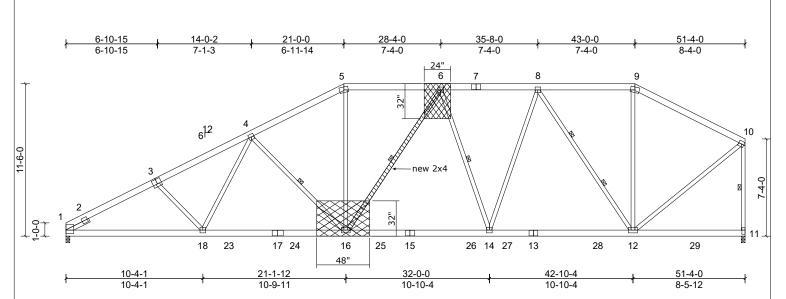
Job	Truss	Truss Type	Qty	Ply	Pro Bldrs / Clayton Craftsman - GR					
72331505REP1	A8	Truss	6	1	Job Reference (optional)					

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Repair to replace web as shown

Cut and fit tight a new 2x4 SP or SPF No.2 Attach 1/2" Plywood or 7/16" OSB (APA Rated Sheathing Exposure 1) gusset to both sides of truss as shown with two rows of 10d (.131" x 3") nails spaced 4" oc in all members from each face, driven through both sheets of plywood.

Plate Offsets (X, Y):	[3:0:4-0;0:4-8], [11:Edge;0:3-8]											
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.89	Vert(LL)	-0.27	16-18	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.93	Vert(CT)	-0.49	16-18	>999	180	M18AHS	186/179
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.91	Horz(CT)	0.12	11	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MSH							Weight: 413 lb	FT = 20%

LUMBER BRACING TOP CHORD 2v6 SP No 2 TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals, and 2-0-0 oc purlins (4-5-5 max.): 5-9.
Rigid ceiling directly applied or 2-2-0 oc bracing. BOT CHORE 2v6 SP No 2 WERS 2x4 SP No.3 WEBS 1 Row at midpt 4-16, 6-16, 6-14, 10-11 SLIDER Left 2x4 SP No.3 -- 1-11-0 2 Rows at 1/3 pts WEBS 8-12

REACTIONS (lb/size) 1=2048/0-3-8, (min. 0-2-7), 11=2048/0-3-8, (min. 0-2-9) Max Horiz 1=331 (LC 10)

Max Uplift 1=-233 (LC 10), 11=-175 (LC 6)

1=2055 (LC 2), 11=2196 (LC 2)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown TORCES | TOP CHORD

1-2=-1446/183, 2-3=-3629/881, 3-4=-3461/859, 4-5=-2854/770, 5-6=-2482/743, 6-7=-2419/681, 7-8=-2419/681, 8-9=-1417/466, 9-10=-1649/436, 10-11=-2063/539 BOT CHORD

1-18=-965/3135, 18-23=-797/2906, 17-23=-797/2906, 17-24=-797/2906, 16-24=-797/2906, 16-25=-568/2524, 15-25=-568/2524, 15-26=-568/2524, 14-26=-568/2524, 14-27=-471/2162, 13-27=-471/2162, 13-28=-4

 $4 - 18 = -40/429, \ 4 - 16 = -684/348, \ 5 - 16 = -140/923, \ 6 - 16 = -275/200, \ 6 - 14 = -455/223, \ 8 - 14 = -84/832, \ 8 - 12 = -1404/346, \ 9 - 12 = 0/411, \ 10 - 12 = -359/1819$

NOTES (10)

WERS

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10: Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior 2) (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Provide adequate drainage to prevent water ponding.
- 3)
- All plates are MT20 plates unless otherwise indicated. 4)
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) 6)
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 1 and 175 lb uplift at joint 11.
- 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- This repair has been prepared based on information and use conditions supplied by client. Designer has made a good faith effort to outline damage and repair conditions as reported by client. When actual field conditions do not approximate those indicated on this drawing, client shall immediately inform the engineer and refrain from applying the repair



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.

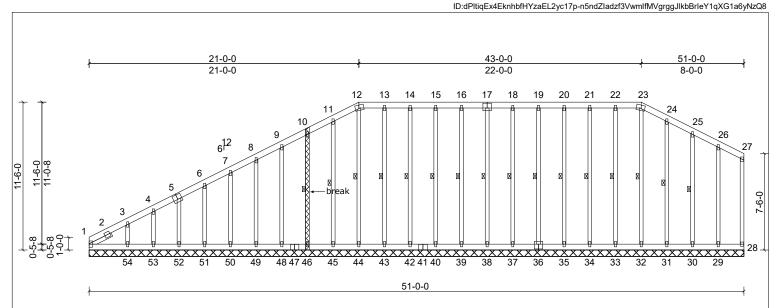


Job	Truss	Truss Type	Qty Ply		Pro Bldrs / Clayton Craftsman - GR					
72331505REP1	A9G	Truss	1	1	Job Reference (optional)					

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Repair for a break in the vertical where indicated.

Attach 2x4 SP or SPF No.2 scab to one face of truss as shown with 2 rows of 10d (.131" x 3") nails spaced 4" oc

Plate Offsets (X, Y): [5:04-0,04-8], [12:04-0,Edge], [17:0-4-0,0-4-8], [23:04-0,04-8]													
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.04	Vert(TL)	n/a	-	n/a	999			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.14	Horiz(TL)	0.00	1	n/a	n/a			
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MSH							Weight: 562 lb	FT = 20%	

LUMBER BRACING TOP CHORD 2x6 SP No 2 TOP CHORD BOT CHORD 2x6 SP No.2 BOT CHORD WERS 2v4 SP No 3 WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.); 12-23. Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

17-38, 18-37, 16-39, 15-40, 14-42, 13-43, 12-44, 11-45, 10-46, 19-36, 20-35, 21-34, 22-33, 23-32, 24-31, 25-30

REACTIONS All bearings 51-0-0

2v4 SP No 3 Left 2x4 SP No.3 -- 1-11-0

> (lb) - Max Horiz 1=334 (LC 10), 55=334 (LC 10)

Max Uplift All uplift 100 (lb) or less at joint(s) 28, 29, 30, 31, 33, 34, 35, 36, 37, 38, 39, 40, 42, 43, 44, 45, 46, 48, 49, 50, 51, 52, 53 except 54=-179 (LC 10) Max Grav All reactions 250 (lb) or less at joint(s) 1, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 42, 43, 44, 45, 46, 48, 49, 50, 51, 52, 53, 54, 55

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown

TOP CHORD 2-3=-354/118, 3-4=-275/78

NOTES (13)

OTHERS

SLIDER

- Unbalanced roof live loads have been considered for this design. 1)
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Truss designed for wind loads in the plane of the truss only. 2)
- 3)
- 4) Provide adequate drainage to prevent water ponding
- All plates are 2x5 MT20 unless otherwise indicated. 5)
- 6) Gable requires continuous bottom chord bearing
- 7) Gable studs spaced at 2-0-0 oc
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 28, 38, 37, 39, 40, 42, 43, 44, 45, 46, 48, 49, 50, 51, 52, 53, 36, 35, 34, 33, 31, 30, 29 except (jt=lb) 54=178. 10)
- 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- This repair has been prepared based on information and use conditions supplied by client. Designer has made a good faith effort to outline damage and repair conditions as reported by client. When actual field conditions do not approximate those indicated on this drawing, client shall immediately inform the engineer and refrain from applying the 13)

This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.

