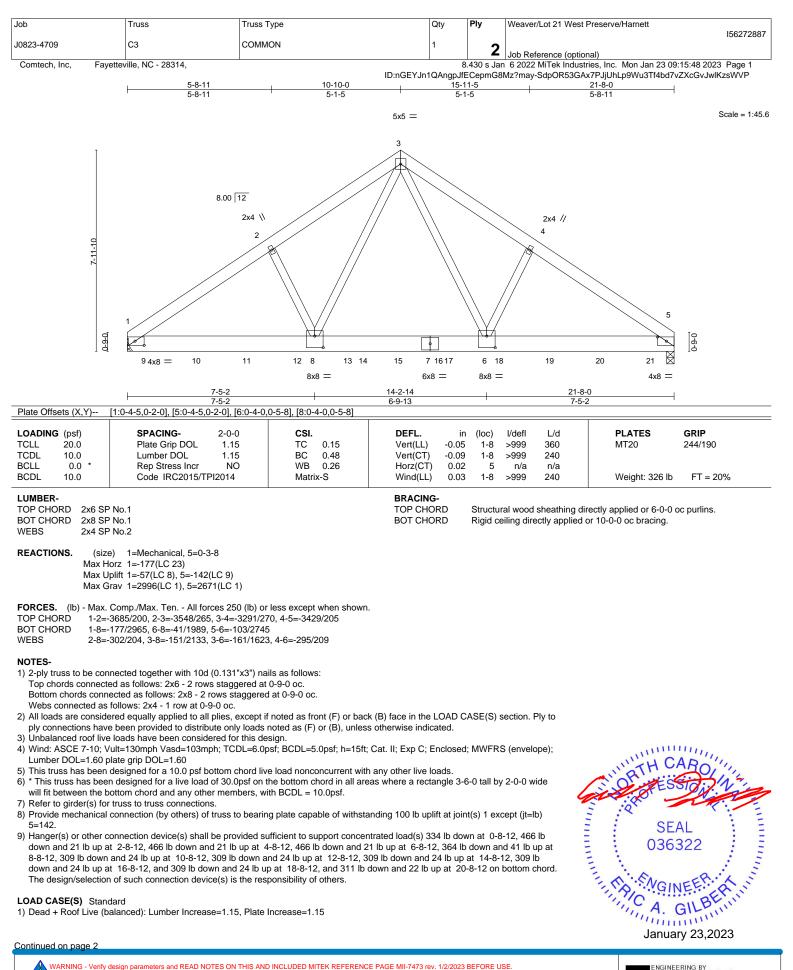




818 Soundside Road

Edenton, NC 27932

A. GILP



Design valid for use only with MiTek® connectors. This design is based only upon parameters have been and the connectors. This design is based only upon parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent pracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TFI1 Quality Criería and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSB Building Component Safety Information** available from the Structural Building Component Addet sociation (www.sbcacomponents.com)

Γ	Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 21 West Preserve/Harnett
						15627288
	J0823-4709	C3	COMMON	1	2	
					_	Job Reference (optional)
	Comtech, Inc, Fayettev	ille, NC - 28314,		8	.430 s Jan	6 2022 MiTek Industries, Inc. Mon Jan 23 09:15:48 2023 Page 2
			ID:nGEYJn ²	1QAngpJf	ECepmG8	Mz?may-SdpOR53GAx7PJjUhLp9Wu3Tf4bd7vZXcGvJwlKzsWVP

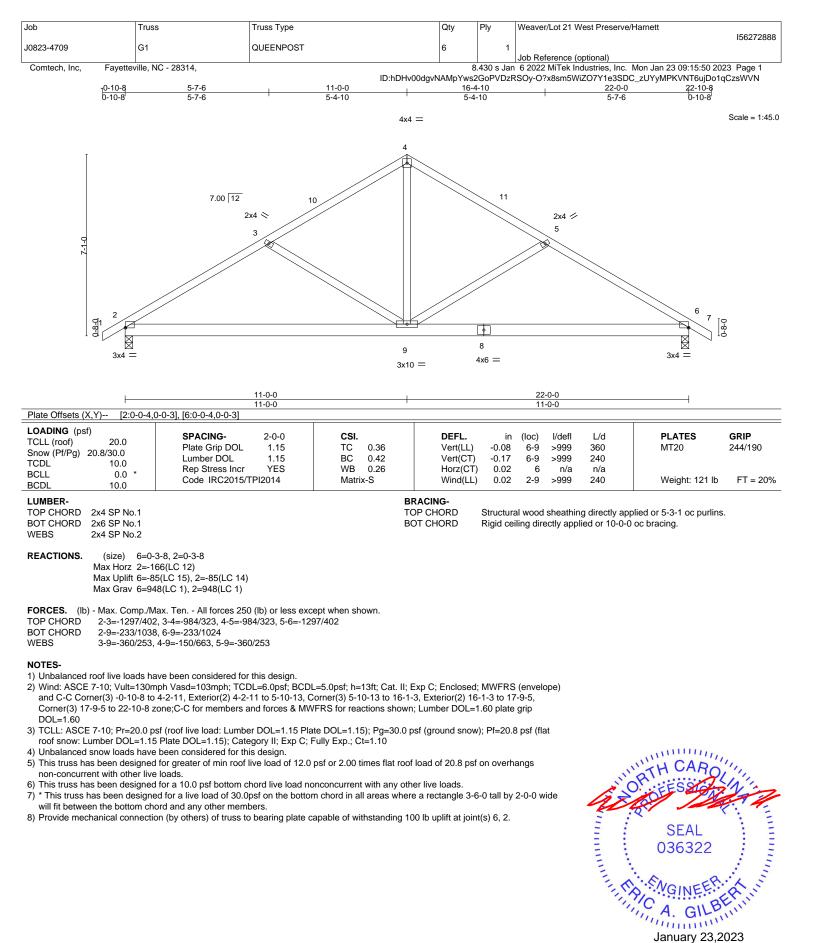
LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-5=-20, 1-3=-60, 3-5=-60 Concentrated Loads (lb)

Vert: 9=-334(F) 10=-466(F) 11=-466(F) 12=-466(F) 13=-364(F) 15=-309(F) 17=-309(F) 18=-309(F) 19=-309(F) 20=-309(F) 21=-311(F)

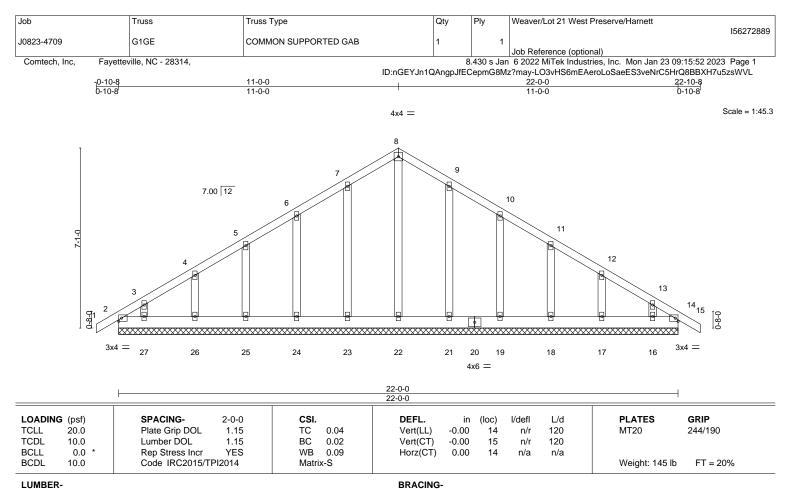
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)







818 Soundside Road



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No 1 BOT CHORD 2x6 SP No.1

OTHERS 2x4 SP No.2

REACTIONS. All bearings 22-0-0.

Max Horz 2=-208(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 23, 24, 25, 26, 21, 19, 18, 17, 16, 14 except 27=-101(LC 10) Max Grav All reactions 250 lb or less at joint(s) 2, 22, 23, 24, 25, 26, 27, 21, 19, 18, 17, 16, 14

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 6-7-3, Corner(3) 6-7-3 to 15-4-13, Exterior(2) 15-4-13 to 18-5-11, Corner(3) 18-5-11 to 22-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 8) will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 23, 24, 25, 26, 21, 19, 18, 17, 16, 14 except (jt=lb) 27=101.



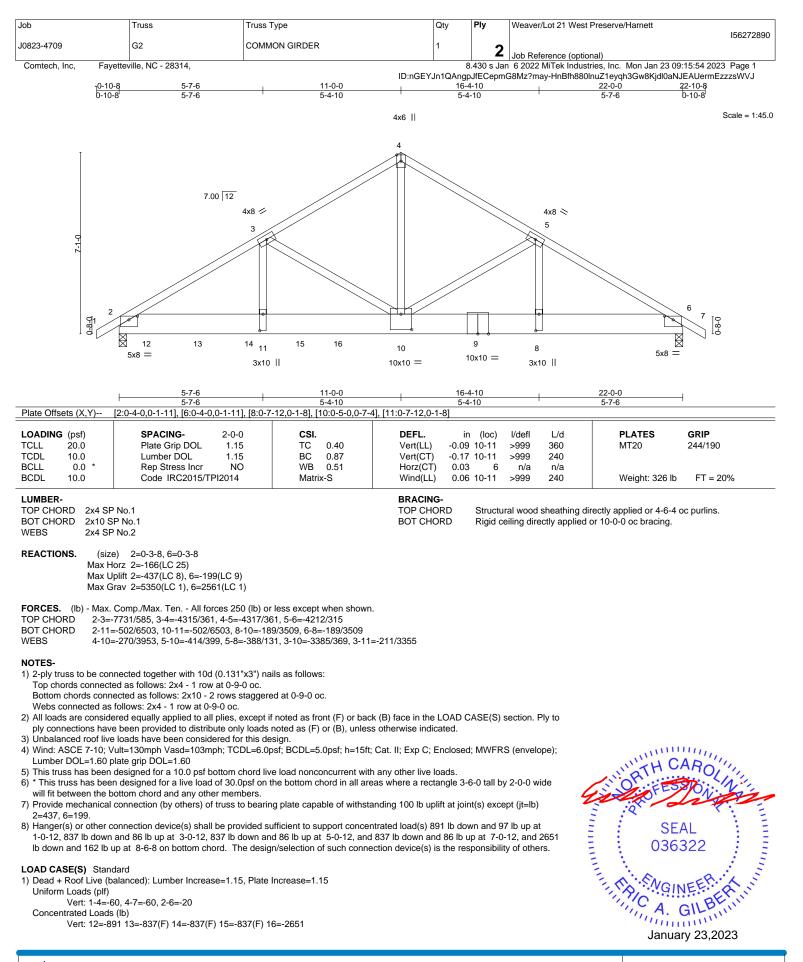
Structural wood sheathing directly applied or 6-0-0 oc purlins.

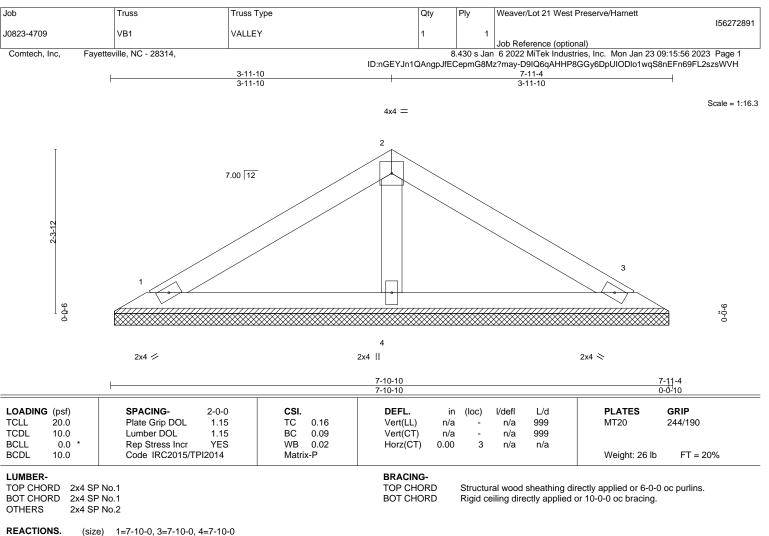
Rigid ceiling directly applied or 10-0-0 oc bracing.

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Max Horz 1=-48(LC 6)

Max Uplift 1=-25(LC 10), 3=-30(LC 11) Max Grav 1=143(LC 1), 3=143(LC 1), 4=258(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

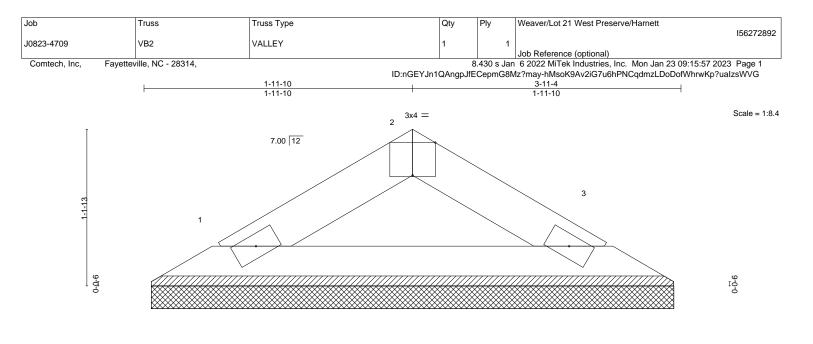
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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2x4 💋

2x4 📚

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Offsets (X,Y)	[2:0-2-0,Edge]		3-10-10 3-10-10	<u>3-11</u> -4 0-0-10
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.02 BC 0.07 WB 0.00 Matrix-P	DEFL.in(loc)l/deflVert(LL)n/a-n/aVert(CT)n/a-n/aHorz(CT)0.003n/a	L/d PLATES GRIP 999 MT20 244/190 999 n/a Weight: 11 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP	No.1		BRACING- TOP CHORD Structural wood	sheathing directly applied or 3-11-4 oc purlins.

BOT CHORD

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1

REACTIONS. (size) 1=3-10-0, 3=3-10-0 Max Horz 1=-20(LC 8) Max Uplift 1=-9(LC 10), 3=-9(LC 11) Max Grav 1=113(LC 1), 3=113(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

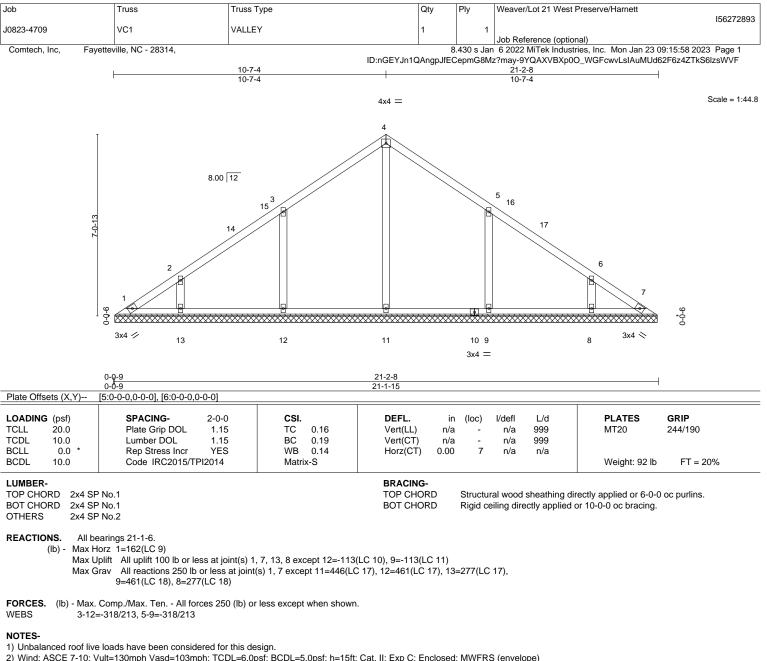
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-15 to 4-10-12, Interior(1) 4-10-12 to 6-2-7, Exterior(2) 6-2-7 to 15-0-1, Interior(1) 15-0-1 to 16-3-12, Exterior(2) 16-3-12 to 20-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

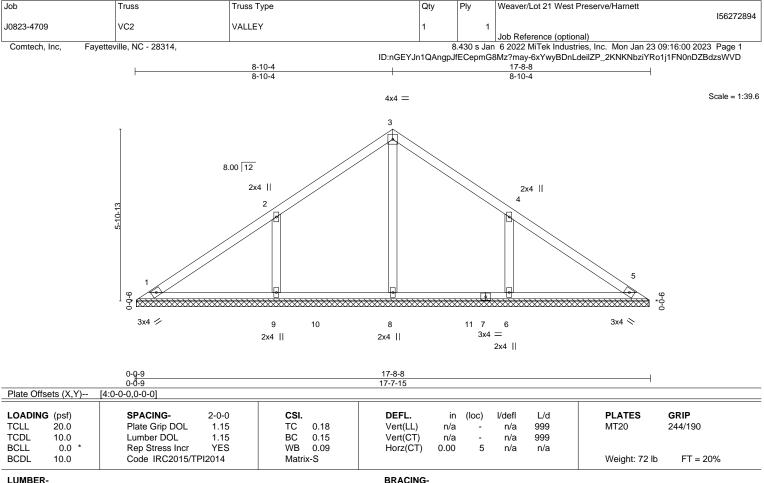
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 13, 8 except (jt=lb) 12=113, 9=113.

7) N/A



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LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2 TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 17-7-6.

(lb) -Max Horz 1=-134(LC 6)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-130(LC 10), 6=-129(LC 11)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=402(LC 17), 9=461(LC 17), 6=461(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-9=-356/249, 4-6=-357/249 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

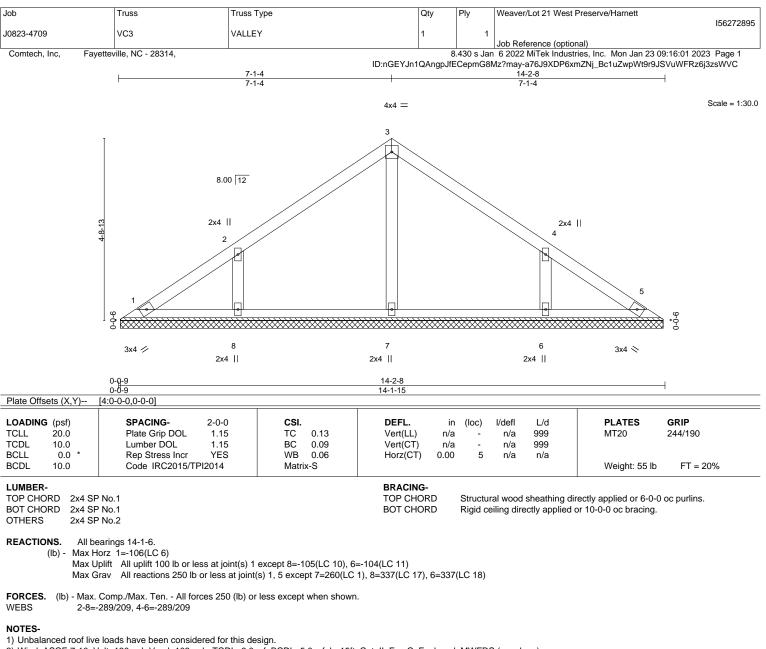
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=130, 6=129.

6) N/A



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2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

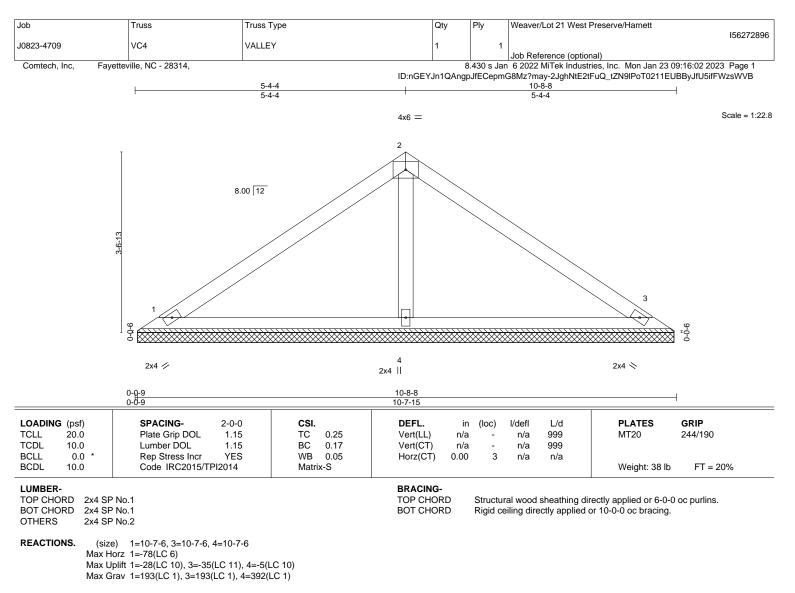
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=105, 6=104.

6) N/A



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

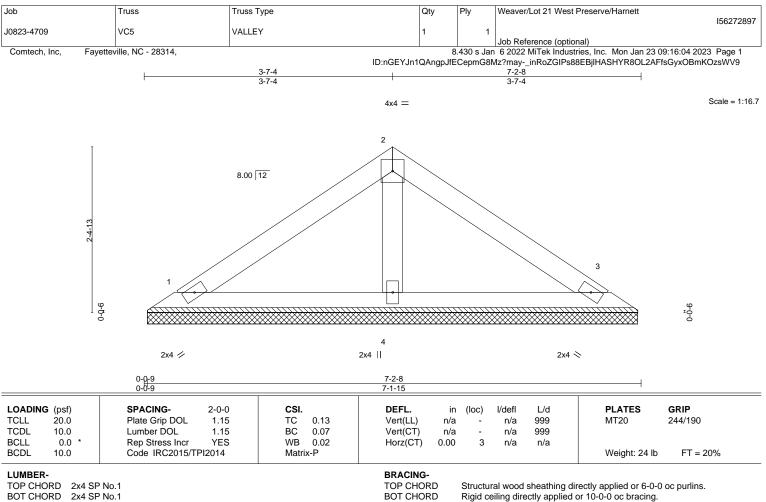
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4. 6) N/A



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BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. 1=7-1-6, 3=7-1-6, 4=7-1-6 (size) Max Horz 1=-50(LC 8) Max Uplift 1=-24(LC 10), 3=-29(LC 11) Max Grav 1=135(LC 1), 3=135(LC 1), 4=227(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

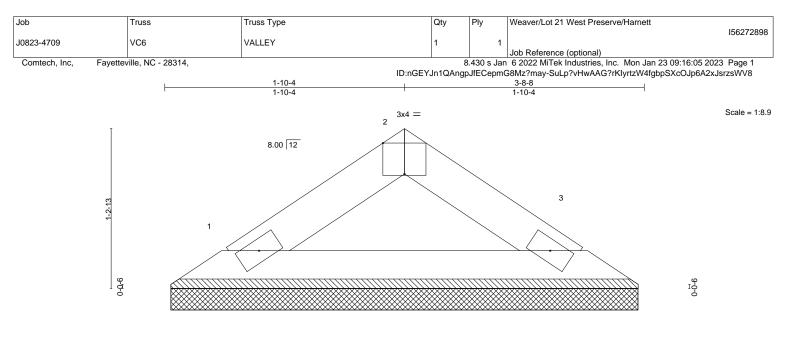
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 6) N/A



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



2x4 🥢

2x4 📎

Structural wood sheathing directly applied or 3-8-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Offsets (X,Y) [0-0-9 0-0-9 2:0-2-0,Edge]		3-8-8 3-7-15					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.02 BC 0.06 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P	()				Weight: 10 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1

REACTIONS. (size) 1=3-7-6, 3=3-7-6 Max Horz 1=-22(LC 6) Max Uplift 1=-8(LC 10), 3=-8(LC 11) Max Grav 1=109(LC 1), 3=109(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=5.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 6) N/A

SEAL 036322 January 23,2023

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