Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 40140 JOB: 23-4837-R01 JOB NAME: LOT 38 PROVIDENCE CREEK Wind Code: 37 Wind Speed: Vult= 115mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. 20 Truss Design(s)

Trusses:

M01, M02, M03, P01, R01, R02, R03, R04, R05, R06, R07, R08, R09, SP01, SP02, V01, V02,



Warning !--- Verify design parameters and read notes before use.



Max Horz 2=75(LC 10) Max Uplift4=-80(LC 10), 2=-91(LC 10) Max Grav 4=354(LC 21), 2=412(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 3-4=-287/144

NOTES- (11)

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- a) relier to girder(s) for truss to truss connections. 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 4 and 91 lb uplift at joint LOAD CASE(S) Standard





Max Uplift4=-80(LC 10), 2=-91(LC 10) Max Grav 4=354(LC 21), 2=412(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-4=-287/144

NOTES- (9)

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left exposed ; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members. 7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 4 and 91 lb uplift at joint 2.

LOAD CASE(S) Standard







FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (12)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4 except (jt=lb) 1=113, 5=113.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard





Installation guide.

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ARK K. MORRI

REACTIONS. All bearings 23-0-0.

(lb) - Max Horz 28=-130(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 28, 16, 23, 24, 25, 26, 27, 21, 20, 19, 18, 17

Max Grav All reactions 250 lb or less at joint(s) 28, 16, 25, 26, 27, 19, 18, 17 except 22=273(LC 27), 23=295(LC

5), 24=279(LC 5), 21=295(LC 6), 20=279(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(14)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 6-8-6, Corner(3R) 6-8-6 to 16-3-10, Exterior(2N) 16-3-10 to 19-0-14, Corner(3E) 19-0-14 to 23-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough PROFESSION Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

- non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 28, 16, 23, 24, 25, 26 , 27, 21, 20, 19, 18, 17.







responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 38 PROVIDENCE CREEK 38 PROVIDENCE CREEK DRIVE FUQUAY-	VARI
23-4837-R01	R03	COMMON GIRDER	1	3	Job Reference (optional) # 40140	
		Run: 8.4 ID:sci	30 s Feb 1 o?uVwdzľ	2 2021 Prin N_6Z0rHb	t: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Jul 24 16:17:13 2023 Page 2 coMyuyZQFk-IRivesCH_eNvo4F68Q9a?KIMQ0cDFcY9MJ4wAyyupY	2 q

NOTES- (14)

11) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 1-5-4 from the left end to 7-5-4 to connect truss(es) R08 (1 ply 2x6 SP) to front face of bottom chord.

12) Use Simpson Strong-Tie HTU26 (20-16d Girder, 11-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 9-5-4 from the left end to 21-5-4 to connect truss(es) R07 (1 ply 2x6 SP) to front face of bottom chord. 13) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 1-7=-20

Concentrated Loads (lb)

Vert: 10=-2181(F) 8=-2256(F) 13=-2181(F) 14=-2181(F) 15=-2181(F) 16=-2256(F) 18=-2256(F) 20=-2256(F) 21=-2256(F) 22=-2256(F) 23=-2256(F)





L	45-4-0											
	45-4-0											
Plate Offsets (X,Y) [2:0-6-1,0-0-5], [8:0-3-0,Edge], [13:0-6-8,0-2-12], [19:0-6-8,0-2-12], [24:0-3-0,Edge], [30:0-6-1,0-0-5]												
LOADING (TCLL (roof) Snow (Pf) TCDL BCLL BCDL	osf) 20.0 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matr	0.08 0.05 0.24 ix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 0.00 0.01	(loc) 30 30 30	l/defl n/r n/r n/a	L/d 180 80 n/a	PLATES MT20 Weight: 383 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHOR BOT CHOR OTHERS SLIDER	D 2x4 SP No.2 D 2x6 SP No.2 2x4 SP No.3 Left 2x6 SP	No.2 - ~ 1-7-12, Right 2x6	6 SP No.2 -~ 1	I-7-12		BRACING- TOP CHORD BOT CHORD WEBS	Struc Rigid 1 Rov	tural w ceiling w at mi	ood shea directly dpt	athing dire applied or 16-4 20-4	ctly applied or 6-0-0 oc 10-0-0 oc bracing. 3, 15-44, 14-45, 12-46, 1 0	ourlins. 17-42, 18-41,

REACTIONS. All bearings 45-4-0.

(lb) - Max Horz 2=117(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 2, 43, 44, 46, 48, 49, 50, 51, 52, 53, 54, 42, 40, 38, 37, 36, 35, 34, 33, 32

Max Grav All reactions 250 lb or less at joint(s) 2, 52, 53, 54, 34, 33, 32, 30 except 43=291(LC 44), 44=299(LC 44), 45=266(LC 52), 46=296(LC 47), 48=293(LC 45), 49=292(LC 45), 50=290(LC 45), 51=299(LC 45), 42=299(LC 45), 42=299(LC 45), 45=299(LC 45), 45=29(LC 45), 45=29(LC 45), 45=29(LC 45), 45=29(LC 45), 45=29 44), 41=257(LC 52), 40=294(LC 49), 38=293(LC 45), 37=292(LC 45), 36=290(LC 45), 35=299(LC 45)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(14)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 13-6-14, Corner(3R) 13-6-14 to 31-9-2, Exterior(2N) 31-9-2 to 41-4-14, Corner(3E) 41-4-14 to 46-2-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TP1 1. 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

PROFESS Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psi potent. e.e. 11) This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a requiring a fit between the bottom chord and any other members, with BCDL = 10.0psf.
 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 43, 44, 46, 48, 49, 50, 54, 52, 53, 54, 42, 40, 38, 37, 36, 35, 34, 33, 32.

LOAD CASE(S) Standard

MORPHS INTERNAL Y24/202 'sd and fo Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Trusse Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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7/24/2023



F	9-2-5		18-4-8 9-2-4		<u> </u>			-1-11 -2-4		<u>45-4-0</u> 9-2-5	
Plate Offset	s (X,Y) [2:0-0	-0,0-3-6], [6:0-6-0,0-2-8]	, [8:0-6-0,0-2·	-8], [12:0-0-	0,0-3-6]					020	
LOADING (p TCLL (roof) Snow (Pf) TCDL BCLL BCDL	osf) 20.0 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/T	2-0-0 1.15 1.15 YES PI2014	CSI. TC BC WB Matri	0.73 0.85 0.40 x-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.28 14-16 -0.44 14-16 0.14 12	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 326 II	GRIP 244/190 b FT = 20%
LUMBER- TOP CHOR BOT CHOR WEBS SLIDER	D 2x4 SP No.1 T3: 2x4 SP I D 2x6 SP No.2 2x4 SP No.3 Left 2x4 SP	BRACING- TOP CHORD BOT CHORD WEBS	Structural w Rigid ceiling 1 Row at mi MiTek reco be installe Installatior	rood sheat g directly a idpt ommends d during tr n guide.	thing direct applied or 1 5-17, 7 that Stabili russ erectio	ly applied or 2-2-0 oc 0-0-0 oc bracing. 7-17, 7-16, 9-16 izers and required cru on, in accordance wit	purlins. oss bracing h Stabilizer				
REACTIONS	REACTIONS. (lb/size) 2=1855/0-3-8 (min. 0-2-11), 12=1855/0-3-8 (min. 0-2-11) Max Horz 2=-117(LC 15) Max Uplift2=-82(LC 14), 12=-82(LC 15) Max Grav 2=2251(LC 39), 12=2251(LC 39)										
FORCES. (TOP CHOR	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-3938/200, 3-20=-3846/216, 4-20=-3814/226, 4-21=-3706/215, 21-22=-3577/228, 5-22=-3474/231, 5-6=-2959/255, 6-23=-2544/254, 7-23=-2544/254, 8-24=-2544/254, 8-9=-2959/255, 9-25=-3474/231, 25-26=-3577/228, 10-26=-3706/215,										
BOT CHOR	D 2-19=-38 17-30=-33/ 32-33=-94/	14/226, 11-27=-3846/21 3338, 19-28=-94/3061, 2560, 30-31=-33/2560, 3061, 14-33=-94/3061.	6, 11-12=-393 28-29=-94/30 16-31=-33/25 12-14=-139/3	88/200 61, 18-29=- 60, 15-16=- 338	94/3061, 17 94/3061, 15	7-18=-94/3061, 5-32=-94/3061,					
WEBS	5-19=-3/39 9-16=-879/	1, 5-17=-879/162, 6-17= 162, 9-14=-3/391	-26/970, 7-17	7=-311/149,	7-16=-311/	149, 8-16=-26/970),				
 NOTES- (11) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-8-4 to 4-1-5, Interior(1) 4-1-5 to 11-7-1, Exterior(2R) 11-7-1 to 33-8-15, Interior(1) 33-8-15 to 41-2-11, Exterior(2E) 41-2-11 to 46-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Unbalanced snow loads have been considered for this design. 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) All plates are 5x5 MT20 unless otherwise indicated. 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 9) This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12. 											
LOAD CASE	E(S) Standard									7/24/202.	3



- REACTIONS. (lb/size) 2=1930/0-3-8 (min. 0-2-12), 12=1888/0-3-8 (min. 0-2-12) Max Horz 2=-121(LC 15) Max Uplift2=-44(LC 14), 12=-35(LC 15) Max Grav 2=2347(LC 45), 12=2312(LC 45)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-4177/130, 3-23=-4100/145, 4-23=-4073/156, 4-24=-4003/144, 24-25=-3794/156,
- 5-25=-3771/159, 5-6=-3297/175, 6-26=-2848/182, 7-26=-2848/182, 7-27=-2848/182, 8-27=-2848/182, 8-9=-3297/175, 9-28=-3773/163, 28-29=-3876/160, 10-29=-4005/147,
 - 10-30=-4059/161, 11-30=-4102/150, 11-12=-4179/135
- BOT CHORD 2-22=-116/3551, 22-31=-17/3344, 31-32=-17/3344, 21-32=-17/3344, 20-21=-17/3344, 20-33=0/2874, 16-33=0/2874, 16-34=0/2874, 34-35=0/2874, 15-35=0/2874, 14-15=-19/3344, 14-36=-19/3344, 36-37=-19/3344, 13-37=-19/3344, 12-13=-75/3554 WEBS 5-22=-12/361. 5-20=-867/168. 6-20=0/1120. 19-20=-347/112. 7-19=-311/147.
- 7-17=-310/150, 15-17=-347/115, 8-15=0/1120, 9-15=-868/169, 9-13=-13/351

NOTES-(11)

1) Unbalanced roof live loads have been considered for this design.

SUMMERTH CAR 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-8-4 to 4-1-5, Interior(1) 4-1-5 to 11-7-1, Exterior(2R) 11-7-1 to 33-8-15, Interior(1) 33-8-15 to 40-6-6, Exterior(2E) 40-6-6 to 45-4-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are 5x5 MT20 unless otherwise indicated.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- AND DIMENSION * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12.

2.4/202 rd and s Warning !—Verify design parameters and read notes before use. This design is based only upon parameters shown, and is to an increase the second state of the second st Zerify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Trusse Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

PROFESS

NOINEE

7/24/2023



F	9-2	-5 -5	9-2-4		4-3-8	+ 26-11-8 + 4-3-8		36-1- 9-2-	-11 -4		45-0-8	
Plate Offsets	(X,Y) [2:0-0	-0,0-3-10], [6:0-6-0,0-2	2-8], [8:0-6-0,0-	2-8]								
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	sf) 20.0 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/	2-0-0 1.15 1.15 YES TPI2014	CSI. TC BC WB Matrix	0.81 0.96 0.46 ←SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.30 -0.50 0.15	(loc) 18 18 12	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 336 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS SLIDER	 2x4 SP No.1 T3: 2x4 SP No.2 2x6 SP No.2 B3: 2x4 SP I 2x4 SP No.3 W6: 2x4 SP Left 2x4 SP SP 	*Except* No.2, T1: 2x6 SP No.2, *Except* No.2 *Except* No.2 No.3 -7 3-4-14, Right 2	. T4: 2x6 SP No x6 SP No.2 -~ 3	o.1 -1-12		BRACING- TOP CHORD BOT CHORD WEBS	Struct Rigid 6-0-0 1 Row MiTe be in Insta	ceiling oc brac v at mic ek reco nstallec allation	ood shea directly cing: 17- dpt mmend during guide.	athing direct applied or 2 -19 5-20, 7 s that Stabili truss erectio	ly applied or 2-2-0 oc -2-0 oc bracing. Exce 7-19, 7-17, 9-15 izers and required croson, in accordance with	purlins. pt: ss bracing Stabilizer
REACTIONS.	REACTIONS. (lb/size) 2=1918/0-3-8 (min. 0-2-12), 12=1877/Mechanical Max Horz 2=-121(LC 19) Max Uplift2=-45(LC 14), 12=-32(LC 15) Max Grav 2=2332(LC 45), 12=2307(LC 45)											
FORCES. (II TOP CHORD	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-4149/129, 3-23=-4071/144, 4-23=-4044/156, 4-24=-3974/143, 24-25=-3765/156, 5-25=-3742/159, 5-6=-3265/174, 6-26=-2820/181, 7-26=-2820/181, 7-27=-2808/181, 8-27=-2808/181, 8-9=-3252/174, 9-28=-3651/161, 28-29=-3754/158, 10-29=-3884/145,											
BOT CHORD	2-22=-117/ 20-33=0/28 14-36=-17/ 5-22=-12/3 7-17=-319/	3526, 22-31=-16/3317 340, 16-33=0/2840, 16 3270, 36-37=-17/3270 62, 5-20=-868/168, 6-2 137, 15-17=-356/102,	, 31-32=-16/33 -34=0/2840, 34 , 13-37=-17/32 20=0/1106, 19- 8-15=0/1101, 9	17, 21-32=-1 -35=0/2840, 70, 12-13=-7 20=-336/120)-15=-819/16	16/3317, 20 15-35=0/2 70/3386), 7-19=-29 \$8, 9-13=-1:	1-21=-16/3317, 840, 14-15=-17/32 9/155, 2/274	270,					
NOTES- (1 1) Unbalance 2) Wind: ASC (envelope) 33-8-15 to plate grip I 3) TCLL: ASC Cat B; Pari 4) Unbalance 5) This truss 10) Provide ad 7) All plates a 8) This truss between th 10) Refer to g 11) From Carl	2) ad roof live load CE 7-16; Vult=) gable end zon 40-2-14, Exter DOL=1.60 CE 7-16; Pr=20 tially Exp.; Cer ad snow loads has been desi shas been desi s has been desi s has been desi s has been desi girder(s) for tru Wecher lical, con	ds have been consider 115mph (3-second gus ne and C-C Exterior(2E rior(2E) 40-2-14 to 45- 0.0 psf (roof LL: Lum D =1.0; Cs=1.00; Ct=1.10 have been considered gned for greater of mir or live loads. ge to prevent water po unless otherwise indica gned for a 10.0 psf bot signed for a 10.0 psf bot signed for a 10.0 psf bot signed for a live load o rd and any other memb ss to truss connection:	ed for this desi st) Vasd=91mp E) -0-8-4 to 4-1- 0-8 zone;C-C for OL=1.15 Plate) for this design n roof live load nding. ated. tom chord live f 30.0psf on th- pers, with BCDI s.	gn. n; TCDL=5.0 5, Interior(1) or members a DOL=1.15); of 12.0 psf of load noncon a bottom cho c = 10.0psf.	Dpsf; BCDL:) 4-1-5 to 1 and forces Pf=20.0 ps r 2.00 times r 2.00 times current with ord in all are	=5.0psf; h=23ft; C 1-7-1, Exterior(2R & MWFRS for rea of (Lum DOL=1.15 s flat roof load of 2 h any other live loa eas where a rectar	at. II; E)) 11-7-1 actions s 5 Plate E 20.0 psf ads. ngle 3-6	xp B; E to 33 shown; DOL=1. on ove 5-0 tall 1	inclosed 8-15, Int Lumber .15); Is= erhangs by 1-0-0	t; MWFRS terior(1) r DOL=1.60 1.0; Rough 1.0; Rough 0 wide will fit	SEAL 28147 7/24/2023 ent to be installed and load	ded
of individual	page 2 pplicability of de web members on	sign parameters and prope	r incorporation of bracing to ensure	component is stability during	responsibility g construction	y of building designer n is the responsibility	r – not tru of the er	uss desig rector.	gner or tr Additiona	uss engineer. Il permanent bi	Bracing shown is for later racing of the overall struct	al support ture is the

responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 38 PROVIDENCE CREEK 38 PROVIDENCE CREEK DRIVE FUQUAY-VA	RI
23-4837-R01	R07	PIGGYBACK BASE	7	1	Job Reference (optional) # 40140	
	·	Run: 8.4 ID:s	30 s Feb 1 co?uVwd	2 2021 Print zN_6Z0rHI	: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Jul 24 16:17:24 2023 Page 2 bcoMyuyZQFk-wYs3ycLBO0mLcmbDHEs9yfiCzRIjKf8muXF?3pyupYf	

LOAD CASE(S) Standard





F	9-2 9-2	2-5 2-5	18-4-8 9-2-4		22-8-0 4-3-8	26-11-8	<u>36-1-11</u> 9-2-4		45-0-8 8-10-13		
Plate Offset	ts (X,Y) [2:0-0	-0,0-3-6], [6:0-6-0,0-2-	8], [8:0-6-0,0-2-8]								
LOADING (TCLL (roof) Snow (Pf) TCDL BCLL BCDL	psf) 20.0 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Inci Code IRC2021/	2-0-0 1.15 1.15 YES TPI2014	CSI. TC BC WB Matri	1.00 0.85 0.40 ix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.28 16-18 >999 -0.46 16-18 >999 0.14 12 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 325 lb	GRIP 244/190 FT = 20%	
LUMBER- TOP CHOR BOT CHOR WEBS SLIDER REACTIONS	LUMBER- BRACING- TOP CHORD 2x4 SP No.2 *Except* TOP CHORD T1,T4: 2x6 SP No.2 BOT CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 BOT CHORD WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No.3 - ~ 3-4-14, Right 2x6 SP No.2 - ~ 3-1-12 WEBS 1 Row at midpt 5-16, 7-16, 7-15, 9-15 REACTIONS. (lb/size) 2=1843/0-3-8 (min. 0-2-10), 12=1801/Mechanical Max Horz 2=-121(LC 19) MiTek recommends that Stabilizer and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. FERECTIONS. (lb/size) 2=1843/0-3-8 (min. 0-2-10), 12=12001(LC 39) 250 (b) and be an and a stabilizer and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.										
FORCES. TOP CHOR BOT CHOR WEBS	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-3914/200, 3-19=-3822/216, 4-19=-3790/227, 4-20=-3681/215, 20-21=-3472/228, 5-21=-3448/231, 5-6=-2930/255, 6-22=-2518/254, 7-23=-2507/254, 8-23=-2507/254, 8-23=-2507/254, 8-23=-2507/254, 8-23=-2507/254, 8-23=-2507/254, 8-23=-2507/254, 8-23=-2507/254, 10-26=-3650/227, 11-26=-3710/223, 11-12=-3794/201 BOT CHORD 2-18=-178/3319, 18-27=-85/3035, 27-28=-85/3035, 17-28=-85/3035, 16-17=-85/3035, 16-29=-23/2528, 15-30=-23/2528, 14-15=-84/2992, 14-31=-84/2992, 31-32=-84/2992, 13-32=-84/2992, 12-13=-129/3183 WEBS 5-18=-3/396, 5-16=-879/162, 6-16=-26/956, 7-16=-301/155, 7-15=-319/139, 8-15=-26/952, 9-16=-871/162, 9-18=-1783/366, 5-16=-871/162, 9-18=-31/366, 5-16=-801/162, 9-18=-301/155, 7-15=-319/139, 8-15=-26/952, 9-18=-31/366, 5-16=-801/162, 9-18=-301/155, 7-15=-319/139, 8-15=-26/952, 9-18=-31/366, 5-16=-801/162, 9-18=-301/155, 7-15=-319/139, 8-15=-26/952, 9-18=-31/366, 5-16=-801/162, 9-16=-301/155, 7-15=-319/139, 8-15=-26/952, 9-18=-31/366, 5-16=-801/162, 9-16=-301/155, 7-15=-319/139, 8-15=-26/952, 9-18=-31/366, 5-16=-801/162, 9-16=-301/155, 7-15=-319/139, 8-15=-26/952, 9-16=-801/162, 9-16=-301/155, 7-15=-319/139, 8-15=-26/952, 9-16=-801/162, 9-16=-301/155, 7-15=-319/139, 8-15=-26/952, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-80/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-801/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-16=-80/162, 9-162, 9-162, 9-162, 9-										
NOTES- 1) Unbaland 2) Wind: AS (enveloped 33-8-15 t plate grip 3) TCLL: AS Cat B; Pa 4) Unbaland 5) This trust non-conc 6) Provide a 7) All plates 8) This trust 9) * This trust between 10) Refer to 11) Provide	(12) ced roof live load SCE 7-16; Vult= e) gable end zoi to 40-2-14, Exte b DOL=1.60 SCE 7-16; Pr=2t artially Exp.; Cei ced snow loads s has been desi surrent with othe adequate drainal s are 5x5 MT20 s has been desi the bottom choro b girder(s) for tru mechanical con	ds have been conside 115mph (3-second gu ne and C-C Exterior(2 rior(2E) 40-2-14 to 45- 0.0 psf (roof LL: Lum I =1.0; Cs=1.00; Ct=1.1 have been considered gned for greater of mi r live loads. ge to prevent water po unless otherwise indic gned for a 10.0 psf bo signed for a 10.0 psf bo signed for a live load of d and any other memi iss to truss connectior nnection (by others) of	red for this design st) Vasd=91mph; E) -0-8-4 to 4-1-5, 0-8 zone;C-C for 00L=1.15 Plate D 0 I for this design. n roof live load of onding. ated. ttom chord live load of 30.0psf on the b pers, with BCDL = s. truss to bearing p	TCDL=5. Interior(1 members OL=1.15) 12.0 psf of ad noncol oottom ch 10.0psf.	0psf; BCDL) 4-1-5 to 1 and forces); Pf=20.0 p or 2.00 time ncurrent with ord in all ar	=5.0psf; h=23ft; C 1-7-1, Exterior(2R & MWFRS for rea sf (Lum DOL=1.15 is flat roof load of 2 th any other live lo eas where a recta standing 100 lb up	at. II; Exp B; Enclosed) 11-7-1 to 33-8-15, Int ctions shown; Lumber Plate DOL=1.15); Is= 20.0 psf on overhangs ads. ngle 3-6-0 tall by 1-0-0 ift at joint(s) 2, 12.	; MWFRS erior(1) DOL=1.60 1.0; Rough	SEAL 28147 7/24/2023	ALL A COMMAND	



⊢	45-0-8											
Plate Offsets (X,Y) [2:0-6-1,0-0-5], [8:0-3-0,Edge], [18:0-6-0,0-2-8], [23:0-3-0,Edge], [38:0-4-0,0-1-4]												
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 20.0 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TI	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matri	0.13 0.04 0.25 ix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 0.00 0.00	(loc) 1 1 29	l/defl n/r n/r n/a	L/d 180 80 n/a	PLATES MT20 Weight: 377 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3 SLIDER Left 2x6 SP No.2 - ~ 1-7-11						BRACING- TOP CHORD BOT CHORD WEBS	Struct end v Rigid 1 Rov	tural w erticals ceiling v at mi	ood shea s. I directly dpt	athing direct applied or 1 18-39, 19-38	tly applied or 6-0-0 oc p 10-0-0 oc bracing. 17-40, 16-41, 15-42, 1	ourlins, except 4-43, 12-44,
 REACTIONS. All bearings 45-0-8. (lb) - Max Horz 2=128(LC 14) Max Uplift All uplift 100 lb or less at joint(s) 2, 40, 41, 42, 44, 46, 47, 48, 49, 50, 51, 52, 38, 36, 35, 34, 33, 32, 31, 30 Max Grav All reactions 250 lb or less at joint(s) 29, 2, 39, 50, 51, 52, 32, 31, 30 except 40=310(LC 44), 41=289(LC 44), 42=298(LC 44), 43=282(LC 52), 44=295(LC 47), 46=293(LC 45), 47=292(LC 45), 48=291(LC 45), 49=298(LC 45), 38=285(LC 49), 36=292(LC 45), 35=292(LC 45), 34=290(LC 45), 33=300(LC 45) 												
FORCES. (Ib	FORCES. (Ib) - Max. Comp./Max. Ten All forces 250 (Ib) or less except when shown.											

NOTES-(14)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 3-11-2, Exterior(2N) 3-11-2 to 13-6-14, Corner(3R) 13-6-14 to 31-9-2, Exterior(2N) 31-9-2 to 40-1-2, Corner(3E) 40-1-2 to 44-10-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- PROFESSION 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

LOAD CASE(S) Standard

24/202 rd and Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Trusse Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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7/24/2023











 Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-6-8 to 5-4-1, Exterior(2R) 5-4-1 to 11-1-1, Exterior(2E) 11-1-1 to 15-10-11 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

between the bottom chord and any other members

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 6.

LOAD CASE(S) Standard









- **NOTES-** (9)
- 1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard





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NOTES- (9)
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- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

