# VININGS



# DUNCANS CROSSING LOT 41

PLAN ID: 020123

# 110 VILLAGE TRAIL SUITE 215 WOODSTOCK, GA. 30188

# DRAWING INDEX

A0.0	COVER SHEET
A1.1	FRONT ELEVATIONS
A2.1	SIDE & REAR ELEVATIONS
A3.1	SLAB FOUNDATIONS
A5.1	FIRST FLOOR PLANS & OPTIONS
A6.1	ROOF PLANS
A7.2	ELECTRICAL PLANS

AREA TABULATION				
FIRST FLOOR	1819			
TOTAL	1819			
GARAGE	411			
FRONT PORCH ELEVATION C F I (COVERED)	144			
REAR PATIO	120			

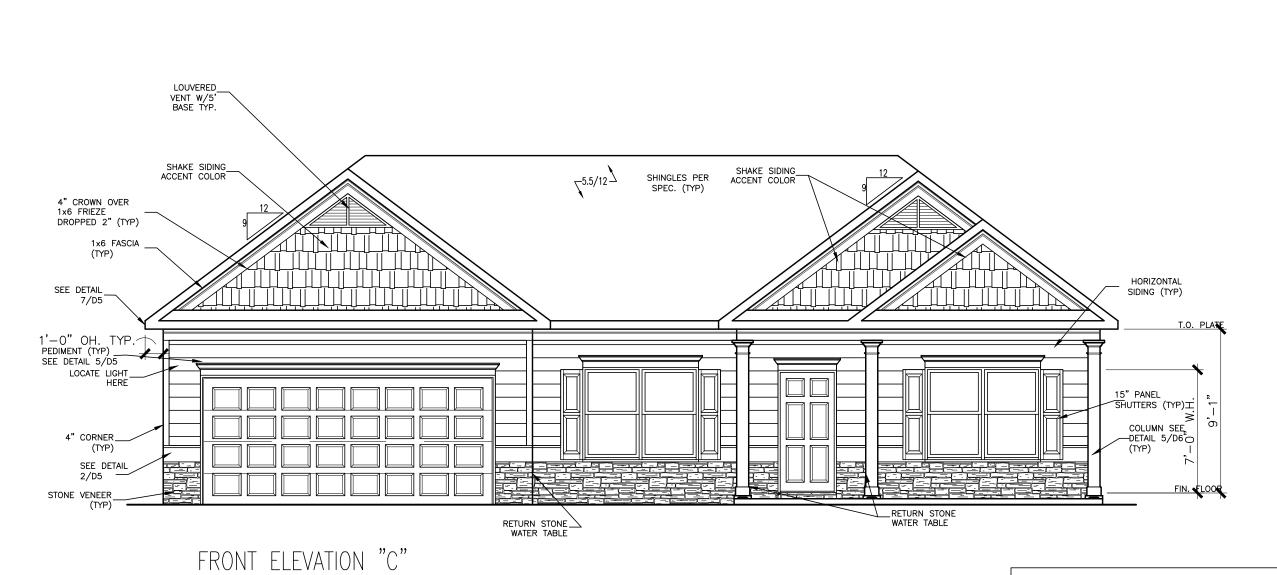
# **GOVERNMENTAL CODES & STANDARDS**

HOME TO BE BUILT TO CONFORM TO ALL APPLICABLE LOCAL CODES, PRACTICES AND STANDARDS

## BUILDING CODE ANALYSIS / DESIGN CRITERIA

HOME TO BE BUILT TO MEET OR EXCEED ALL LOCAL CODES AND DESIGN CRITERIA

PLAN REVISIONS					
DATE	TE BY REVISION				
1/10/2019	MM	Added optional unfinished basement	A4.1		
1/18/2019	AW	Prototype walk revisions - see revision sheet for changes	A3.1, A5.1, A7.2		
3/27/2019	MM	Added callout for detail 3/D5.1 for A massing	A1.1-A1.9.1		
8/2/2019	AW	PCR # 3105 Removed column at Foyer/Dining	A5.1, A5.1.1, A5.1.2, A7.2, A8.1		
10/11/2019	AW	PCR #3301 Relocated door for optional 2nd flr to the top of the stairs (see revision sht.)	A5.1.1, A5.3, A5.4.1, A7.2, A7.3		
12/18/2019	AW	PCR #3464 Remove outlet on knee wall behind kitchen sink and reduce overhang at end of peninsula to 6" per code	A5.1-A5.1.2, A7.2		
2/19/2020	AW	PCR #3621 Remove hdr. between Dining & Lndry Hall and reduce length of wall next to refrig. 12"	A5.1-A5.1.2, A7.2, A8.1		
2/20/2020	AW	Added new Obath configuration to allow for separate tub and shower and created basement plan only Obath options	A3.1, A3.1.1, A5.1-A5.1.2, A7.2, A8.1		
11/1/2020	MM	PCR #4201 Relocated pendant lights	A7.2		
5/1/2021	MM	Removed unfinished 2nd flr option	A5.3,A7.3		
9/20/2021	AW	Added dim to wall next to refrig. and shifted opening over 4"	A5.1-A5.1.2		
12/1/2021	AW	Noted applicable walls on opt. room over garage to be 2x6 and built into the attic truss	A5.3		
12/5/2022	BB	REVISED ROOF PITCHES ON A, B AND C MASSING AND ALL ELEVATIONS OF RANCH PLAN	A1.19, A2.1-A2.3 A6.1-A6.3		

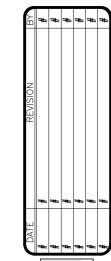


SCALE : 3/16" = 1'-0"

ALL NON-MASONRY RETURNS TO BE HORIZONTAL SIDING

SEE SHEET D3 OF SDH TYPICAL DETAILS FOR SOFFIT DETAILS PER SOFFIT MATERIAL

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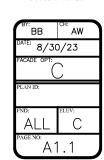


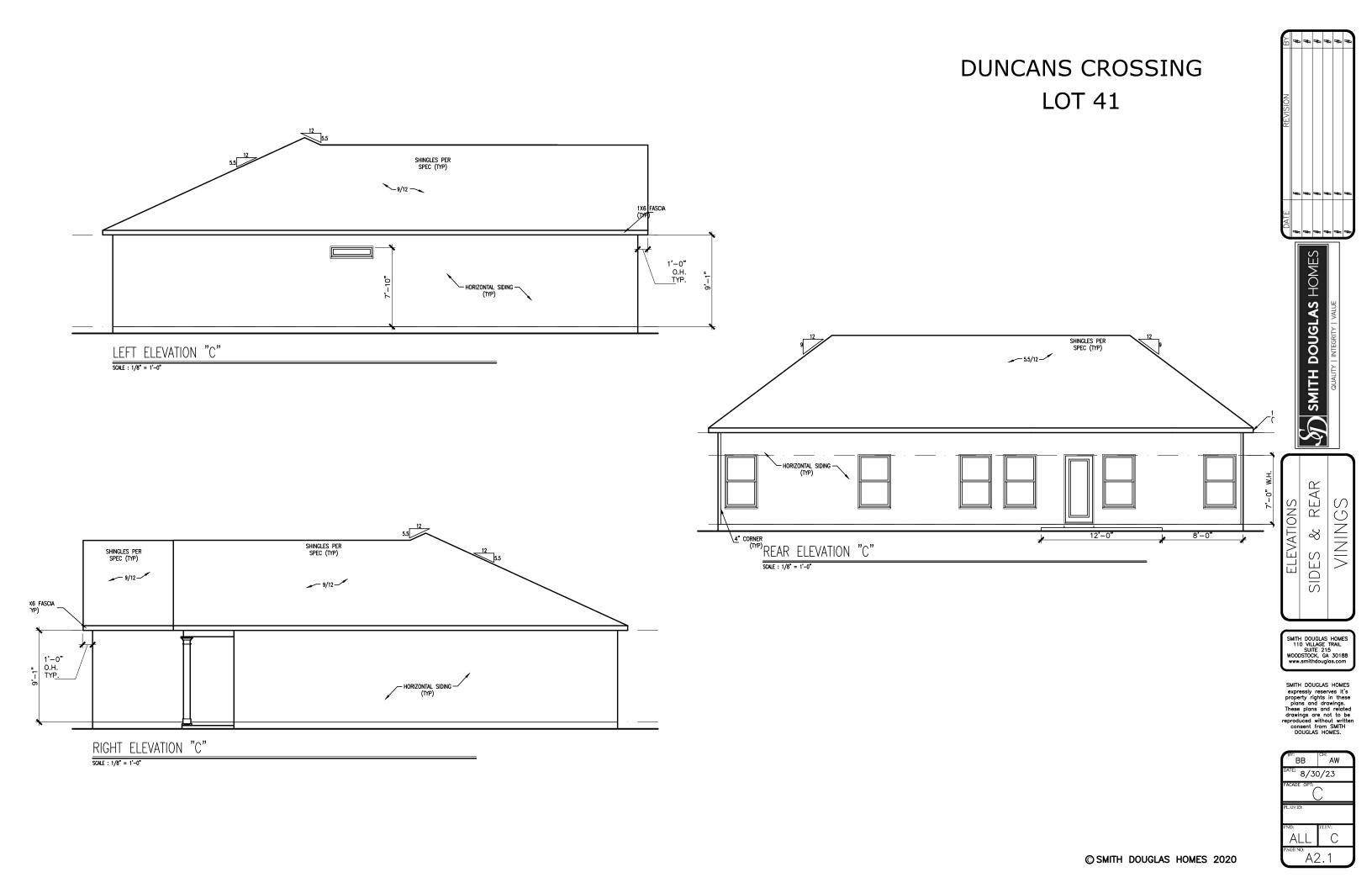
SMITH DOUGLAS HOMES

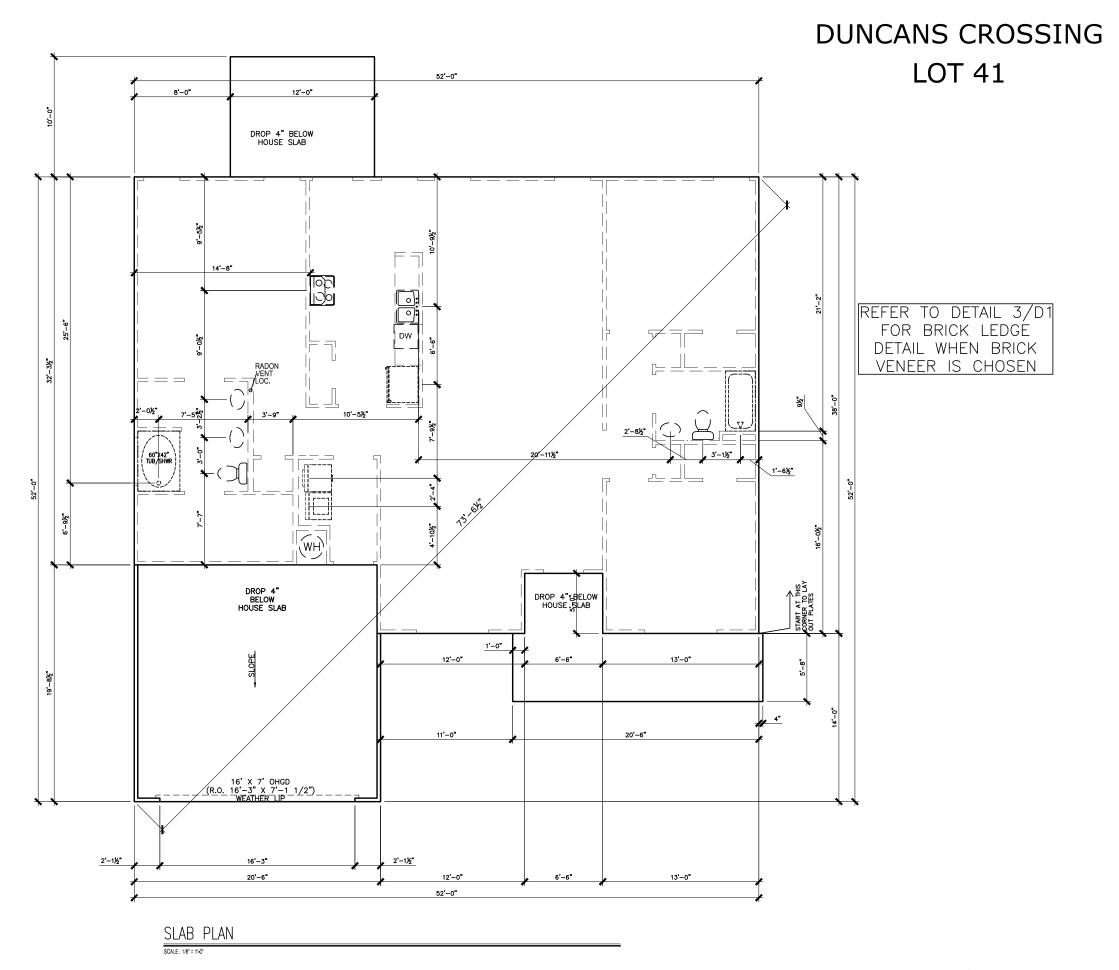
ELEVATIONS FRONT ELEVATION VININGS

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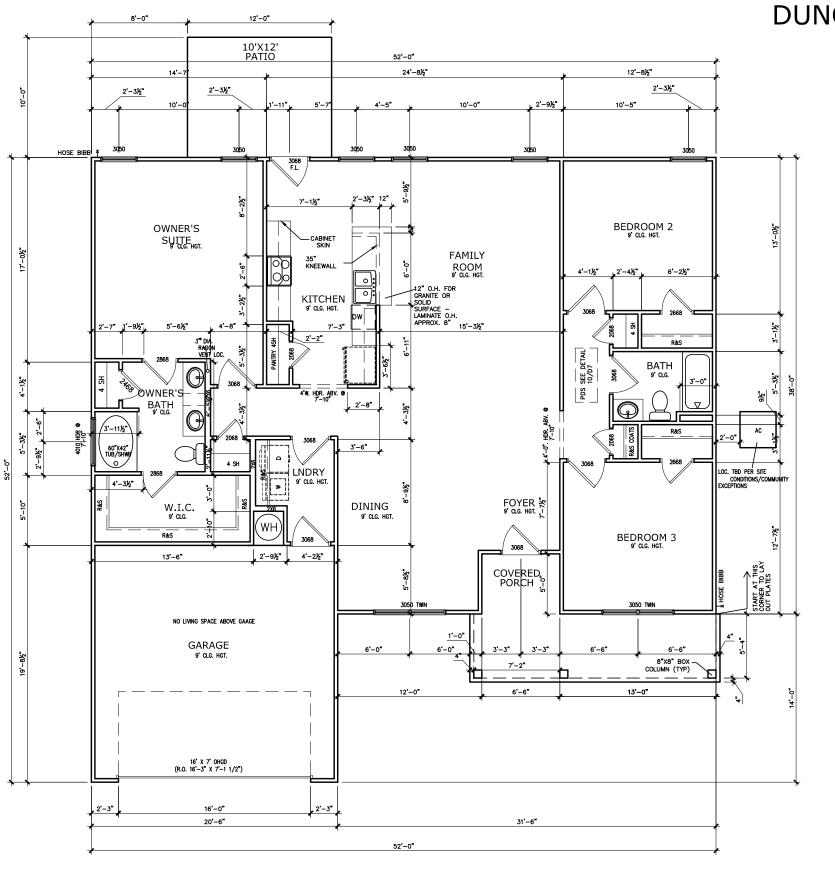
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FOUNDATION PLAN SLAB PLAN VININGS

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BY: BB	CH: AW
	0/23
FACADE OPT:	$\supset$
PLAN ID:	
FND:	C C
PAGE NO:	3.1



\*RADON VENT PROVIDED PER LOCAL CODE

REFER TO MANUFACTURER'S SPECS. FOR DRAIN LOCATIONS ON DETAIL SHEETS D12, D12.1, D12.2 & D12.3

FIRST FLOOR PLAN

SCALE: 188" = 17-0"

BB CH: AW

DATE: 8/30/23

FACADE OPT:

PLAN ID:

FND: ELEV:

ALL C

PAGE NO:

A5.1

SMITH DOUGLAS HOMES

FLOOR

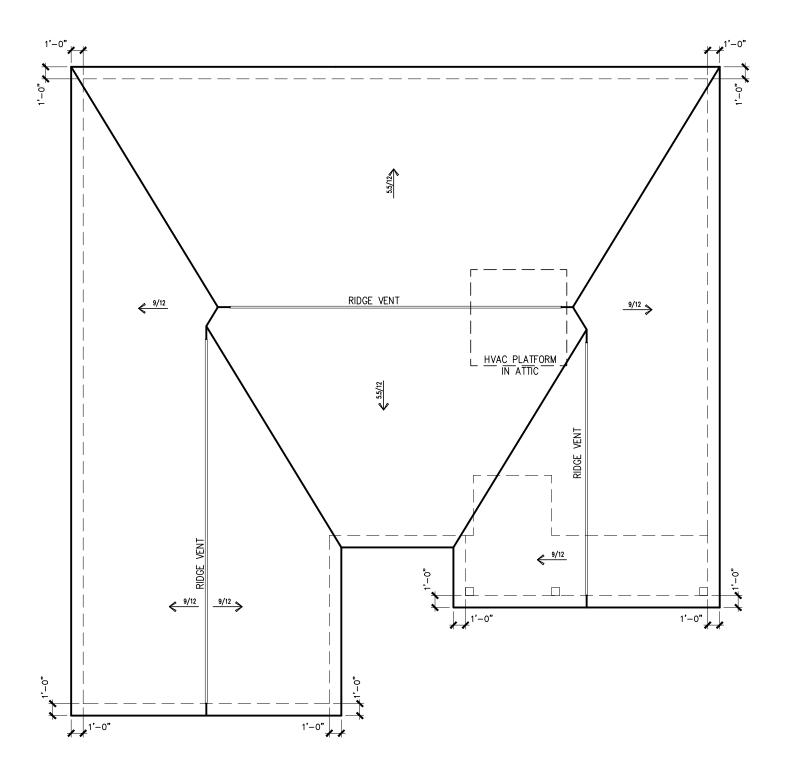
FIRST

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VININGS

FLOOR PLAN



ROOF PLAN "C"

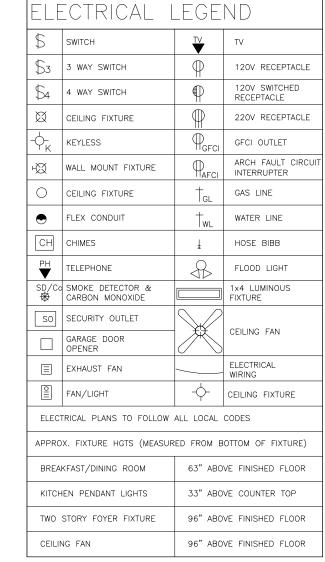
SCALE: 1/8" = 1'-0"



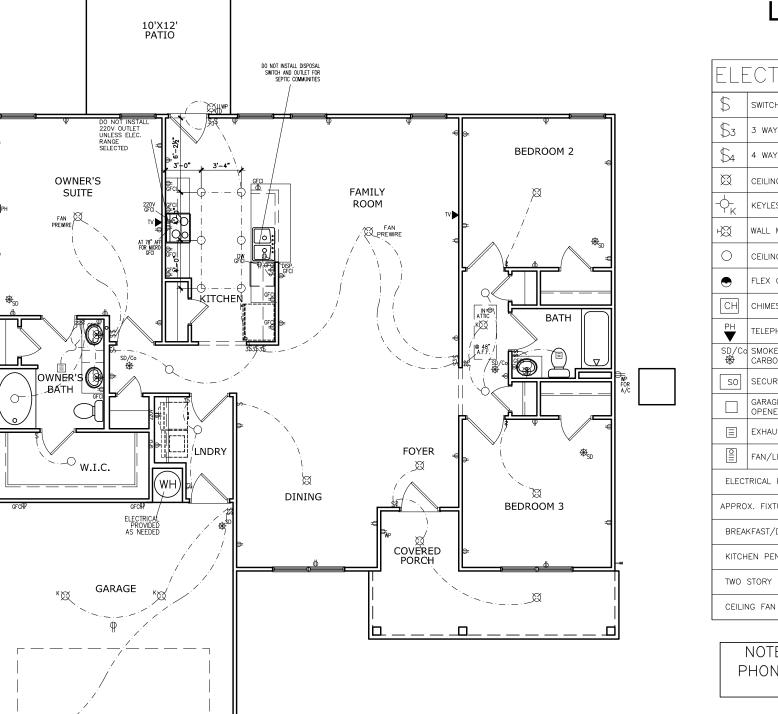
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NOTE: FINAL PLACEMENT OF PHONE/CABLE T.B.D. ON SITE BY THE BUILDER



FIRST FLOOR ELECTRICAL PLAN

SCALE: 1/8" = 1'-0"

BB CH: AW

DATE: 8/30/23

FACADE OPT:

PLAN ID:

FND: ELEV:

ALL C

PAGE NO:

A7.2

SMITH DOUGLAS HOMES

ELECTRICAL PLAN

FLOOR

FIRST

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VININGS

### DESIGN SPECIFICATIONS:

Construction Type: Commerical ☐ Residential ☒

### Applicable Building Codes:

- 2018 North Carolina Residential Building Code
- ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

### Design Loads:

1.	Roc	f		
		11	Ī	íve

I.I FIVE	20 1-0
1.2 Dead	
1.3 Snow	15 PSF
1.3.1 Importance Factor	1.0
2. Floor Live Loads	
2.1 Typ. Dwelling	40 PSF
2.2 Sleeping Areas	30 PSF
2.3 Balconies (exterior) and Decks	
2.4 Garage Parking	50 PSF
3. Floor Dead Loads	
3.1 Conventional 2x	10 PSF
3.2  -Joist	15 PSF
3.3 Floor Truss	15 PSF
4. Ultimate Wind Speed (3 sec. gust)	130 MPH
4.1 Exposure	В
4.2 Importance Factor	1.0
4.3 Wind Base Shear	

# 4.3.1 $\forall$ x = 4.3.2 $\forall$ y = 5. Component and Cladding (in PSF)

MEAN ROOF HT.	UP TO 30'	3Ø'1"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

### 6. Seismic

6. Site Class	D
62 Design Category	C
6.3 Importance Factor	1.0
6.4 Seismic Use Group	1

6.5 Spectral Response Acceleration

6.5.1 Sms = %q

6.5.2 Sml = %g 66 Seismic Base Shear

6.6.1 Vx =

6.6.2 Vy =

6.7 Basic Structural System (check one)

☑ Bearing Wall☑ Building Frame

☐ Moment Frame

☐ Dual w/ Special Moment Frame

☐ Dual w/ Intermediate R/C or Special Steel

☐ Inverted Pendulum

6.8 Arch/Mech Components Anchored?\_\_\_\_\_No

6.9 Lateral Design Control: Seismic ☐ Wind ☑

7. Assumed Soil Bearing Capacity......

...... 2000psf

20 065



STRUCTURAL PLANS PREPARED FOR:

# VININGS

PROJECT ADDRESS: TBD

OWNER:

Smith Douglas Homes - Raleigh 2520 Reliance Ave Apex, NC 27539

### ARCHITECT/DESIGNER:

Smith Douglas Homes 110 Village Trail, Suite 215 Woodstock, GA 30188

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineer of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory & Testing, P.C. before construction begins.

### PLAN ABBREVIATIONS:

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
ΕE	EACH END	SYP	SOUTHERN YELLOW PINE
ΕW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
OC	ON CENTER	TYP	TYPICAL
PSF	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory 4 Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by SMITH DOUGLAS HOMES. Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

### SHEET LIST:

Sheet No.	Description
CS1	Cover Sheet, Specifications, Revisions
C52	Specifications Continued
S1.Øm	Monolithic Slab Foundation
S1.Øs	Stem Wall Foundation
51.Øc	Crawl Space Foundation
51.0b	Basement Foundation
S2.Ø	Basement Framing Plan
S3.Ø	First Floor Framing Plan
S4.Ø	Second Floor Framing Plan
S5.Ø	Roof Framing Plan
S6.Ø	Basement Bracing Plan
S7.Ø	First Floor Bracing Plan
58.Ø	Second Floor Bracing Plan

### REVISION LIST:

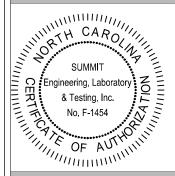
Revision No.	Date	Project No.	Description
1	10.29.18	3832.154R	Added optional bonus room.
2	2.21.19	3832.226	Added optional unfinished basement.
3	3/5/19	3832.226R	Made corrections to header sizes
4	10/17/19	3832.226R2	Moved door to second floor to top of stairs
5	Ø7/Ø7/2Ø 21		Added LIB Option

Duncans Lot 41





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Coversheet client Smith Douglas Homes - Rale 2520 Reliance Ave Apex, NC 21539

CURRENT DRAWING

PROJECT Vinings (LH)

DATE: 10/17/2019

SCALE: 1/8"=1'-0"

PROJECT \*: 3832226R2

DRAWN BY: MEB

CHECKED BY: CNB

ORIGINAL DRAWING

**DATE PROJECT \* 08/07/2018** 3832,154

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

CS1

### GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.
- The structure is only stable in its completed form. The
  contractor shall provide all required temporary bracing
  during construction to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- 5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- This structure and all construction shall conform to all applicable sections of the international residential code.
- 8. This structure and all construction shall conform to all applicable sections of the 2018 North Carolina Residential Code (NCRC) and any local codes or restrictions

### FOUNDATIONS:

- Foundations shall be constructed in accordance with chapter 4 of the 2018 NC Residential Building Code (Special consideration shall be given to Chapter 45 in wind zones above 130mph)
- Footing sizes based on a presumptive soil bearing capacity of 2000 PSF. Contractor is solely responsible for verifying the suitability of the site soil conditions at the time of construction
- Maximum depth of unbalanced fill against masonry walls to be as specified in section R404.1 of the 2018 NCRC
- The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
   The bottom of all footings shall extend below the frost line
- for the region in which the structure is to be constructed.

  However, the bottom of all footings shall be a minimum of 12" below grade.
- Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
- Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- 8. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.
- Each crawl space pier shall bear in the middle third of its respective footing and each girder shall bearing in the middle third of the piers. Pilasters to be bonded to perimeter foundation wall
- IO. Crawl spaced to be graded level and clear of all debris
   II. Provide foundation waterproofing and drain with positive slope to outlet as required by site conditions
- 12. Energy efficiency compliance and insulation of the structure to be in accordance with chapter 11 of the 2018 NCRC

### CONCRETE:

- Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:
  - 3.1. Footings: 5%
- 3.2. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER
- Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab Construction".
- 6. The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- 7. Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
- 8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- Reinforcing steel may not extend through a control joint.
   Reinforcing steel may extend through a saw cut joint.
- 10. All welded wire fabric (WWF.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WWF. shall be securely supported during the concrete pour. Fibermesh may be used in lieu of WWF.

### CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard)
- Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry standard.
- Steel Řeinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- 6. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"
- Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice.
- Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement.
   The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

### WOOD FRAMING:

- Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Spruce-Pine-Fir (SPF) #2.
- LVL or PSL engineered wood shall have the following minimum design values:
- 2.1. E = 1,900,000 psi
- 2.2. Fb = 2600 psi
- 2.3.  $F_V = 285 \text{ psi}$
- 2.4. Fc = 700 psi
- Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- 4. Nails shall be common wire nails unless otherwise noted.
- Lag screws shall conform to ANSI/ASME standard BI82.1-1981.
   Lead holes for lag screws shall be in accordance with NDS specifications.
- All beams shall have full bearing on supporting framing members unless otherwise noted.
- . Exterior and load bearing stud walls are to be 2x4 SPF\*2 @16"
  O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- Individual studs forming a column shall be attached with one 10d nail #6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be fully blocked at all floor levels to ensure proper load transfer.
- 9. Multi-ply beams shall have each ply attached wth (3) lod nails @ 24" OC
- IO. Flitch beams and four and five ply beams shall be bolted together with (2) rows of 1/2" dia. through bolts staggered @24" O.C. w/ 2" edge distance and (2) bolts located at 6" from each end, unless noted otherwise.

### 1000 TRUSSES:

- The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- 3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

### WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards.
- All structurally required wood sheathing shall bear the mark of the APA.
- b. Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- b. Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T4G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

### STRUCTURAL FIBERBOARD PANELS:

- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
- Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

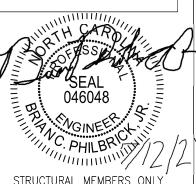
### EXTERIOR WOOD FRAMED DECKS:

Duncans Lot 41

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

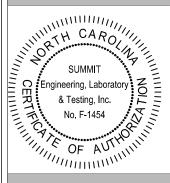
STRUCTURAL STEEL:

- Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and of the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- . All steel shall have a minimum yield stress (Fy) of 36 ksi unless otherwise noted.
- Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D.I.
  Electrodes for shopt and field welding shall be class ETØXX.
  All welding shall be performed by a certified welder per the above standards.





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Coversheet client Smith Douglas Homes - Raleigh 2520 Reliance Ave Apex, NC 21539

### CURRENT DRAWING

/inings (LH)

DATE: 10/17/2019

SCALE: 1/8"=1'-@"

PROJECT \*: 3832226R2

DRAWN BY: MEB

CHECKED BY: CNB

ORIGINAL DRAWING

**DATE PROJECT \*** 08/07/2018 3832.154

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEE

CS2

### FOUNDATION NOTES:

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL
- APENDRENS
  APENDR
- BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE CODE BEPORCEPTION OF CIPICIAL.

  4. FOOTING SYZES BASED ON A PRESIMPTIVE SOIL BEARING CAPACITY OF 2000 PSF. CONTRACTOR IS SOLELY RESPONSIBLE FOR VERRITING THE SUITABILITY OF THE SITE SOIL CANDITIONS AT THE TIME OF CONSTRUCTION.

  5. FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS, PROVIDE 2" WINNING PROJECTION FROOT THE FACE OF MASCARY.

  6. MAXIMM DEPTH OF UNBALANCED FILL AGAINST MASCARY WALLS TO BE AS SPECIFIED IN SECTION RADAL OF THE 2018 NORTH CAROLINA RESIDENTIAL BILLIONS CODE

- SPECIFIED IN SECTION RAGA! OF THE 2006 NORTH CAROLINA RESIDENTIAL BUILDING CODE.

  1. PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL. SPROVIDE FOUNDATION WATERPROCHING, AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.

  9. PROVIDED PERIMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH CAROLINA RESIDENTIAL BUILDING COORD.

  10. CORREL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK YENERS.

- VENETES.

  VENETES.

  1. CRAIL 9FACE TO BE GRADED LEVEL AND CLEARED OF ALL DERRIS.

  12. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 20% NORTH CAROLINA RESIDENTIAL CODE SECTION Register INTERM 19° DIA BOLTS 9FACED AT 6-0° ON CENTER WITH A 1" MINIMAN EMBEDMENT INTO MASOARY OR CONCRETE. ANCHOR BOLTS 9HALL BE 10" PROOT THE BOLD OF EACH PLATE SECTION NINIMAN (2) ANCHOR BOLTS 15 HALL BE 10" PROOT THE BOLD OF EACH PLATE SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.

  13. ABBREVIATIONS.
- DJ = DOUBLE JOIST GT = GIRDER TRUSS 9C = 9TUD COLUMN EE = EACH END SJ = SINGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER TR = TRIPLE RAFTER TJ = TRIPLE JOIST CL = CENTER LINE OC = ON CENTER PL = POINT LOAD

- 4. ALL PIERS TO BE 16"x16" MASONRY AND ALL PILASTERS TO BE 8"x6" MASONRY, TYPICAL. (INKO)

  5. WALL FOOTINGS TO BE CONTINUOUS CONCRETE, SIZES PER STRUCTURAL PLAN.

  6. A FOUNDATION EXCANATION OSSERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER, OR HIS GUALIFIED

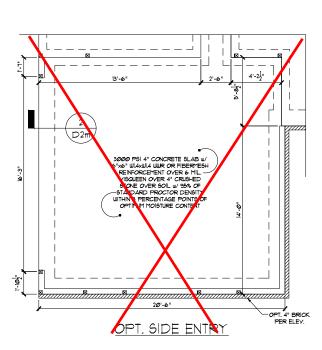
  REPRESENTATIVE. FIREOLATED AREA OF YTELDING MATERIALIS AND/OR POTENTIALLY EXPANSIVE SOLIS ARE OBSERVED IN THE FOOTING EXCANATIONS AT THE TITLE OF CONSTRUCTION SHOTHED TEMPLETING, LABORATORY A TESTING, P.C. MUST BE PROVIDED THE OPPORTINITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE PLACEMENT.

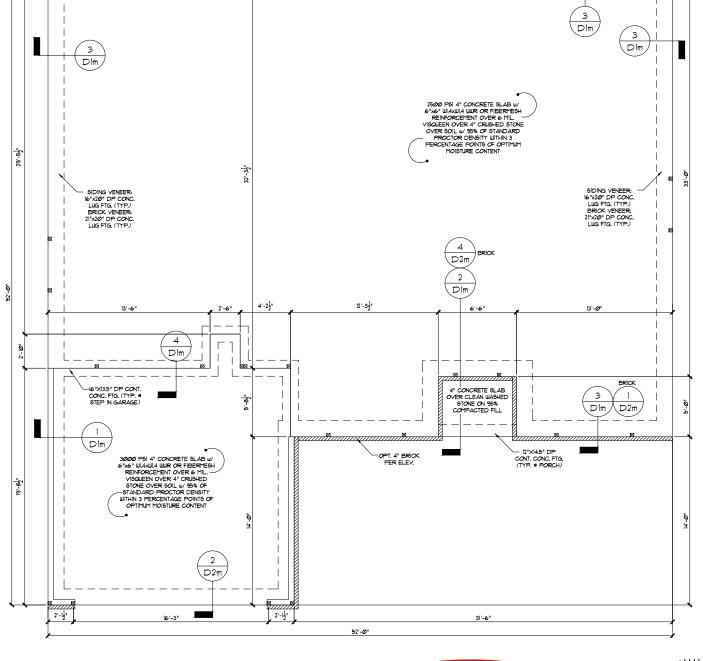
  11. ALL FOOTINGS 6 SLABS ARE TO BEAR ON UNDISTURBED SOLI OR 95% COMPACTED FILL, VERTIFIED BY ENGINEER OR CODE OFFICIAL.

NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER, UNO

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS HOMES COMPLETED REVISED ON BOTLODS. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT SHANEERING, LABORATORY & TESTING, P.C., F. ANY CHANGES ARE HADE TO THE RECHITECTURAL PLANS PRIOR TO CONSTRUCTION SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS HERD USED WITH ACKITECTURAL PLANS HERD MECHITECTURAL PLANS HERD DATE LISTED ABOVE.

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP I PER TABLE R405.1





32'-016"

OPT. 4" CONC. PATIO SLAB

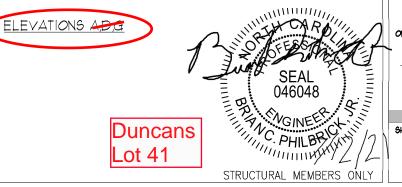
11'-115"

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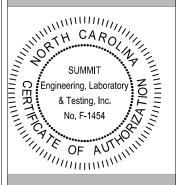
STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

MONOLITHIC SLAB FOUNDATION





3070 Hammond Business Place Suite 171, RALEIGH, NC 27603 OFFICE: 919 380 9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM



<u>g</u>

<u>o</u> 0<u>7</u> Douglas Homes . Reliance Ave x, NC 21539 Fnd lab S  $\widehat{\mathbb{L}}$ Monolithic Vinings Дрех, Smith 1 2520

CURRENT DRAWING

DATE: 10/17/2019

SCALE: 1/8"=1'-@"

PROJECT \*: 3832226R2

DRAWN BY: MEB

CHECKED BY: CNB

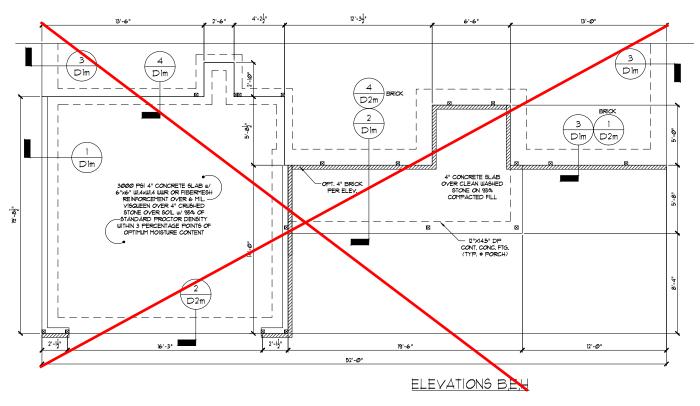
ORIGINAL DRAWING

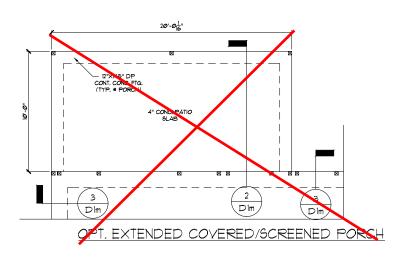
DATE PROJECT \* 08/07/2018 3832.154

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S1.0m

SEE SHEET SI.OM FOR NOTES AND MORE INFORMATION





11'-115"

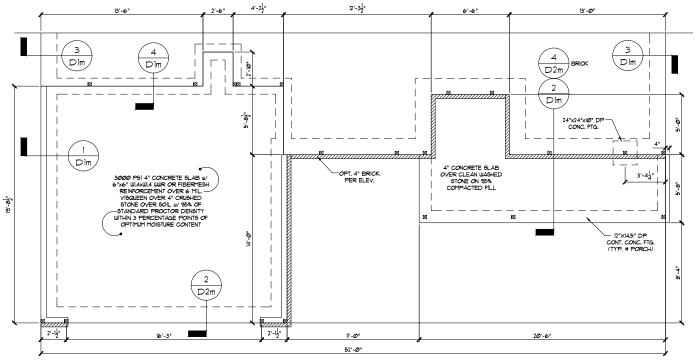
— 12"X14,5" DP CONT. CONC. FTG. (TYP. ⊕ PORCH

COVERED/SCREENED PORCH

INC. PATIO

3

\ Dim /



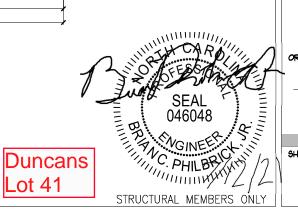
ELEVATIONS C.F.I

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

MONOLITHIC SLAB FOUNDATION



Lot 41

n Douglas Homes . 9 Reliance Ave x, NC 21539 Fnd Slab Vinings (LH) Monolithic Smith D 2520 R Apex, I

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Suite 171, RALEIGH, NC 27603 OFFICE: 919.380.9991

FAX: 919.380.9993

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CE Engineering, Laboratory

No. F-1454

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Raleigh

### CURRENT DRAWING

DATE: 10/17/2019

SCALE: 1/8"=1'-@"

PROJECT \*: 3832.226R2

DRAWN BY: MEB

CHECKED BY: CNB

ORIGINAL DRAWING

DATE PROJECT \*

08/07/2018 3832.154

REFER TO COVER SHEET FOR A

COMPLETE LIST OF REVISIONS

S1.2m

### GENERAL STRUCTURAL NOTES:

- GENERAL STRUCTURAL NOTES

  1. CONSTRUCTURAL NOTES

  2. CONTRACTOR SHALL CAL AMENOMENTS.

  2. CONTRACTOR SHALL LOCAL AMENOMENTS.

  2. CONTRACTOR SHALL VERRY ALL DIMENSIONS, CONTRACTOR SHALL COMPLY WITH THE CONTRIBUTE OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY EVALUATION FROM THIS SPECIFIC PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY EVALUATION FROM THIS SPECIFIC PROJECT, STRUCTURED TO RESPONSIBLE FOR PROVIDING EITHORARY BRACING REQUIRED TO REPORT IN SECOND FROM THE STRUCTURE OF THE SHALL SHALL BE 2 SPECIFIC MAN 14 CONTRIBUTED TO REPORTED WITH A COTEX AND SHALL SHALL BE CONSTRUCTED FOR THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION REQUIRED FOR THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION REQUIRED FOR THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION REQUIRED FOR THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION REGISTAL THE CONSTRUCTED FOR THE 2018 NORTH CAROLINA RESIDENTIAL CODE SECTION REGISTAL THE END OF EACH PLATE SECTION MINIMUM (2) ANCHOR DATE SHALL BE 10 FROM THE END OF EACH PLATE SECTION MINIMUM (2) ANCHOR DATE SHALL BE 10 FROM THE END OF EACH PLATE SECTION MINIMUM (2) ANCHOR DATE SHALL BE CECTION ANCHOR SOUTH OF THE FLATE.

  CONTRACTOR TO PROVIDE THE PLATE SECTION ANGEONED FOR PLATE SECTION PLATE CONTRACTOR TO PROVIDED LOCACIDE HIM DET CELIUM JOISTS SHALL BE DUTTED OF THE PLATE.

  CONTRACTOR TO PROVIDE DLOCACIDI HERD CELIUM JOISTS SHALL BE BOLTED.
- 9. CONTRACTOR TO PROVIDED LOXICATS WHEN CELLING JOISTS SPAN PERFENDICILAR TO RAFTERS.

  10. FLITCH BEAYS, 4-FLY I.V.I.S. AND 3-FLY SIDE LOADED L.V.I.S. SHALL BE BOLTED TOSETHER WITH I/I.P. IAV. THRU BOLTES PACED AT 12\* C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS FER DETAIL (1931 MN) EDGE DISTANCE SHALL BE 2' AND (2) BOLTES SHALL BE L.CATED MINITH IN FROM EACH BND OF THE BEAM.

  11. ALL NON-LOAD BEARNA' LEADERS SHALL BE (1) FLAT 2x4 SFF 9, DROFFED, FOR ONL LOAD BEARNA' LEADERS SHALL BE (2) FLAT 2x4 SFF 9, DROFFED. CONNECTION CONTRACTOR SHALL BE (2) FLAT 2x4 SFF 9, DROFFED.

  12. ARDREW IATIONS.

(ELEV. C.F.I - ONLY)

A

- DJ = DOUBLE JOIST GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END TJ = TRIPLE JOIST CL = CENTER LINE
- SJ = SINGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER TR = TRIPLE RAFTER OC = ON CENTER PL = POINT LOAD

NOTE: NOTE:

DESIGNATES JOIST SUPPORTED LOAD
BEARING WALL ABOVE, PROVIDE BLOCKING UNDER
JOIST SUPPORTED LOAD BEARING WALL.

NOTE: SHADED WALLS INDICATE LOAD BEARING WALLS

JOIST 4 BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL HESE PLANS AND LESIGNED IN ACCOMMENDE WITH ARCHITECTURAL PLANS PROVIDED BY SHITL POXIL AS HOTEL COMPLETED PROPERTIED ON <u>2017/2019</u>. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SHITLING PLANS AND ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION JUMPIT ENGINEERING, LEADATORY I TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WEN USED WITH ACHITECTURAL PLANS OF THE ADEQUACY OF THESE STRUCTURAL PLANS WEN DEAT WITH THE DATE LISTED ABOVE.

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN SCALE: 1/8"=1"

KING STUD REQUIREMENTS OPENING WIDTH 16" O.C. 24" O.C. LESS THAN (1) (1) KING STUD REQUIREMENTS ABOVE DO NOT APPLY TO PORTAL FRAMED OPENINGS

HEADER TAG	BEAM TAG	SIZE	JACKS (EACH END)
-	BI	(1) 14" FLOOR JOIST	(2)
-	B2	(2) 14" FLOOR JOIST	(2)
Α	B3	(2) 2x6	(I)
В	B4	(2) 2x8	(2)
С	B5	(2) 2xlØ	(2)
D	B6	(2) 2xl2	(2)
E	B1	(2) 9-1/4" LVL	(3)
F	B8	(2) II-7/8" LVL	(3)
G	B9	(2) 14" LVL	(3)
H	BIØ	(2) 16" LVL	(3)
	BII	(2) 18" LVL	(3)
J	B12	(2) 24" LVL	(4)
K	B13	(3) 9-1/4" LVL	(3)
L	B14	(3) 11-7/8" LVL	(3)
М	B16	(3) I4" LVL	(3)
N	BIT	(3) 16" LVL	(3)
0	BIS	(3) 18" LVL	(3)
P	BIS	(3) 24" LVL	(4)
HEADER/BEA	M SITES SHOW	N ON PLANS ARE MINI	MUMS GREATER

HEADER/BEAM SCHEDULE

HEADER/BEAM 91259 SHOWN ON PLANS ARE MINIMANS, GREATER HEADER/BEAM 91255 NAY BE USED FOR EASE OF CONSTRUCTION. ALL HEADERS TO BE DROMPED UNLESS NOTED OTHERWISE, ALL BEAMS TO BE FLUSH UNLESS NOTED OTHERWISE.

LINTEL SCHEDULE			
TAG SIZE OF		OPENING SIZE	
0	L3×3×1/4"	LESS THAN 6'-0"	
2	L5x3x1/4"	6'-0" TO 10'-0"	
3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"	
4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS	
SECURE LINTEL TO HEADER w/ (2) 1/2"			

DIAMETER LAG SCREWS STAGGERED . 16" O.C. (TYP FOR 3) ALL HEADERS WITH BRICK ABOVE: (1)(UNO)

WALL STUD SCHEDULE 1ST & 2ND FLOOR LOAD BEARING WALLS: 2x6 STUDS @ 24" O.C. OR 2x4 STUDS @ 16" O.C. 2X6 STUDS \* 14" O.C. OR 2X4 STUDS \* 16" O.C. 1ST FLOOR LOAD BEARING WALLS SUPPORTING: 2ND FLOOR \* WALK-UP ATTIC: 2X6 STUDS \* 16" O.C. OR 2X4 STUDS \* 12" O.C. 246 51UD5 6 (8° O.C. OR 744 51UD5 0 (2° O.C. BASETINE I OAD BEARING HULLS: 246 51UD5 6 (8° O.C. OR 744 51UD5 6 (8° O.C. OR 744 51UD5 6 (8° O.C. OR 744 51UD5 6 (8° O.C. TUD 51 OCK HULLS: (ALL FLOORS): 244 51UD5 6 (8° O.C. VERTICALLY CAS 51UD5 6 (8° O.C. VERTICALLY CAS 45 UD5 6 (8° O.C. VERTICALY CAS 45 UD5 6 (8° O.

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<u>g</u> <u>o</u> 0<u>7</u> Douglas Homes . Reliance Ave x, NC 27539 Framing  $\widehat{\mathbb{L}}$ Floor Vinings Дрех, Smith 2520 First

### CURRENT DRAWING

DATE: 10/17/2019

SCALE: 1/8"=1'-@"

PROJECT \*: 3832226R2

DRAWN BY: MEB

CHECKED BY: CNB

### ORIGINAL DRAWING

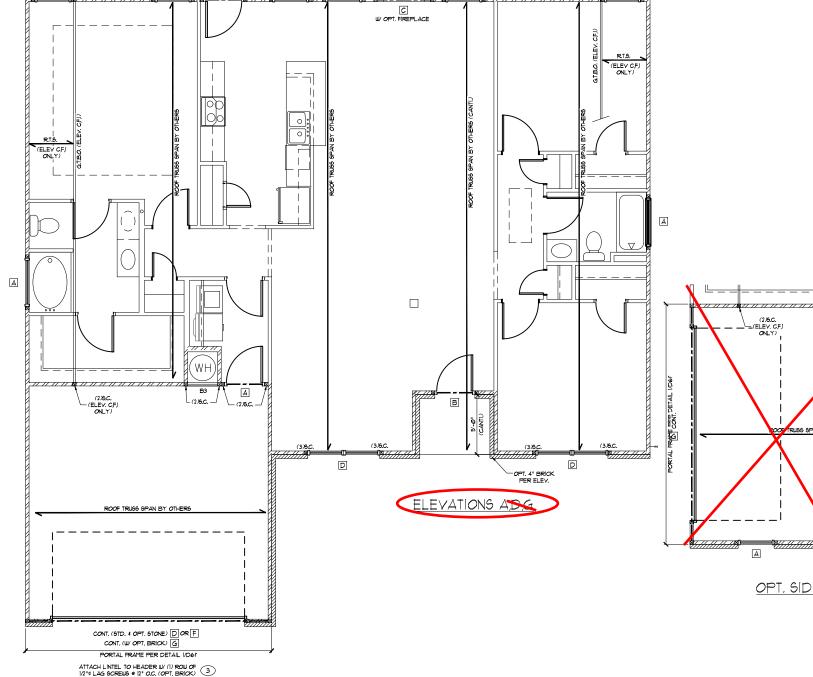
DATE PROJECT \*

08/07/2018 3832.154

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

STRUCTURAL MEMBERS ONLY

S3.0



ROOF TRUSS SPAN BY OTHERS OR 2X4 C.J. @ 16" O.C. (W/ OPT. FIREPLACE ONLY)

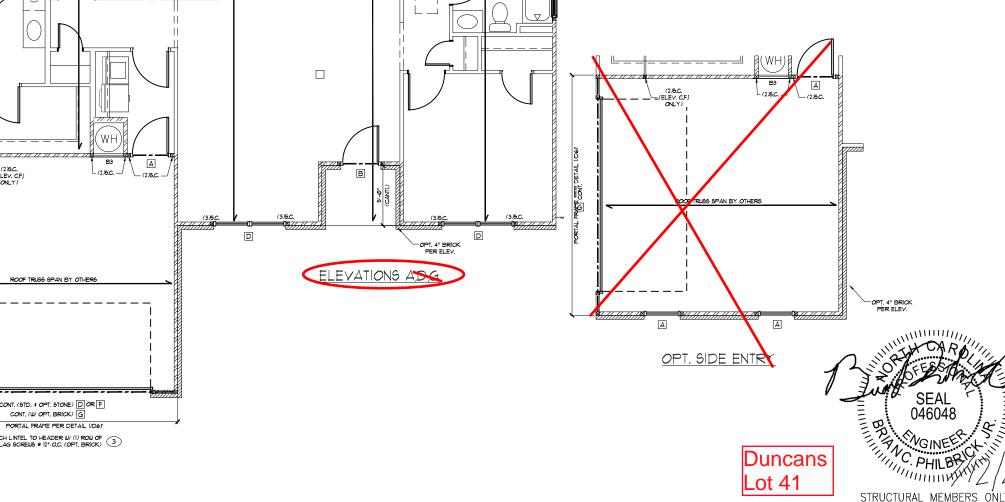
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-- (2)S.C. -

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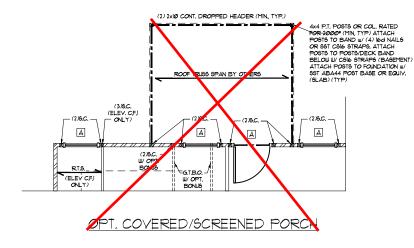
(3.6.C. (ELEV. C.F.) -

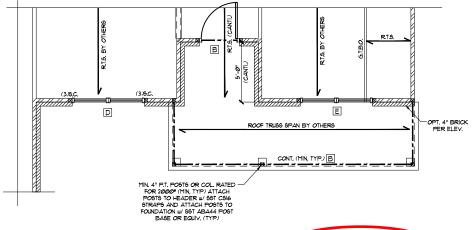
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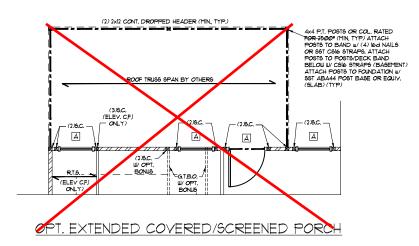
Duncans Lot 41

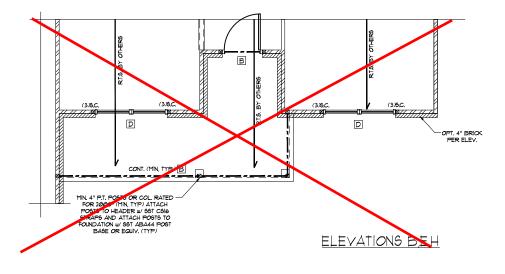
SEE SHEET 53.0 FOR NOTES AND MORE INFORMATION







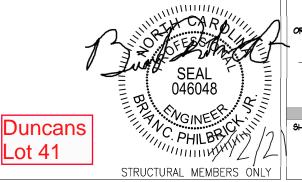




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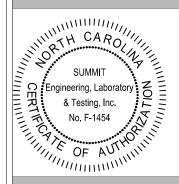
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.



Lot 41

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<u>6</u> 0<u>x</u> Douglas Homes . Reliance Ave x, NC 21539 Framing First Floor Vinings Smith D 2520 R Apex, I

### CURRENT DRAWING

DATE: 10/17/2019

SCALE: 1/8"=1'-@"

PROJECT \*: 3832226R2

DRAWN BY: MEB

CHECKED BY: CNB

ORIGINAL DRAWING

DATE PROJECT \*

Ø8/Ø7/2Ø18 3832.154

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S3.3

FIRST FLOOR FRAMING PLAN SCALE: 1/8"=1"

TRUSS	TRUSS UPLIFT CONNECTOR SCHEDULE			
MAX, UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FND	
535 LB6	H2.5A	PER WALL SHEATHIN	IG 4 FASTENERS	
1070 LB6	(2) H2.5A	CSI6 (END = 13")	DTT2Z	
1245 LBS	HTS2Ø	CSI6 (END = 13")	DTT2Z	
112Ø LBS	(2) MT52Ø	(2) C916 (END = 13")	DŤŤ2Z	
249Ø LB6	(2) HT52Ø	(2) C6I6 (END = I3")	HTT4	
2365 LB6	LGT3-9D62.5	(2) CSI6 (END = 13")	HTT4	
A LL PRODUCTS LISTED ARE SINDS OF STRONG THE POUNTAL FAIR PRODUCTS				

L ALL PRODUCTS LISTED ARE SIMPSON STRONG-TIE. EQUIVALENT PRODUCTS MAY BE USED FIR MANIFACTURERS SPECIFICATIONS.

2. IMPLIFY VALUES LISTED ARE FOR SHF 2 (RADDE HEPGERS).

3. RETER TO TRUSS LATOUT FEER MANIF. FOR WILLT VALUES AND TRUSS TO TRUSS CANCILORS SPECIFIED BY TRUSS MANIFACTURER.

I RUSS CONNECTIONS CONNECTORS SPECIFIED BY TRUSS FRANCACTURER OVERRIDE THOSE LISTED ABOVE.

4. CONTACT SUMMIT FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.

NOTE: IST PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

NOTE: TRUSS UPLIFT LOADS SHALL BE DETERMINED FER TRUSS
MANIFACTURER IN ACCORDANCE WITH SECTION REQUIL! WALL SHEATHING
AND FASTENERS HAVE BEEN DEBIGNED TO RESIST THE WIND UPLIFT LOAD
PAIT IN ACCORDANCE WITH METHOD 3 OF SECTION REQUISS OF THE 2018
NOCE, REFER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER
REQUIREMENTS.

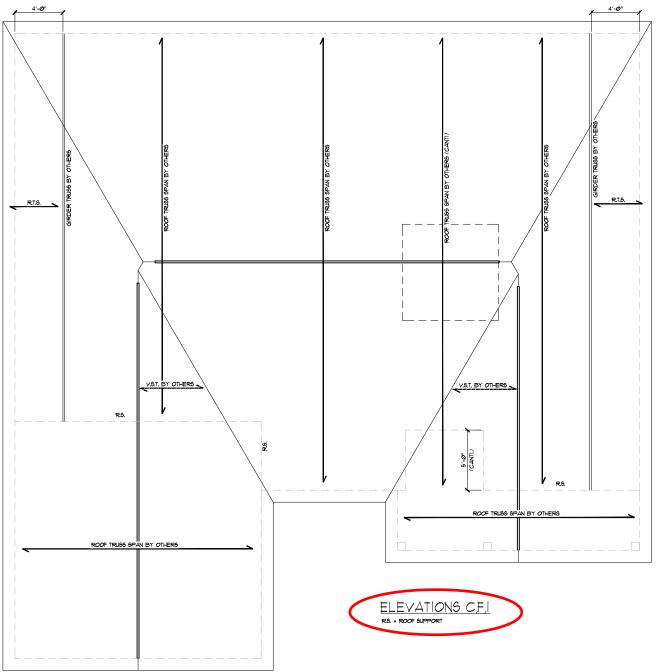
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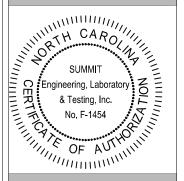
STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

ROOF FRAMING PLAN SCALE: 1/8"=1"





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<u>6</u> 0<u>K</u> Douglas Homes . Reliance Ave x, NC 21539 Plan Roof Framing PROJECT Vinings (LH) Smith D 2520 R Apex, 1

### CURRENT DRAWING

DATE: 10/17/2019

SCALE: 1/8"=1'-@"

PROJECT \*: 3832226R2

DRAWN BY: MEB

CHECKED BY: CNB

ORIGINAL DRAWING

DATE PROJECT \*

08/07/2018 3832.154

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SEAL 046048

O46048

TRUCTURAL MEMBERS

STRUCTURAL MEMBERS ONLY

Duncans

Lot 41

S5.2



REQUIRED BRACED WALL PANEL CONNECTIONS				
		MIN. REQUIRED		CONNECTION
METHOD	MATERIAL	THICKNESS	# PANEL EDGES	# INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS © 6" O.C.	6d COMMON NAILS 12" O.C.
GB	GYPSUM BOARD	1/2"	5d COOLER NAIL5** # 7" O.C.	5d COOLER NAILS** ® 7" O.C.
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS # 6" O.C.	6d COMMON NAILS 9 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.1	PER FIGURE R602.10.1
**OR EQUIVALENT PER TABLE R102.3.5				

### BRACED WALL NOTES:

- I. WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602/0
  FROM THE 2009 NORTH CAROLINA RESIDENTIAL CODE.

  2. WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND
  SPEEDS UP TO 300 MPH.

  3. REFER TO ARCHITECTIRAL PLAN FOR DOORWINDOW OPENING SIZES.

  4. BRACKING MATERIALS, METHODS AND FASTENIERS SHALL BE IN
  ACCORDANCE WITH TABLE R602/30.

  5. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL
  NOT EXCEED OF HEIT FOR ISOLATED PANEL METHOD AND 12 HEET FOR
  CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING
  CALCULATIONS.
- NOT EXCEED ID FIET FOR ISOLATED PANEL INTETHOD AND IS FIET FOR CONTINUOUS SHEATHING RETHOD WITHOUT ADDITIONAL PRISHERING CALCULATIONS.

  CALCULATIONS.

  (A) MINIMUM PANEL LENGTH SHALL BE FER TABLE R602/0/1.

  1. THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHATHED CONTINUOUS! Y WITH MINIMUM IS' MYPSIM BOARD (IND).

  FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHING METHOD, EXTERIOR SHILL AREAS SHEWARD RACKED WALL PANELS, AND ON GABLE END WALL.

  FIN. ORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARNS WALL BELOW WITHOUT ADDITIONAL DISINEERING CALCULATIONS.

  (B) A BRACED WALL PANEL SHALL BELOCATED WITHIN IS FEET OF EACH END OF A BRACED WALL INTO.

  11. THE MAXIMM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED I FEET.

  12. MASONRY OR CONCRETE SITEM WALLS WITH A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANELS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMENT.

  13. BRACED WALL PANEL CONNECTIONS OF THE 200 NORC.

  14. BOSTRICTED IN ACCORDANCE WITH SECTION REQUIPMENT.

  15. BRACED WALL PANEL CONNECTION TO FLOORCEPILING SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMENT.

  16. BRACED WALL PANEL OWN ACCORDANCE WITH SECTION REQUIPMENT.

  17. BRACED WALL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REGULES AND WALL SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMENT.

  18. BRACED WALL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REQUIPMENT WALL SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMENT.

  18. BRACED WALL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REQUIPMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMENT.

  18. BRACED WALL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REQUIPMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMENT.

- ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.
   ABBREVIATIONS:

GB = GYP9UM BOARD
C5-XXX = CONT, SHEATHED
FF = PORTAL FRAME
FF = PORTAL FRAME
FF-ENG = ENG, PORTAL FRAME

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SHITH DOKALAS HOTES COMPLETED REVISED ON DOLLOWS. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT EXISTERING, LADORATORY I TESTING, P.C. FANY CHANCES ARE MODE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION, SUMMIT EXISTERING, LADORATORY I TESTING, P.C. CANNOT GLARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

FIRST FLOOR BRACING (FT)			
CONTINUOUS SHEATHING METHOD: RECTANGLE 1			
	REQUIRED PROVIDED		
FRONT	5.8	IT.Ø	
RIGHT	7.4	13.1	
REAR	5.8	34.0	
LEET	74	323	

FIRST FLOOR BRACING (FT)				
CONTINUOUS SHEATHING METHOD: RECTANGLE 2				
	REQUIRED PROVIDED			
FRONT	3.0	IT.Ø		
RIGHT	3.1	13,1		
REAR	3.0	34.0		
LEFT	3.1	3.1 32.3		

INSTALL HOLD-DOWNS PER SECTION R602:10.4 AND FIGURE R602:10.3(4) OF THE 2018 NCRC.

### STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS.

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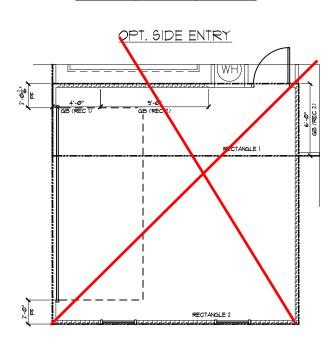
STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

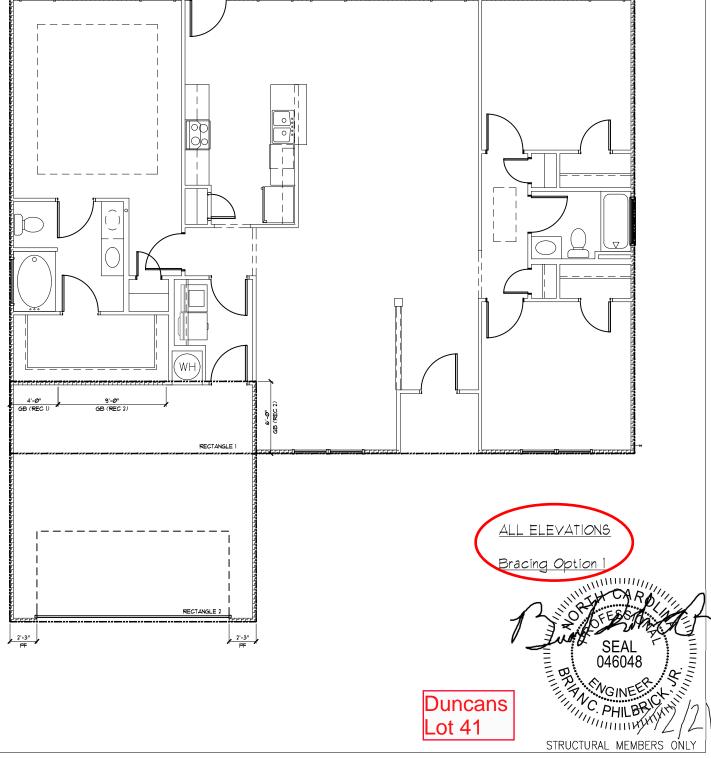
FIRST FLOOR BRACING PLAN SCALE: 1/8"=1"



FIRST FLOOR BRACING (FT)				
CONTINUOUS SHEAT	CONTINUOUS SHEATHING METHOD: SIDE ENTRY RECTANGLE I			
REQUIRED PROVIDED				
FRONT	5.8	15.0		
RIGHT	34.0			
REAR	5.8	24.6		
LEFT 1.4 28.3				

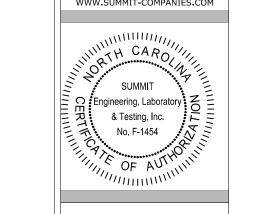
FIRST FL	FIRST FLOOR BRACING (FT)			
CONTINUOUS SHEAT	CONTINUOUS SHEATHING METHOD: SIDE ENTRY RECTANGLE 2			
	REQUIRED PROVIDED			
FRONT	3.Ø	14.5		
RIGHT	3.1 IT.Ø			
REAR	3.0	4.5		
LEFT	3.1	60		







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<u>ģ</u> <u>o</u> 0X Douglas Homes . Reliance Ave x, NC 21539 Bracing Floor É E Vinings CLIENT Smith I 2520 R Apex, 1 First

### CURRENT DRAWING

DATE: 10/17/2019

SCALE: 1/8"=1'-@"

PROJECT \*: 3832226R2

DRAWN BY: MEB

CHECKED BY: CNB

### ORIGINAL DRAWING

DATE PROJECT \* Ø8/Ø7/2Ø18 3832.154

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S7.0

REQ	REQUIRED BRACED WALL PANEL CONNECTIONS			
		MIN	REQUIRED C	ONNECTION
METHOD	MATERIAL	THICKNESS	# PANEL EDGES	<ul> <li>INTERMEDIATE</li> <li>SUPPORTS</li> </ul>
C5-W5P	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS 6 O.C.	6d COMMON NAILS # 12" O.C.
GB	GYPSUM BOARD	1/2"	5d COOLER NAILS # 7" O.C.	5d COOLER NAILS** # T" O.C.
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS 9 6" O.C.	6d COMMON NAILS # 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.1	PER FIGURE R602.10.1
OR EQUIVALENT PER TABLE R102.3.5				

### BRACED WALL NOTES:

- WALLE SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602/0
  FROM THE 2019 NORTH CAROLINA RESIDENTIAL CODE.

  WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND SPEEDS UP TO 190 MPH.

  REFER TO ARCHITECTURAL, PLAN FOR DOORNUNDOUL OPENNAS SIZES.

  RACKING MATERIALS, METHOOS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R607/20.

  ALL BRACED WALTERIALS, PRICES SHALL BE FILL WALL HEIGHT AND SHALL NOT EXCEED WO FEET FOR ISOLATED PANEL METHOD AND IS FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.

  MINIMUM PANEL LEDRICH SHALL BE FER TABLE R607/20.

  THE NITERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLD SHALL BE BLEATHED CONTINUOUSLY WITH MINIMUM 12" GYPSWI
- BOARD (UNO), FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE
- 8. FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHABLE BURKACES INCLUDION SHIFL AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE BND WALLS.

  9. FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL DISNIESRING CALCULATIONS.

  10. A BRACED WALL PANEL SHALL BE LOCATED WITHIN IZ FEET OF EACH BID OF A BRACED WALL INE.

  11. THE MAXIMM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEPT 21 SHEFT.

- 11. THE MAXIMM EDAE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 3 FEBT.

  12. MASONRY OR CONCRETE STEM WALLS WITH A LINGTH OF 49' OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE READ 343 OF THE 2008 NORC.

  13. BRACED WALL PANEL CONNECTIONS TO FLOORICELING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION READ 304.4

  14. BRACED WALL PANEL CONNECTIONS TO REOP SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION READ 304.5

  15. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION READ 304.0

  16. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE READ 310 (IND)

- R602.01 (UNO)

  11. ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.

  18. ABBREVIATIONS:

GB = GYPSIM BOARD WSP = WOOD STRUCTURAL PANEL C5-XXX = CONT. SHEATHED ENG = ENG. PORTAL FRAME
FF = PORTAL FRAME
FF = ENG. PORTAL FRAME

THESE FLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS HOMES COMPLETED REVISED ON SOLIDONS. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SWITHIN FACE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION SWITHIN TRANSERING, LABORATORY I TESTING, P.C. RANY CHANGES ARE HADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION SWITHIN TRANSERING, LABORATORY I TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS OF THE ARCHITECTURAL PLANS OF THE ARCHITECTURAL PLANS OF THE DATE LIGHTED ABOVE.

FIRST FLOOR BRACING (FT)				
CONTINUOUS	CONTINUOUS SHEATHING METHOD: RECTANGLE 1			
REQUIRED PROVIDED		PROVIDED		
FRONT	5.8	15.0		
RIGHT	1.4	12.00		
REAR	5.8	13.6		
LEFT	1,4	IT.3		

FIRST FLOOR BRACING (FT)				
CONTINUOUS	CONTINUOUS SHEATHING METHOD: RECTANGLE 2			
	REQUIRED	PROVIDED		
FRONT	3.0	17.5		
RIGHT	3.1	12.0		
REAR	3.0	13.6		

INSTALL HOLD-DOUNS PER SECTION R602.10.4 AND FIGURE R602.103(4) OF THE 2018 NCRC.

### STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS.

ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

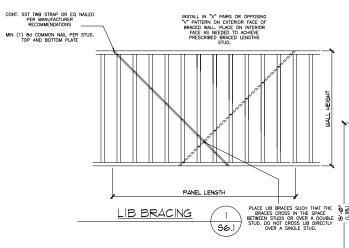
### FIRST FLOOR BRACING PLAN

SCALE: 1/8"=1"

INSTALL HOLD-DOWNS PER SECTION R602.10.4 AND FIGURE

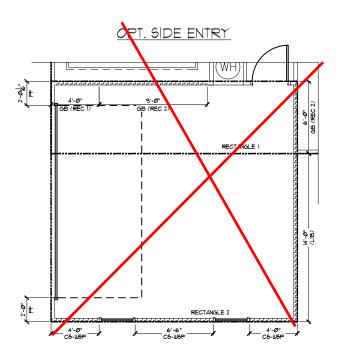
SEE SHEET ST.Ø FOR NOTES AND MORE INFORMATION

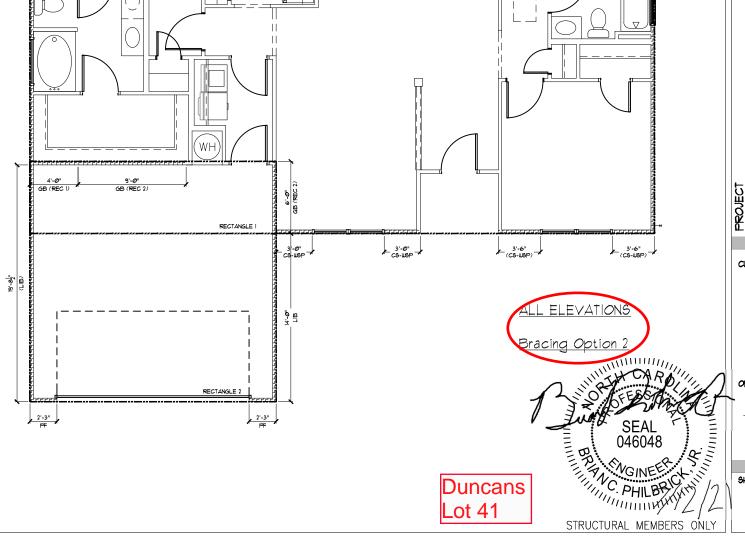
\_ 1Ø'-2½". (LIB)



REAR	,			
	<u></u>	FIRST FL	OOR BRAC	ING (FT)
HOUSE	<del>     </del>	CONTINUOUS SHEAT	THING METHOD: SIDE EI	NTRY RECTANGLE
	l 📆		REQUIRED	PROVIDED
		FRONT	5.8	15.0
FRONT		RIGHT	1.4	12
		REAR	5.8	13.6

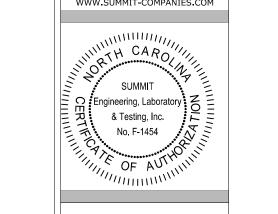
FIRST FLOOR BRACING (FT)			
CONTINUOUS SHEAT	HING METHOD: SIDE EN	NTRY RECTANGLE 2	
	REQUIRED	PROVIDED	
FRONT	3.0	27.5	
RIGHT	3.1	12	
REAR	3.0	13.6	
1	.,	140	







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9 <u>o</u> 0X Douglas Homes . Reliance Ave x, NC 21539 Bracing Floor  $\widehat{\mathbb{L}}$ Vinings Smith D 2520 A Apex, First

### CURRENT DRAWING

DATE: 10/17/2019

SCALE: 1/8"=1'-@"

PROJECT \*: 3832226R2

DRAWN BY: MEB

CHECKED BY: CNB

ORIGINAL DRAWING

DATE PROJECT \* 08/07/2018 3832.154

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S7.1

### GENERAL STRUCTURAL NOTES:

- 1. The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.
- The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- 3. The SER is not responsible for construction sequences. methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- 5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- This structure and all construction shall conform to all applicable sections of the international residential code.
- This structure and all construction shall conform to all applicable sections of the 2018 North Carolina Residential Code (NCRC) and any local codes or restrictions

### FOUNDATIONS:

- 1. Foundations shall be constructed in accordance with chapter 4 of the 2018 NC Residential Building Code (Special consideration shall be given to Chapter 45 in wind zones above 130mph)
- 2. Footing sizes based on a presumptive soil bearing capacity of 2000 PSF. Contractor is solely responsible for verifying the suitability of the site soil conditions at the time of construction
- Maximum depth of unbalanced fill against masonry walls to be as specified in section R404.1 of the 2018 NCRC
- The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- 6. Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
- 7. Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- 8. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.
- 9. Each crawl space pier shall bear in the middle third of its respective footing and each girder shall bearing in the middle third of the piers. Pilasters to be bonded to perimeter foundation wall
- 10. Crawl spaced to be graded level and clear of all debris
- 11. Provide foundation waterproofing and drain with positive slope to outlet as required by site conditions
- 12. Energy efficiency compliance and insulation of the structure to be in accordance with chapter 11 of the 2018 NCRC

- 1. Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- 2. Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- 3. Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows: 3.1. Footings: 5%
  - 3.2. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER
- 5. Concrete slabs—on—grade shall be constructed in accordance with ACI 302.1R-96: "Guide for Concrete Slab and Slab Construction".
- 6. The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions
- 7. Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
- 8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint.
- 10. All welded wire fabric (W.W.F.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The W.W.F. shall be securely supported during the concrete pour. Fibermesh may be used in lieu of W.W.F.

### CONCRETE REINFORCEMENT:

- 1. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strenath.
- 2. Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement
- 3. Application of fibermesh per cubic yard of concrete shall egual a minimum of 0.1% by volume (1.5 pounds per cubic yard)
- 4. Fibermesh shall comply with ASTM C1116, any local building code requirements, and shall meet or exceed the current industry standard.
- 5. Steel Reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- 6. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"
- Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice.
- 8. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- 9. Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

### WOOD FRAMING:

- 1. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Spruce-Pine-Fir (SPF) #2.
- 2. LVL or PSL engineered wood shall have the following minimum design values:
  - 2.1. E = 1.900.000 psi
  - 2.2. Fb = 2600 psi
  - 2.3. Fy = 285 psi
- 2.4. Fc = 700 psi3. Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in

accordance with AWPA standard C-2

- 4. Nails shall be common wire nails unless otherwise noted.
- 5. Lag screws shall conform to ANSI/ASME standard B18.2.1-1981. Lead holes for lag screws shall be in accordance with NDS specifications.
- 6. All beams shall have full bearing on supporting framing members unless otherwise noted.
- 7. Exterior and load bearing stud walls are to be 2x4 SPF#2 @16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall he continuous
- 8. Individual studs forming a column shall be attached with one 10d nail @6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be fully blocked at all floor levels to ensure proper load transfer
- 9. Multi-ply beams shall have each ply attached wth (3)10d nails @ 24" O.C.
- 10. Flitch beams and four and five ply beams shall be bolted together with (2) rows of 1/2" dia. through bolts staggered @24" O.C. w/ 2" edge distance and (2) bolts located at 6" from each end, unless noted otherwise.

- 1. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- 3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design" Specification for Metal Plate Connected Wood Trusses
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing. both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- 5. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

### WOOD STRUCTURAL PANELS:

- 1. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards.
- All structurally required wood sheathing shall bear the mark of the APA.
- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building
- 6. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

### STRUCTURAL FIBERBOARD PANELS:

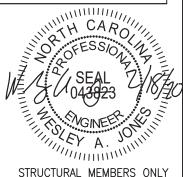
- 1. Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards
- 2. Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

### EXTERIOR WOOD FRAMED DECKS:

1. Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

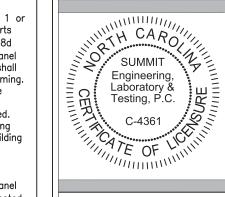
### STRUCTURAL STEEL:

- 1. Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and of the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- All steel shall have a minimum yield stress (Fy) of 36 ksi unless otherwise noted.
- Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D1.1. Electrodes for shopt and field welding shall be class E70XX. All welding shall be performed by a certified welder per the above standards.





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Specifications 21. glas Homes Trail, Suite , GA 30188 110 Village T Woodstock, ( Dougl and Notes Smith |

2

CURRENT DRAWING

Details

Standard

DATE: 2/18/20

SCALE: NTS

PROJECT #: 3832 DRAWN BY: LBV

CHECKED BY: WAI

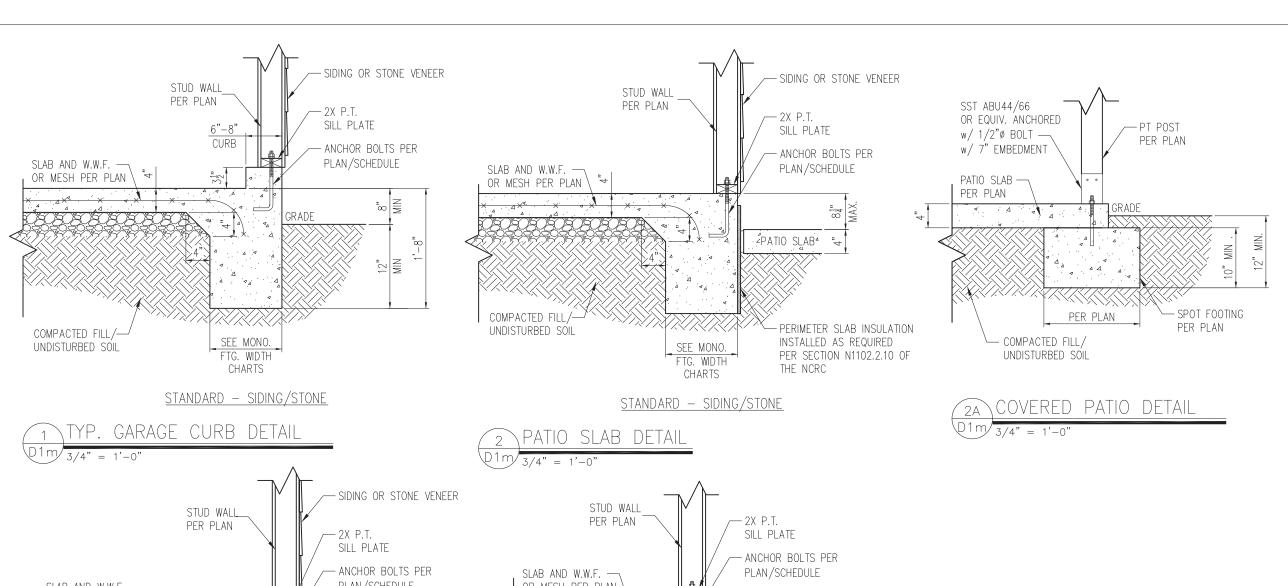
ORIGINAL DRAWING

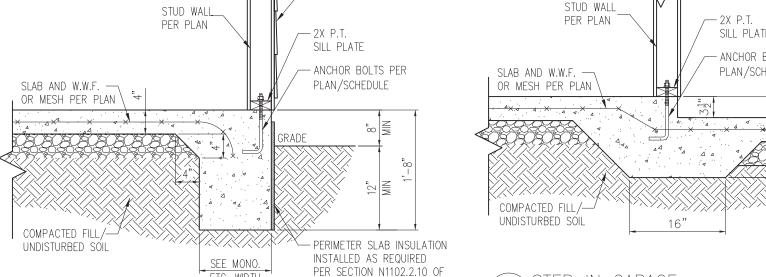
DATE PROJECT # 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

CS<sub>2</sub>





CHARTS STANDARD - SIDING/STONE

THE NCRC

TVD CLAD DETAIL	WALL ANCHOR SCHEDULE
3 TYP. SLAB DETAIL	TYPE OF ANCHOR
$01m\sqrt{3/4"} = 1'-0"$	
	1/2"ø A307 BOLTS w/
TES:	STD. 90° BEND
	II

FTG. WIDTH

- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.

WALL ANCHOR SCHEDULE				
TYPE OF ANCHOR	MIN. CONC.	SPACING	INTERIOR	EXTERIOR
	EMBEDMENT	EMBEDMENT	WALL	WALL
1/2"ø A307 BOLTS w/	7"	6'-0"	YES	YES
STD. 90° BEND				
SST - MAS	4"	5'-0"	NO	YES
HILTI KWIK BOLT KBI 1/2-2-3/4	2-1/4"	6'-0"	YES	NO
1/2"ø HILTI THREADED ROD	7"	6'-0"	YES	YES
w/ HIT HY150 ADHESIVE				

NOTE: INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.

### MONOLITHIC FOOTING WIDTH

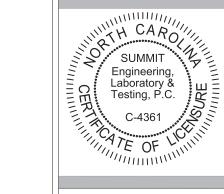
# OF STORIES	WIDTH BASED ON SOIL BEARING CAPACITY				
	1500 PSF	2000 PSF	2500 PSF		
1 STORY - STD.	16"	16"	16"		
1 STORY - BRICK VENEER	21"*	21"*	21"*		
2 STORY - STD.	20"	16"	16"		
2 STORY - BRICK VENEER	25"*	21"*	21"*		
*5" BRICK LEDGE HAS BEEN ADDED TO THE MONOLITHIC					

FOOTING WIDTH FOR BRICK SUPPORT





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2 21 Slab Details Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Standard Details Monolithic

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PRO1ECT # · 3832

DRAWN BY: LBV

CHECKED BY: WAJ

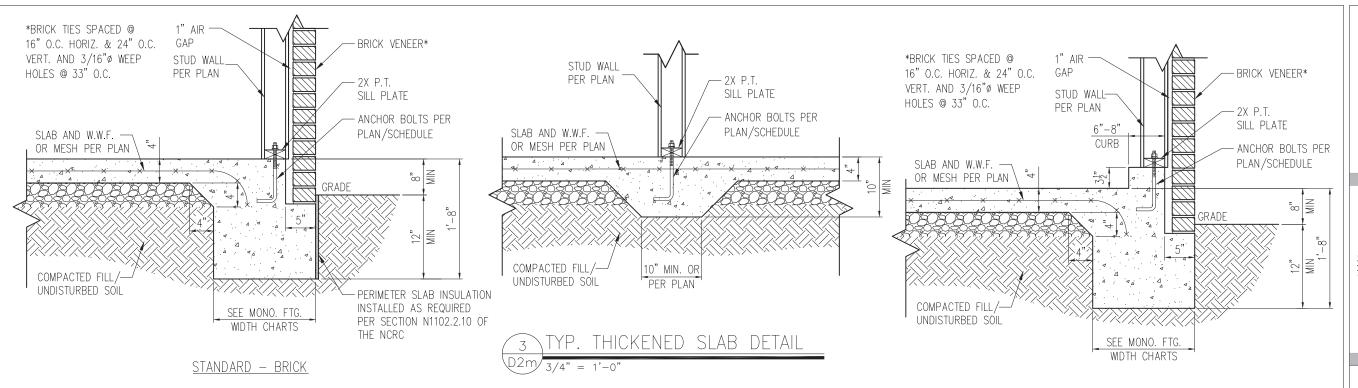
ORIGINAL DRAWING

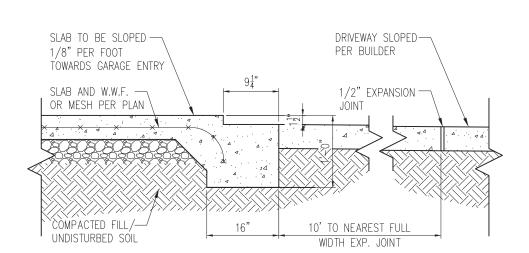
DATE PROJECT # 1/7/16

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

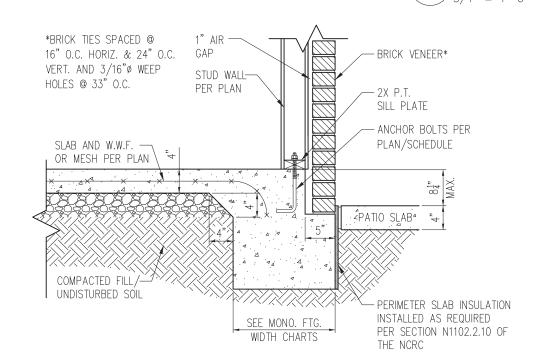
D<sub>1</sub>m





YP. SLAB DETAIL W/ BRICK VENEER



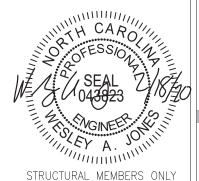


STANDARD - BRICK

PATIO SLAB DETAIL W/BRICK VENEER

### NOTES

- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.



STANDARD - BRICK

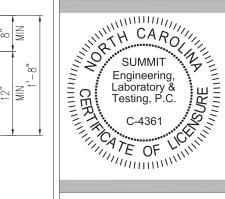
TYP. GARAGE CURB DETAIL

W/ BRICK VENEER

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OFFICE: 919.380.9991

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# Standard Details Monolithic Slab Details Smith Douglas Homes 110 Village Trail, Suite 215 Woodstock, GA 30188

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PROJECT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

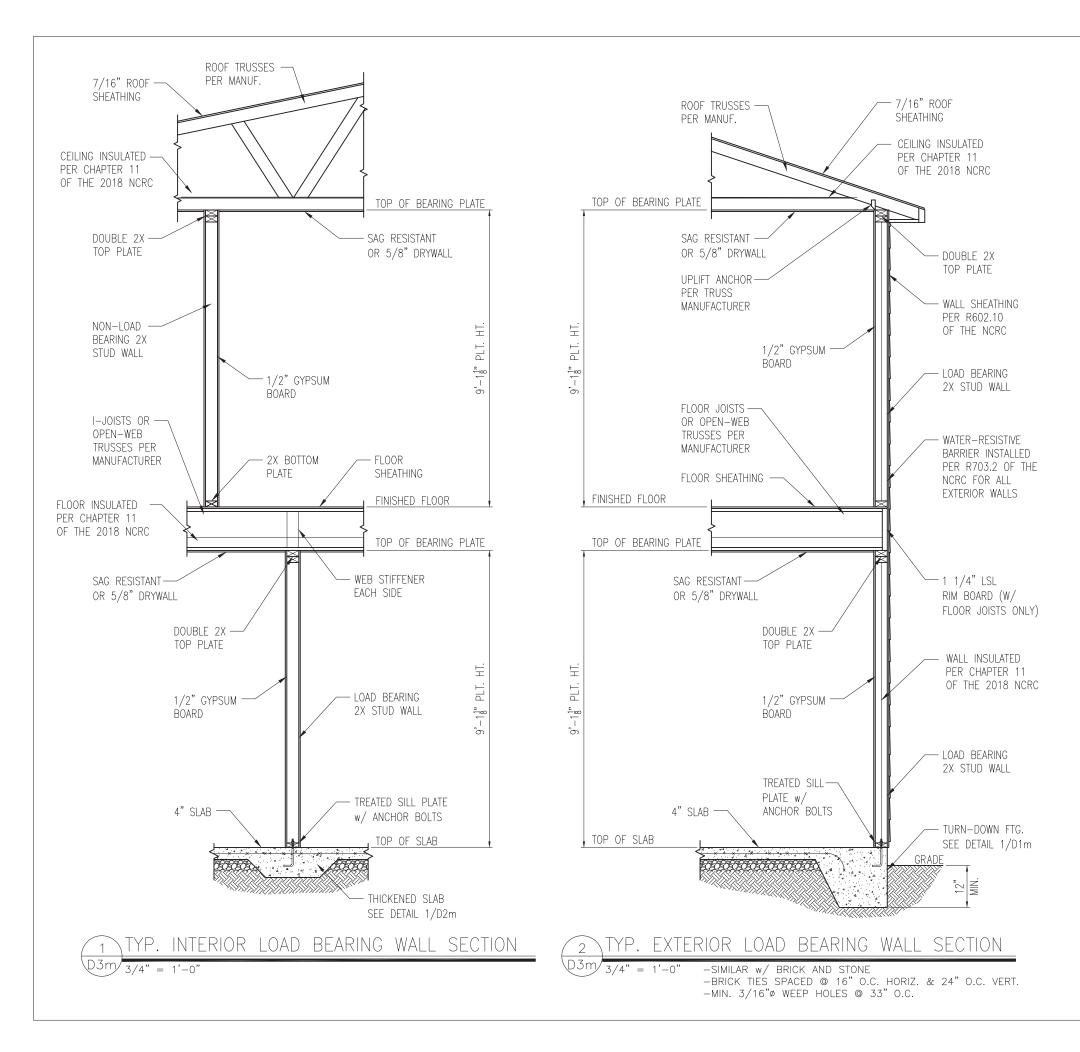
ORIGINAL DRAWING

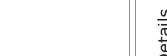
NO. DATE PROJECT # 0 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

D<sub>2</sub>m





Slab Details Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Standard Details Monolithic

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2

21

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CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PRO1FCT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

ORIGINAL DRAWING

DATE PROJECT # 1/7/16

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

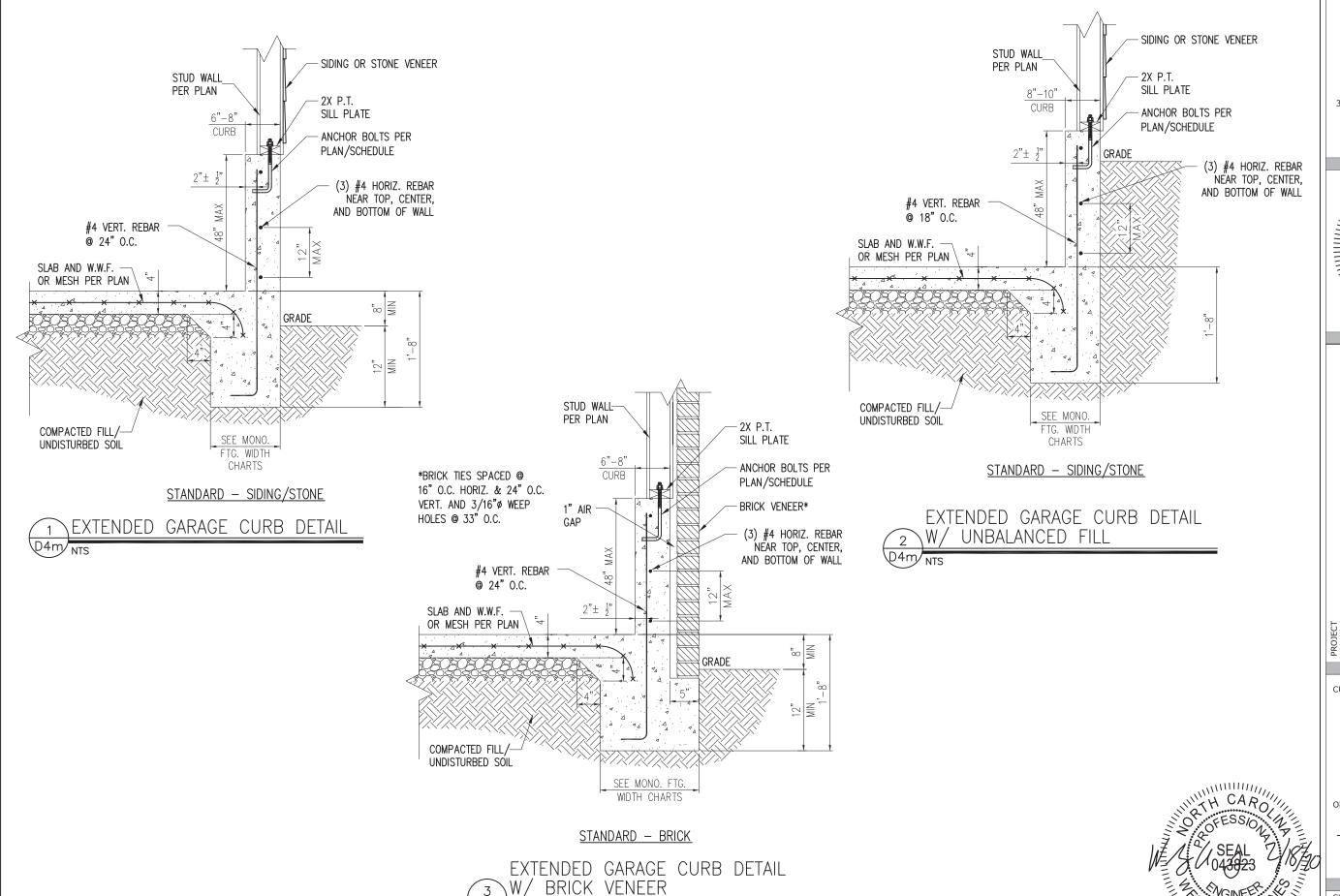
D<sub>3</sub>m

NOTES:
1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.

2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.

3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.

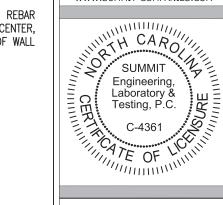




D4m/NTS



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Standard Details

Monolithic Slab Details

Smith Douglas Homes
110 Village Trail, Suite 21!
Woodstock, GA 30188

2

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PROJECT #: 3832

DRAWN BY: LBV

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ORIGINAL DRAWING

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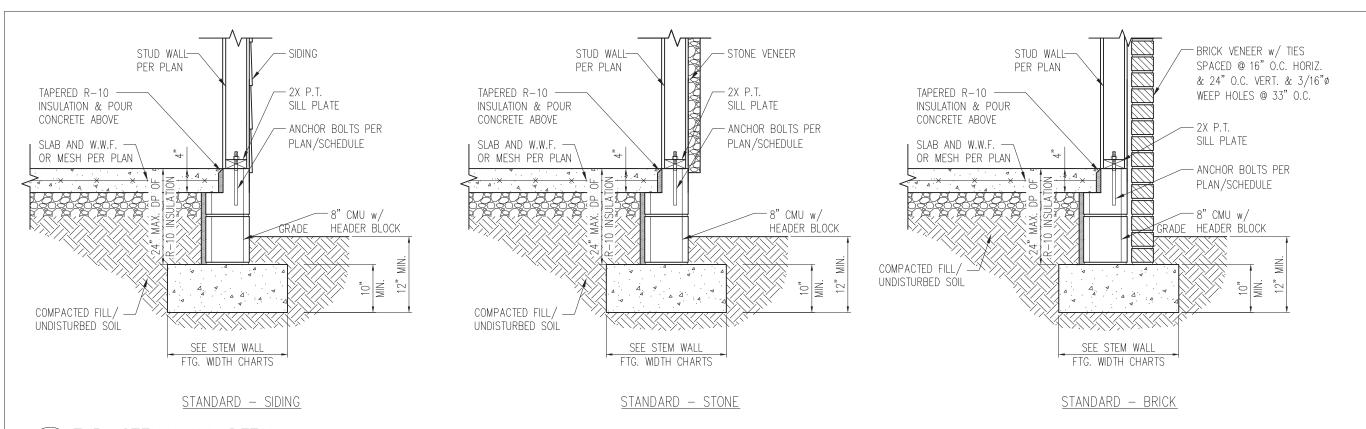
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

THEY A. JOHN

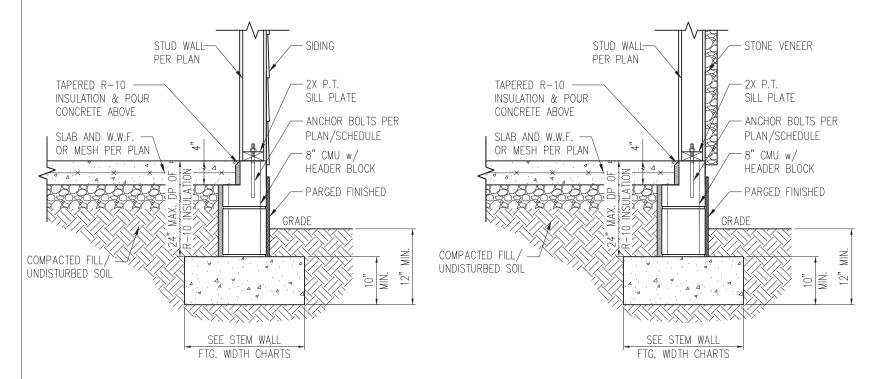
STRUCTURAL MEMBERS ONLY

D4m



STANDARD - STONE

# TYP. STEM WALL DETAIL D1s 3/4" = 1'-0"



STANDARD - SIDING

1a STEM WALL DETAIL W/ PARGED FINISH

3/4" = 1'-0"

### STEM WALL FOOTING WIDTH

# OF STORIES	WIDTH BASED ON SOIL BEARING CAPACITY				
	1500 PSF	2000 PSF	2500 PSF		
1 STORY - STD.	16"	16"	16"		
1 STORY - BRICK VENEER	21"*	21"*	21"*		
2 STORY - STD.	20"	16"	16"		
2 STORY - BRICK VENEER	25"*	21"*	21"*		
*5" BRICK LEDGE HAS BEEN ADDED TO THE STEM WALL					
FOOTING WIDTH FOR BRICK SUPPORT					

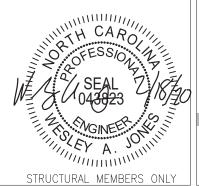
### WALL ANCHOR SCHEDULE

TYPE OF ANCHOR	MIN. CONC.	SPACING	INTERIOR	EXTERIOR
	EMBEDMENT	EMBEDMENT	WALL	WALL
1/2"ø A307 BOLTS w/	7"	6'-0"	YES	YES
STD. 90° BEND				
SST - MAS	4"	5'-0"	NO	YES
HILTI KWIK BOLT KBI 1/2-2-3/4	2-1/4"	6'-0"	YES	NO
1/2"ø HILTI THREADED ROD	7"	6'-0"	YES	YES
w/ HIT HY150 ADHESIVE				

NOTE: INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.

### NOTES

- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPS AND DEPRESSIONS.

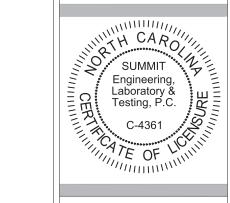


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Standard Details
Stemwall Details
Smith Douglas Homes
110 Village Trail, Suite 21!
Woodstock, GA 30188

2

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PROJECT #: 3832

DRAWN BY: LBV
CHECKED BY: WAJ

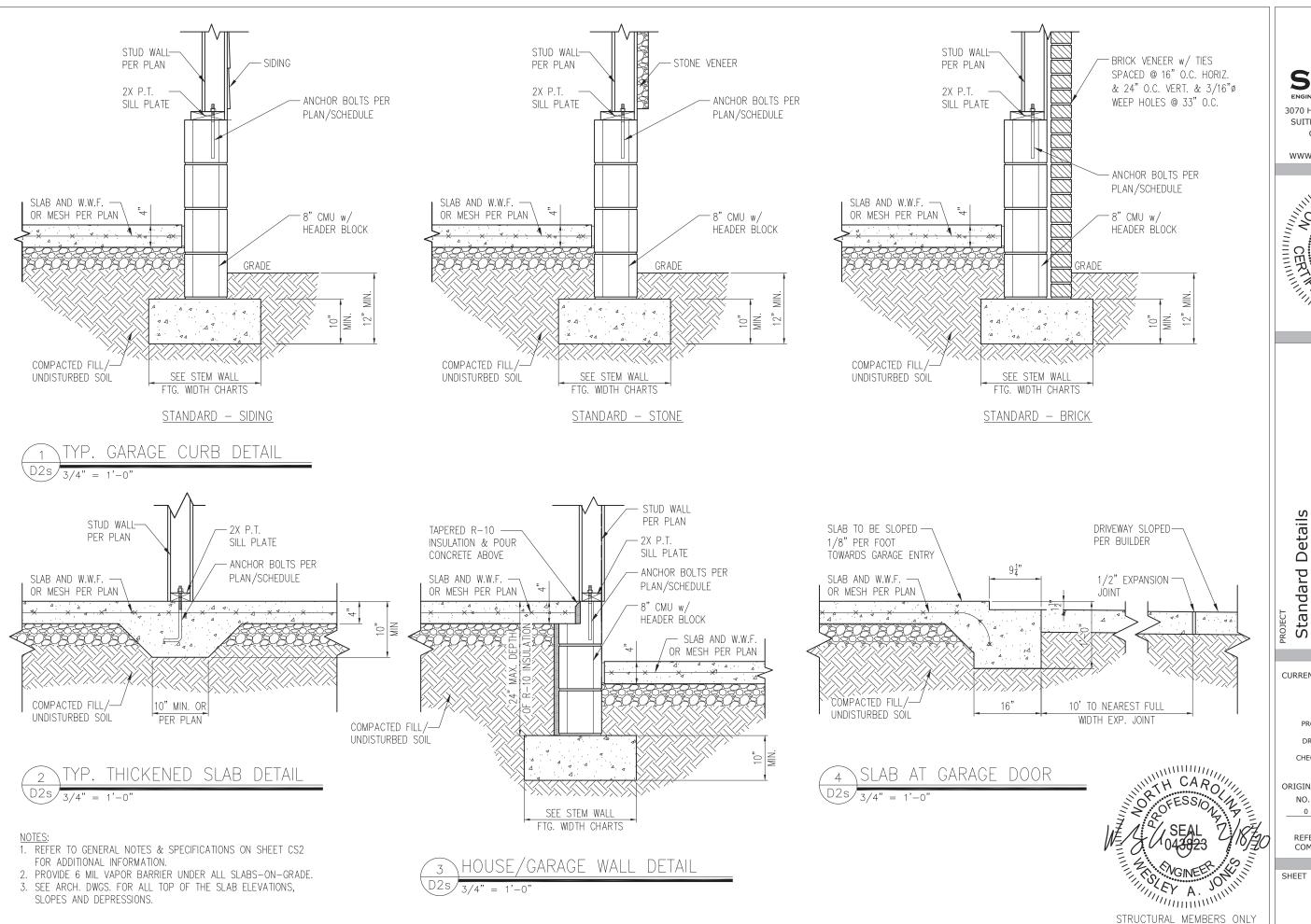
ORIGINAL DRAWING

NO. DATE PROJECT #
0 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

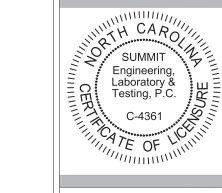
SHEET

D1s





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2 21 Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Stemwall Details

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PRO1ECT # · 3832

DRAWN BY: LBV

CHECKED BY: WAJ

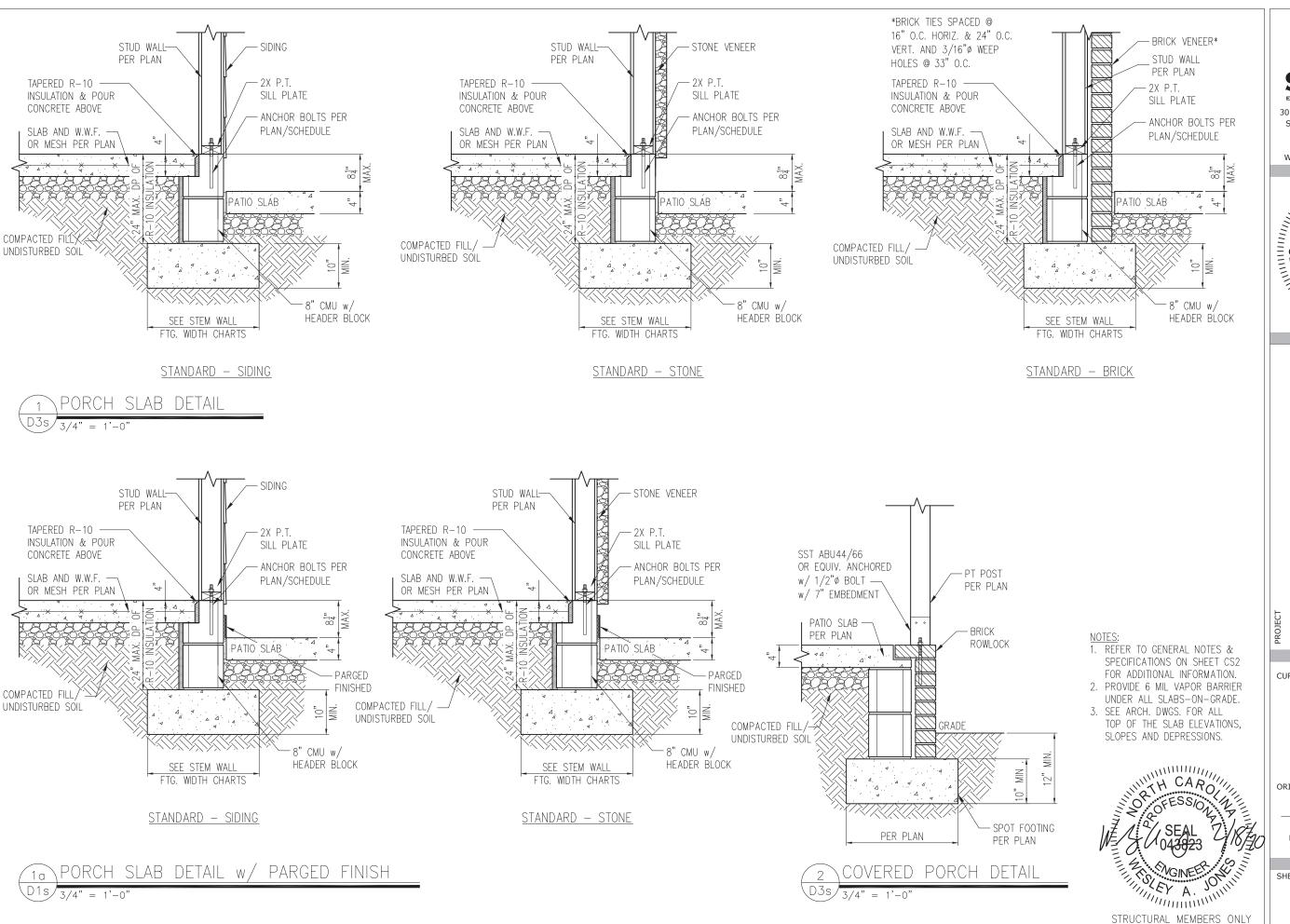
ORIGINAL DRAWING

DATE PROJECT # 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

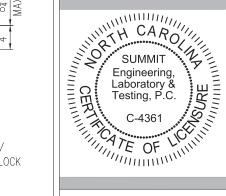
SHEET

D2s





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21 Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Stemwall Details Standard Details

2

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PRO1FCT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

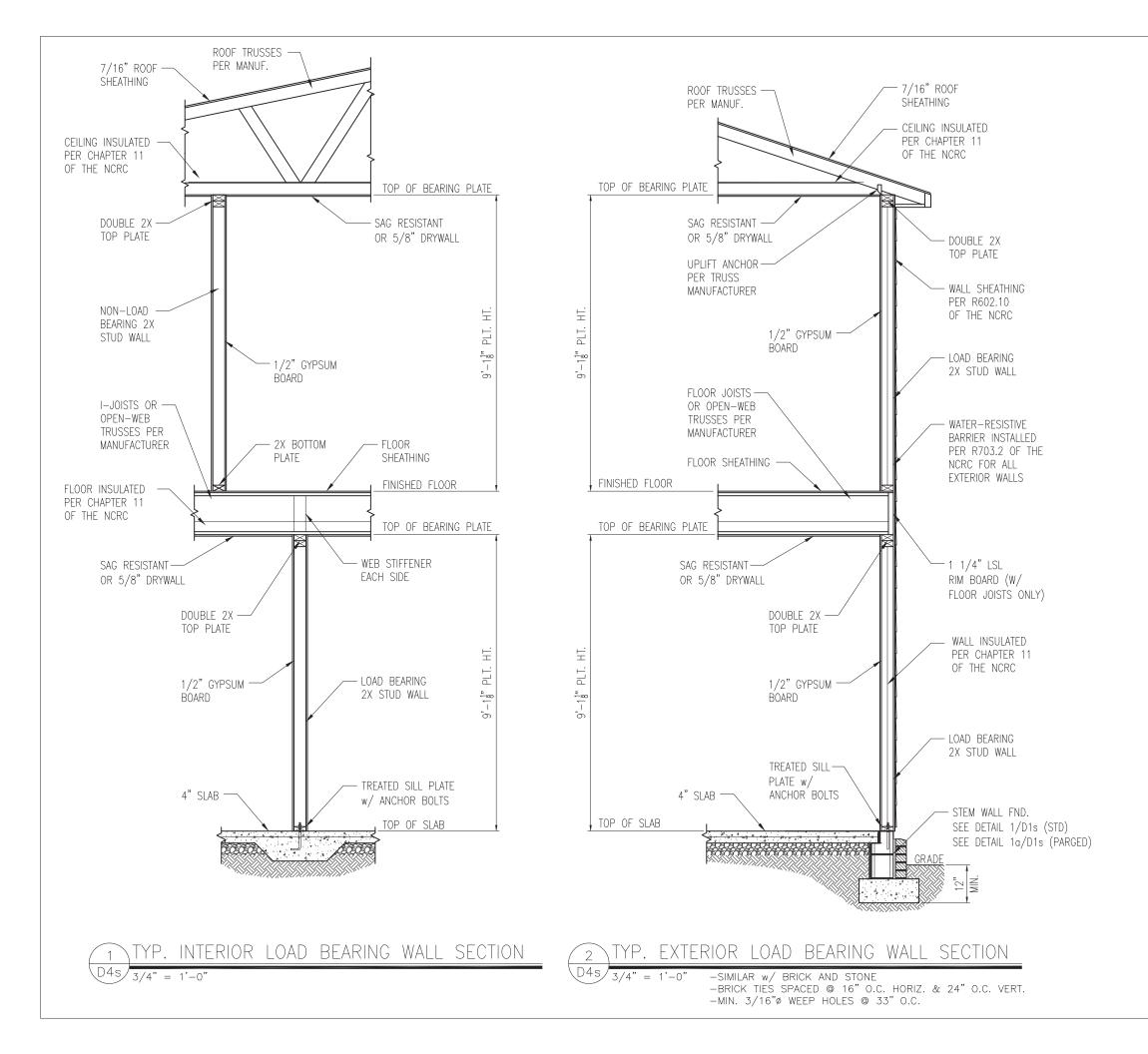
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DATE PROJECT # 1/7/16

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

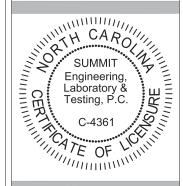
SHEET

D3s





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21 Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Stemwall Details

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CURRENT DRAWING

Standard Details

DATE: 2/18/20

SCALE: NTS

PRO1ECT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

ORIGINAL DRAWING

DATE PROJECT # 1/7/16 3832

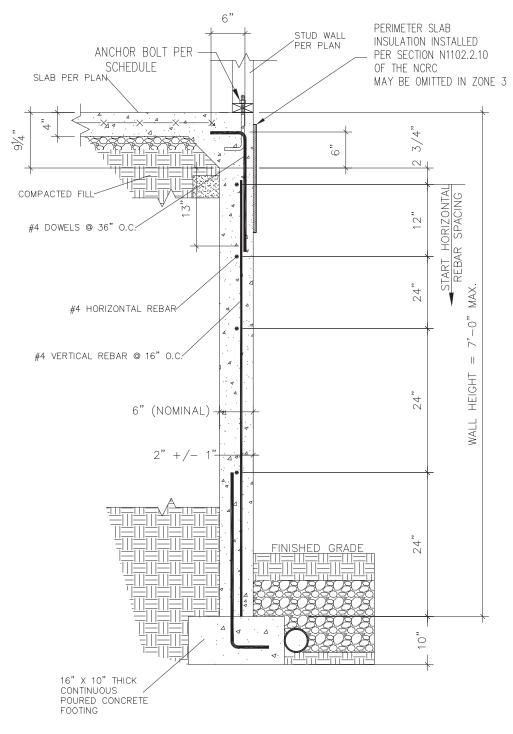
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

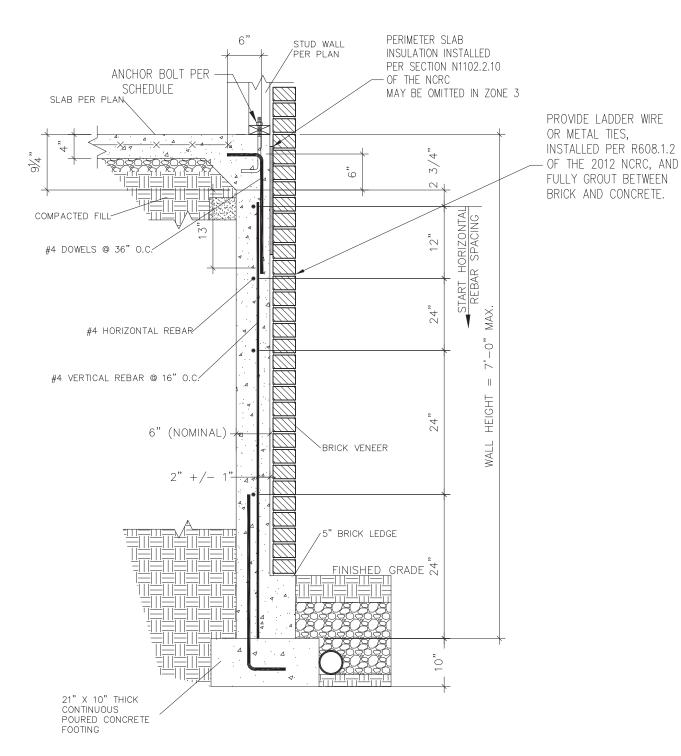
D4s

- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.









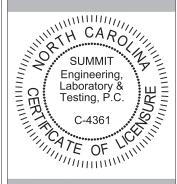
SUBWALL FOUNDATION W/ BRICK VENEER

3/4" = 1'-0"





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Stemwall Details

Smith Douglas Homes
110 Village Trail, Suite 2
Woodstock, GA 30188

2

21

### CURRENT DRAWING

Standard Details

DATE: 2/18/20

SCALE: NTS

PROJECT #: 3832

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CHECKED BY: WAJ

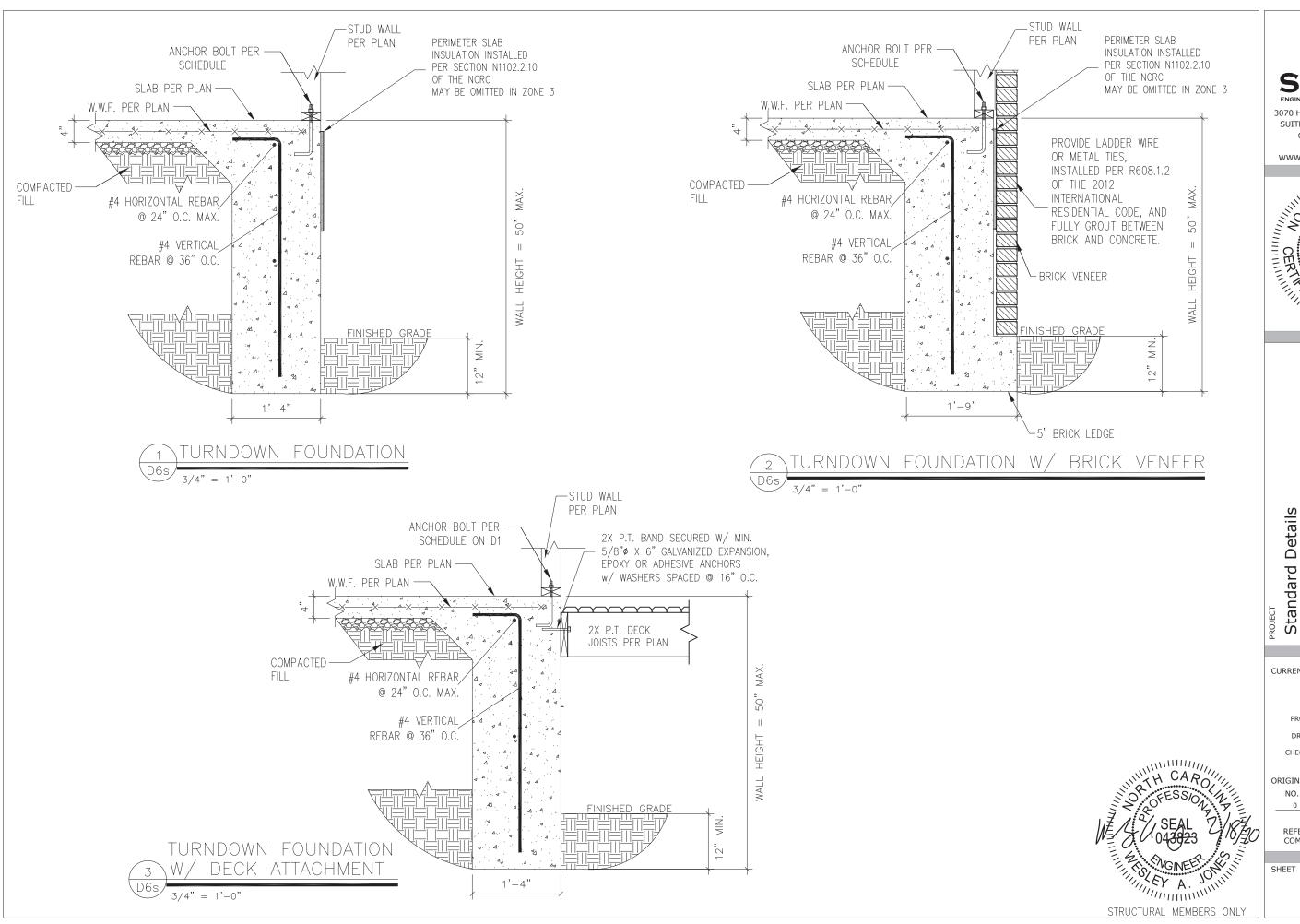
### ORIGINAL DRAWING

NO. DATE PROJECT # 0 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

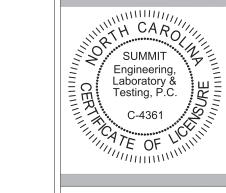
SHEET

D5s





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21 Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Stemwall Details

2

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DATE: 2/18/20

SCALE: NTS

PRO1ECT #: 3832

DRAWN BY: LBV

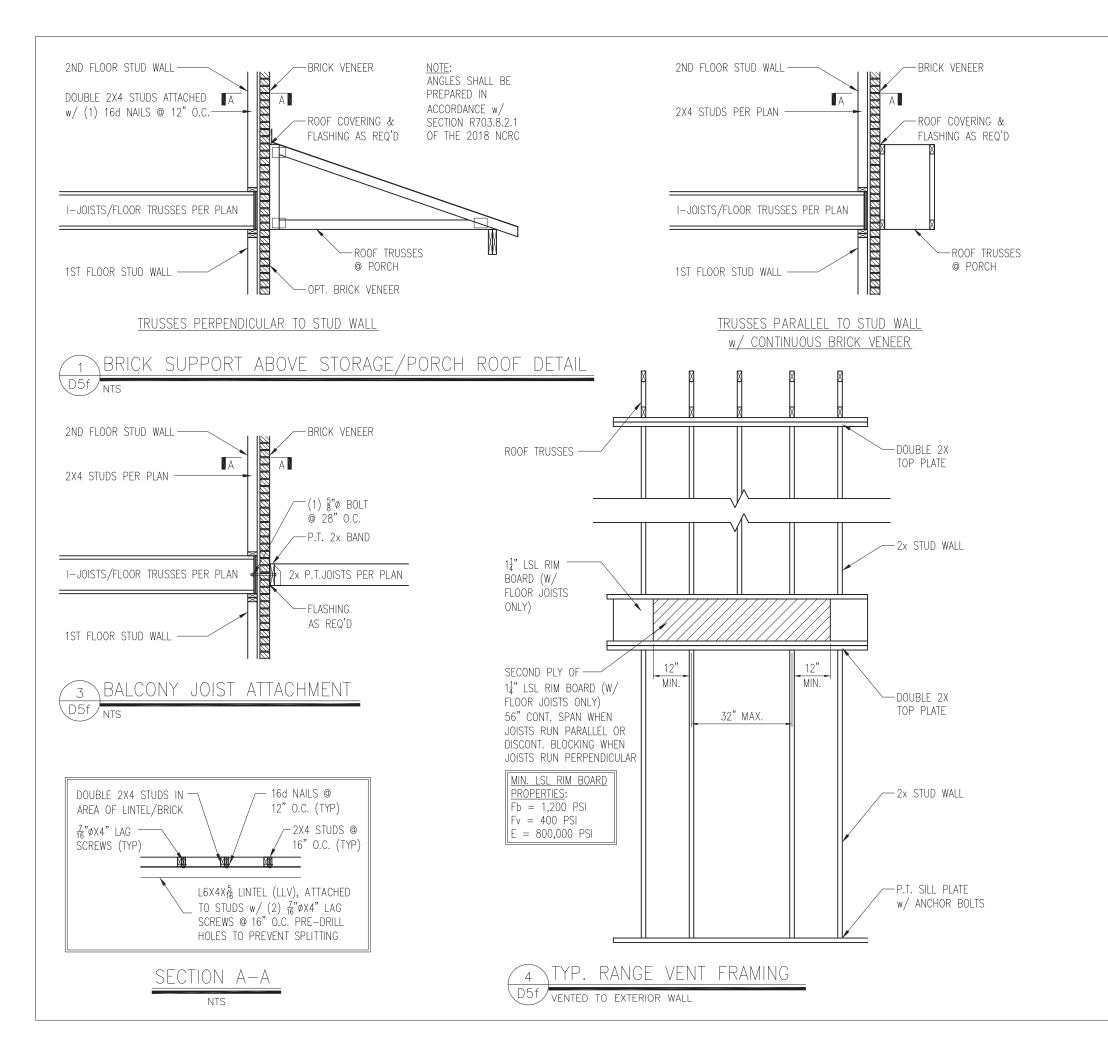
CHECKED BY: WAJ

ORIGINAL DRAWING

DATE PROJECT # 1/7/16 3832

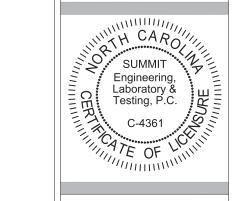
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D6s





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Standard Details
Framing Details
Smith Douglas Homes
110 Village Trail, Suite 21:
Woodstock, GA 30188

2

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS
PROJECT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

ORIGINAL DRAWING

O. DATE PROJECT # 0 1/7/16 3832

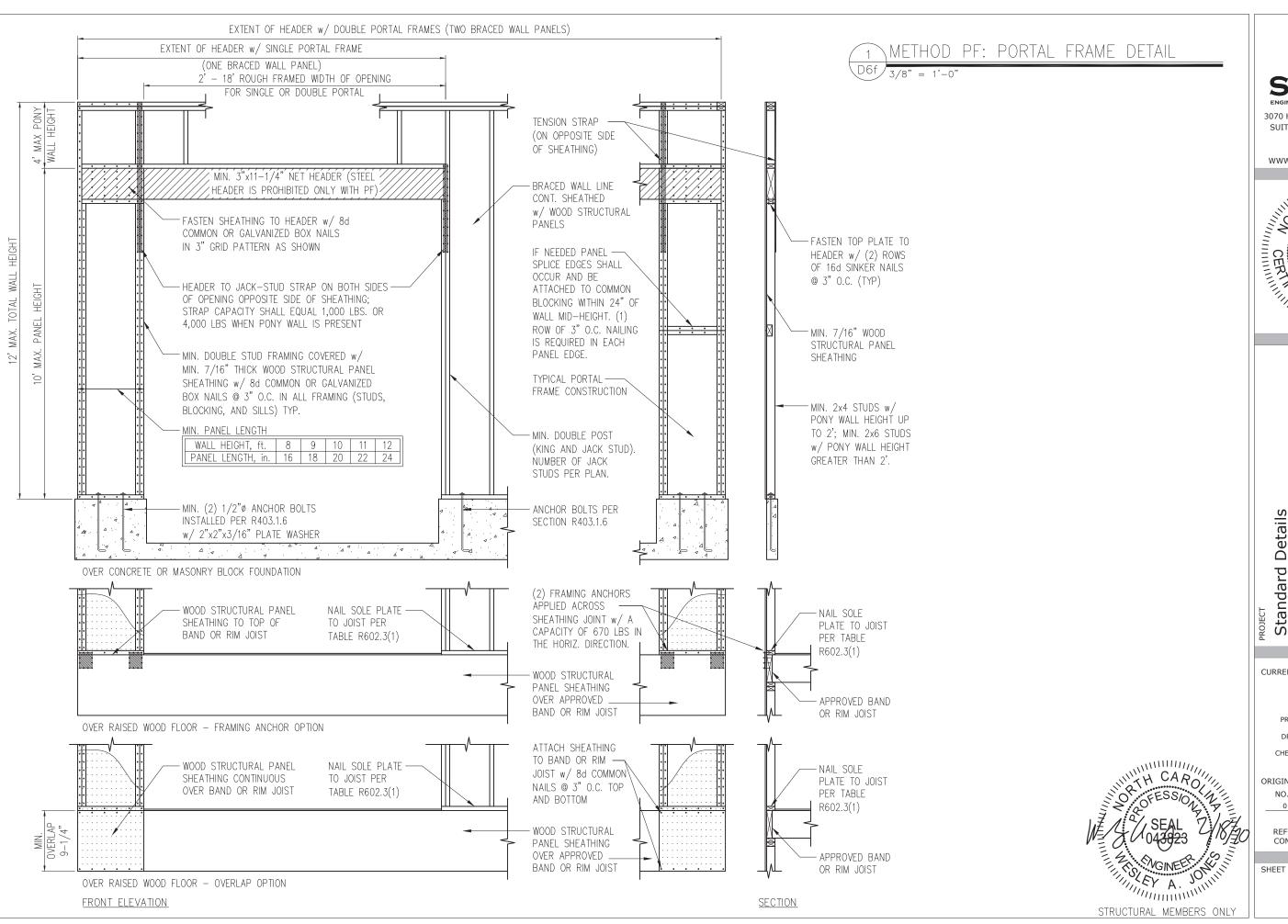
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

THEY A. JOHN

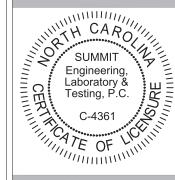
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D5f





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# Bracing 2 21 glas Homes Trail, Suite 7, GA 30188 Details Details Smith Dougla 110 Village T Woodstock, C Framing Standard

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PRO1ECT # · 3832

DRAWN BY: LBV

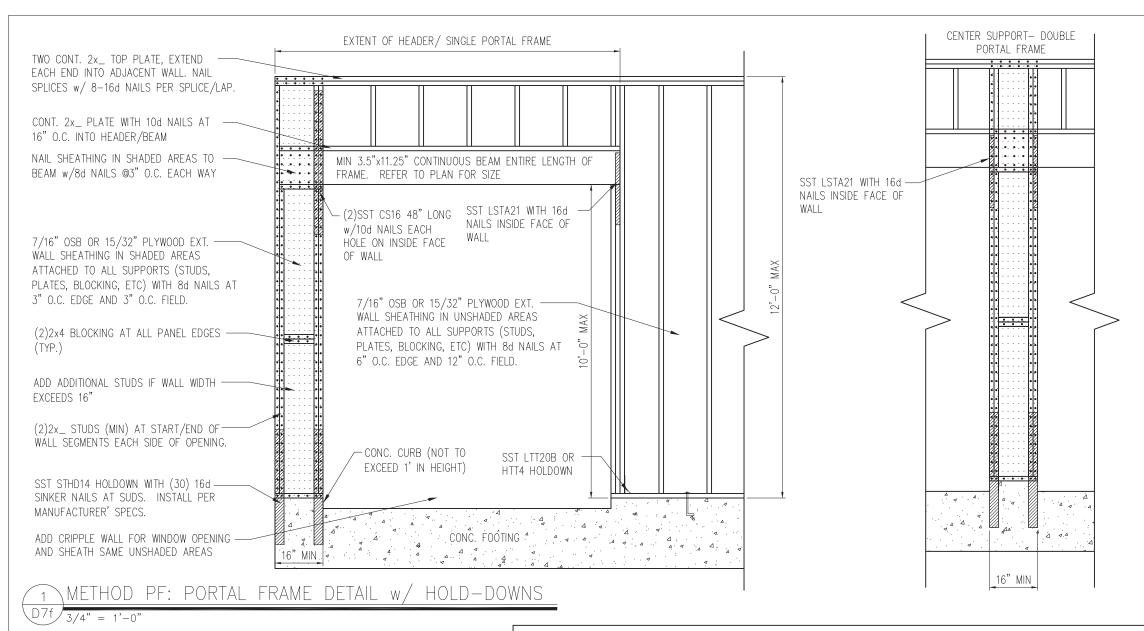
CHECKED BY: WAJ

ORIGINAL DRAWING

DATE PROJECT # 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

D6f



**ELEVATION VIEW** 

MULTI-PLY BEAM CONNECTION DETAIL

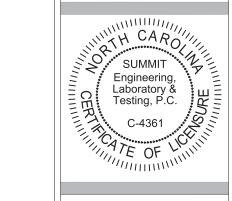
MINIMUM FASTENING REQUIREMENTS FOR TOP- AND SIDE-LOADED MEMBERS		3¹/₂″ WIDE	51/4" WIDE		7" WIDE			
FASTENER TYPE	LVLDEPTH	2-Ply 13/4"	3-Ply 13/4"	13/4" + 31/2"	4-Ply 13/4"	2-Ply 13/4" + 31/2"	2-Ply 31/2"	
10d (0.128" x 3")	7¼"≤d<14"	3 rows @ 12" o.c.	3 rows @ 12" o.c. (ES)	3 rows @ 12" o.c.		3 rows @ 12" o.c. (ES)	-	
Nails	d≥14″	4 rows @ 12" o.c.	4 rows @ 12" o.c. (ES)	4 rows @ 12" o.c.		4 rows @ 12" o.c. (ES)	-	
16d (0.162" x 31/2")	7¼"≤d<14"	2 rows @ 12" o.c.	2 rows @ 12" o.c. (ES)	2 rows @ 12" o.c.	-	2 rows @ 12" o.c. (ES)	-	
Nails	d≥14″	3 rows @ 12" o.c.	3 rows @ 12" o.c. (ES)	3 rows @ 12" o.c.	-	3 rows @ 12" o.c. (ES)	-	
½" Through Bolts		2 rows @ 24" o.c.	2 row	2 rows @ 24" o.c.		2 rows @ 24" o.c.		
SDS 1/4" x 31/2", WS35, 33/6" TrussLok	3.71///	2 rows @ 24" o.c.	2 rows @ 24" o.c. <b>(ES)</b>	2 rows @ 24" o.c.	-	2 rows @ 24" o.c. (ES)	-	
SDS 1/4" x 6", WS6	d≥7¼″	-	2 rows @ 24" o.c.		2 rows @ 24" o.c. (ES)			
5" TrussLok		-			-			
6¾" TrussLok	6¾″ TrussLok		-		2 rows @ 24" o.c.			

### NOTES:

- 1. All fasteners must meet the minimum requirements in the table above. Side-loaded multiple-ply members must meet the minimum fastening and side-loading capacity requirements given on page 48.
- . Minimum fastening requirements for depths less than  $7\frac{1}{4}$ " require special consideration. Please contact your technical representative.
- Three general rules for staggering or offsetting for a certain fastener schedule:
   if staggering or offsetting is not referenced, then none is required;
- (2) if staggering is referenced, then fasteners installed in adjacent rows on the front side are to be staggered up to one-half the o.c. spacing, but maintaining the fastener clearances above: and
- (3) if "ES" is referenced, then the fastener schedule must be repeated on each side, with the fasteners on the back side offset up to one-half the o.c. spacing of the front side (whether or not it is staggered).



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Standard Details
Framing Details - Bracing
Smith Douglas Homes
110 Village Trail, Suite 215
Woodstock, GA 30188

CURRENT DRAWING

DATE: 2/18/20 SCALE: NTS

PROJECT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

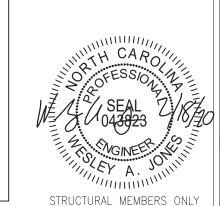
ORIGINAL DRAWING

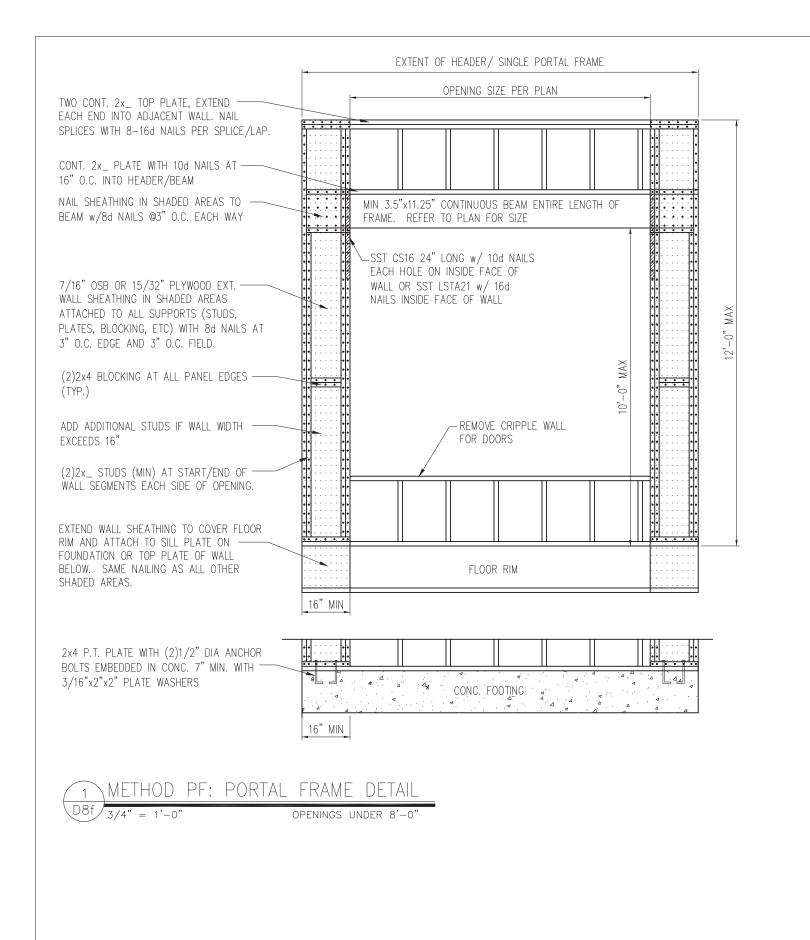
NO. DATE PROJECT # 0 1/7/16 3832

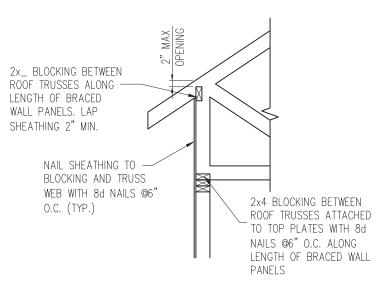
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

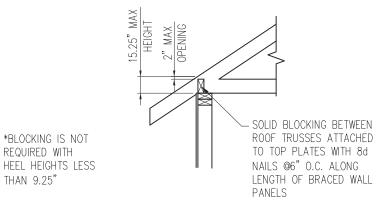
D7f







### HEEL HEIGHT GREATER THAN 15.25"

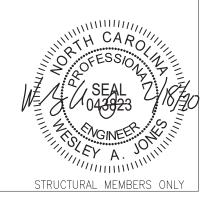


HEEL HEIGHT LESS THAN 15.25" \*

YP. WALL PANEL TO ROOF TRUSS CONNECTION

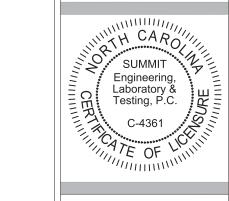
REQUIRED WITH

THAN 9.25"





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- Bracing 2 21 Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Details Standard Details Framing

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PRO1ECT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

ORIGINAL DRAWING

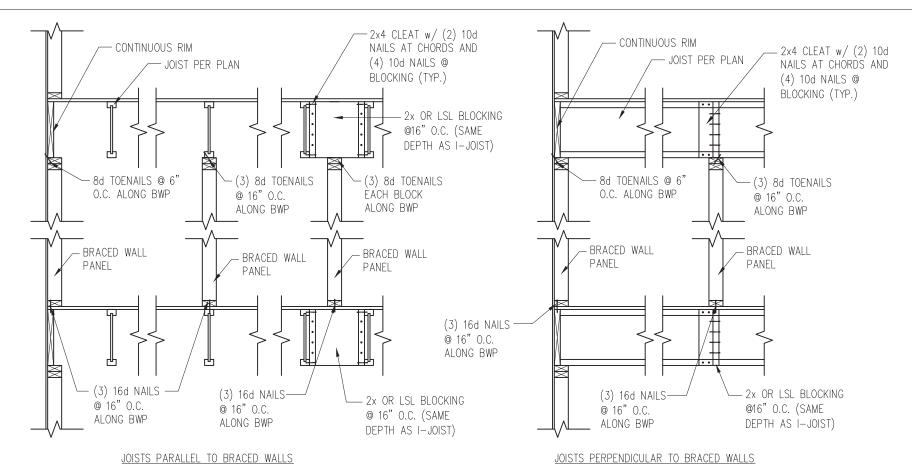
DATE PROJECT #

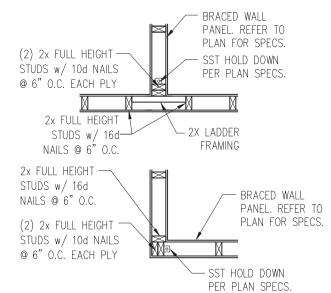
1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

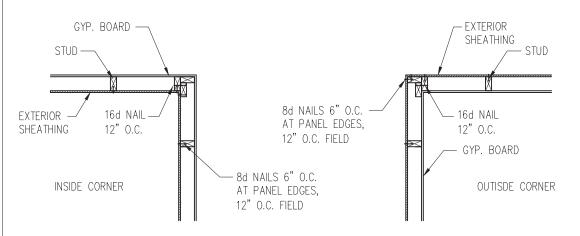
D8f

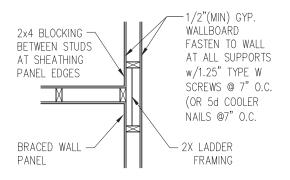




TYP. HOLD DOWN DETAIL
D9f

# 1 TYP. WALL PANEL TO FLOOR/CEILING CONNECTION D9f/1" = 1'-0"

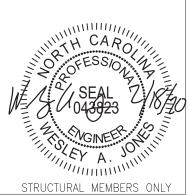




TYP. EXTERIOR CORNER FRAMING

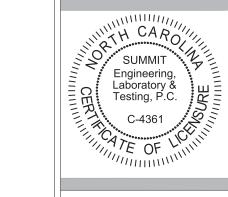
3 INTERIOR 3-STUD WALL INTERSECTION

D9f 1" = 1'-0"





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# Standard Details Framing Details - Bracing Smith Douglas Homes 110 Village Trail, Suite 215 Woodstock, GA 30188

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PROJECT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

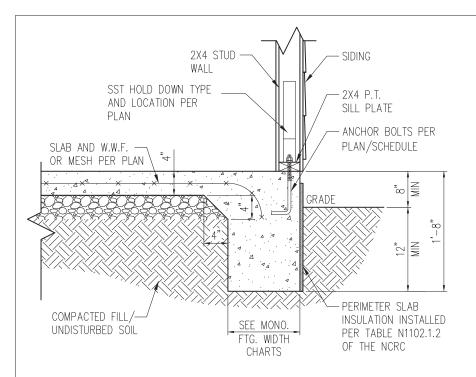
ORIGINAL DRAWING

NO. DATE PROJECT # 0 1/7/16 3832

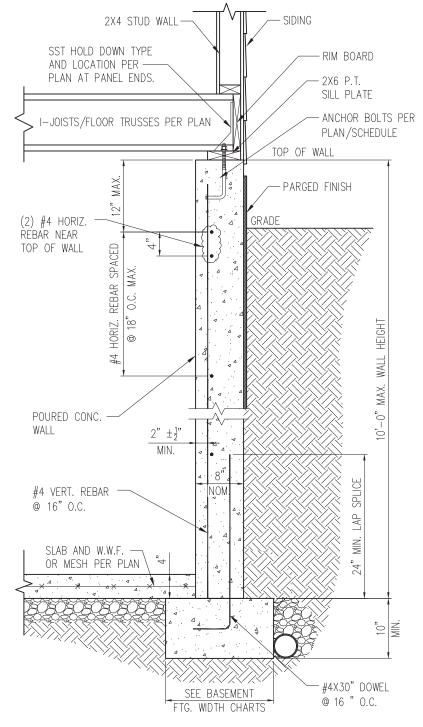
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

D9f

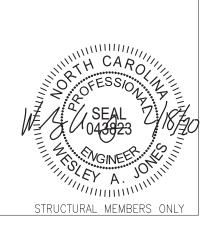


SLAB DETAIL w/ HOLD-DOWN



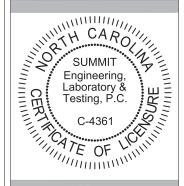
STANDARD - SIDING

BASEMENT FOUNDATION WALL DETAIL W/ HOLD-DOWN





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- Bracing 21 Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Framing Details Standard Details

2

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PRO1ECT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

ORIGINAL DRAWING

DATE PROJECT # 1/7/16 3832

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SHEET

D10f

