

(BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute



component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



































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Job	Truss		Truss Type			Qty	Qty Ply PROFESSIONAL\PLAN # 6 THE RAI					6 THE RALEIGI	H ROOF		
72333198	V1		Truss			3	3	1	Job Reference (optional)						
UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Micah Clayton Run: 8.86 S 8.62 Sep 22 2022 Print: 8.600 S Sep 22 2022 MiTek Industries, Inc. Wed Sep 13 12:42:52 Page:											Page: 1				
							<u>1-2-2</u> 1-2-2	2 2003300000 2-0-1 0-10-0 0	2-4-3	viygze	y-03598	48000		<u>nv_ràdeaux</u>	BHGye2VII
				0-8-5	-0- 4		2 3x4 2	3x4 = 2 B1 B1 3x4	*						
						1	:	2-4-3	\rightarrow						
Plate Offsets (X, Y): [2:	0-2-0,Edg	e]													
Loading TCLL (roof) TCDL BCLL BCDL	(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	IRC20	2-0-0 1.15 1.15 YES 15/TPI2014	CSI TC BC WB Matrix-MP	0.04 0.04 0.00	DEF Vert(Vert(Horiz	L (LL) (TL) z(TL)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 7 lb	GRIP 244/190 FT = 20%	
TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 REACTIONS (lb/siz Max I FORCES 1) Unbalanced roof live load 2) Wind: ASCE 7-10; Vult=1 exterior zone and C-C Ex for reactions shown; Lum 3) Gable requires continuou 4) This truss has been desig 5) * This truss has been desig 5) * This truss has been desig 5) * This truss is designed in TPI 1.	2 ze) 1= Horiz 1= Uplift 1= (lb) - Max ds have be 30mph (3 kterior (2) z is bottom (2) gned for a signed for a signed for a cordance accordance	=94/2-4-3, (min. 0-1-8), 5 =-21 (LC 8) =-12 (LC 10), 3=-12 (LC & Comp./Max. Ten All even considered for this c -second gust) Vasd=10 zone; cantilever left and 1.60 plate grip DOL=1.6 chord bearing. 10.0 psf bottom chord I a live load of 20.0psf or mibers. • others) of truss to bear the with the 2015 Internation with the 2015 International second s	3=94/2-4-3, (m 11) forces 250 (lb lesign. 3mph; TCDL=1 right exposed 30 ive load nonco n the bottom ch ing plate capal tional Residen	in. 0-1-8)) or less exce 5.0psf; BCDI ; end vertica ncurrent with ord in all are ole of withsta tial Code sec	ept when shown. =6.0psf; h=35ft; Cat. II I left and right exposed any other live loads. as where a rectangle 3 inding 12 lb uplift at join tions R502.11.1 and R	; Exp B; ; C-C for -06-00 ta t1 and 1 802.10.2	RD RD	St Ri sed; MWFR vers and for 2-00-00 wide uplift at joint eferenced s	ructural w gid ceiling S (envelo ces & MV e will fit b 3. standard <i>i</i>	vood shin g direct! vFRS etween ANSI/	pathing c y applied	lirectly lor 10-	applied or 2-4-3 oc 0-0 oc bracing.	AROLINA AL 1010111 AL 1954 / AL	
This design is based upon para component is responsibility of th governing codes and ordinance truss is fabricated by a UFPI pla (BCSI) for general guidance reg	meters sh he Building es. Buildin ant. Bracii garding sto	own, and is for an indivi g Designer. Building De g Designer accepts resp ng shown is for lateral s orage, erection and brac	idual building o esigner shall ve ponsibility for t upport of truss sing available f	component to prify all designed the correctne members or rom SBCA a	be installed and loade n information on this sh ss or accuracy of the de nly and does not replace nd Truss Plate Institute	d vertica leet for c esign info e erectio	illy. Ap conforr ormation and	pplicability on ance with on as it may permanent	of design condition y relate to bracing.	parame s and re a spec Refer to	ters and quireme ific buildi o Building	proper nts of ti ng. Ce g Comp	incorporation of he specific building rtification is valid or ponent Safety Infor	and hly when mation	围

Job	Truss	Truss Type		Qty	Ply	PROFESSIO	NAL\PLAN #	6 THE RALEIGH	I ROOF		
72333198	V2	Truss		3	1	Job Reference	ce (optional)				
UFP Mid Atlantic LLC, 5631 S. N	IC 62, Burlington, NC, Micah Clay	rton	Run: 8.86 S 8.62 Sep 2	22 2022 Pr	rint: 8.620 S S	ep 22 2022 MiTe	ek Industries, In	c. Wed Sep 13 12:4	2:52 Page: 1		
ID:cJyxz6Q3S0hflYG6zOATvTyg2Sy-85S9a48OOuNjUxcrQ3?oJeL2jHIPLyqe5mXBHGye2vn											
			<u>}</u>	<u>2-4-8</u> 2-4-8		4-4-14 2-0-6	4-9-0 				
			12	τ.	3x4= 2	X					
	2-0-0		10 ⊢ 1		B1		3				
			3x4	4		3x4	! ∢				
			×		4-9-0		\rightarrow				
Plate Offsets (X, Y): [2:	0-2-0,Edge]		I								
Loading TCLL (roof) TCDL BCLL BCDL	(psf)Spacing20.0Plate Grip DOL10.0Lumber DOL0.0*Rep Stress Incr10.0Code	2-0-0 1.15 1.15 YES IRC2015/TPI2014	CSI TC BC WB Matrix-MP	0.16 Ve 0.14 Ve 0.00 Hc	E FL ert(LL) ert(TL) oriz(TL)	in (loc) n/a - n/a - 0.01 3	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 15 lb	GRIP 244/190 FT = 20%		
LUMER i I of PC HORD 24 S PN 2 TO HORD 24 S PN 2 BO T CHORD Rigid celling directly applied or 10-0-0 oc bracing. REACTIONS (bbix): 1 = 192/4-9-10, (min. 0-1-8), 3=192/4-9-10, (min. 0-1-8) Max LUpit 1 = -23 (LC 0) Max Upit 1 = -23 (LC 10) 3=-23 (LC 11) FORCES (bb) -Max. Comp.Max. Ten All forces 250 (bb) or less except when shown. TO P CHORD 1 2-2-88/65 NOTES 9 Unbalanced root live loads have been considered for this design. 9 Unbalanced root live loads have been considered for this design. 9 Unbalanced root live loads have been considered for this design. 9 Wint: ASSE 7-10; VUHE 300 (bb) for loss prove less and long for live loads have been considered for this design. 9 Wint: ASSE 7-10; VUHE 300 (bb) for loss prove less and long for live loads have been considered for this design. 9 Wint: ASSE 7-10; VUHE 300 (bb) for loss prove less and long for live loads have been considered for this design. 9 Wint: ASSE 7-10; VUHE 300 (bb) for loss prove less and long for live loads have been considered for this design. 9 Kint: ASSE 7-10; VUHE 300 (bb) for loss prove less and long for live loads have been considered for this design. 9 Kint: ASSE for live loads have been considered for this design. 9 Kint: ASSE for live loads do 20,0pf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord bearing. 9 For its trues have been designed for a live load of 20,0pf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord in the load in joint (1), 1. 9 For vide mechanical connection (by others) of trues to bearing plate capable of withstanding 23 lb upilit at joint 3. 9 Eveled plate or a live load of 20,0pf on the bottom chord at joint(1), 1. 9 This trues in a bean designed for a live load at low plate capable of withstanding 23 lb upilit at joint 3. 9 Eveled plate or a live load the 2015 International Res											
							Jul	SEA 0250	AROLIN P AL 1994/23 EEB LE JUIN		

























