Location: ROOF- Hip st Bedroom 2 Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 9.25 IN x 20.75 FT (4.1 + 16.7) (Actual 25.2 FT) 1.9E Microllam - iLevel Trus Joist Section Adequate By: 99.9% Controlling Factor: Moment

Location: ROOF- Valley over Dining Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 9.25 IN x 16.91 FT (4.1 + 12.8) 1.9E Microllam - iLevel Trus Joist Section Adequate By: 40.4% Controlling Factor: Moment

Location: ROOF- Hip over Family Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] 1.5 IN x 9.25 IN x 3.83 FT #2 - Spruce-Pine-Fir (South) - Dry Use Section Adequate By: 111.6% Controlling Factor: Shear

Location: ROOF-Valley at Master Bath Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.5 IN x 11.25 IN x 13.17 FT (5.7 + 7.5) (Actual 15.7 FT) #2 - Spruce-Pine-Fir (South) - Dry Use Section Adequate By: 37.1% Controlling Factor: Moment

Location: ROOF- Hip at laundry Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.5 IN x 11.25 IN x 9.0 FT (Actual 10.6 FT) #2 - Spruce-Pine-Fir (South) - Dry Use Section Adequate By: 14.3% Controlling Factor: Moment

Location: ROOF-Valley at Playroom Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 11.875 IN x 19.08 FT (4.3 + 14.8) (Actual 22.7 FT) 1.9E Microllam - iLevel Trus Joist Section Adequate By: 5.5% Controlling Factor: Moment

Location: FL2- Roof beam at Foyer Combination Roof And Floor Beam [2015 International Building Code(2015 NDS)] (3) 1.5 IN x 9.25 IN x 11.0 FT #2 - Spruce-Pine-Fir (South) - Dry Use Section Adequate By: 17.1% Controlling Factor: Moment



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Location: FL! -Garage Door header Uniformly Loaded Floor Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 16.0 IN x 18.67 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 74.4% Controlling Factor: Moment

Location: FL1-Beam at Rear of garage Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 9.25 IN x 12.0 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 44.7% Controlling Factor: Deflection

Location: FL1-Beam between Dining and Family Uniformly Loaded Floor Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 9.25 IN x 10.0 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 19.6% Controlling Factor: Shear

Location: FL1- Beam at Dining Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] 1.75 IN x 16.0 IN x 10.33 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 6.1% Controlling Factor: Shear

Location: FL1-Beam between Kitchen and Family Uniformly Loaded Floor Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 18.0 IN x 17.83 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 8.5% Controlling Factor: Moment

Location: FLI- Beam at breakfast room Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (3) 1.75 IN x 18.0 IN x 15.0 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 13.6% Controlling Factor: Moment

Location: FL1-Front porch header Combination Roof And Floor Beam [2015 International Building Code(2015 NDS)] (2) 1.5 IN x 9.25 IN x 11.0 FT #2 - Spruce-Pine-Fir (South) - Dry Use Section Adequate By: 28.0% Controlling Factor: Moment

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Location: FL1-Gsrage beam Uniformly Loaded Floor Beam [2015 International Building Code(2015 NDS)] (3) 1.75 IN x 24.0 IN x 24.67 FT Versa-Lam 2800 Fb DF - Boise Cascade Section Adequate By: 116.7% Controlling Factor: Moment

Location: FL1- header at side load garage Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 24.0 IN x 18.67 FT Versa-Lam 2800 Fb DF - Boise Cascade Section Adequate By: 19.2% Controlling Factor: Moment

Shear:

1901 lb

7074 lb

Location: ROOF- Hip st Bedroom 2 Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 9.25 IN x 20.75 FT (4.1 + 16.7) (Actual 25.2 FT) 1.9E Microllam - iLevel Trus Joist Section Adequate By: 99.9% Controlling Factor: Moment



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CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to all members DEFLECTIONS Left Center LOADING DIAGRAM Live Load -0.02 IN L/3637 0.25 IN L/964 Dead Load -0.01 in 0.26 in Total Load -0.03 IN L/1986 0.51 IN L/476 Total Load Deflection Criteria: L/180 Live Load Deflection Criteria: L/240 REACTIONS В С Α Live Load 389 lb 2196 lb 334 lb Dead Load -291 lb 2221 lb 379 lb Total Load 98 lb 4417 lb 713 lb Uplift (1.5 F.S) -936 lb 0 **lb** 0 **lb** TR Bearing Length 0.04 in 1.68 in 0.27 in BEAM DATA Left Center 16.67 ft Span Length 4.08 ft 16.67 ft 4.08 ft Ā Unbraced Length-Top 0 ft 0 ft Unbraced Length-Bottom 4.08 ft 16.67 ft **Beam End Elevation Difference** 14.25 ft UNIFORM LOADS Left Center Live Load Duration Factor 1.15 Uniform Live Load 0 plf 0 plf Notch Depth 0.00 Uniform Dead Load 0 plf 0 plf **MATERIAL PROPERTIES** Beam Self Weight 10 plf 10 plf 1.9E Microllam - iLevel Trus Joist Total Uniform Load 10 10 plf plf **Base Values Adjusted TRAPEZOIDAL LOADS - LEFT SPAN** Bending Stress: 2600 psi Fb' = Fb = 2651 psi Load Number One Cd=1.15 CI=0.86 CF=1.04 Left Live Load 212 plf Shear Stress: 328 psi Fv = 285 psi Fv' = Left Dead Load 159 plf Cd=1.15 **Right Live Load** 177 plf Modulus of Elasticity: F = 1900 ksi F' = 1900 ksi Right Dead Load 132 plf Comp. \perp to Grain: Fc - ⊥ = 750 psi Fc - ⊥' = 750 psi Load Start 0 ft Load End 4.08 ft **Controlling Moment:** -5516 ft-lb Load Length 4.08 ft Over left support of span 2 (Center Span) CENTER SPAN Created by combining all dead loads and live loads on span(s) 1, 2 Load Number One Controlling Shear: 1901 lb Left Live Load 177 plf At left support of span 2 (Center Span) Left Dead Load 132 plf Created by combining all dead loads and live loads on span(s) 1, 2 **Right Live Load** 0 plf Right Dead Load 0 plf Comparisons with required sections: Req'd Provided Load Start 0 ft Section Modulus: 49.91 in3 24.97 in3 Load End 16.67 ft Area (Shear): 8.7 in2 32.38 in2 Load Length 16.67 ft 87.22 in4 Moment of Inertia (deflection): 230.84 in4 Moment: -5516 ft-lb 11026 ft-lb

Location: ROOF- Valley over Dining Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 9.25 IN x 16.91 FT (4.1 + 12.8) 1.9E Microllam - iLevel Trus Joist Section Adequate By: 40.4% Controlling Factor: Moment



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CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to all members DEFLECTIONS Left Center LOADING DIAGRAM Live Load -0.02 IN L/2408 0.26 IN L/593 Dead Load -0.02 in 0.20 in Total Load -0.04 IN L/1374 0.46 IN L/334 Total Load Deflection Criteria: L/180 Live Load Deflection Criteria: L/240 REACTIONS С А В Live Load 69 lb 3345 lb 2151 lb Dead Load -855 lb 2646 lb 1663 lh Total Load -786 lb 5991 lb 3814 lb TR2 Uplift (1.5 F.S) -2039 lb 0 **lb** 0 **Ib** TR1 Bearing Length 0.00 in 2.28 in 1.45 in BEAM DATA Left Center 12.83 ft 4.08 ft Span Length 4.08 ft 12.83 ft A Unbraced Length-Top 0 ft 0 ft Unbraced Length-Bottom 4.08 ft 12.83 ft Live Load Duration Factor 1.15 UNIFORM LOADS Left Center Notch Depth 0.00 Uniform Live Load 0 plf 0 plf MATERIAL PROPERTIES Uniform Dead Load 0 plf 0 plf 1.9E Microllam - iLevel Trus Joist Beam Self Weight 10 plf 10 plf Total Uniform Load 10 plf **Base Values** Adjusted 10 plf Fb' = Bending Stress: Fb = 2600 psi 2926 psi **TRAPEZOIDAL LOADS - LEFT SPAN** Cd=1.15 Cl=0.94 CF=1.04 Load Number One Two Shear Stress: Fv = 285 psi Fv' = 328 psi Left Live Load 0 plf 0 plf Cd=1 15 Left Dead Load 0 plf 0 plf Modulus of Elasticity: 1900 ksi E = 1900 ksi F' = **Right Live Load** 60 plf 52 plf Comp. \perp to Grain: Fc - ⊥ = 750 psi Fc - ⊥' = 750 psi Right Dead Load 39 plf 45 plf Load Start 0 ft 0 ft **Controlling Moment:** -8667 ft-lb Load End 4.08 ft 4.08 ft Over left support of span 2 (Center Span) Load Length 4.08 ft 4.08 ft Created by combining all dead loads and live loads on span(s) 1, 2 **CENTER SPAN Controlling Shear:** -3814 lb Load Number One Two 13.0 Ft from left support of span 2 (Center Span) Left Live Load 60 plf 52 plf Created by combining all dead loads and live loads on span(s) 2 Left Dead Load 45 plf 39 plf **Right Live Load** 320 plf 215 plf Comparisons with required sections: Req'd **Provided** 161 plf Right Dead Load 240 plf Section Modulus: 49.91 in3 35.54 in3 Load Start 0 ft 0 ft 17.45 in2 Area (Shear): 32.38 in2 Load End 12.83 ft 12.83 ft Moment of Inertia (deflection): 230.84 in4 124.49 in4 Load Length 12.83 ft 12.83 ft Moment: -8667 ft-lb 12170 ft-lb Shear: -3814 lb 7074 lb

page

Project: 230609B Fletcher Location: ROOF- Hip over Family Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] 1.5 IN x 9.25 IN x 3.83 FT #2 - Spruce-Pine-Fir (South) - Dry Use Section Adequate By: 111.6% Controlling Factor: Shear	StruCalc 9.0 page StruCalc Version 10.0.1.6 8/28/2023 9:23:43 AM
DEFLECTIONS Center Live Load 0.01 IN L/6418 Dead Load 0.01 in Total Load 0.01 IN L/3642 Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240 REACTIONS A B Live Load 335 Ib 282 Ib Dead Load 255 Ib 215 Ib Total Load 590 Ib 497 Ib	
Bearing Length 1.17 in 0.99 in BEAM DATA Center Span Length 3.83 ft Unbraced Length-Top 0 ft Unbraced Length-Bottom 3.83 ft Live Load Duration Factor 1.00	A -3.83 ft B
Notch Depth 0.00 MATERIAL PROPERTIES #2 - Spruce-Pine-Fir (South) Bending Stress: Fb = 775 psi Fb = 775 psi Fb' = 853 psi	UNIFORM LOADSCenterUniform Live Load0plfUniform Dead Load0plfBeam Self Weight2plfTotal Uniform Load2plf
Cd=1.00 CF=1.10 Shear Stress: $Fv = 135 \text{ psi}$ Fv = 135 psi $Fv' = 135 \text{ psi}$ Cd=1.00 $Cd=1.00$ Modulus of Elasticity: $E = 1100 \text{ ksi}$ $E' = 1100 \text{ ksi}$ Comp. \perp to Grain: $Fc - \perp = 335 \text{ psi}$ $Fc - \perp' = 335 \text{ psi}$ Controlling Moment: 521 ft-lb	TRAPEZOIDAL LOADS - CENTER SPANLoad NumberOneLeft Live Load60 plf143 plfLeft Dead Load45 plf107 plfRight Live Load0 plf119 plfRight Dead Load0 plf89 plf
 1.84 Ft from left support of span 2 (Center Span) Created by combining all dead loads and live loads on span(s) 2 Controlling Shear: 590 lb At left support of span 2 (Center Span) Created by combining all dead loads and live loads on span(s) 2 	Load Start 0 ft 0 ft Load End 3.83 ft 3.83 ft Load Length 3.83 ft 3.83 ft
Comparisons with required sections:Req'dProvidedSection Modulus:7.34 in321.39 in3Area (Observe)2.50 in240.90 in3	

Section Modulus:	7.34 in3	21.39 in3	
Area (Shear):	6.56 in2	13.88 in2	
Moment of Inertia (deflection):	6.52 in4	98.93 in4	
Moment:	521 ft-lb	1520 ft-lb	
Shear:	590 lb	1249 lb	

Shear:

-2073 lb

3493 lb

Location: ROOF-Valley at Master Bath Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.5 IN x 11.25 IN x 13.17 FT (5.7 + 7.5) (Actual 15.7 FT) #2 - Spruce-Pine-Fir (South) - Dry Use Section Adequate By: 37.1% Controlling Factor: Moment



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CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to all members DEFLECTIONS Center Left LOADING DIAGRAM Live Load -0.02 IN L/4842 0.06 IN L/1831 Dead Load -0.01 in 0.05 in Total Load -0.03 IN L/2959 0.11 IN L/985 Total Load Deflection Criteria: L/180 Live Load Deflection Criteria: L/240 REACTIONS Α В С Live Load 126 lb 1792 lb 1303 lb Dead Load -80 lb 1652 lb 1163 lb Total Load 46 lb 3444 lb 2466 lb Uplift (1.5 F.S) -309 lb 0 **Ib** 0 **lb** 2.45 in **Bearing Length** 0.05 in 3.43 in TR1 BEAM DATA Left Center 5.67 ft 7.5 ft Span Length 5.67 ft 7.5 ft Ā B Unbraced Length-Top 0 ft 0 ft Unbraced Length-Bottom 5.67 ft 7.5 ft **Beam End Elevation Difference** 8.5 ft UNIFORM LOADS Left Center Live Load Duration Factor 1.15 Uniform Live Load 0 plf 0 plf Notch Depth 0.00 Uniform Dead Load 0 plf 0 plf **MATERIAL PROPERTIES** Beam Self Weight 5 plf 5 plf #2 - Spruce-Pine-Fir (South) Total Uniform Load 5 plf 5 plf **Base Values** Adjusted TRAPEZOIDAL LOADS - LEFT SPAN Bending Stress: Fb = 775 psi Fb' = 891 psi Load Number One Cd=1.15 CF=1.00 Left Live Load 0 plf Shear Stress: Fv = 135 psi Fv' =155 psi Left Dead Load 0 plf Cd=1.15 **Right Live Load** 161 plf Modulus of Elasticity: F = 1100 ksi F' = 1100 ksi Right Dead Load 121 plf Comp. \perp to Grain: Fc-⊥= 335 psi Fc - ⊥' = 335 psi Load Start 0 ft Load End 5.67 ft **Controlling Moment:** 3429 ft-lb Load Length 5.67 ft 4.579 Ft from left support of span 2 (Center Span) CENTER SPAN Created by combining all dead loads and live loads on span(s) 2 Load Number One Controlling Shear: -2073 lb Left Live Load 161 plf 7.562 Ft from left support of span 2 (Center Span) Left Dead Load 121 plf Created by combining all dead loads and live loads on span(s) 2 Right Live Load 510 plf Right Dead Load 382 plf Comparisons with required sections: Req'd Provided Load Start 0 ft Section Modulus: 46.16 in3 63.28 in3 Load End 7.5 ft Area (Shear): 20.02 in2 33.75 in2 Load Length 7.5 ft Moment of Inertia (deflection): 65.03 in4 355.96 in4 Moment: 3429 ft-lb 4700 ft-lb

Location: ROOF- Hip at laundry Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.5 IN x 11.25 IN x 9.0 FT (Actual 10.6 FT) #2 - Spruce-Pine-Fir (South) - Dry Use Section Adequate By: 14.3% Controlling Factor: Moment StruCalc 9.0

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CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to a	all members
DEFLECTIONSCenterLive Load0.10IN L/1328Dead Load0.09inTotal Load0.18IN L/691Live Load Deflection Criteria: L/360Total Load Deflection Criteria: L/240	LOADING DIAGRAM
REACTIONS A B Live Load 968 lb 675 lb Dead Load 886 lb 627 lb Total Load 1854 lb 1302 lb Bearing Length 1.84 in 1.30 in	TR2
BEAM DATA Center Span Length 9 ft Unbraced Length-Top 0 ft Unbraced Length-Bottom 9 ft Beam End Elevation Difference 5.67 ft	A 9 ft B
Live Load Duration Factor 1.00 Notch Depth 0.00	UNIFORM LOADS Center Uniform Live Load 0 plf
MATERIAL PROPERTIES #2 - Spruce-Pine-Fir (South) Base Values Adjusted	Uniform Dead Load 0 plf Beam Self Weight 5 plf Total Uniform Load 5 plf
Bending Stress: Fb = 775 psi Fb' = 775 psi Cd=1.00 CF=1.00	TRAPEZOIDAL LOADS - CENTER SPAN
Shear Stress: $Fv = 135 \text{ psi}$ $Fv' = 135 \text{ psi}$ Cd=1.00	Load NumberOneTwoLeft Live Load138 plf142 plfLeft Dead Load104 plf106 plf
Modulus of Elasticity: $E = 1100$ ksi $E' = 1100$ ksiComp. \perp to Grain: $Fc - \perp = 335$ psi $Fc - \perp' = 335$ psi	Right Live Load0 plf85 plfRight Dead Load0 plf64 plf
Controlling Moment:3577 ft-lb4.137 Ft from left support of span 2 (Center Span)Created by combining all dead loads and live loads on span(s) 2Controlling Shear:1568 lbAt left support of span 2 (Center Span)Created by combining all dead loads and live loads on span(s) 2	Load Start0 ft0 ftLoad End9 ft9 ftLoad Length9 ft9 ft

Comparisons with required sections:	Req'd	Provided
Section Modulus:	55.38 in3	63.28 in3
Area (Shear):	17.43 in2	33.75 in2
Moment of Inertia (deflection):	123.64 in4	355.96 in4
Moment:	3577 ft-lb	4087 ft-lb
Shear:	1568 lb	3038 lb

Location: ROOF-Valley at Playroom Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 11.875 IN x 19.08 FT (4.3 + 14.8) (Actual 22.7 FT) 1.9E Microllam - iLevel Trus Joist Section Adequate By: 5.5% Controlling Factor: Moment

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CAUTIONS

Shear:

* Laminations are to be fully c	onnected to provide uniform transfer of loads to a	all members				
DEFLECTIONS Left	Center	LOADING DIAGR	AM			
Live Load -0.02 IN L/2						
Dead Load -0.02 in	0.32 in					
Total Load -0.05 IN L/1						
Live Load Deflection Criteria:						
REACTIONS A	<u>B</u> <u>C</u>					
	791 lb 2767 lb					
Dead Load -1532 lb 4						
	1300 lb 5320 lb					TR2
Uplift (1.5 F.S) -3293 Ib Bearing Length 0.00 in	0 lb 0 lb 3.54 in 2.03 in					TR1
BEAM DATA	Left Center	4.33 ft		14 *	75 ft	
	33 ft 14.75 ft 0 ft 0 ft	A ^{4.33} " E	8	- 14.	75 IL	C
Unbraced Length-Top	33 ft 14.75 ft					
Unbraced Length-Bottom 4. Beam End Elevation Difference						
Live Load Duration Factor	1.00	UNIFORM LOADS		<u>Center</u>		
Notch Depth	0.00	Uniform Live Load		0 plf		
·	0.00	Uniform Dead Loa		0 plf		
MATERIAL PROPERTIES		Beam Self Weight		13 plf		
1.9E Microllam - iLevel Trus Jo		Total Uniform Load	d 13 plf	13 plf		
	Base Values Adjusted	TRAPEZOIDAL L	OADS - LEF	T SPAN		
Bending Stress:	Fb = 2600 psi Fb' = 2281 psi	Load Number	<u>One</u>	Two	Three	
	Cd=1.00 Cl=0.88 CF=1.00	Left Live Load	0 plf	93 plf	0 plf	
Shear Stress:	Fv = 285 psi Fv' = 285 psi	Left Dead Load	0 plf	70 plf	0 plf	
Madulus of Electicity	Cd=1.00 F = 1900 ksi F' = 1900 ksi	Right Live Load	47 plf	113 plf	57 plf	
Modulus of Elasticity: Comp.	E = 1900 ksi E' = 1900 ksi Fc - ⊥ = 750 psi Fc - ⊥' = 750 psi	Right Dead Load	35 plf	85 plf	42 plf	
Comp to Grain:	FC = 750 psi $FC = 750 psi$	Load Start	0 ft	3 ft	0 ft	
Controlling Moment:	-14825 ft-lb	Load End	3 ft	4.33 ft	4.33 ft	
Over left support of span 2 (0		Load Length	3 ft	1.33 ft	4.33 ft	
	ad loads and live loads on span(s) 1, 2	CENTER SPAN				
Controlling Shear:	4550 lb	Load Number	<u>One</u>	<u>Two</u>		
At left support of span 2 (Cer		Left Live Load	113 plf	57 plf		
	ad loads and live loads on span(s) 1, 2	Left Dead Load	85 plf	42 plf		
		Right Live Load	343 plf	240 plf		
Comparisons with required	sections: Reg'd Provided	Right Dead Load	257 plf	180 plf		
Section Modulus:	77.98 in3 82.26 in3	Load Start	0 ft	0 ft		
Area (Shear):	23.95 in2 41.56 in2	Load End	14.75 ft	14.75 ft		
Moment of Inertia (deflection):		Load Length	14.75 ft	14.75 ft		
Moment:	-14825 ft-lb 15639 ft-lb					

4550 lb

7897 lb

page

Location: FL2- Roof beam at Foyer Combination Roof And Floor Beam [2015 International Building Code(2015 NDS)] (3) 1.5 IN x 9.25 IN x 11.0 FT #2 - Spruce-Pine-Fir (South) - Dry Use Section Adequate By: 17.1% Controlling Factor: Moment

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CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to a	all members
DEFLECTIONSCenterLive Load0.19IN L/698Dead Load0.15inTotal Load0.34IN L/384Live Load Deflection Criteria: L/360Total Load Deflection Criteria: L/240	LOADING DIAGRAM
REACTIONS A B Live Load 1031 lb 1031 lb Dead Load 841 lb 841 lb Total Load 1872 lb 1872 lb Bearing Length 1.24 in 1.24 in	
BEAM DATACenterSpan Length11Unbraced Length-Top0Roof Pitch10Floor Duration Factor1.00Roof Duration Factor1.15	A 11 ft B
Notch Depth 0.00 MATERIAL PROPERTIES #2 - Spruce-Pine-Fir (South) Base Values Adjusted Bending Stress: Fb = 775 psi Fb' = 1127 psi	Side 1Side 2Roof Live LoadRLL =25 psf0 psfRoof Dead LoadRDL =15 psf0 psfRoof Tributary WidthRTW =7.5 ft0 ft
Shear Stress: $Fv = 135 \text{ psi}$ $Fb = 1127 \text{ psi}$ $Cd=1.15 \ CF=1.10 \ Cr=1.15$ Shear Stress: $Fv = 135 \text{ psi}$ $Fv' = 155 \text{ psi}$ $Cd=1.15$ Modulus of Elasticity: $E = 1100 \text{ ksi}$ $E' = 1100 \text{ ksi}$ Comp. \perp to Grain: $Fc - \perp = 335 \text{ psi}$ $Fc - \perp' = 335 \text{ psi}$	FLOOR LOADINGSide 1Side 2Floor Live LoadFLL =0psfFloor Dead LoadFDL =0psf0Floor Tributary WidthFTW =0ft0
Controlling Moment:5149 ft-lb5.5 ft from left supportCreated by combining all dead and live loads.	Wall Load WALL = 0 plf BEAM LOADING Roof Uniform Live Load: wL-roof = 188 plf
Controlling Shear: 1872 lb At support. Created by combining all dead and live loads.	Roof Uniform Dead Load:wD-roof =146plfFloor Uniform Live Load:wL-floor =0plfFloor Uniform Dead Load:wD-floor =0plfBeam Self Weight:BSW =6plf
Comparisons with required sections:Req'dProvidedSection Modulus:54.81 in364.17 in3Area (Shear):18.09 in241.63 in2Moment of Inertia (deflection):185.34 in4296.79 in4Moment:5149 ft-lb6029 ft-lbShear:1872 lb4308 lb	Combined Uniform Live Load:wL =188plfCombined Uniform Dead Load:wD =153plfCombined Uniform Total Load:wT =340plf

/--3-

Moment:

Shear:

Location: FL! -Garage Door header Uniformly Loaded Floor Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 16.0 IN x 18.67 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 74.4% Controlling Factor: Moment StruCalc 9.0

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CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to all members DEFLECTIONS Center LOADING DIAGRAM 0.27 IN L/820 Live Load Dead Load 0.22 in Total Load 0.49 IN L/455 Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240 REACTIONS А В Live Load 2117 lb 2117 lb Dead Load 1704 lb 1704 lb Total Load 3821 lb 3821 lb Bearing Length 1.46 in 1.46 in w **BEAM DATA** Center Span Length 18.67 ft 18.67 ft Unbraced Length-Top 0 ft Ā B Floor Duration Factor 1.00 Notch Depth 0.00 FLOOR LOADING **MATERIAL PROPERTIES** 1.9E Microllam - iLevel Trus Joist Side 2 Side 1 Floor Live Load FLL = Base Values 40 psf 0 psf **Adjusted** Bending Stress: Fb =2600 psi Fb' = 2500 psi Floor Dead Load FDL = 15 psf 0 psf Cd=1.00 CF=0.96 Floor Tributary Width FTW = 5.7 ft 0 ft Shear Stress: Fv = 285 psi Fv' =285 psi Cd=1.00 Wall Load WALL = 80 plf Modulus of Elasticity: E = E' = 1900 ksi 1900 ksi **BEAM LOADING** Comp. \perp to Grain: Fc - ⊥ = 750 psi Fc - ⊥' = 750 psi Beam Total Live Load: wL = 227 plf Beam Total Dead Load: wD = 165 plf **Controlling Moment:** 17836 ft-lb Beam Self Weight: BSW = 18 plf 9.335 ft from left support Total Maximum Load: wT = 409 plf Created by combining all dead and live loads. Controlling Shear: 3821 lb At support. Created by combining all dead and live loads. Comparisons with required sections: Req'd Provided Section Modulus: 85.61 in3 149.33 in3 Area (Shear): 20.11 in2 56 in2 Moment of Inertia (deflection): 630.84 in4 1194.67 in4

31114 ft-lb

10640 lb

17836 ft-lb

3821 lb

Location: FL1-Beam at Rear of garage Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 9.25 IN x 12.0 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 44.7% Controlling Factor: Deflection StruCalc 9.0

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CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to all members DEFLECTIONS Center LOADING DIAGRAM 0.28 IN L/521 Live Load Dead Load 0.08 in Total Load 0.36 IN L/404 Total Load Deflection Criteria: L/240 Live Load Deflection Criteria: L/360 REACTIONS В А Live Load 1694 lb 1457 lb Dead Load 486 lb 426 lb Total Load 2180 lb 1883 lb Bearing Length 0.83 in 0.72 in TR1 **BEAM DATA** Center Span Length 12 ft Unbraced Length-Top 12 ft 0 ft Ā Unbraced Length-Bottom 12 ft Live Load Duration Factor 1.00 Notch Depth 0.00 UNIFORM LOADS Center **MATERIAL PROPERTIES** Uniform Live Load 0 plf 1.9E Microllam - iLevel Trus Joist Uniform Dead Load 0 plf **Base Values** Adjusted Beam Self Weight 10 plf Bending Stress: 2600 psi 2694 psi Total Uniform Load 10 plf Fb =Fb' =Cd=1.00 CF=1.04 **TRAPEZOIDAL LOADS - CENTER SPAN** Shear Stress: 285 psi 285 psi Fv = Fv' = Load Number Two <u>One</u> Cd=1.00 Left Live Load 227 plf 80 plf Modulus of Elasticity: 1900 ksi E' = 1900 ksi F = Left Dead Load 57 plf 20 plf Comp. \perp to Grain: Fc - ⊥ = 750 psi 750 psi Fc - ⊥' = 80 plf **Right Live Load** 227 plf **Right Dead Load** 57 plf 20 plf **Controlling Moment:** 6028 ft-lb Load Start 0 ft 0 ft 5.64 Ft from left support of span 2 (Center Span) Load End 12 ft 5.33 ft Created by combining all dead loads and live loads on span(s) 2 Load Length 12 ft 5.33 ft **Controlling Shear:** 2179 lb At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:	<u>Req'd</u>	Provided
Section Modulus:	26.85 in3	49.91 in3
Area (Shear):	11.47 in2	32.38 in2
Moment of Inertia (deflection):	159.54 in4	230.84 in4
Moment:	6028 ft-lb	11204 ft-lb
Shear:	2179 lb	6151 lb

Location: FL1-Beam between Dining and Family Uniformly Loaded Floor Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 9.25 IN x 10.0 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 19.6% Controlling Factor: Shear

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<u>CAUTIONS</u> * Laminations are to be fully connected to provide uniform transfer of loads to a	all members
DEFLECTIONSCenterLive Load0.12IN L/979Dead Load0.10inTotal Load0.22IN L/534Live Load Deflection Criteria: L/360Total Load Deflection Criteria: L/240	
REACTIONS A B Live Load 502 lb 2843 lb Dead Load 447 lb 2300 lb Total Load 949 lb 5143 lb Bearing Length 0.36 in 1.96 in	1
BEAM DATACenterSpan Length10 ftUnbraced Length-Top0 ftFloor Duration Factor1.00Notch Depth0.00	A 10 ft B
<u>MATERIAL PROPERTIES</u> 1.9E Microllam - iLevel Trus Joist	FLOOR LOADING Side 1 Side 2
Base Values Adjusted Bending Stress: Fb = 2600 psi Fb' = 2694 psi Cd=1.00 CF=1.04 Cd=1.04 Cd=1.04	Floor Live LoadFLL =0psf0psfFloor Dead LoadFDL =0psf0psfFloor Tributary WidthFTW =0ft0ft
Shear Stress: Fv = 285 psi Fv' = 285 psi <i>Cd=1.00</i>	Wall Load WALL = 0 plf
Modulus of Elasticity: $E =$ 1900 ksi $E' =$ 1900 ksiComp. $^{\perp}$ to Grain: $Fc - ^{\perp} =$ 750 psi $Fc - ^{\perp} =$ 750 psi	BEAM LOADING Beam Total Live Load: wL = 0 plf
Controlling Moment:7703 ft-lb5.0 ft from left supportCreated by combining all dead and live loads.	Beam Total Dead Load:wD =0plfBeam Self Weight:BSW =10plfTotal Maximum Load:wT =10plf
Controlling Shear: -5143 lb At support. Created by combining all dead and live loads.	POINT LOADS - CENTER SPANLoad NumberOneLive Load3345 lbDead Load2646 lb
Comparisons with required sections:Req'dProvidedSection Modulus:34.32 in349.91 in3Area (Shear):27.07 in232.38 in2Moment of Inertia (deflection):103.67 in4230.84 in4Moment:7703 ft-lb11204 ft-lbShear:-5143 lb6151 lb	Location 8.5 ft * Load obtained from Load Tracker. See Summary Report for details.



Shear:

Location: FL1- Beam at Dining Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] 1.75 IN x 16.0 IN x 10.33 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 6.1% Controlling Factor: Shear



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DEFLECTIONS Center		LOADING DIAGRAM
Live Load 0.12 IN L/1066		
Dead Load 0.07 in		1
Total Load 0.19 IN L/656		
Live Load Deflection Criteria: L/360 To	otal Load Deflection Criteria: L/240	
REACTIONS A B		
Live Load 3037 lb 2469 lb		TR3
Dead Load 1977 lb 1504 lb		TR2
Total Load 5014 lb 3973 lb		TR1
Bearing Length 3.82 in 3.03 in		
		w
BEAM DATA Center		
Span Length 10.33 ft		10.33 ft
Unbraced Length-Top 0 ft		A B
Unbraced Length-Bottom 10.33 ft		
Live Load Duration Factor 1.00		
Notch Depth 0.00		UNIFORM LOADS Center
MATERIAL PROPERTIES		Uniform Live Load 0 plf
1.9E Microllam - iLevel Trus Joist		Uniform Dead Load 64 plf
Base	Values Adjusted	Beam Self Weight 9 plf
Bending Stress: Fb =	2600 psi Fb' = 2500 psi	Total Uniform Load 73 plf
	CF=0.96	
Shear Stress: Fv =	285 psi Fv' = 285 psi	POINT LOADS - CENTER SPAN
Cd=1.00		Load Number <u>One</u> *
Modulus of Elasticity: E =	1900 ksi E' = 1900 ksi	Live Load 1031 lb
	750 psi Fc - \perp ' = 750 psi	Dead Load 841 lb
		Location 1.58 ft
Controlling Moment: 10987 ft-	-lb	* Load obtained from Load Tracker. See Summary Report for details.
4.75 Ft from left support of span 2 (Cer		TRAPEZOIDAL LOADS - CENTER SPAN
Created by combining all dead loads ar		Load Number <u>One Two</u> <u>Three</u>
Controlling Shear: 5015 lb		Left Live Load 230 plf 110 plf 110 plf
At left support of span 2 (Center Span)		Left Dead Load 58 plf 55 plf 82.5 plf
Created by combining all dead loads ar	nd live loads on span(s) 2	Right Live Load 230 plf 110 plf 110 plf
		Right Dead Load 58 plf 55 plf 82.5 plf
Comparisons with required sections:	Reg'd Provided	Load Start 0 ft 0 ft 1.58 ft
Section Modulus:	52.73 in3 74.67 in3	Load End 10.33 ft 10.33 ft 10.33 ft
Area (Shear):	26.39 in2 28 in2	Load Length 10.33 ft 10.33 ft 8.75 ft
Moment of Inertia (deflection):	218.44 in4 597.33 in4	
Moment:	10987 ft-lb 15557 ft-lb	

5015 lb

5320 lb

Jage

Moment:

Shear:

Location: FL1-Beam between Kitchen and Family Uniformly Loaded Floor Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 18.0 IN x 17.83 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 8.5% Controlling Factor: Moment

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CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to a	Il members
DEFLECTIONSCenterLive Load0.45IN L/476Dead Load0.18inTotal Load0.63IN L/338Live Load Deflection Criteria: L/360Total Load Deflection Criteria: L/240	LOADING DIAGRAM
REACTIONS A B Live Load 5702 lb 5702 lb Dead Load 2314 lb 2314 lb Total Load 8016 lb 8016 lb Bearing Length 3.05 in 3.05 in	
BEAM DATACenterSpan Length17.83ftUnbraced Length-Top0ftFloor Duration Factor1.001.00Notch Depth0.001.00	A 17.83 ft B
MATERIAL PROPERTIES 1.9E Microllam - iLevel Trus Joist <u>Base Values</u> <u>Adjusted</u>	FLOOR LOADING Side 1 Side 2 Floor Live Load FLL = 40 psf 40 psf
Bending Stress: $Fb = 2600 \text{ psi}$ $Fb' = 2460 \text{ psi}$ $Cd=1.00 \ CF=0.95$ $Cd=1.00 \ CF=0.95$ Shear Stress: $Fv = 285 \text{ psi}$ $Fv' = 285 \text{ psi}$ $Cd=1.00$ $Cd=1.00 \ Cd=1.00 \ C$	Floor Dead LoadFDL =15psf15psfFloor Tributary WidthFTW =6.6ft9.4ftWall LoadWALL =0plf
Modulus of Elasticity: $E = 1900$ ksi $E' = 1900$ ksiComp. \perp to Grain: $Fc - \perp = 750$ psi $Fc - \perp' = 750$ psi	BEAM LOADING Beam Total Live Load: wL = 640 plf Beam Total Dead Load: wD = 240 plf
Controlling Moment: 35730 ft-lb 8.915 ft from left support Created by combining all dead and live loads.	Beam Self Weight: BSW = 20 plf Total Maximum Load: wT = 899 plf
Controlling Shear: -8016 lb At support. Created by combining all dead and live loads.	
Comparisons with required sections:Req'dProvidedSection Modulus:174.26 in3189 in3Area (Shear):42.19 in263 in2Moment of Inertia (deflection):1287.79 in41701 in4	

35730 ft-lb

-8016 lb

38752 ft-lb

11970 lb

Location: FLI- Beam at breakfast room Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (3) 1.75 IN x 18.0 IN x 15.0 FT 1.9E Microllam - iLevel Trus Joist Section Adequate By: 13.6% Controlling Factor: Moment StruCalc 9.0

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CAUTIONS

Moment of Inertia (deflection):

Moment:

Shear:

CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to a	II members
DEFLECTIONS Center Live Load 0.27 IN L/671 Dead Load 0.16 in Total Load 0.43 IN L/423 Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240	LOADING DIAGRAM 1
REACTIONS A B Live Load 6807 lb 9590 lb Dead Load 4316 lb 5446 lb Total Load 11123 lb 15036 lb Bearing Length 2.83 in 3.82 in	TR2
BEAM DATACenterSpan Length15ftUnbraced Length-Top0ftUnbraced Length-Bottom15ftLive Load Duration Factor1.00Notch Depth0.00	A 15 ft B
MATERIAL PROPERTIES 1.9E Microllam - iLevel Trus Joist Base Values Adjusted Bending Stress: Fb = 2600 psi Fb' = 2460 psi	UNIFORM LOADSCenterUniform Live Load0plfUniform Dead Load0plfBeam Self Weight30plfTotal Uniform Load30plf
$Cd=1.00 \ CF=0.95$ Shear Stress: $Fv = 285 \ psi \ Cd=1.00$ Modulus of Elasticity: $E = 1900 \ ksi \ E' = 1900 \ ksi$ Comp. \perp to Grain: $Fc - \perp = 750 \ psi \ Fc - \perp' = 750 \ psi$	POINT LOADS - CENTER SPAN Load Number One Live Load 5702 lb Dead Load 2314 lb Location 11.16 ft
Controlling Moment:51148 ft-lb9.15 Ft from left support of span 2 (Center Span)Created by combining all dead loads and live loads on span(s) 2Controlling Shear:-15035 lbAt right support of span 2 (Center Span)Created by combining all dead loads and live loads on span(s) 2	TRAPEZOIDAL LOADS - CENTER SPANLoad NumberOneTwoThreeLeft Live Load253 plf315 plf145 plfLeft Dead Load158 plf236 plf73 plfRight Live Load253 plf315 plf145 plfRight Dead Load158 plf236 plf73 plfRight Dead Load158 plf236 plf73 plfLoad Start0 ft0 ft0 ft
Comparisons with required sections:Req'dProvidedSection Modulus:249.46 in3283.5 in3Area (Shear):79.13 in294.5 in2Monte of Monte of	Load End 15 ft 15 ft 15 ft Load Length 15 ft 15 ft 15 ft

2551.5 in4

17955 lb

58128 ft-lb

1448.08 in4

-15035 lb

51148 ft-lb

page

Location: FL1-Front porch header Combination Roof And Floor Beam [2015 International Building Code(2015 NDS)] (2) 1.5 IN x 9.25 IN x 11.0 FT #2 - Spruce-Pine-Fir (South) - Dry Use Section Adequate By: 28.0% Controlling Factor: Moment StruCalc 9.0

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CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to a	all members
DEFLECTIONSCenterLive Load0.17IN L/775Dead Load0.10inTotal Load0.27IN L/483Live Load Deflection Criteria:L/360Total Load Deflection Criteria:	
REACTIONSABLive Load619lb619lbDead Load374lb374lbTotal Load993lb993lbBearing Length0.99in0.99in	
BEAM DATACenterSpan Length11Unbraced Length-Top0Roof Pitch3State1.00Roof Duration Factor1.15	A 11 ft B
Notch Depth 0.00 MATERIAL PROPERTIES #2 - Spruce-Pine-Fir (South) Base Values Adjusted	Side 1 Side 2 Roof Live Load RLL = 25 psf 0 psf Roof Dead Load RDL = 15 psf 0 psf Roof Tributary Width RTW = 2.5 ft 0 ft
Bending Stress: Fb = 775 psi Fb' = 980 psi $Cd=1.15$ CF=1.10 Shear Stress: Fv = 135 psi Fv' = 155 psi $Cd=1.15$	FLOOR LOADING Side 1 Side 2 Floor Live Load FLL = 20 psf 0 psf
$Cd=7.75$ Modulus of Elasticity: $E = 1100$ ksi $E = 1100$ ksi $E' = 1100$ ksiComp. \perp to Grain: $Fc - \perp = 335$ psi $Fc - \perp = 335$ psi	Floor Live LoadFLL =20psf0psfFloor Dead LoadFDL =10psf0psfFloor Tributary WidthFTW =2.5ft0ft
Controlling Moment: 2730 ft-lb 5.5 ft from left support Created by combining all dead and live loads. Controlling Shear: -993 lb	Wall Load WALL = 0 plf BEAM LOADING Roof Uniform Live Load: wL-roof = 63 plf
Controlling Shear: -993 lb At support. Created by combining all dead and live loads.	Roof Uniform Dead Load:wD-roof =39plfFloor Uniform Live Load:wL-floor =50plfFloor Uniform Dead Load:wD-floor =25plfBeam Self Weight:BSW =4plf
Comparisons with required sections:Req'dProvidedSection Modulus:33.41 in342.78 in3Area (Shear):9.59 in227.75 in2Moment of Inertia (deflection):98.26 in4197.86 in4Moment:2730 ft-lb3495 ft-lbShear:-993 lb2872 lb	Beam Self Weight:BSW =4plfCombined Uniform Live Load:wL =113plfCombined Uniform Dead Load:wD =68plfCombined Uniform Total Load:wT =180plf

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Location: FL1-Gsrage beam Uniformly Loaded Floor Beam [2015 International Building Code(2015 NDS)] (3) 1.75 IN x 24.0 IN x 24.67 FT Versa-Lam 2800 Fb DF - Boise Cascade Section Adequate By: 116.7% Controlling Factor: Moment

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CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to all members DEFLECTIONS Center LOADING DIAGRAM 0.31 IN L/947 Live Load Dead Load 0.14 in Total Load 0.45 IN L/651 Total Load Deflection Criteria: L/240 Live Load Deflection Criteria: L/360 REACTIONS В А Live Load 5595 lb 5595 lb Dead Load 2551 lb 2551 lb Total Load 8146 lb 8146 lb Bearing Length 2.07 in 2.07 in **BEAM DATA** Center Span Length 24.67 ft 24.67 ft Unbraced Length-Top 0 ft Ā Floor Duration Factor 1.00 Notch Depth 0.00 FLOOR LOADING **MATERIAL PROPERTIES** Versa-Lam 2800 Fb DF - Boise Cascade Side 1 Side 2 Floor Live Load FLL = **Base Values** 40 psf 40 psf **Adjusted** Bending Stress: Fb =2800 psi Fb' = 2592 psi Floor Dead Load FDL = 15 psf 15 psf Cd=1.00 CF=0.93 Floor Tributary Width FTW = 5.7 ft 5.7 ft Shear Stress: Fv = 285 psi Fv' = 285 psi Wall Load Cd=1.00 WALL = 0 plf Modulus of Elasticity: E = E' = 2000 ksi 2000 ksi **BEAM LOADING** Comp. \perp to Grain: Fc-⊥= 750 psi Fc - ⊥' = 750 psi Beam Total Live Load: wL = 454 plf Beam Total Dead Load: wD = 170 plf **Controlling Moment:** 50244 ft-lb Beam Self Weight: BSW = 37 plf 12.335 ft from left support Total Maximum Load: wT = 660 plf Created by combining all dead and live loads. **Controlling Shear:** -8147 lb At support. Created by combining all dead and live loads. Comparisons with required continuou Doald Dravidad

<u>Req a</u>	Provided
232.57 in3	504 in3
42.88 in2	126 in2
2298.18 in4	6048 in4
50244 ft-lb	0 108883 ft-lb
-8147 lb	23940 lb
	232.57 in3 42.88 in2 2298.18 in4 50244 ft-lk

Moment:

Shear:

Location: FL1- header at side load garage Multi-Loaded Multi-Span Beam [2015 International Building Code(2015 NDS)] (2) 1.75 IN x 24.0 IN x 18.67 FT Versa-Lam 2800 Fb DF - Boise Cascade Section Adequate By: 19.2% Controlling Factor: Moment

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CAUTIONS * Laminations are to be fully connected to provide uniform transfer of loads to all members DEFLECTIONS Center LOADING DIAGRAM 0.28 IN L/811 Live Load Dead Load 0.14 in Total Load 0.42 IN L/539 Total Load Deflection Criteria: L/240 Live Load Deflection Criteria: L/360 1 REACTIONS А В Live Load 6143 lb 5871 lb Dead Load 3180 lb 3045 lb Total Load 9323 lb 8916 lb TR3 Bearing Length 3.55 in 3.40 in TR1 **BEAM DATA** Center Span Length 18.67 ft 18.67 ft Unbraced Length-Top 0 ft Ā Unbraced Length-Bottom 18.67 ft Live Load Duration Factor 1.00 Notch Depth 0.00 UNIFORM LOADS Center **MATERIAL PROPERTIES** Uniform Live Load 0 plf Versa-Lam 2800 Fb DF - Boise Cascade Uniform Dead Load 0 plf **Base Values** Adjusted Beam Self Weight 25 plf Bending Stress: 2800 psi 2592 psi Total Uniform Load Fb =Fb' =25 plf Cd=1.00 CF=0.93 **POINT LOADS - CENTER SPAN** Shear Stress: 285 psi 285 psi Fv =Fv' =Load Number One ' Cd=1.00 Live Load 5595 lb Modulus of Elasticity: 2000 ksi E' = 2000 ksi F = Dead Load 2551 lb Comp. \perp to Grain: Fc - ⊥ = 750 psi Fc - ⊥' = 750 psi Location 9.33 ft * Load obtained from Load Tracker. See Summary Report for details. **Controlling Moment:** 60896 ft-lb **TRAPEZOIDAL LOADS - CENTER SPAN** 9.34 Ft from left support of span 2 (Center Span) Created by combining all dead loads and live loads on span(s) 2 Load Number One Two Three Left Live Load 163 plf 82 plf 243 plf Controlling Shear: 9323 lb Left Dead Load 81 plf 41 plf 122 plf At left support of span 2 (Center Span) 243 plf **Right Live Load** 163 plf 82 plf Created by combining all dead loads and live loads on span(s) 2 Right Dead Load 81 plf 41 plf 122 plf Load Start 4.33 ft Comparisons with required sections: 0 ft 0 ft Req'd Provided Load End 18.67 ft 18.67 ft 4.33 ft Section Modulus: 281.88 in3 336 in3 Load Length 4.33 ft 14.34 ft 18.67 ft Area (Shear): 49.07 in2 84 in2 1794.34 in4 4032 in4 Moment of Inertia (deflection):

60896 ft-lb

9323 lb

72589 ft-lb

15960 lb