Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 40537 JOB: 23-5652-R01 JOB NAME: LOT 39 PROVIDENCE CREEK Wind Code: 37 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. *30 Truss Design(s)*

Trusses:

PB01, PB02, PB03, R01, R02, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, SP01, SP02, SPJ01, SPJ02, SPJ03, VS01, VS02, VT01, VT02, VT03, VT04, VT05, VT06, VT07



Warning !--- Verify design parameters and read notes before use.



Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEPT WAY FUQUAY-VARINA,	ŅС
23-5652-R01	PB01	GABLE	2	1	Job Reference (optional) # 40537	
		Run: 8.4	30 s Feb 12	2021 Print	: 8,430 s Feb 12 2021 MiTek Industries, Inc. Wed Aug. 9 11:05:39 2023 Page 2	<i>.</i>

un: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Aug 9 11:05:39 2023 Page 2 ID:kHdPkcON9g3_0lfrDBlgKRzexCS-v472eFoBJMzvzSaVi2yg6MK9VBfDsKTk33SFuUypccw

14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard









Vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEPT WAY FUQUAY-VA	RINA, NC
23-5652-R01	R01	Piggyback Base Supported Gable	1	1	Job Reference (optional) # 40537	
		Run: 8.43	0 s Feb 12	2021 Print:	: 8,430 s Feb 12 2021 MiTek Industries, Inc. Wed Aug. 9 11:05:42 2023	Page 2

: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Aug 9 11:05:42 2023 Page 2 ID:kHdPkcON9g3_0lfrDBlgKRzexCS-JfpAGHq4bHLUqwl3NAVNj?yhsOi63dvBl1hvVoypcct

14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Scale = 1:102.5



L	7-4-5	14-5-3	21-6-0	27-0-0	31-0-0) 32-0 ₋ 0 36-6-4	44-6-13		51-7-11	59-0-0		
	7-4-5	7-0-13	7-0-13	5-6-0	4-0-0	1-0-0 4-6-4	8-0-9		7-0-13	7-4-5	1	
Plate Offsets	s (X,Y) [7:0-5	-4,0-3-0], [20:0-4-0,0-5	-0]									
LOADING (p	sf)	CDACING	2.0.0	001		DEEL	in (las)	1/-l 41	1./-1			
TCLL (roof)	20.0	Diete Crip DOI	2-0-0		0.90				L/U 240	PLATES	GRIP	
Snow (Pf)	20.0	Flate Grip DOL	1.10		0.00	Vert(LL)	-0.20 23-24	>999	240	IVI I ZU	244/190	
TCDL	10.0	Luiliber DOL Bon Stross Inor	1.10 VEQ		0.95		-0.41 23-24	>999	100			
BCLL	0.0 *	Code IBC2021/		VVD Matri	0.90 v SH		0.06 15	n/a	n/a	Weight: 503 lb	ET - 20%	
BCDL	10.0		1112014	Iviau	X-011					Weight. 505 lb	11-2070	
LUMBER-						BRACING-						
TOP CHORE) 2x6 SP No 2					TOP CHORD	Structural w	ood she	athing directl	v applied or 4-6-3 oc	purlins	
BOT CHORE	2x6 SP No.2	*Except*				BOT CHORD	Rigid ceiling	directly	applied or 2	-2-0 oc bracing. Exce	ept:	
	B4: 2x4 SP I	No.2					6-0-0 oc bra	cina: 22	-24	5	•	
WEBS	2x4 SP No.3	*Except*				WEBS	1 Row at mid	dpt	6-27, 7	-25, 11-20, 9-20		
	W8: 2x4 SP SS 2 Rows at 1/3 pts 8-22											
SLIDER	IDER Left 2x4 SP No.3 -° 4-0-9, Right 2x4 SP No.3 -° 4-0-9											
	he installed during trues erections in accordance with Stabilizer											
							Installation	auide.			o tabilizo	
REACTIONS	. (lb/size) 2=	1297/0-3-8 (min. 0-1-	12), 15=562/0-	5-8 (min. 0	-1-8), 20=31	46/0-3-8 (min. 0-	-2-11)	0				
	Max Horz 2=	-148(LC 15)		,		,	,					
	Max Uplift2=	-130(LC 14), 15=-143	(LC 15)									
	Max Grav 2=	1509(LC 92), 15=734	LC 41), 20=39	03(LC 43)								
FORCES. (I	b) - Max. Com	o./Max. Ten All force	s 250 (lb) or le	ss except w	hen shown.							
TOP CHORE) 2-3=-2467/	183, 3-4=-2370/203, 4	-5=-1944/201,	5-31=-1835	/212, 6-31=-	-1831/230,						
	6-32=-1187	7/217, 7-32=-1052/246	, 7-33=-701/20	1, 33-34=-7	00/201, 34-3	35=-700/201,						
	8-35=-700/	201, 8-36=0/958, 9-36	=0/958, 9-10=-	104/264, 10)-3/=-11//29	94, 11-37=-297/22	22,					
	11-38=-128	3/388, 12-38=-265/329	, 12-13=-351/3	07, 13-14=-	835/221, 14	-15=-940/201	-					
BOT CHORL) 2-30=-237/	2072, 29-30=-237/207	2, 28-29=-116/	1645, 28-39	9=-116/1645	, 27-39=-116/164	5,					
	26-27=-33/	1030, 26-40=-33/1030	, 25-40=-33/10	30, 25-41=-	81/429, 41-4	42=-81/429,						
	42-43=-81/	429, 21-43=-81/429, 2 420, 20, 47= 200/226	1-44=-81/429,	44-45=-81/4	129, 45-46=-	-81/429, 19- 110/717						
	20-4001/	429, 20-47299/220, 0/747	19-47299/22	0, 10-192	99/220, 17-	10112/141,						
WERS	10-17112	2//4/ \	-0/514 6 27-	1152/104 7	7 27- 101/01	20 7 25- 1010/1	50					
WEDS	4-30-0/270	1, 4-29300/141, 0-29 1538 8 21-0/1661 8	-0/314, 0-27- 22- 2221/171	20 22- 220	-27101/92	20, 7-201019/10	50,					
	24-2021/	1000, 0-24-0/1001, 0- 6 13 18- 678/130 13	222224/171,	20-22228	0/140, 11-2	01270/203, 1006 21 23- 288	/0					
	11-10-0/50	0, 13-10070/139, 10	-17-0/301, 9-2		, 9-1100/	1000, 21-23200	10			MULLININI (1/1/10)		
NOTES- (1	17-20)									WINTH CARO	11.	
1) Unbalance	ed roof live load	ds have been consider	ed for this des	an					1	0	114	
2) Wind: AS(CF 7-16: Vult=	120mph (3-second que	t) Vasd=95mp	h: TCDI =5	0psf ⁻ BCDI =	=5 0psf ⁻ h=23ft ⁻ C	at II Exp B E	nclosed	· Gable 🌋	A OFESSION.	7 12	
Roof Hip	Truss: MWFR	S (envelope) gable end	zone and C-C	Exterior(2F	-0-10-8 to	5-0-5 Interior(1)	5-0-5 to 15-7-3	3 Exteri	or(2R)	101 13		
15-7-3 to 2	27-4-13 Interio	r(1) 27-4-13 to 31-7-3	Exterior(2R) 3	1-7-3 to 43-	4-13 Interio	r(1) 43-4-13 to 53	-11-11 Exterio	r(2F) 5	3-11-11 To	SEAL	TE	
59-10-8 zo	one: end vertication	al left and right expose	d:C-C for mem	bers and fo	rces & MWF	RS for reactions	shown: Lumbe	er DOL=	1.60 plate	00147	=	
grip DOL=	1.60	5 1	,				,		III	20147	1 Ē.	
3) TCLL: AS	CE 7-16; Pr=20).0 psf (roof LL: Lum D	OL=1.15 Plate	DOL=1.15)	; Pf=20.0 ps	sf (Lum DOL=1.15	5 Plate DOL=1	.15); ls=	1.0; Rough	1		
Cat B; Par	tially Exp.; Ce	=1.0; Cs=1.00; Ct=1.10)	,		,		,.	1 m	A NOWER .	11	
4) Unbalance	ed snow loads	have been considered	for this design						1	AD, """"""	2 ME	
5) This truss	has been desi	gned for greater of mir	roof live load	of 12.0 psf o	or 2.00 times	s flat roof load of 2	20.0 psf on ove	erhangs		MALK. MONIN	ler.	
non-concu	irrent with othe	r live loads.								attribute to the second		
6) Provide ac	dequate draina	ge to prevent water po	nding.							8/0/2023		
7) All plates a	are 5x5 MT20	unless otherwise indica	ated.							0/9/2023		
8)Whiteitres	-Nvasibeetrsidesi Dage 2	grædetørsæitið. Gepst fibod	som for house live	soladi groja dal	sectoreystypyith	p araynothe rs hive rlp	adsis for an indi-	vidual bui	lding compone	ent to be installed and loa	ded	
Vertically. A	ppličability of de	sign parameters and prope	r incorporation of	component is	s responsibility	y of building designe	r – not truss desi	gner or tr	uss engineer.	Bracing shown is for late	ral support	
of individual	web members on	ly. Additional temporary	bracing to ensure	stability durir	ng construction	n is the responsibility	of the erector.	Additiona	l permanent br	acing of the overall struc	ture is the	
responsibility	of the building d	lesigner. For general guid	ance regarding fa	brication, qua	ity control, sto	orage, delivery, erec	tion and bracing,	consult A	NSI/TPI 1 Na	tional Design Standard J	for Metal	

Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583

D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEPT	WAY FUQUAY-VARINA, NC
23-5652-R01	R02	Piggyback Base	4	1	Job Reference (optional)	# 40537
		Run: 8.4 IE	30 s Feb 12):kHdPkcO	2021 Print: N9g3_0lfr	: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Au DBIgKRzexCS-nrNYUdriMaTLR4tGxu0dGCV	g 9 11:05:43 2023 Page 2 groptov6K_hQS1Fypccs

NOTES- (17-20)

9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

10) Bearing at joint(s) 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

- Provide metal plate or equivalent at bearing(s) 20 to support reaction shown.
 Provide metal plate or equivalent at bearing(s) 20 to support reaction shown.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=130, 15=143.

13) Load case(s) 86, 87, 88, 89, 90, 91, 92, 93, 94 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss. 14) MULTIPLE LOADCASES - This design is the composite result of multiple load cases.

- 15) User moving load cases exist: Review the load cases for details.
- 16) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

. . . .

- 17) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 18) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 19) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trustes for additional bracing guidelines, including diagonal bracing. 20) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

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LOAD CASE(S) Standard Except:

86)	1st User Defined Moving Load - Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-7=-60(F), 7-10=-60(F), 10-16=-60(F), 2-15=-20(F), 22-24=-20(F)
	Concentrated Loads (Ib)
	Vert: 25=-150 42=-150
87)	2nd User Defined Moving Load - Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-7=-60(F), 7-10=-60(F), 10-16=-60(F), 2-15=-20(F), 22-24=-20(F)
	Concentrated Loads (Ib)
	Vert: 42=-150 43=-150
88)	3rd User Defined Moving Load - Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-7=-60(F), 7-10=-60(F), 10-16=-60(F), 2-15=-20(F), 22-24=-20(F)
	Concentrated Loads (Ib)
	Vert: 43=-150 44=-150
89)	4th User Defined Moving Load - Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
	Vert: 1-7=-60(F), 7-10=-60(F), 10-16=-60(F), 22-15=-20(F), 22-24=-20(F)
001	Vert: 44=-150 46=-150
90)	Sur Oser Delined Noving Load - Dead + Show (Dalanced). Lumber Increase - 1.15, Plate Increase - 1.15
	$V_{\text{out}} = 1.7 \pm 60(\text{E}) - 7.10 \pm 60(\text{E}) - 10.46 \pm 60(\text{E}) - 2.15 \pm 20(\text{E}) - 20(\text{E})$
	Velt. 1-700(F), 7-1000(F), 10-1000(F), 2-1320(F), 22-2420(F)
	Vart 20=150.45=150
91)	7th Linhal 1st Liser Defined Moving Load - Dead + Snow (balanced) Parallel: Lumber Increase=1.15 Plate Increase=1.15
51)	Uniform Loads (olf)
	Vert 1.7=-32(F=-20) 7-10=-101(F=-20) 10-16=-32(F=-20) 2-15=-20(F) 22-24=-20(F)
	Concentrated Loads (b)
	Vert: 25=-150 42=-150
92)	8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15. Plate Increase=1.15
- /	Uniform Loads (plf)
	Vert: 1-5=-60(F=-20), 5-7=-101(F=-20), 7-10=-32(F=-20), 10-12=-101(F=-20), 12-16=-60(F=-20), 2-15=-20(F), 22-24=-20(F)
	Concentrated Loads (lb)
	Vert: 25=-150 42=-150
93)	7th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15
	Uniform Loads (plf)
	Vert: 1-7=-32(F=-20), 7-10=-101(F=-20), 10-16=-32(F=-20), 2-15=-20(F), 22-24=-20(F)
	Concentrated Loads (lb)
	Vert: 25=-150 42=-150
94)	8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15
	Vert: 1-5=-60(F=-20), 5-7=-101(F=-20), 7-10=-32(F=-20), 10-12=-101(F=-20), 12-16=-60(F=-20), 2-15=-20(F), 22-24=-20(F)
	Volicentrated Loads (ib) Volt 25 = 150.42 = 150
	Vert. 25150 42150





D'Onofrio Drive, Madison, WI 53719.

- 101 manung, msummg & dracing of metal rule Connected wood trusses from

Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEP	TWAY FUQUAY-VARINA, NC
23-5652-R01	R03	Piggyback Base	5	1	Job Reference (optional)	# 40537
	·	Run: 8.43 ID:kl	0 s Feb 12 dPkcON9	2021 Print: 0g3 0lfrDE	8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Au BIgKRzexCS-F1xxhzrK7ubC3ESSUbXspQ1g	ug 9 11:05:44 2023 Page 2 6C9RXObTDLA0Zhypccr

NOTES- (17-20)

7) All plates are 5x5 MT20 unless otherwise indicated.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

10) Bearing at joint(s) 21 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

11) Provide metal plate or equivalent at bearing(s) 21 to support reaction shown.

12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=116, 16=152.

13) Load case(s) 86, 87, 88, 89, 90, 91, 92, 93, 94 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss. 14) MULTIPLE LOADCASES - This design is the composite result of multiple load cases.

15) User moving load cases exist: Review the load cases for details.

- 16) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 17) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 18) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 19) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 20) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard Except:

86) 1st User Defined Moving Load - Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-8=-60(F), 8-11=-60(F), 11-17=-60(F), 2-33=-20(F), 30-32=-20(F), 16-29=-20(F), 23-25=-20(F)

- Concentrated Loads (lb) Vert: 26=-150 44=-150 87) 2nd User Defined Moving Load - Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
- Vert: 1-8=-60(F), 8-11=-60(F), 11-17=-60(F), 2-33=-20(F), 30-32=-20(F), 16-29=-20(F), 23-25=-20(F) Concentrated Loads (lb)

Vert: 44=-150 45=-150

88) 3rd User Defined Moving Load - Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-8=-60(F), 8-11=-60(F), 11-17=-60(F), 2-33=-20(F), 30-32=-20(F), 16-29=-20(F), 23-25=-20(F) Concentrated Loads (lb)

Vert: 45=-150 46=-150

89) 4th User Defined Moving Load - Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-8=-60(F), 8-11=-60(F), 11-17=-60(F), 2-33=-20(F), 30-32=-20(F), 16-29=-20(F), 23-25=-20(F) Concentrated Loads (Ib)

Vert: 46=-150 48=-150

90) 5th User Defined Moving Load - Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-8=-60(F), 8-11=-60(F), 11-17=-60(F), 2-33=-20(F), 30-32=-20(F), 16-29=-20(F), 23-25=-20(F) Concentrated Loads (Ib)

Vert: 21=-150 47=-150

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91) 7th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15
   Uniform Loads (plf)
```

Vert: 1-8=-32(F=-20), 8-11=-101(F=-20), 11-17=-32(F=-20), 2-33=-20(F), 30-32=-20(F), 16-29=-20(F), 23-25=-20(F)

Concentrated Loads (lb) Vert: 26=-150 44=-150

92) 8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-6--60(F=-20), 6-8=-101(F=-20), 8-11=-32(F=-20), 11-13=-101(F=-20), 13-17=-60(F=-20), 2-33=-20(F), 30-32=-20(F), 16-29=-20(F), 23-25=-20(F)

Concentrated Loads (lb)

Vert: 26=-150 44=-150

93) 7th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel; Lumber Increase=1.15. Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-8=-32(F=-20), 8-11=-101(F=-20), 11-17=-32(F=-20), 2-33=-20(F), 30-32=-20(F), 16-29=-20(F), 23-25=-20(F) Concentrated Loads (lb) Vert: 26=-150 44=-150

94) 8th Unbal.1st User Defined Moving Load - Dead + Snow (balanced)-Parallel: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-60(F=-20), 6-8=-101(F=-20), 8-11=-32(F=-20), 11-13=-101(F=-20), 13-17=-60(F=-20), 2-33=-20(F), 30-32=-20(F), 16-29=-20(F), 23-25=-20(F)

Concentrated Loads (Ib)

Vert: 26=-150 44=-150





Scale = 1:102.6



	7-4-5		21-6-0	+ 27-0-0 3	1-0-0 36-6-4	44-6-13	51-7-11 59-0-0
Plate Off	sets (X.Y) [7:0-5	-4.0-3-0]. [19:0-4-0.0-4	1-121	5-0-0	+-0-0 5-0-4	0-0-9	7-0-13 7-4-3
	(nef)	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	3				
TCLL (roo	of) 20.0	SPACING-	2-0-0	CSI.	DEFL.	in (loc) l/defl L/	d PLATES GRIP
Snow (Pf) 20.0	Plate Grip DOL	1.15	TC 0.79	Vert(LL)	-0.28 21-22 >999 24	0 MT20 244/190
TCDL	10.0	Lumber DOL	1.15	BC 0.92	Vert(CT)	-0.40 21-22 >999 18	0
BCLL	0.0 *	Code IBC2021/		WB 0.96	Horz(CT)	0.05 15 n/a n/	a Weight: 501 lb ET - 20%
BCDL	10.0		1 F12014	Matrix-SH			Weight: 501 lb F1 = 20%
LUMBER	-				BRACING-		
TOP CHO	ORD 2x6 SP No.2				TOP CHORD	Structural wood sheathin	g directly applied or 4-7-5 oc purlins.
BOT CHO	ORD 2x6 SP No.2	*Except*			BOT CHORD	Rigid ceiling directly appl	ied or 2-2-0 oc bracing. Except:
	B4: 2x4 SP I	No.2				6-0-0 oc bracing: 21-23	
WEBS	2x4 SP No.3	*Except*			WEBS	1 Row at midpt	6-26, 7-24, 11-19, 9-19
	W9: 2x4 SP	55 No 2 ° 4 0 0 Diabt 2v		0.0		2 Rows at 1/3 pts	8-21
SLIDER	Leit 2x4 SP	NO.3 - 4-0-9, Right 2x	4 SP N0.3 - 4	-0-9		MiTek recommends that	t Stabilizers and required cross bracing
						be installed during truss	s erection, in accordance with Stabilizer
REACTIC	NS (lb/size) 2	1208/0-3-8 (min 0-1-	11) 15=500/0-	5_{-8} (min 0_{-1}_{-8}) 10	$3=31/15/0_{-}3_{-}8$ (min 0_{-}		
REA0110	Max Horz 2=	-150(I C 15)	11), 10-000/0	0-0 (11111: 0-1-0), 13	0.	2 11)	
	Max Uplift2=	-130(LC 14), 15=-129	(LC 15)				
	Max Grav 2=	1454(LC 39), 15=681	(LC 43), 19=39	02(LC 45)			
FORCES	. (lb) - Max. Com	o./Max. Ten All force	es 250 (lb) or le	ss except when sho	Wn.		
TOP CHO	JRD 2-3=-2357	184, 3-4=-2263/204, 4	-5=-1911/202,	5-30=-1824/213, 6-	30=-1819/231,		
	8-31=-1184	+/218,7-31=-1050/247 202 8-35=0/0/11 0-35	,7-32=-699/20 =0/0/1_0_10=-	2, 32-33=-098/202, 105/265 10-36=-11	33-34=-098/202, 17/205 11_36=_207/22	23	
	11-37=-13	202, 0-33-0/341, 3-33	12-13=-354/3	04 13-14=-842/224	14-15=-945/203		
BOT CHO	ORD 2-29=-236/	1979, 28-29=-236/197	9. 27-28=-115/	1647.27-38=-115/	1647. 26-38=-115/164	7.	
	25-26=-32/	1031, 25-39=-32/1031	, 24-39=-32/10	31, 24-40=-138/43	, 20-40=-138/431,	- ,	
	20-41=-138	3/431, 19-41=-138/431	, 19-42=-296/2	28, 18-42=-296/228	3, 17-18=-296/228,		
	16-17=-112	2/753, 15-16=-112/753					
WEBS	4-29=0/270), 4-28=-512/141, 6-28	=0/515, 6-26=-	1151/194, 7-26=-10)2/921, 7-24=-1106/15	51,	
	23-24=-15/	1562, 8-23=0/1665, 8-	21=-2182/169,	19-21=-2311/143,	11-19=-1283/202, - 205/0_0_1165/085		
	11-17-0/56	52, 13-17075/140, 13	5-10-0/301, 9-	19–-037/102, 20-22	295/0, 9-1105/962	2	
NOTES-	(13-16)						MUMMINIA
1) Unbala	anced roof live loa	ds have been consider	ed for this desi	gn.			WHATH CAROLING
2) Wind: .	ASCE 7-16; Vult=	120mph (3-second gue	st) Vasd=95mp	h; TCDL=5.0psf; B0	CDL=5.0psf; h=23ft; C	at. II; Exp B; Enclosed; Ga	ble Startessid Valle
Roof [.] H	Hip Truss; MWFR	S (envelope) gable end	d zone and C-C	Exterior(2E) -0-10-	8 to 5-0-5, Interior(1)	5-0-5 to 15-7-3, Exterior(2F	R) I PAL
	1. 07 4 40 Later	r(1) 27-4-13 to 31-7-3,	Exterior(2R) 3	1-7-3 to 43-4-13, In	terior(1) 43-4-13 to 53	-1-3, Exterior(2E) 53-1-3 to	59-0-0
15-7-3	to 27-4-13, Interic	1) · · · · · · · · · · · · · · · · · · ·	r members and	torces & MWERS	for reactions shown; L	umber DOL=1.60 plate gri	p SEAL : E
15-7-3 zone; e	end vertical left an	d right exposed;C-C fo					
15-7-3 zone; e DOL=1	end vertical left an l.60	d right exposed;C-C fo)∩l =1 15 Plate	DOI =1 15). Pf=20	$0 \operatorname{pef}(I \operatorname{um} D \cap I = 1.15)$	Plate DOI = 1.15 $ls = 1.0$	Bottop 28147
15-7-3 zone; e DOL=1 3) TCLL: Cat B	to 27-4-13, interio end vertical left an I.60 ASCE 7-16; Pr=20 Partially Exp : Ce:	d`right exposed;C-C fo).0 psf (roof LL: Lum E =1 0: Cs=1 00: Ct=1 1(OL=1.15 Plate	DOL=1.15); Pf=20	.0 psf (Lum DOL=1.15	Plate DOL=1.15); ls=1.0;	Rough 28147
15-7-3 zone; e DOL=1 3) TCLL: Cat B; 4) Unbala	to 27-4-13, Interio end vertical left an I.60 ASCE 7-16; Pr=20 Partially Exp.; Ce anced snow loads	d right exposed;C-C fo).0 psf (roof LL: Lum E =1.0; Cs=1.00; Ct=1.1(have been considered	OL=1.15 Plate) for this design	DOL=1.15); Pf=20	.0 psf (Lum DOL=1.15	5 Plate DOL=1.15); ls=1.0;	Rough 28147
15-7-3 zone; e DOL=1 3) TCLL: Cat B; 4) Unbala 5) This tru	to 27-4-13, Interio end vertical left an I.60 ASCE 7-16; Pr=20 Partially Exp.; Ce anced snow loads uss has been desi	d right exposed;C-C fo 0.0 psf (roof LL: Lum E =1.0; Cs=1.00; Ct=1.10 have been considered gned for greater of mir	OL=1.15 Plate) for this design roof live load	DOL=1.15); Pf=20 of 12.0 psf or 2.00 t	.0 psf (Lum DOL=1.15 imes flat roof load of 2	9 Plate DOL=1.15); ls=1.0; 20.0 psf on overhangs	Rough 28147
15-7-3 zone; e DOL=1 3) TCLL: Cat B; 4) Unbala 5) This tri non-co	to 27-4-13, interic end vertical left an I.60 ASCE 7-16; Pr=20 Partially Exp.; Ce- anced snow loads uss has been desi incurrent with othe	d right exposed;C-C fo 0.0 psf (roof LL: Lum E =1.0; Cs=1.00; Ct=1.10 have been considered gned for greater of mir r live loads.	OL=1.15 Plate) for this design n roof live load	DOL=1.15); Pf=20 of 12.0 psf or 2.00 t	.0 psf (Lum DOL=1.15 imes flat roof load of 2	i Plate DOL=1.15); ls=1.0; 20.0 psf on overhangs	Rough 28147
15-7-3 zone; e DOL=1 3) TCLL: Cat B; 4) Unbala 5) This tri non-co 6) Provid	to 27-4-13, interio end vertical left an 1.60 ASCE 7-16; Pr=20 Partially Exp.; Ce- anced snow loads uss has been desi incurrent with othe e adequate draina	d right exposed;C-C for 2.0 psf (roof LL: Lum E =1.0; Cs=1.00; Ct=1.10 have been considered gned for greater of mir r live loads. ge to prevent water po	OOL=1.15 Plate) for this design n roof live load	DOL=1.15); Pf=20 of 12.0 psf or 2.00 t	0 psf (Lum DOL=1.15 imes flat roof load of 2	i Plate DOL=1.15); ls=1.0; 20.0 psf on overhangs	Rough 28147
15-7-3 zone; e DOL=1 3) TCLL: Cat B; 4) Unbala 5) This tri non-cc 6) Provid 7) All plat	to 27-4-13, interic end vertical left an 1.60 ASCE 7-16; Pr=20 Partially Exp.; Ce- anced snow loads uss has been desi ncurrent with othe e adequate draina res are 5x5 MT20	d right exposed;C-C for clopsf (roof LL: Lum E =1.0; Cs=1.00; Ct=1.10 have been considered gned for greater of mir r live loads. ge to prevent water po unless otherwise indice	DOL=1.15 Plate) for this design n roof live load nding. ated.	DOL=1.15); Pf=20 of 12.0 psf or 2.00 t	0 psf (Lum DOL=1.15 imes flat roof load of 2	i Plate DOL=1.15); ls=1.0; 20.0 psf on overhangs	Rough 28147
15-7-3 zone; e DOL=1 3) TCLL: Cat B; 4) Unbala 5) This tri non-cc 6) Providi 7) All plat 8) This tri 7) All plat	to 27-4-13, interic end vertical left an 1.60 ASCE 7-16; Pr=20 Partially Exp.; Ce- anced snow loads uss has been desi ncurrent with othe e adequate draina ies are 5x5 MT20 uss has been desi - 1. Vorific dea ⁺	d right exposed;C-C for 0.0 psf (roof LL: Lum E =1.0; Cs=1.00; Ct=1.10 have been considered gned for greater of mir r live loads. ge to prevent water po unless otherwise indic: gned for a 10.0 psf boo	OL=1.15 Plate of this design or roof live load anding. ated. ttom chord live	DOL=1.15); Pf=20 of 12.0 psf or 2.00 t	0 psf (Lum DOL=1.15 imes flat roof load of 2	i Plate DOL=1.15); Is=1.0; 20.0 psf on overhangs ads.	Rough 28147
15-7-3 zone; e DOL=1 3) TCLL: Cat B; 4) Unbala 5) This tri non-co 6) Provid 7) All plat 8) This tri Warning Continued	to 27-4-13, interic end vertical left an 1.60 ASCE 7-16; Pr=20 Partially Exp.; Ce- anced snow loads uss has been desi ncurrent with othe e adequate draina res are 5x5 MT20 uss has been desi ?Verify design p On page 2	d right exposed;C-C for 0.0 psf (roof LL: Lum D =1.0; Cs=1.00; Ct=1.10 have been considered gned for greater of mir r live loads. ge to prevent water po unless otherwise indic: <u>gned for a 10.0 psf boi</u> arameters and read note	OL=1.15 Plate of this design or roof live load anding. ated. ttom chord live s before use. This	DOL=1.15); Pf=20 of 12.0 psf or 2.00 t load nonconcurren s design is based only	0 psf (Lum DOL=1.15 imes flat roof load of 2 t with any other live loa upon parameters shown, a	i Plate DOL=1.15); Is=1.0; 20.0 psf on overhangs ads. and is for an individual building	Rough 28147 NoneERRS NORR N
15-7-3 zone; e DOL=1 3) TCLL: Cat B; 4) Unbala 5) This tri non-cc 6) Provid 7) All plat 8) This tri Continue Continue Continue Continue Continue	to 27-4-13, interic end vertical left an 1.60 ASCE 7-16; Pr=20 Partially Exp.; Ce- anced snow loads uss has been desi ncurrent with othe e adequate draina es are 5x5 MT20 <u>uss has been desi</u> <u>1-Verify design p</u> On page.	d right exposed;C-C for 0.0 psf (roof LL: Lum E =1.0; Cs=1.00; Ct=1.10 have been considered gned for greater of mir r live loads. ge to prevent water po unless otherwise indic: <u>gned for a 10.0 psf boi</u> arameters and read note sign parameters and prope	DOL=1.15 Plate of this design or roof live load anding. ated. ttom chord live s before use. Thi r incorporation of	DOL=1.15); Pf=20 of 12.0 psf or 2.00 t load nonconcurren s design is based only 'component is respons	0 psf (Lum DOL=1.15 imes flat roof load of 2 t with any other live loa upon parameters shown, a ibility of building designe	5 Plate DOL=1.15); Is=1.0; 20.0 psf on overhangs ads. and is for an individual building r – not truss designer or truss er of the groups. Additional arm	Rough 28147 NoneEnrore 8/9/2023 component to be installed and loaded ngineer. Bracing shown is for lateral support means because of the current strengthene in the

Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583

D'Onofrio Drive, Madison, WI 53719.

ŀ	Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEPT WAY FUQUAY-VARINA, N	С
2	23-5652-R01	R04	PIGGYBACK BASE	1	1	Job Reference (optional) # 40537	
			Run: 8.43 ID:I	0 s Feb 12 (HdPkcOl	2021 Print: V9g3_0lfr[8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Aug 9 11:05:45 2023 Page 2 DBIgKRzexCS-kEVJuIsyuCj3hO1e2J25Lda?OcVwGpddS_vZ67ypccq	

NOTES- (13-16)

- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Bearing at joint(s) 19 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide metal plate or equivalent at bearing(s) 19 to support reaction shown.
 Provide metal plate or equivalent at bearing(s) 19 to support reaction shown.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=130, 15=129.
- 13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the
- loads indicated. 15) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
- 16) SEE BCSI-B3 SUMMARY SHEET PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard







L	7-4-5	14-5-3	21-6-0	29-0-2	36-6-4	44-6-13	48-10-4	58-6-8				
	7-4-5	7-0-13	7-0-13	7-6-2	7-6-2	8-0-9	4-3-7	9-8-4				
Plate Offsets	s (X,Y) [7:0-5	-4,0-3-0]										
LOADING (p: TCLL (roof) Snow (Pf) TCDL BCLL	sf) 20.0 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Inc Code IRC2021	2-0-0 - 1.15 1.15 r YES /TPI2014	CSI. TC 0.51 BC 0.50 WB 1.00 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.12 22-24 >999 -0.21 22-24 >999 0.05 15 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190 FT = 20%			
BCDL	10.0							5				
LUMBER- TOP CHORE BOT CHORE WEBS) 2x6 SP No.2 2x6 SP No.2 2x4 SP No.3 W5: 2x4 SP	*Except* No.2			BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sho Rigid ceiling directl 1 Row at midpt 2 Rows at 1/3 pts	eathing direct y applied or 6 6-22, 7 9-19	ly applied or 4-4-2 oc p 5-0-0 oc bracing. 7-21, 8-21, 9-21, 11-19,	urlins. 9-11			
SLIDER	Left 2x4 SP	No.3 -° 4-0-9, Right 2	x6 SP No.2 -° 4-			MiTek recommen be installed during Installation guide.	ds that Stabili g truss erectio	izers and required cross on, in accordance with S	ঃ bracing Stabilizer			
REACTIONS (Ib)	REACTIONS. All bearings 0-3-8 except (jt=length) 15=Mechanical. (lb) - Max Horz 2=-150(LC 15) Max Uplift All uplift 100 lb or less at joint(s) 19, 16, 15 except 2=-135(LC 14) Max Grav All reactions 250 lb or less at joint(s) except 2=1575(LC 39), 19=2880(LC 45), 16=691(LC 43), 15=482(LC 43) ECRCES. (lb) - Max Comp (Max Ten - All forces 250 (lb) or less except when shown											
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2601/232, 3-4=-2500/253, 4-5=-2093/257, 5-26=-2007/267, 6-26=-1993/285, 6-27=-1357/270, 7-27=-1210/298, 7-28=-709/269, 8-28=-708/269, 8-29=-708/269, 29-30=-708/269, 30-31=-708/269, 9-31=-708/269, 9-10=-200/358, 10-32=-224/414, 11-32=-399/331, 12-33=-303/152, 12-13=-386/411, 13-14=-424/173, 14-15=-513/141												
BOT CHORE	2-25=-244/ 22-35=-32/ 19-37=-13	2184, 24-25=-244/21 1134, 21-35=-32/113 1/258, 18-37=-131/25	84, 23-24=-125/ 4, 21-36=-444/1 8, 17-18=-131/2	1795, 23-34=-125/1795 53, 20-36=-444/153, 19 58, 15-16=-96/380	5, 22-34=-125/179)-20=-444/153,	5,						
WEBS	4-25=0/275 8-21=-835/ 13-16=-614	5, 4-24=-492/139, 6-2 174, 9-21=-155/1810 4/164, 9-11=-47/606	4=0/470, 6-22=- , 9-19=-2114/25	1135/200, 7-22=-73/11 0, 11-19=-763/152, 13-	11, 7-21=-1124/1 [·] 17=-106/256,	16,						
NOTES- (1 1) Unbalance 2) Wind: ASC Roof; Hip 15-7-12 to zone; end plate grip I 3) TCLL: ASC Cat B; Par 4) Unbalance 5) This truss non-concu 6) Provide ac 8) This truss 9)* This truss 9)* This truss Warning: # Conneget # Conneget #	 NOTES- (12-15) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; Hip Trus; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 4-11-12, Interior(1) 4-11-12 to 15-7-12, Exterior(2R) 31-7-12 to 43-4-4, Interior(1) 4-11-12 to 15-7-12, Exterior(2R) 31-7-12 to 43-4-4, Interior(1) 4-11-12 to 15-7-12, Exterior(2E) 52-8-4 to 58-64 to											
of individual	web members on	ly. Additional temporary	bracing to ensure	stability during construction	n is the responsibility	of the erector. Addition	al permanent b	racing of the overall structu	re is the			
responsibility	responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPT 1 National Design Standard for Metal											
D'Onofrio Di	rive, Madison, W	1 53719.	1-05 Guide to G00	а 1 racuce jor папашig, Ir	isialling & Dracing (y metai r iaie Connected	a wood 1russes	nom muss riate insutute,	505			

Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEPT WAY FUQUAY-V	ARINA, NC
23-5652-R01	R05	Piggyback Base	5	1	Job Reference (optional) # 40537	
		Run: 8.4	30 s Feb 12 ID:kHdPk	2021 Print cON9g3_0	8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Aug 9 11:05:46 2023 DlfrDBIgKRzexCS-CQ2h6etafVrvIXcrc0aKur7Fc?yi?GMmgef6	Page 2 aypccp

NOTES- (12-15)

10) Refer to girder(s) for truss to truss connections.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 19, 16, 15 except (it=lb) 2=135.

 Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

 Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEPT WAY FUQUAY-VARINA, N		
23-5652-R01	R06	PIGGYBACK BASE	5	1	Job Reference (optional) # 40537		
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Aug 9 11:05:47 2023 Page 2							

10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 11) Bearing symbols are not considered in the structural design of the truss to support the

loads indicated. 12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

12) Yes blacking shown is to hater support of individual web members only. Telef to boot a back to boot a factor of hatering, instanting, nestanting a blacking of index a factor of hatering of individual web members only. Telef to boot a back to boot a factor of hatering, nestanting a blacking of index a factor of hatering of individual web members only. Telef to boot a back to boot a factor of hatering, nestanting a blacking of index a factor of hatering of individual web members only. Telef to boot a back to boot a factor of hatering, nestanting a blacking of individual web members only. Telef to boot a back to boot a factor of hatering, nestanting, nestanting a blacking of individual web members only. Telef to boot a back to boot a factor of individual web members only. Telef to boot a back to boot a factor of individual web members only. Telef to boot a back to boot a factor of individual web members only. Telef to boot a back to boot a factor of individual web members on a distribution of the boot a back to boot a factor of the boot a back to boot a ba

LOAD CASE(S) Standard





Scale = 1:99.7



L	7-4-5	14-5-3	21-6-0	29-0-2	36-6-4	44-6-13	51-4-0	58-6-8					
	7-4-5	7-0-13	7-0-13	7-6-2	7-6-2	8-0-9	6-9-3	7-2-8					
Plate Offsets	s (X,Y) [7:0-5	-4,0-3-0]			1								
LOADING (p	osf)	SPACING-	2-0-0	CSI	DEEL	in (loc) l/defl	L/d	PLATES	GRIP				
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC 0.51	Vert(LL)	-0.12 22-24 >999	240	MT20	244/190				
Snow (Pf)	20.0	Lumber DOL	1.15	BC 0.48	Vert(CT)	-0.20 22-24 >999	180		211/100				
	10.0	Rep Stress Inc	r YES	WB 0.94	Horz(CT)	0.06 15 n/a	n/a						
BCDI	0.0	Code IRC2021	/TPI2014	Matrix-SH				Weight: 490 lb	FT = 20%				
	10.0												
LUMBER-					BRACING-								
BOT CHORE	J 2X6 SP NO.2	*Evcent*			BOT CHORD	Structural wood snea	athing directly a	applied or 4-6-1 oc	ouriins.				
BOT CHOIL	B2: 2x6 SP I	- Except			BOT CHORD	6-0-0 oc bracing 19-	-21 17-19	0-0 00 bracing, LN	Jept.				
WEBS	2x4 SP No.3	*Except*			WEBS	1 Row at midpt	6-22, 8-2	1, 9-21, 11-19					
	W5: 2x4 SP	No.2				2 Rows at 1/3 pts	7-21, 9-1	9					
SLIDER	Left 2x4 SP	No.3 -° 4-0-9, Right 2	k6 SP No.2 -° 4-	0-12		MiTek recommend	s that Stabilize	rs and required cros	ss bracing				
	be installed during truss erection, in accordance with Stabilizer												
	Installation guide.												
REACTIONS	EACTIONS. (Ib/size) 2=1350/0-3-8 (min. 0-1-13), 19=2778/0-3-8 (min. 0-3-5), 15=608/Mechanical												
	Max Horz 2=	=-150(LC 19)		(1045)									
	Max Opini2=	-142(LC 14), 19=-31(-1511/LC 30) 10-330	LU 14, $15=-12$	SO(LC 43)									
	Wax Glav 2-	-1311(LC 39), 19-330	H(LC 43), 13-7	JU(LC 43)									
FORCES. (I	lb) - Max. Com	p./Max. Ten All force	es 250 (lb) or le	s except when shown									
TOP CHORE	D´ 2-3=-2473/	215, 3-4=-2376/235, 4	4-5=-1973/238,	5-26=-1887/249, 6-26=	-1874/266,								
	6-27=-1252	2/250, 7-27=-1116/27	9, 7-28=-615/24	6, 8-28=-614/246, 8-29)=-615/246,								
	29-30=-61	5/246, 30-31=-615/24	6, 9-31=-615/24	6, 9-10=-112/394, 10-3	32=-126/453,								
	11-32=-303	3/371, 11-33=-316/19	5, 12-33=-455/1	78, 12-13=-533/168, 13	3-14=-883/203,								
	14-15=-10	07/182 2076 24 25- 256/20	76 22 24- 120/	1600 22 24- 129/160	0 22 24- 120/160	0							
BUICHURL	22-35=-250/	2070, 24-25-250/20	70, 23-24138/ 5 21-36=-645/1	1090, 23-34138/1090 32 20-36=-645/162 19	9-20=-645/162	Ο,							
	19-37=-103	3/395, 18-37=-103/39	5, 17-18=-103/3	95. 16-17=-87/836. 15	-16=-87/836								
WEBS	4-25=0/277	7, 4-24=-504/137, 6-24	4=0/474, 6-22=-	1138/199, 7-22=-73/11	15, 7-21=-1188/10	08,							
	8-21=-832/	174, 9-21=-167/1844,	9-19=-2194/27	2, 11-19=-1226/195, 1	1-17=0/529,								
	13-17=-589	9/130, 13-16=0/282, 9	-11=0/730										
	10.15							MANINE CONTRACTOR					
NULES- (12-15) od roof live lee	da hava haan aanaida	rad for this dasi	20				WINTH CARO	4.				
2) Wind: AS	CE 7-16 Vult=	120mph (3-second qu	st) Vasd=95mn	yıı. n: TCDI =5 (Insf: BCDI)	=5 0nsf: h=23ft: C	at II: Exp B: Enclosed	l' Gable	OR TERRITA	111				
Roof: Hip	Truss: MWFR	S (envelope) dable en	d zone and C-C	Exterior(2E) -0-10-8 to	0 4-11-12. Interior(1) 4-11-12 to 15-7-12.	Exterior(2R)	20FESSIPA	9 12				
15-7-12 to	27-4-4, Interic	r(1) 27-4-4 to 31-7-12	, Exterior(2R) 3	1-7-12 to 43-4-4, Interio	or(1) 43-4-4 to 52-8	8-4, Exterior(2E) 52-8-	4 to 58-6-8	is the	4				
zone; end	vertical left ex	posed;C-C for membe	ers and forces &	MWFRS for reactions	shown; Lumber D	OL=1.60 plate grip DO	L=1.60	SEAL					
3) TCLL: AS	CE 7-16; Pr=20	0.0 psf (roof LL: Lum I	DOL=1.15 Plate	DOL=1.15); Pf=20.0 p	sf (Lum DOL=1.15	i Plate DOL=1.15); ls=	1.0; Rough	28147	IW				
Cat B; Pa	rtially Exp.; Ce	=1.0; Cs=1.00; Ct=1.1	0				HIII						
4) Unbalance	ed snow loads	nave been considered	a for this design	of 12.0 not or 2.00 time	a flat reaf land of (0.0 not on overhange	III	S. Su al	Ing				
5) This truss	irrent with othe	gried for greater of mi r live loads	II TOOL IIVE IOAU	or 12.0 psi or 2.00 time		20.0 psi on overhangs	THE R	VOINEE C	AN AN				
6) Provide a	dequate draina	de to prevent water p	ondina.				11	AK K MORM	Inte				
7) All plates	are 5x5 MT20	unless otherwise indic	ated.					All the state of t					
8) This truss	has been desi	gned for a 10.0 psf bo	ottom chord live	load nonconcurrent wit	th any other live loa	ads.		8/0/2022					
9 <u>) * This trus</u>	s has been de	signed for a live load	of 30.0psf on the	e bottom chord in all ar	eas where a recta	ngle 3-6-0 tall by 1-0-0	wide will fit	0/9/2023					
Noetwiegn-t	Wighwing 1-the bill to many and terry and terry and the ratio matching to the second states (with second states) with second states (with second states) with second states (with second states) with second states) with second states (with second states) with second states) with second states (with second states) with second states) with second states (with second states) with second states) with second states (with second states) with second states) with second states (with second states) with second states) with second states) with second states (with second states) with second s												
- vertically. A	vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support												
of individual	web members or	ly. Additional temporary	bracing to ensure	stability during construction	on is the responsibility	of the erector. Additiona	I permanent brac	ing of the overall struct	ure is the				
responsibility	responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal												
Plate Conne	cted Wood Truss	Construction and BCSI	1-03 Guide to Goo	d Practice for Handling, I	nstalling & Bracing o	f Metal Plate Connected	Wood Trusses fr	om Truss Plate Institute	, 583				
D'Onofrio D	rive, Madison, W	T 53719.											

Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEPT WAY FUQ	UAY-VARINA, NC
23-5652-R01	R07	Piggyback Base	4	1	Job Reference (optional) # 403	537
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Aug 9 11:05:48 2023 Page 2 ID:kHdPkcON9g3_0lfrDBlgKRzexCS-8pARXKvqB76dYrmDjRcozGCb9peVTBf38y8DiSypcc						

NOTES- (12-15)

- 10) Refer to girder(s) for truss to truss connections.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 19 except (jt=lb) 2=142, 15=121.
- 12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





(lb) - Max Horz 2=-150(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 2, 52, 53, 54, 55, 57, 58, 60, 61, 62, 63, 64, 65, 66, 51, 49, 48, 46, 45, 43, 42, 41, 40, 39, 38, 37 except 67=-103(LC 14), 36=-108(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 2, 64, 65, 66, 67, 39, 38, 37, 36, 35 except 52=292(LC 44), 53=292(LC 44), 54=295(LC 44), 55=288(LC 44), 56=266(LC 53), 57=297(LC 47), 58=294(LC 45), 60=291(LC 45), 61=292(LC 45), 62=301(LC 45), 63=270(LC 45), 51=292(LC 44), 49=294(LC 44), 48=294(LC 44), 47=257(LC 52), 46=289(LC 49), 45=295(LC 45), 43=292(LC 45), 42=292(LC 45), 41=293(LC 45), 40=284(LC 45)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

12-13=-127/276, 14-15=-121/266, 15-16=-121/266, 16-17=-121/266, 17-18=-121/266, TOP CHORD 18-69=-121/266, 19-69=-121/266, 19-20=-121/266, 20-21=-121/266, 21-22=-121/266,

22-23=-121/266, 23-24=-128/276, 24-25=-115/251

NOTES-(15-18)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16: Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed: Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 5-3-4, Exterior(2N) 5-3-4 to 15-7-12, Corner(3R) 15-7-12 to 27-3-4, Exterior(2N) 27-3-4 to 31-7-12, Corner(3R) 31-7-12 to 43-3-4, Exterior(2N) 43-3-4 to 52-8-4, Corner(3E) 52-8-4 to 58-6-8 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 17 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 50 Unbalanced snow loads have been considered for this design.
 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 7) Provide adequate drainage to prevent water ponding.
 8) All plates are 2x4 MT20 unless otherwise indicated.
 9) Gable studs spaced at 2-0-0 oc.
 11) This truss has been designed for a live load of 30.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0 psf.
 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb unlife at the true to a true to the set of the true to a true to the set of true to the

MORPHS INTERNAL Y9/2023 's'd and ' for WAR HAR 20 Verte 18 Stander 18 Stander 18 Stander 18 Stander 18 Stander 19 St of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEPT WAY	FUQUAY-VARINA, NO
23-5652-R01	R08	Piggyback Base Supported Gable	1	1	Job Reference (optional) #	40537
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NOTES- (15-18)

- 14) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 35.
- 15) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 16) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 17) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 18) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENCINEER FOR ADDITIONAL DEACING CONCEPTENTIONS. ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEP	PT WAY FUQUAY-VARINA, NC
23-5652-R01	R11	Common Supported Gable	1	1	Job Reference (optional)	# 40537
Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Aug 9 11:05:51 2023 Page 2					Aug 9 11:05:51 2023 Page 2	

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Wed Aug 9 11:05:51 2023 Page 2 ID:kHdPkcON9g3_0lfrDBlgKRzexCS-YOsa9MxjU2UCPJUoPZ9VbuqCQ0megmGVqwMtJnypcck

14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEPT WAY FUQUAY-VARINA, N
23-5652-R01	R12	GABLE	1	1	Job Reference (optional) # 40537
		Run: 8.43	30 s Feb 12	2021 Print	: 8,430 s Feb 12 2021 MiTek Industries, Inc. Wed Aug. 9 11:05:51 2023 Page 2

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12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 39 PROVIDENCE CREEK 343 WINDSWEPT	WAY FUQUAY-VARINA, NO
23-5652-R01	R13	Common Girder	1	3	Job Reference (optional)	# 40537
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NOTES- (15-18)

- 12) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 13-5-4 from the left end to 17-5-4 to connect truss(es) R09 (1 ply 2x4 SP) to front face of bottom chord.
- 13) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 19-5-4 from the left end to 27-5-4 to connect truss(es) R05 (1 ply 2x6 SP) to front face of bottom chord.
- 14) Fill all nail holes where hanger is in contact with lumber.
- 15) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 16) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 17) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
- 18) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf)
 - Vert: 1-6=-60, 6-11=-60, 1-11=-20
 - Concentrated Loads (lb)

Vert: 18=-740(F) 19=-740(F) 20=-740(F) 21=-740(F) 22=-867(F) 24=-867(F) 25=-347(F) 26=-347(F) 27=-347(F) 29=-462(F) 31=-462(F) 32=-462(F) 33=-462(F) 34=-462(F) 34=-4





vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





Max Uplift5=-2(LC 14), 3=-20(LC 14), 4=-7(LC 11) Max Grav 5=208(LC 21), 3=57(LC 21), 4=34(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=10ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; end vertical left exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply LOT 39 PROVIDENCE CRE	EK 343 WINDSWEPT WAY FUQUAY-VARINA, NO
23-5652-R01	SPJ02	Half Hip Girder	2		# 40537
			Run: 8.430 s Feb 12	Job Reference (optional) 2 2021 Print: 8.430 s Feb 12 2021 MiTek I	ndustries, Inc. Wed Aug 9 11:05:55 2023 Page 1
	-0-10-8	2-0-0	ID:97bQtTov	vZ8dTuLxBq9ksq7zUUzD-R955?j_C 4-0-0	PYG_etwoZePERIk?uae85cZe5IYK5SYypccg
	0-10-8	2-0-0		2-0-0	
				3×6 Îl	Scale = 1:12.6
Ŧ			4	5300 %	
		6.00 12		T2	
		3		×	т
		T1		VN/1	
o o T	0		W1	W2	
Ŕ	2	HW1			3- 3-
	1				
F			B1		
			ПП	ব	
				X	
			7 _{2x4}	6	
				4.4 -	
	c	3x8		4X4 —	
		2-0-0		4-0-0	
		2-0-0		2-0-0	
Plate Offsets (X,Y) [2:0	<u>-6-1,0-0-5], [4:0-5-0,0-2-0]</u>		1		
LOADING (psf) TCLL (roof) 20.0	SPACING-	2-0-0 CSI .	DEFL.	in (loc) l/defl L/d	PLATES GRIP
Snow (Pf) 20.0	Plate Grip DOL	1.15 TC 0.11 1.15 BC 0.03	Vert(LL)	-0.00 2 >999 240 -0.00 7 >999 180	MT20 244/190
TCDL 10.0	Rep Stress Incr	NO WB 0.03	Horz(CT)	0.00 5 n/a n/a	
BCDL 10.0	Code IRC2021/TP	12014 Matrix-P			Weight: 27 lb FT = 20%
LUMBER-			BRACING-		
TOP CHORD 2x4 SP No	0.2		TOP CHORD	Structural wood sheathing direct	tly applied or 4-0-0 oc purlins, except
WEBS 2x4 SP No	0.2 0.3		BOT CHORD	Rigid ceiling directly applied or	10-0-0 oc bracing.
SLIDER Left 2x6 S	P No.2 -° 1-6-12			MiTek recommends that Stab	ilizers and required cross bracing
				be installed during truss erect	ion, in accordance with Stabilizer
REACTIONS. (lb/size)	5=56/Mechanical, 6=92/Me	echanical, 2=213/0-3-0 (min. 0-1-8)		Installation guide.	
Max Horz	2=37(LC 12)				
Max Uplift Max Grav	5=-14(LC 8), 6=-6(LC 12), 5=94(I C 33) 6=101(I C 33	2=-14(LC 12) () $2=302(LC 34)$			
		,, , , _ , _ , _ , _ , _			
FORCES. (lb) - Max. Co	mp./Max. Ten All forces 2	250 (lb) or less except when shown.			
NOTES- (13)					
1) Wind: ASCE 7-16; Vul	t=115mph (3-second gust)	Vasd=91mph; TCDL=5.0psf; BCDL	=5.0psf; h=11ft; C	at. II; Exp B; Enclosed; MWFRS	
2) TCLL: ASCE 7-16; Pr=	20.0 psf (roof LL: Lum DOI	L=1.15 Plate DOL=1.15); Pf=20.0 ps	sf (Lum DOL=1.15	Plate DOL=1.15); ls=1.0; Rough	
Cat B; Partially Exp.; C	e=1.0; Cs=1.00; Ct=1.10	n Alaia ala aliana			
4) This truss has been de	signed for greater of min ro	oof live load of 12.0 psf or 2.00 times	s flat roof load of 2	20.0 psf on overhangs	
non-concurrent with ot	her live loads.				
 5) Provide adequate drail 6) This truss has been de 	hage to prevent water pond	ing. m chord live load nonconcurrent witl	h any other live loa	ads	
7) * This truss has been of	lesigned for a live load of 3	0.0psf on the bottom chord in all are	eas where a rectar	ngle 3-6-0 tall by 1-0-0 wide will fi	t
between the bottom ch 8) Refer to girder(s) for tr	ord and any other member	S.			
9) Provide mechanical co	nnection (by others) of trus	s to bearing plate capable of withsta	anding 100 lb uplif	t at joint(s) 5, 6, 2.	AND CA STATISTICS
10) Gap between inside of	of top chord bearing and firs	st diagonal or vertical web shall not (exceed 0.500in.		WINGRTH CAROLINI
11) "NAILED" Indicates 3 12) In the LOAD CASE(S) section. loads applied to t	the face of the truss are noted as fro	allnes. ont (F) or back (B).	1111	OFESSION SHE
,			(· ,). 2000 (D).	nun.	1 the second second
1) Dead + Snow (balance	d): Lumber Increase=1 15	Plate Increase=1 15		n na	SEAL E
Uniform Loads (plf)		a.e meredee - 1.10		10,111	20147
Vert: 1-4=-60,	4-5=-60, 2-6=-20			1111	No. al
Vert: 7=0(F)	0)			3	A GINEE OF IN
(.)					Min K. MORIMUN
					X/U/2023



REACTIONS. (lb/size) 5=221/0-3-0 (min. 0-1-8), 3=101/Mechanical, 4=43/Mechanical Max Horz 5=51(LC 14) Max Uplift3=-38(LC 14), 4=-9(LC 11) Max Crav(5=22)(LC 24), 2=154(LC 21), 4=72(LC 7)

Max Grav 5=322(LC 21), 3=154(LC 21), 4=72(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-5=-293/78

NOTES- (9)

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=11ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; end vertical left exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members. 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.

LOAD CASE(S) Standard



















