

RE: MP10

DRHORTON/WILMINGTON; LOT 10 MCKAY PLACE

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: MP10 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.6 Wind Speed: 120 mph Floor Load: N/A psf

This package includes 12 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	159163116	A01AG	6/26/2023
2	159163117	A02	6/26/2023
3	159163118	A02A	6/26/2023
4	159163119	A03	6/26/2023
5	159163120	A04V	6/26/2023
6	159163121	A05AV	6/26/2023
7	159163122	A05V	6/26/2023
8	159163123	A06AVG	6/26/2023
9	159163124	B01G	6/26/2023
10	159163125	B02GR	6/26/2023
11	159163126	P01G	6/26/2023
12	159163127	P02	6/26/2023

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Builders FirstSource-Apex,NC.

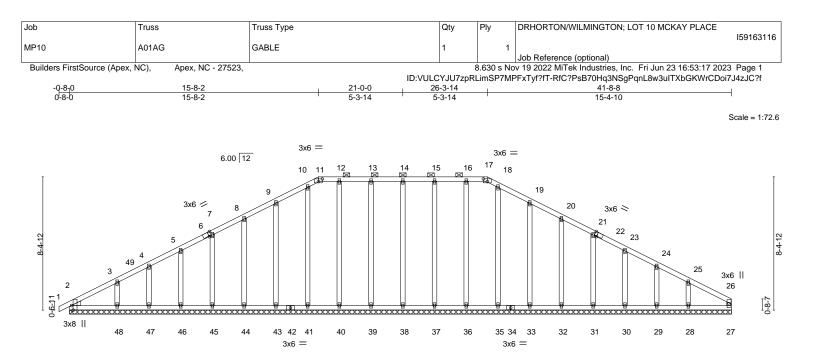
Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2023

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





L			41-8-8				J
I			41-8-8				
late Offsets (X,Y)	[2:0-3-8,Edge], [6:0-1-13,Edge], [11:0-3	3-0,0-2-0], [17:0-3-0,0-2-0]	, [22:0-1-13,Edge]				
.OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. ir	n (loc)	l/defl L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.10	Vert(LL) -0.00) 1	n/r 120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.07	Vert(CT) 0.00) 1	n/r 120		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.16	Horz(CT) 0.01	27	n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S				Weight: 284 lb	FT = 20%
UMBER-			BRACING-				
FOP CHORD 2x4 SF	P No.2		TOP CHORD	Structura	al wood sheathing dir	ectly applied or 6-0-0	oc purlins,
BOT CHORD 2x4 SF	P No.2					0 oc purlins (6-0-0 ma	
WEBS 2x4 SF	P No.3		BOT CHORD		ling directly applied of		
OTHERS 2x4 SF	P No 3			g			
NEDGE	11010						

Left: 2x4 SP No.3

- Max Uplift All uplift 100 lb or less at joint(s) 2, 38, 39, 40, 43, 44, 45, 46, 47, 48, 37, 36, 33, 32, 31, 30, 29, 28
- Max Grav All reactions 250 lb or less at joint(s) 27, 2, 38, 39, 40, 41, 43, 44, 45, 46, 47, 48, 37, 36, 35, 33, 32, 31, 30, 29, 28

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-0 to 4-1-10, Exterior(2) 4-1-10 to 15-8-2, Corner(3) 15-8-2 to 20-5-12, Exterior(2) 20-5-12 to 26-3-14, Corner(3) 26-3-14 to 31-0-0, Exterior(2) 31-0-0 to 41-6-12 zone; cantilever left and right exposed; c-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 38, 39, 40, 43, 44, 45, 46, 47, 48, 37, 36, 33, 32, 31, 30, 29, 28.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

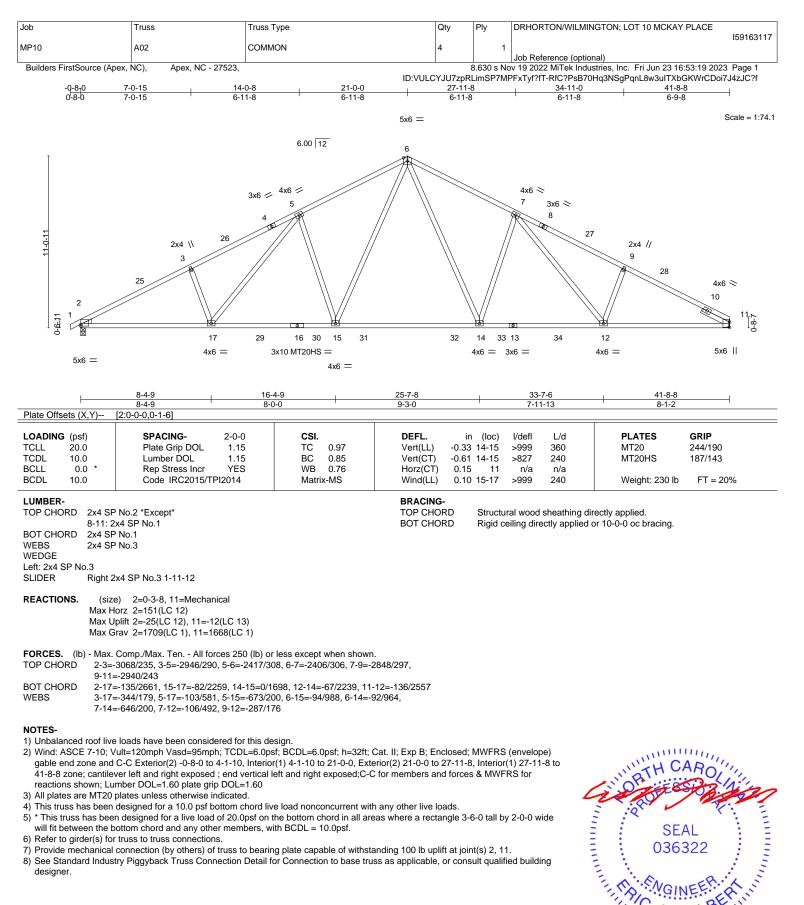


TRENCO A MITEK Atfiliate 818 Soundside Road

Edenton, NC 27932

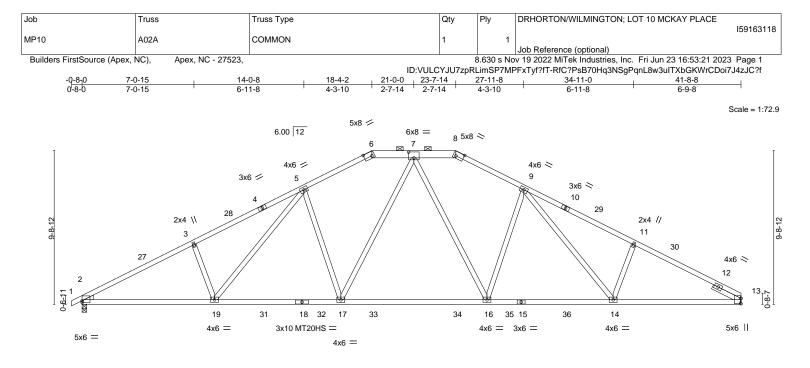
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

REACTIONS. All bearings 41-8-8. (Ib) - Max Horz 2=121(LC 16)



June 26,2023

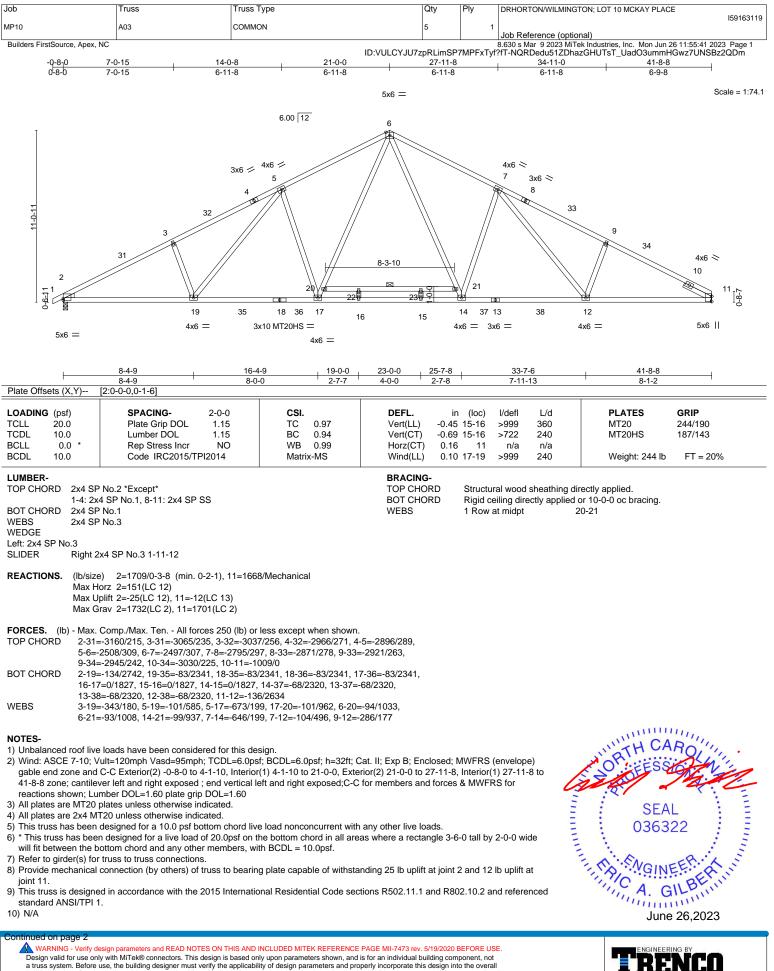
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L	8-4-9	16-4-9	25-7-8	33-7-6	41-8-8
	8-4-9	8-0-0	9-3-0	7-11-13	8-1-2
Plate Offsets (X,Y)	[2:0-0-0,0-1-6], [6:0-5-14,0	-3-4], [7:0-4-0,0-4-8], [8:0-5-14,0-3-	4]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 CODI 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 CSI. 1.15 TC 0.97 1.15 BC 0.87 YES WB 0.60	Vert(LL) -0.40 1 Vert(CT) -0.77 1 Horz(CT) 0.15	16-17 >649 240 13 n/a n/a	PLATES GRIP MT20 244/190 MT20HS 187/143
BCDL 10.0	Code IRC2015/TPI	2014 Matrix-MS	Wind(LL) 0.14	17 >999 240	Weight: 230 lb FT = 20%
6-8: 2xi BOT CHORD 2x4 SP WEBS 2x4 SP WEDGE Left: 2x4 SP No.3			2	Structural wood sheathing dir 2-0-0 oc purlins (3-2-2 max.): Rigid ceiling directly applied c	6-8.
Max H Max U Max G FORCES. (lb) - Max. TOP CHORD 2-3=- 11-13 BOT CHORD 2-19= WEBS 3-19=	3064/269, 3-5=-2945/324, 3=-2935/276, 6-7=-2066/32 190/2660, 17-19=-122/22	C 13) 58(LC 1) es 250 (lb) or less except when sho 5-6=-2355/339, 8-9=-2346/336, 9-1 5, 7-8=-2056/322 18, 16-17=-28/1710, 14-16=-107/21 5-17=-534/187, 7-17=-82/858, 7-16	1=-2847/331, 199, 13-14=-168/2556		
 Wind: ASCE 7-10; V gable end zone and 41-8-8 zone; cantileu reactions shown; Luu Provide adequate dr All plates are MT20 This truss has been * This truss has been * This truss has been * Refer to girder(s) for Provide mechanical See Standard Indust designer. 	C-C Exterior(2) -0-8-0 to 4 ver left and right exposed ; mber DOL=1.60 plate grip plates unless otherwise inc designed for a 10.0 psf bo n designed for a 10.0 psf bo n designed for a live load c ottom chord and any other truss to truss connections connection (by others) of t ry Piggyback Truss Connections	TCDL=6.0psf; BCDL=6.0psf; h=32 -1-10, Interior(1) 4-1-10 to 18-4-2, E end vertical left and right exposed; DOL=1.60 mding. ticated. ttom chord live load nonconcurrent v f 20.0psf on the bottom chord in all members, with BCDL = 10.0psf.	Exterior(2) 18-4-2 to 30-5-5, In C-C for members and forces & with any other live loads. areas where a rectangle 3-6-0 nstanding 100 lb uplift at joint(truss as applicable, or consul	nterior(1) 30-5-5 to & MWFRS for	SEAL 036322



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besign valid bit des only with with with exercise constructions. This design is based only upon parameters shown, and is for an individual biting component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	DRHORTON/WILMINGTON; LOT 10 MCKAY PLACE	
MP10	A03	COMMON	5	1		l59163119
Builders FirstSource, Ape	ex. NC				Job Reference (optional) 8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Jun 26 11:55	:41 2023 Page 2
NOTES-			ID:VULCYJU7zpRLimSP	7MPFxTyf	?fT-NQRDedu51ZDhazGHUTsT_UadO3ummHGwz7U	JNSBz2QDm
11) See Standard In		ction Detail for Connection to base tr the face of the truss are noted as fro		sult qualifi	ied building designer.	
LOAD CASE(S) 1) Dead + Roof Live Uniform Loads (p	(balanced): Lumber Increase=	1.15, Plate Increase=1.15				
2) Dead + 0.75 Root Uniform Loads (p	lf)	b. Attic Storage: Lumber Increase=1.				
 Dead + Uninhabit Uniform Loads (p 	able Attic Without Storage: Lur	36=-50, 36-37=-20, 37-38=-50, 27-38 ber Increase=1.25, Plate Increase=1	, , , , , , , , , , , , , , , , , , , ,			
4) Dead + 0.6 C-C V Uniform Loads (p	Vind (Pos. Internal) Case 1: Lui	nber Increase=1.60, Plate Increase=	1.60			
Horz: 1-2	2=-59, 2-31=-37, 6-31=-26, 6-7= Vind (Pos. Internal) Case 2: Lui		1.60			
Vert: 1-2 Horz: 1-2	9, 2-5=14, 5-6=25, 6-34=14, 1 2=-21, 2-5=-26, 5-6=-37, 6-34=2		1 60			
Uniform Loads (p Vert: 1-2						
7) Dead + 0.6 C-C V Uniform Loads (p	Vind (Neg. Internal) Case 2: Lu	mber Increase=1.60, Plate Increase= =-20	1.60			
	. ,	imber Increase=1.60, Plate Increase	=1.60			
Horz: 1-2 9) Dead + 0.6 MWF	, , , , , , , , , , , , , , , , , , ,	umber Increase=1.60, Plate Increas	e=1.60			
Horz: 1-2	2=4, 2-6=9, 6-11=-2, 24-27=-12 2=-16, 2-6=-21, 6-11=10		4.00			
Uniform Loads (Vert: 1-		Lumber Increase=1.60, Plate Increas =-20	e=1.60			
11) Dead + 0.6 MWI Uniform Loads (FRS Wind (Neg. Internal) Right	: Lumber Increase=1.60, Plate Increa	ase=1.60			
12) Dead + 0.6 MWI Uniform Loads (plf)	arallel: Lumber Increase=1.60, Plate	Increase=1.60			
Horz: 1	· · · · ·		Increase=1.60			
Vert: 1- Horz: 1-	2=-2, 2-6=3, 6-33=11, 11-33=2 -2=-10, 2-6=-15, 6-33=23, 11-3		Increase=1 60			
Uniform Loads (Vert: 1-	. , , , , , , , , , , , , , , , , , , ,					
Uniform Loads (. , , , , , , , , , , , , , , , , , , ,	arallel: Lumber Increase=1.60, Plate	Increase=1.60			
16) Dead + 0.6 MWI Uniform Loads (plf)	arallel: Lumber Increase=1.60, Plate	Increase=1.60			
Horz: 1- 17) Dead + 0.6 MWI			Increase=1.60			
Horz: 1	2=-10, 2-6=-15, 6-33=-6, 11-33 -2=-10, 2-6=-5, 6-33=14, 11-33					
Uniform Loads (Vert: 1-	plf) 6=-20, 6-11=-20, 24-35=-20, 38	5-36=-60, 36-37=-20, 37-38=-60, 27-3 ttic Storage + 0.75(0.6 MWFRS Wind		Increase=	=1.60. Plate	
Increase=1.60 Uniform Loads (plf)	5=-20, 35-36=-50, 36-37=-20, 37-38-				
	-2=-4, 2-6=-0, 6-11=8	,	, , , , , , , , , , , , , , , , , , , ,	· /		

ntinued on page 3

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MP10 A03 COMMON 5 1	
	I59163119
Job Reference (optional)	

Builders FirstSource, Apex, NC

8.630 s Mar 9 2023 MiTek Industries, Inc. Mon Jun 26 11:55:41 2023 Page 3 ID:VULCYJU7zpRLimSP7MPFxTyf?fT-NQRDedu51ZDhazGHUTsT_UadO3ummHGwz7UNSBz2QDm

LOAD CASE(S)

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf) Vert: 1-2=-38, 2-6=-42, 6-11=-50, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

Horz: 1-2=-12, 2-6=-8, 6-11=0

21) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-28, 2-32=-32, 6-32=-40, 6-11=-46, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

Horz: 1-2=-22, 2-32=-18, 6-32=-10, 6-11=4

22) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-43, 2-6=-46, 6-33=-40, 11-33=-32, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

- Horz: 1-2=-7, 2-6=-4, 6-33=10, 11-33=18 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf)
 - Vert: 1-6=-60, 6-11=-20, 24-27=-20
- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

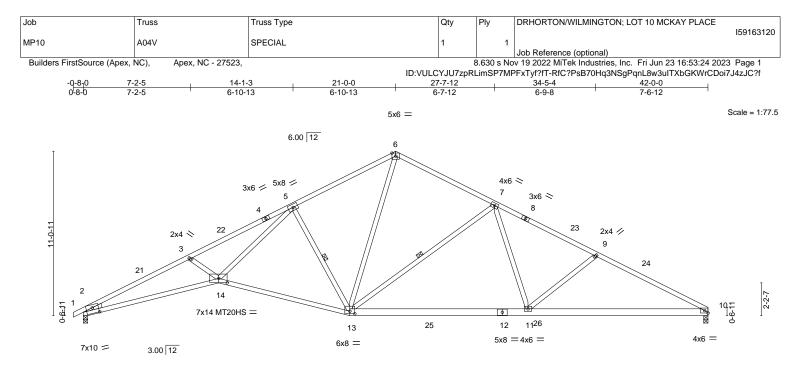
Vert: 1-6=-20, 6-11=-60, 24-27=-20

- 25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
- Vert: 1-6=-50, 6-11=-20, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)
- 26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-20, 6-11=-50, 24-35=-20, 35-36=-50, 36-37=-20, 37-38=-50, 27-38=-20, 20-21=-30(F)

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F	<u>9-1-4</u> 9-1-4		17-11-0 8-9-12		29-10-12				<u>42-0-0</u> 12-1-4	
Plate Offsets (X,	Y) [2:0-5-0,0-4-8], [13:0-4-	0,0-2-0], [14:0-	6-12,0-3-8]							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES IPI2014	CSI. TC BC WB Matrix	0.93 0.86 0.70 (-MS	- ()	in (loc) -0.44 11-13 -0.94 13-14 0.35 10 0.23 13-14	l/defl >999 >537 n/a >999	L/d 360 240 n/a 240	PLATES MT20 MT20HS Weight: 236 lb	GRIP 244/190 187/143 FT = 20%
BOT CHORD	2x4 SP SS *Except* 8-10: 2x4 SP No.2 2x6 SP No.2 *Except* 2-14: 2x4 SP SS, 13-14: 2x4 S 2x4 SP No.3 *Except* 5-14: 2x4 SP No.2 3	P No.1			BRACING- TOP CHORI BOT CHORI WEBS	D Rigid o			directly applied. d or 10-0-0 oc bracing. 5-13, 7-13	
	(size) 2=0-3-8, 10=0-3-8 Max Horz 2=149(LC 16) Max Uplift 2=-24(LC 12), 10=- Max Grav 2=1720(LC 1), 10=-									
FORCES. (lb) TOP CHORD BOT CHORD WEBS	- Max. Comp./Max. Ten All fr 2-3=-5379/348, 3-5=-5101/32 9-10=-3079/265 2-14=-262/4846, 13-14=-115 3-14=-264/184, 5-14=-77/286 7-11=0/671, 9-11=-410/186	27, 5-6=-2199/2 /2602, 11-13=-	287, 6-7=-1766 55/2193, 10-1	6/270, 7-9=-2 1=-148/2671	728/239,					
2) Wind: ASCE gable end zor	oof live loads have been consi 7-10; Vult=120mph Vasd=95m ne and C-C Exterior(2) -0-8-0 tr cantilever left and right expose	ph; TCDL=6.0p o 4-1-10, Interio d ; end vertical	osf; BCDL=6.0 or(1) 4-1-10 to	21-0-0, Exte	rior(2) 21-0-0 to 27	7-7-12, Interio	r(1) 27-7-	be) 12 to	ORTH C	AD LAND

reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

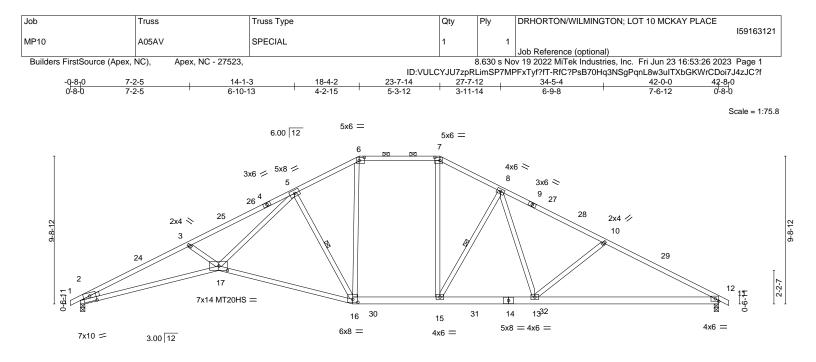
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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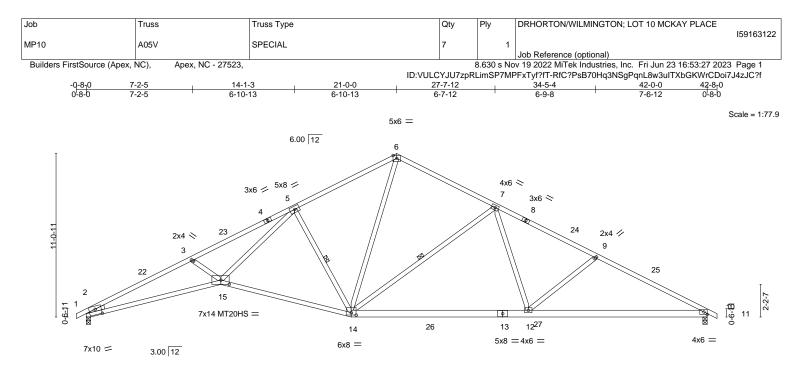


9:14 8:9:12 5:8:14 6:2:14 12:1.4 12:1.4 Plate Offsets (X,Y) [2:0-5:0,0-4:8], [6:0-4:0,0-2:8], [7:0-4:0,0-2:8], [16:0-4:0,0-2:4], [17:0-6:12,0-3:8] DEFL. in (loc) Vdeft L/d PLATES GRIP TCLL 20:0 Plate Grip DOL 1.15 TC 1.00 Vert(LL) -0:58 16:17 >875 360 MT20 244/190 TCDL 0.0 Lumber DOL 1.15 BC 0.95 Vert(CT) -1:10 16:17 >475 360 MT20 244/190 BCLL 0.0 * Rep Stress Incr YES WB 0.71 Horz(CT) 0.35 11 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16:17 >999 240 Weight: 241 lb FT = 20% LUMBER- 2:44 SP No.1 *Except* TOP CHORD 2:45 SP No.2 Structural wood sheathing directly applied, except 2:0:0 co putins (3:6-6 max): 6:7. BOT CHORD 2:44 SP No.3 *Except* Structural w
LOADING (psf) TCLL SPACING- 20.0 2-0.0 Plate Grip DOL CSI. 1.15 DEFL. in (loc) V/defl L/d TCLL 20.0 Plate Grip DOL 1.15 TC 1.00 Vert(LL) -0.58 16-17 >875 360 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 BC 0.95 Wert(CT) -1.10 16-17 >875 360 MT20 244/190 BCDL 10.0 Rep Stress Incr YES WB 0.71 Wort(CT) -1.10 16-17 >879 240 WE20HS 187/143 BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16-17 >999 240 Weight: 241 lb FT = 20% LUMBER- TOP CHORD 2x4 SP No.1 *Except* 2-0-0 oc purlins (3-6-4 max.): 6-7. BOT CHORD Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-6-4 max.): 6-7. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 13-15. WEBS 1 Row at midpt 5-16, 8-15 5-16, 8-15
TCLL 20.0 Plate Grip DOL 1.15 TC 1.00 Vert(LL) -0.58 16-17 >875 360 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 BC 0.95 WB 0.71 Horz(CT) 0.35 11 n/a MT20HS 187/143 BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16-17 >999 240 Weight: 241 lb FT = 20% LUMBER- Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16-17 >999 240 Weight: 241 lb FT = 20% LUMBER- Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16-17 >999 240 Weight: 241 lb FT = 20% LUMBER- Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16-17 >999 240 Weight: 241 lb FT = 20% LUMBER- Code IRC2015/TPI2014 Matrix-MS Structural wood sheathing directly applied, except 2-0-0 co purlins (3-6-4 max.): 6-7. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 13-15. WEBS 1 Row at mid
TCDL 10.0 Lumber DOL 1.15 BC 0.95 Vert(CT) -1.10 16-17 >458 240 MT20HS 187/143 BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16-17 >458 240 MT20HS 187/143 BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16-17 >458 240 Weight: 241 lb FT = 20% LUMBER- TOP CHORD 2x4 SP No.1 *Except* 2-00 cc purlins (3-64 max.): 6-7. BOT CHORD Structural wood sheathing directly applied, except 2-00 oc purlins (3-64 max.): 6-7. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 13-15. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 13-15. WEBS 1 Row at midpt 5-16, 8-15 5-16, 8-15 WEDGE Left: 2x4 SP No.3 Keactions. (size) 2=0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13) 1 No 1 No 1 1 1 1
BCLL BCDL0.0 * Code IRC2015/TPI2014Rep Stress Incr YES Code IRC2015/TPI2014WB0.71 Matrix-MSHorz(CT) Wind(LL)0.3511 n/a N/an/a N/aLUMBER- TOP CHORDTOP CHORD2x4 SP No.1 *Except* 1-4,9-12: 2x4 SP SS, 6-7: 2x4 SP No.2BOT CHORD2x6 SP No.2 *Except* 2-17: 2x4 SP SS, 16-17: 2x4 SP No.1WEBS2x4 SP No.3 *Except*
BCDL 10.0 Code IRC2015/TPI2014 Matrix-MS Wind(LL) 0.31 16-17 >999 240 Weight: 241 lb FT = 20% LUMBER- TOP CHORD 2x4 SP No.1 *Except* 1-4,9-12: 2x4 SP SS, 6-7: 2x4 SP No.2 BRACING- TOP CHORD TOP CHORD 2x6 SP No.2 *Except* 2-0-0 oc purlins (3-6-4 max.): 6-7. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-17: 2x4 SP SS, 16-17: 2x4 SP No.1 WEBS 2x4 SP No.3 *Except* 5-17: 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 13-15. WEBS 1 Row at midpt 5-16, 8-15 WEDGE Left: 2x4 SP No.3 Structural wood sheathing tirectly applied or 10-0-3-8 Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13) Matrix-MS Weight: 241 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.1 *Except* 1-4,9-12: 2x4 SP SS, 6-7: 2x4 SP No.2 BRACING- TOP CHORD Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-6-4 max.): 6-7. BOT CHORD 2x6 SP No.2 *Except* 2-17: 2x4 SP SS, 16-17: 2x4 SP No.1 BOT CHORD Rigid ceiling directly applied, except 2-2-0 oc bracing, Except: 2-2-0 oc bracing: 13-15. WEBS 2x4 SP No.3 *Except* 5-17: 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied, except 2-2-0 oc bracing: 13-15. WEDGE Left: 2x4 SP No.3 Except* 5-16, 8-15 5-16, 8-15 MEACINDS. (size) 2=0-3-8, 11=0-3-8 Max Horz Max Horz Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13) Max Horz
TOP CHORD 2x4 SP No.1 *Except* TOP CHORD Structural wood sheathing directly applied, except 1-4,9-12: 2x4 SP SS, 6-7: 2x4 SP No.2 BOT CHORD 2x6 SP No.2 *Except* BOT CHORD 2-17: 2x4 SP SS, 16-17: 2x4 SP No.1 BOT CHORD Rigid ceiling directly applied or 10-0 oc bracing, Except: 2-17: 2x4 SP No.3 *Except* BOT CHORD WEBS 1 Row at midpt 5-17: 2x4 SP No.2 WEBS 1 Row at midpt 5-16, 8-15 WEDGE Left: 2x4 SP No.3 Except 2-0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13) Structural wood sheathing directly applied or 10-0 oc bracing, Except:
TOP CHORD 2x4 SP No.1 *Except* 1-4,9-12: 2x4 SP SS, 6-7: 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-6-4 max.): 6-7. BOT CHORD 2x6 SP No.2 *Except* 2-17: 2x4 SP SS, 16-17: 2x4 SP No.1 BOT CHORD Rigid ceiling directly applied or 10-0 oc bracing, Except: 2-2-0 oc bracing: 13-15. WEBS 2x4 SP No.3 *Except* 5-17: 2x4 SP No.2 WEBS 1 Row at midpt 5-16, 8-15 WEDGE Left: 2x4 SP No.3 Except 2-0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13) Max Horz 2=134(LC 12)
1-4,9-12: 2x4 SP SS, 6-7: 2x4 SP No.2 2-0-0 oc purlins (3-6-4 max.): 6-7. BOT CHORD 2x6 SP No.2 *Except* BOT CHORD 2-17: 2x4 SP SS, 16-17: 2x4 SP No.1 BOT CHORD WEBS 2x4 SP No.3 *Except* 2-2-0 oc bracing: 13-15. WEDGE 2-17: 2x4 SP No.2 Left: 2x4 SP No.3 Except* REACTIONS. (size) 2=0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13)
BOT CHORD 2x6 SP No.2 *Except* 2-17: 2x4 SP SS, 16-17: 2x4 SP No.1 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 13-15. WEBS 2x4 SP No.3 *Except* 5-17: 2x4 SP No.2 WEBS 1 Row at midpt 5-16, 8-15 WEDGE Left: 2x4 SP No.3 REACTIONS. (size) 2=0-3-8, 11=0-3-8 Max Horz Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13)
2-17: 2x4 SP SS, 16-17: 2x4 SP No.1 WEBS 2x4 SP No.3 *Except* 5-17: 2x4 SP No.2 WEDGE Left: 2x4 SP No.3 REACTIONS. (size) 2=0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13)
WEBS 2x4 SP No.3 *Except* WEBS 1 Row at midpt 5-16, 8-15 WEDGE Left: 2x4 SP No.3 REACTIONS. (size) 2=0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13) Kenter State
WEDGE Left: 2x4 SP No.3 REACTIONS. (size) 2=0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13)
Left: 2x4 SP No.3 REACTIONS. (size) 2=0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13)
REACTIONS. (size) 2=0-3-8, 11=0-3-8 Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13)
Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13)
Max Horz 2=134(LC 12) Max Uplift 2=-15(LC 12), 11=-15(LC 13)
Max Uplift 2=-15(LC 12), 11=-15(LC 13)
Max Grav 2=1720(LC 1), 11=1720(LC 1)
FORCES. (Ib) - Max. Comp./Max. Ten All forces 250 (Ib) or less except when shown.
TOP CHORD $2-3=-5388/427, 3-5=-5102/409, 5-6=-2190/315, 7-8=-2133/319, 8-10=-2709/279,$
10-11=-3069/299. 6-7=-1857/302
BOT CHORD 2-17-320/3/259, 01/2-10/1/302 BOT CHORD 2-17-320/4856, 16-17-140/2587, 15-16=-19/1857, 13-15=-86/2183, 11-13=-190/2665
WEBS 3-17=-269/190, 5-17=-117/2875, 5-16=-1312/220, 6-16=-50/712, 8-15=-802/163,
8-13=-41(631, 10-13=-431/186, 7-15=-51/741 8-13=-41(631, 10-13=-431/186, 7-15=-51/741
0-13=-41/031, 10-13=-431/100, 1-13=-31/141
NOTES-
 Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 4-1-10 to 18-4-2, Exterior(2) 18-4-2 to 30-5-5, Interior(1) 30-5-5 to
2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope)
gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10. Interior(1) 4-1-10 to 18-4-2. Exterior(2) 18-4-2 to 30-5-5. Interior(1) 30-5-5 to
42-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for
reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
will fit between the bottom chord and any other members, with BCDL = 10.0psf.
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capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11.
9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building
designer.
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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June 26,2023





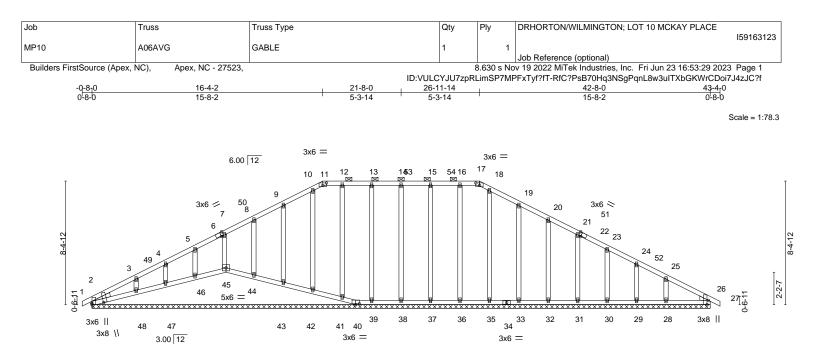
 	9-1-4		-11-0	29-10-12					42-0-0	
Plate Offsets (X,Y)	9-1-4 [2:0-5-0,0-4-8], [14:0-4-0,0		9-12 12 0-3-8]	11-11-12	2		•		12-1-4	
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.93		-0.43 1		>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.86		-0.93 1		>542	240	MT20HS	187/143
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.70	Horz(CT)	0.35	10	n/a	n/a		
BCDL 10.0	Code IRC2015/TP	12014	Matrix-MS	Wind(LL)	0.23 1	4-15	>999	240	Weight: 237 lb	FT = 20%
BOT CHORD 2x6 S 2-15: WEBS 2x4 S	SP SS SP No.2 *Except* 2x4 SP SS, 14-15: 2x4 SP P No.3 *Except* 2x4 SP No.2	No.1		BRACING- TOP CHORI BOT CHORI WEBS	D F		ling dire	ctly applied	rectly applied. or 10-0-0 oc bracing. 5-14, 7-14	
WEDGE										
Left: 2x4 SP No.3										
Max Max	ize) 2=0-3-8, 10=0-3-8 Horz 2=145(LC 16) Uplift 2=-24(LC 12), 10=-24 Grav 2=1720(LC 1), 10=172	· /								
TOP CHORD 2-3	x. Comp./Max. Ten All forc =-5378/342, 3-5=-5100/318, 0=-3077/255									
WEBS 3-1	5=-228/4845, 14-15=-93/260 5=-264/185, 5-15=-69/2861, 2=0/671, 9-12=-412/186			4=-893/139,						
NOTES-										
 Wind: ASCE 7-10; gable end zone an 42-8-0 zone; cantil reactions shown; L 	ve loads have been conside Vult=120mph Vasd=95mph id C-C Exterior(2) -0-8-0 to 4 lever left and right exposed ; .umber DOL=1.60 plate grip 0 plates unless otherwise inc	; TCDL=6.0psi I-1-10, Interior ; end vertical le DOL=1.60	f; BCDL=6.0psf; h=32ft; (1) 4-1-10 to 21-0-0, Ext	erior(2) 21-0-0 to 27	7-7-12, I	Interior(1) 27-7-1		NIN TH C	AROLIN

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5) will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.
 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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[2:0-1-8,0-9-8], [2:0-0-14,Edge], [6:0-1-13,Edge], [11:0-3-0,0-2-0], [17:0-3-0,0-2-0], [22:0-1-13,Edge], [26:0-3-8,Edge], [40:0-3-0,0-0-12]

DEFL

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

in (loc)

27

27

26

0.00

0.00

0.01

42-8-0

24-1-0

L/d

120

120

n/a

2-0-0 oc purlins (6-0-0 max.): 11-17

Rigid ceiling directly applied or 10-0-0 oc bracing.

l/defl

n/r

n/r

n/a

12)	Graphical purlin re	epresentation does not de	pict the size or the orientation of the r	purlin along the top and/or bottom chord.
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18-7-0

8-9-12

CSI.

тс

BC

WB

Max Uplift All uplift 100 lb or less at joint(s) 45, 40, 2, 38, 39, 41, 43, 44, 46, 47, 48, 37, 36, 33, 32, 31,

2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-8-0 to 4-1-10, Interior(1) 4-1-10 to 15-8-2, Exterior(2) 15-8-2 to 22-5-9, Interior(1) 22-5-9 to 26-3-14, Exterior(2) 26-3-14 to 33-0-0, Interior(1) 33-0-0 to 42-8-0 zone; cantilever left and right exposed ; end vertical left and right

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 45, 40, 2, 38, 39,

11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building

exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

All reactions 250 lb or less at joint(s) 45, 40, 2, 26, 38, 39, 41, 42, 43, 44, 46, 47, 48, 37, 36

Matrix-S

0.09

0.07

0.15

2-0-0

1.15

1.15

YES

-0<u>-8-</u>0 0-8-0

20.0

10.0

10.0

TOP CHORD 2x4 SP No.2

(lb) -

2x4 SP No.2

2x4 SP No.3

Max Grav

All bearings 42-0-0. Max Horz 2=123(LC 12)

30, 29, 28

1) Unbalanced roof live loads have been considered for this design.

4) Provide adequate drainage to prevent water ponding.5) All plates are 2x4 MT20 unless otherwise indicated.6) Gable requires continuous bottom chord bearing.

will fit between the bottom chord and any other members.

41, 43, 44, 46, 47, 48, 37, 36, 33, 32, 31, 30, 29, 28,

7) Gable studs spaced at 2-0-0 oc.

35, 33, 32, 31, 30, 29, 28

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

0.0

Plate Offsets (X,Y)-

LOADING (psf)

TCLL

TCDL

BCLL

BCDL

LUMBER-

OTHERS

WEDGE

NOTES-

9)

designer.

BOT CHORD

REACTIONS.

9-9-4 9-1-4

SPACING-

Plate Grip DOL

Rep Stress Incr

Code IRC2015/TPI2014

Lumber DOL



43-4_r0

0-8-0

GRIP

244/190

FT = 20%

PLATES

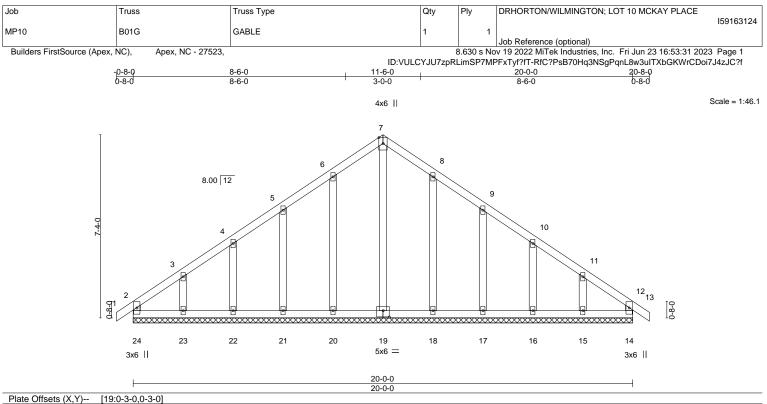
Weight: 275 lb

MT20

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2015/TPI2014	CSI. TC 0.06 BC 0.05 WB 0.13 Matrix-R	DEFL. Vert(LL) -0. Vert(CT) -0. Horz(CT) 0.		l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 120 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP	No.2		BRACING- TOP CHORD	Struct	ural wood	sheathing d	irectly applied or 6-0-0	oc purlins,

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3		

REACTIONS. All bearings 20-0-0.

(lb) - Max Horz 24=167(LC 11)

Max Uplit All uplit 100 lb or less at joint(s) 24, 14, 20, 21, 22, 23, 18, 17, 16, 15 Max Grav All reactions 250 lb or less at joint(s) 24, 14, 19, 20, 21, 22, 23, 18, 17, 16, 15

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

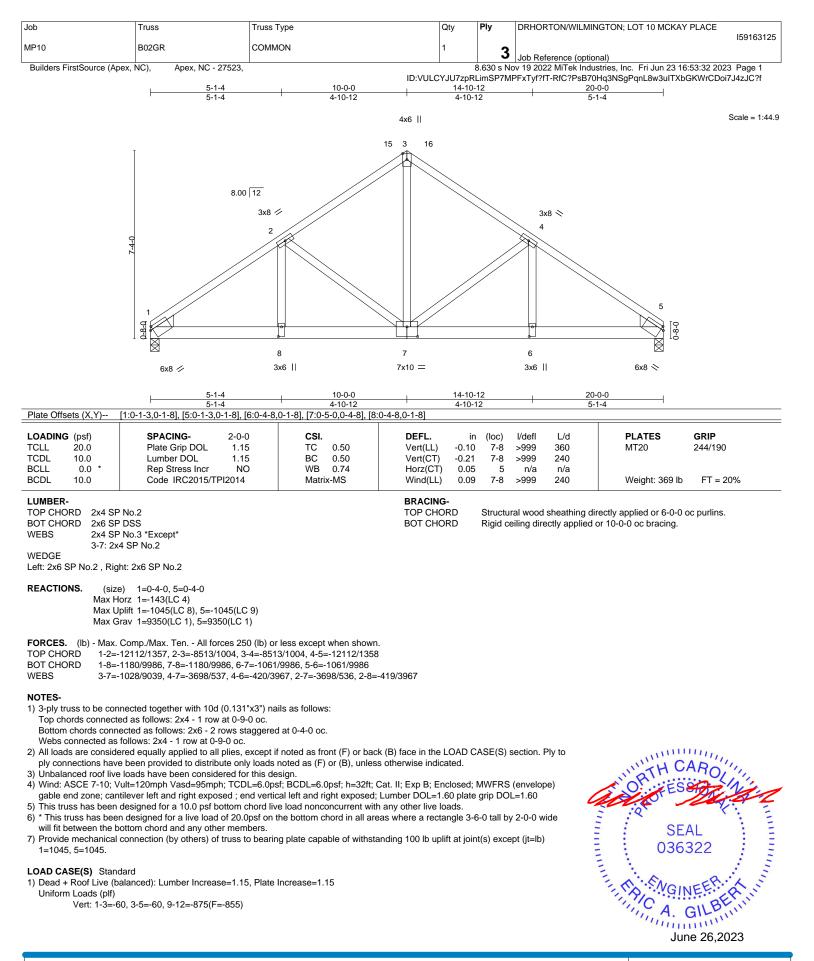
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-0 to 4-0-0, Exterior(2) 4-0-0 to 10-0-0, Corner(3) 10-0-0 to 14-9-10, Exterior(2) 14-9-10 to 20-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 14, 20, 21, 22, 23, 18, 17, 16, 15.



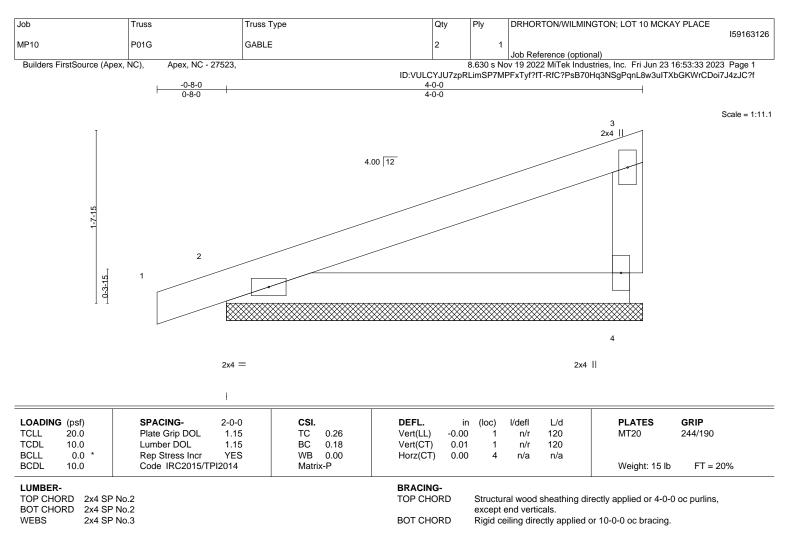
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI (Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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A MiTek A



REACTIONS. (size) 2=4-0-0, 4=4-0-0 Max Horz 2=52(LC 9) Max Uplift 2=-39(LC 8), 4=-21(LC 12) Max Grav 2=198(LC 1), 4=151(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

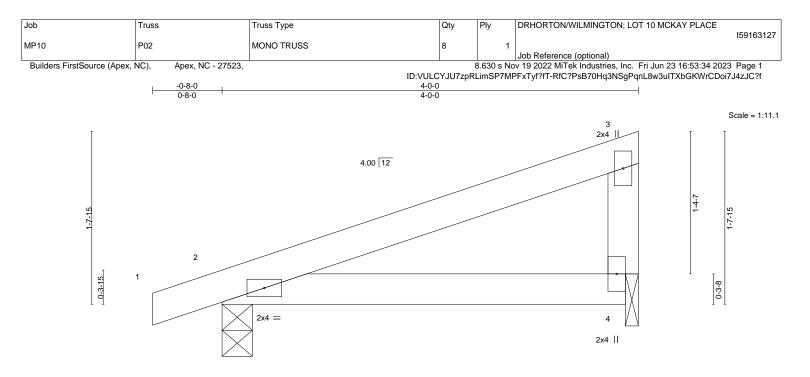
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



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			4-0-0 4-0-0				I
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.19	DEFL. Vert(LL) -0.0	in (loc) 1 4-7	l/defl >999	L/d 360	PLATES GRIP MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.17	Vert(CT) -0.0	2 4-7	>999	240	M120 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-MP	Horz(CT) 0.0 Wind(LL) 0.0		n/a >999	n/a 240	Weight: 15 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS

2x4 SP No.3

REACTIONS. 2=0-3-8, 4=0-1-8 (size) Max Horz 2=52(LC 11) Max Uplift 2=-39(LC 8), 4=-21(LC 12) Max Grav 2=198(LC 1), 4=151(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=120mph Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=32ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



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818 Soundside Road Edenton, NC 27932

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

