



StruCalc Version 10.0.1.6

4/17/2025 7:59:04 AM

Location: ROOF- Hip st Bedroom 2
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 9.25 IN x 20.75 FT (4.1 + 16.7) (Actual 25.2 FT)
1.9E Microllam - iLevel Trus Joist
Section Adequate By: 99.9%
Controlling Factor: Moment

Location: ROOF- Valley over Dining
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 9.25 IN x 16.91 FT (4.1 + 12.8)
1.9E Microllam - iLevel Trus Joist
Section Adequate By: 40.4%
Controlling Factor: Moment

Location: ROOF- Hip over Family
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
1.5 IN x 9.25 IN x 3.83 FT
#2 - Spruce-Pine-Fir (South) - Dry Use
Section Adequate By: 111.6%
Controlling Factor: Shear

Location: ROOF-Valley at Master Bath
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.5 IN x 11.25 IN x 13.17 FT (5.7 + 7.5) (Actual 15.7 FT)
#2 - Spruce-Pine-Fir (South) - Dry Use
Section Adequate By: 37.1%
Controlling Factor: Moment

Location: ROOF- Hip at laundry
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.5 IN x 11.25 IN x 9.0 FT (Actual 10.6 FT)
#2 - Spruce-Pine-Fir (South) - Dry Use
Section Adequate By: 14.3%
Controlling Factor: Moment

Location: ROOF-Valley at Playroom
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 11.875 IN x 19.08 FT (4.3 + 14.8) (Actual 22.7 FT)
1.9E Microllam - iLevel Trus Joist
Section Adequate By: 5.5%
Controlling Factor: Moment

Location: FL2- Roof beam at Foyer
Combination Roof And Floor Beam
[2015 International Building Code(2015 NDS)]
(3) 1.5 IN x 9.25 IN x 11.0 FT
#2 - Spruce-Pine-Fir (South) - Dry Use
Section Adequate By: 17.1%
Controlling Factor: Moment

Location: FL1-Beam at Rear of garage
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 9.25 IN x 12.0 FT
1.9E Microllam - iLevel Trus Joist
Section Adequate By: 44.7%
Controlling Factor: Deflection

Location: FL1-Beam between Dining and Family
Uniformly Loaded Floor Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 9.25 IN x 10.0 FT
1.9E Microllam - iLevel Trus Joist
Section Adequate By: 19.6%
Controlling Factor: Shear

Location: FL1- Beam at Dining
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
1.75 IN x 16.0 IN x 10.33 FT
1.9E Microllam - iLevel Trus Joist
Section Adequate By: 6.1%
Controlling Factor: Shear

Location: FL1-Front porch header
Combination Roof And Floor Beam
[2015 International Building Code(2015 NDS)]
(2) 1.5 IN x 9.25 IN x 11.0 FT
#2 - Spruce-Pine-Fir (South) - Dry Use
Section Adequate By: 28.0%
Controlling Factor: Moment

Location: FL1-Garage beam
Uniformly Loaded Floor Beam
[2015 International Building Code(2015 NDS)]
(3) 1.75 IN x 24.0 IN x 24.67 FT
Versa-Lam 2800 Fb DF - Boise Cascade
Section Adequate By: 116.7%
Controlling Factor: Moment

Location: FL1- header at side load garage
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 24.0 IN x 18.67 FT
Versa-Lam 2800 Fb DF - Boise Cascade
Section Adequate By: 19.2%
Controlling Factor: Moment

Location: FL1- Ceiling joists at breakfast room
Floor Joist
[2015 International Building Code(2015 NDS)]
(2) 1.5 IN x 9.25 IN x 15.0 FT @ 16 O.C.
#2 - Southern Pine - Dry Use
Section Adequate By: 22.9%
Controlling Factor: Moment



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Location: FL2- Roof beam at Bedroom 4
Roof Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 9.25 IN x 13.33 FT
Versa-Lam 2800 Fb DF - Boise Cascade
Section Adequate By: 120.5%
Controlling Factor: Deflection

Location: FI1- Ceiling joists above Family room
Floor Joist
[2015 International Building Code(2015 NDS)]
1.5 IN x 9.25 IN x 17.83 FT @ 12 O.C.
#2 - Spruce-Pine-Fir - Dry Use
Section Adequate By: 29.7%
Controlling Factor: Moment

Location: FI1- Beam at family room ceiling
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 16.0 IN x 18.67 FT
Versa-Lam 2800 Fb DF - Boise Cascade
Section Adequate By: 25.8%
Controlling Factor: Deflection

Location: FL1- Beam at breakfast room
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(3) 1.75 IN x 9.25 IN x 15.0 FT
Versa-Lam 2800 Fb DF - Boise Cascade
Section Adequate By: 38.7%
Controlling Factor: Deflection

Location: FL1- Header at bedroom hallway
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 9.25 IN x 4.25 FT
Versa-Lam 2800 Fb DF - Boise Cascade
Section Adequate By: 97.9%
Controlling Factor: Shear

Location: FL1-Beam between Kitchen and Family
Uniformly Loaded Floor Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 16.0 IN x 17.83 FT
1.9E Microllam - iLevel Trus Joist
Section Adequate By: 11.5%
Controlling Factor: Shear

Location: FL2- Beam at fl2 bath
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.5 IN x 9.25 IN x 9.67 FT
#2 - Spruce-Pine-Fir - Dry Use
Section Adequate By: 3.3%
Controlling Factor: Moment

Location: FL1- beam at master entry
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(3) 1.5 IN x 9.25 IN x 5.0 FT
#2 - Spruce-Pine-Fir - Dry Use
Section Adequate By: 35.8%
Controlling Factor: Shear

Location: FL1- Header at Breakfast room door
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.5 IN x 9.25 IN x 6.0 FT
#2 - Southern Pine - Dry Use
Section Adequate By: 23.4%
Controlling Factor: Moment

Location: FL1- Beam at stairs in garage
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 9.25 IN x 11.67 FT
Versa-Lam 2800 Fb DF - Boise Cascade
Section Adequate By: 68.1%
Controlling Factor: Deflection

Location: FI1- Beam between garages
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.5 IN x 9.25 IN x 3.67 FT
#2 - Southern Pine - Dry Use
Section Adequate By: 36.1%
Controlling Factor: Shear

Project: 250304B Fletcher

Location: ROOF- Hip st Bedroom 2

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(2) 1.75 IN x 9.25 IN x 20.75 FT (4.1 + 16.7) (Actual 25.2 FT)

1.9E Microllam - iLevel Trus Joist

Section Adequate By: 99.9%

Controlling Factor: Moment



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

	Left	Center
Live Load	-0.02 IN L/3637	0.25 IN L/964
Dead Load	-0.01 in	0.26 in
Total Load	-0.03 IN L/1986	0.51 IN L/476
Live Load Deflection Criteria: L/240 Total Load Deflection Criteria: L/180		

REACTIONS

	A	B	C
Live Load	389 lb	2196 lb	334 lb
Dead Load	-291 lb	2221 lb	379 lb
Total Load	98 lb	4417 lb	713 lb
Uplift (1.5 F.S)	-936 lb	0 lb	0 lb
Bearing Length	0.04 in	1.68 in	0.27 in

BEAM DATA

	Left	Center
Span Length	4.08 ft	16.67 ft
Unbraced Length-Top	0 ft	0 ft
Unbraced Length-Bottom	4.08 ft	16.67 ft
Beam End Elevation Difference	14.25 ft	
Live Load Duration Factor	1.15	
Notch Depth	0.00	

MATERIAL PROPERTIES

1.9E Microllam - iLevel Trus Joist

	Base Values	Adjusted
Bending Stress:	Fb = 2600 psi Cd=1.15 Cl=0.86 CF=1.04	Fb' = 2651 psi
Shear Stress:	Fv = 285 psi Cd=1.15	Fv' = 328 psi
Modulus of Elasticity:	E = 1900 ksi	E' = 1900 ksi
Comp. $\perp$ to Grain:	Fc - $\perp$ = 750 psi	Fc - $\perp$ = 750 psi

Controlling Moment:

-5516 ft-lb

Over left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 1, 2

Controlling Shear:

1901 lb

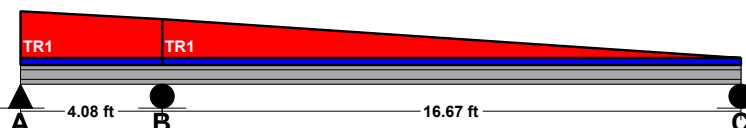
At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 1, 2

Comparisons with required sections:

	Req'd	Provided
Section Modulus:	24.97 in ³	49.91 in ³
Area (Shear):	8.7 in ²	32.38 in ²
Moment of Inertia (deflection):	87.22 in ⁴	230.84 in ⁴
Moment:	-5516 ft-lb	11026 ft-lb
Shear:	1901 lb	7074 lb

LOADING DIAGRAM



UNIFORM LOADS

	Left	Center
Uniform Live Load	0 plf	0 plf
Uniform Dead Load	0 plf	0 plf
Beam Self Weight	10 plf	10 plf
Total Uniform Load	10 plf	10 plf

TRAPEZOIDAL LOADS - LEFT SPAN

Load Number	One
Left Live Load	212 plf
Left Dead Load	159 plf
Right Live Load	177 plf
Right Dead Load	132 plf
Load Start	0 ft
Load End	4.08 ft
Load Length	4.08 ft

CENTER SPAN

Load Number	One
Left Live Load	177 plf
Left Dead Load	132 plf
Right Live Load	0 plf
Right Dead Load	0 plf
Load Start	0 ft
Load End	16.67 ft
Load Length	16.67 ft

Project: 250304B Fletcher

Location: ROOF- Valley over Dining

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(2) 1.75 IN x 9.25 IN x 16.91 FT (4.1 + 12.8)

1.9E Microllam - iLevel Trus Joist

Section Adequate By: 40.4%

Controlling Factor: Moment



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

	Left	Center
Live Load	-0.02 IN L/2408	0.26 IN L/593
Dead Load	-0.02 in	0.20 in
Total Load	-0.04 IN L/1374	0.46 IN L/334
Live Load Deflection Criteria: L/240 Total Load Deflection Criteria: L/180		

REACTIONS

	A	B	C
Live Load	69 lb	3345 lb	2151 lb
Dead Load	-855 lb	2646 lb	1663 lb
Total Load	-786 lb	5991 lb	3814 lb
Uplift (1.5 F.S)	-2039 lb	0 lb	0 lb
Bearing Length	0.00 in	2.28 in	1.45 in

BEAM DATA

	Left	Center
Span Length	4.08 ft	12.83 ft
Unbraced Length-Top	0 ft	0 ft
Unbraced Length-Bottom	4.08 ft	12.83 ft
Live Load Duration Factor	1.15	
Notch Depth	0.00	

MATERIAL PROPERTIES

1.9E Microllam - iLevel Trus Joist

	Base Values	Adjusted
Bending Stress:	Fb = 2600 psi Cd=1.15 Cl=0.94 CF=1.04	Fb' = 2926 psi
Shear Stress:	Fv = 285 psi Cd=1.15	Fv' = 328 psi
Modulus of Elasticity:	E = 1900 ksi	E' = 1900 ksi
Comp. $\perp$ to Grain:	Fc - $\perp$ = 750 psi	Fc - $\perp$ = 750 psi

Controlling Moment:

-8667 ft-lb

Over left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 1, 2

Controlling Shear:

-3814 lb

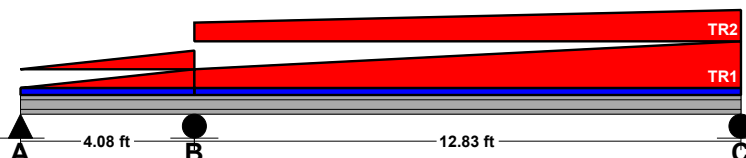
13.0 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

	Req'd	Provided
Section Modulus:	35.54 in3	49.91 in3
Area (Shear):	17.45 in2	32.38 in2
Moment of Inertia (deflection):	124.49 in4	230.84 in4
Moment:	-8667 ft-lb	12170 ft-lb
Shear:	-3814 lb	7074 lb

LOADING DIAGRAM



UNIFORM LOADS

	Left	Center
Uniform Live Load	0 plf	0 plf
Uniform Dead Load	0 plf	0 plf
Beam Self Weight	10 plf	10 plf
Total Uniform Load	10 plf	10 plf

TRAPEZOIDAL LOADS - LEFT SPAN

Load Number	One	Two
Left Live Load	0 plf	0 plf
Left Dead Load	0 plf	0 plf
Right Live Load	60 plf	52 plf
Right Dead Load	45 plf	39 plf
Load Start	0 ft	0 ft
Load End	4.08 ft	4.08 ft
Load Length	4.08 ft	4.08 ft

CENTER SPAN

Load Number	One	Two
Left Live Load	60 plf	52 plf
Left Dead Load	45 plf	39 plf
Right Live Load	320 plf	215 plf
Right Dead Load	240 plf	161 plf
Load Start	0 ft	0 ft
Load End	12.83 ft	12.83 ft
Load Length	12.83 ft	12.83 ft

Project: 250304B Fletcher

Location: ROOF- Hip over Family
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
1.5 IN x 9.25 IN x 3.83 FT
#2 - Spruce-Pine-Fir (South) - Dry Use
Section Adequate By: 111.6%
Controlling Factor: Shear



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DEFLECTIONS

Center

Live Load	0.01	IN L/6418
Dead Load	0.01	in
Total Load	0.01	IN L/3642
Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240		

REACTIONS

A

B

Live Load	335 lb	282 lb
Dead Load	255 lb	215 lb
Total Load	590 lb	497 lb
Bearing Length	1.17 in	0.99 in

BEAM DATA

Center

Span Length	3.83 ft
Unbraced Length-Top	0 ft
Unbraced Length-Bottom	3.83 ft
Live Load Duration Factor	1.00
Notch Depth	0.00

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir (South)

	Base Values	Adjusted
Bending Stress:	Fb = 775 psi Cd=1.00 CF=1.10	Fb' = 853 psi
Shear Stress:	Fv = 135 psi Cd=1.00	Fv' = 135 psi
Modulus of Elasticity:	E = 1100 ksi	E' = 1100 ksi
Comp. $\perp$ to Grain:	Fc $\perp$ = 335 psi	Fc $\perp$ = 335 psi

Controlling Moment: 521 ft-lb

1.84 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: 590 lb

At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

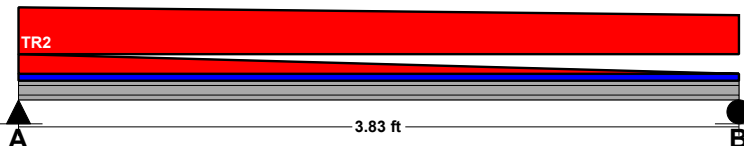
Comparisons with required sections:

Req'd

Provided

Section Modulus:	7.34 in ³	21.39 in ³
Area (Shear):	6.56 in ²	13.88 in ²
Moment of Inertia (deflection):	6.52 in ⁴	98.93 in ⁴
Moment:	521 ft-lb	1520 ft-lb
Shear:	590 lb	1249 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load	0 plf
Uniform Dead Load	0 plf
Beam Self Weight	2 plf
Total Uniform Load	2 plf

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number	One	Two
Left Live Load	60 plf	143 plf
Left Dead Load	45 plf	107 plf
Right Live Load	0 plf	119 plf
Right Dead Load	0 plf	89 plf
Load Start	0 ft	0 ft
Load End	3.83 ft	3.83 ft
Load Length	3.83 ft	3.83 ft

Project: 250304B Fletcher

Location: ROOF-Valley at Master Bath

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(2) 1.5 IN x 11.25 IN x 13.17 FT (5.7 + 7.5) (Actual 15.7 FT)

#2 - Spruce-Pine-Fir (South) - Dry Use

Section Adequate By: 37.1%

Controlling Factor: Moment



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

	Left	Center
Live Load	-0.02 IN L/4842	0.06 IN L/1831
Dead Load	-0.01 in	0.05 in
Total Load	-0.03 IN L/2959	0.11 IN L/985
Live Load Deflection Criteria: L/240 Total Load Deflection Criteria: L/180		

REACTIONS

	A	B	C
Live Load	126 lb	1792 lb	1303 lb
Dead Load	-80 lb	1652 lb	1163 lb
Total Load	46 lb	3444 lb	2466 lb
Uplift (1.5 F.S)	-309 lb	0 lb	0 lb
Bearing Length	0.05 in	3.43 in	2.45 in

BEAM DATA

	Left	Center
Span Length	5.67 ft	7.5 ft
Unbraced Length-Top	0 ft	0 ft
Unbraced Length-Bottom	5.67 ft	7.5 ft
Beam End Elevation Difference		8.5 ft
Live Load Duration Factor		1.15
Notch Depth		0.00

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir (South)

	Base Values	Adjusted
Bending Stress:	Fb = 775 psi Cd=1.15 CF=1.00	Fb' = 891 psi
Shear Stress:	Fv = 135 psi Cd=1.15	Fv' = 155 psi
Modulus of Elasticity:	E = 1100 ksi	E' = 1100 ksi
Comp. ⊥ to Grain:	Fc - ⊥ = 335 psi	Fc - ⊥' = 335 psi

Controlling Moment:

3429 ft-lb

4.579 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear:

-2073 lb

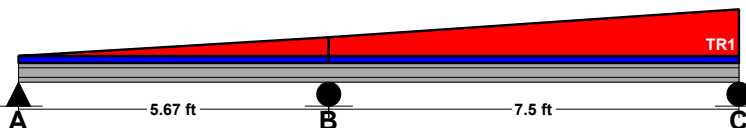
7.562 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

	Req'd	Provided
Section Modulus:	46.16 in3	63.28 in3
Area (Shear):	20.02 in2	33.75 in2
Moment of Inertia (deflection):	65.03 in4	355.96 in4
Moment:	3429 ft-lb	4700 ft-lb
Shear:	-2073 lb	3493 lb

LOADING DIAGRAM



UNIFORM LOADS

	Left	Center
Uniform Live Load	0 plf	0 plf
Uniform Dead Load	0 plf	0 plf
Beam Self Weight	5 plf	5 plf
Total Uniform Load	5 plf	5 plf

TRAPEZOIDAL LOADS - LEFT SPAN

Load Number	One
Left Live Load	0 plf
Left Dead Load	0 plf
Right Live Load	161 plf
Right Dead Load	121 plf
Load Start	0 ft
Load End	5.67 ft
Load Length	5.67 ft

CENTER SPAN

Load Number	One
Left Live Load	161 plf
Left Dead Load	121 plf
Right Live Load	510 plf
Right Dead Load	382 plf
Load Start	0 ft
Load End	7.5 ft
Load Length	7.5 ft

Project: 250304B Fletcher

Location: ROOF- Hip at laundry

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(2) 1.5 IN x 11.25 IN x 9.0 FT (Actual 10.6 FT)

#2 - Spruce-Pine-Fir (South) - Dry Use

Section Adequate By: 14.3%

Controlling Factor: Moment



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.10 IN L/1328

Dead Load 0.09 in

Total Load 0.18 IN L/691

Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 968 lb 675 lb

Dead Load 886 lb 627 lb

Total Load 1854 lb 1302 lb

Bearing Length 1.84 in 1.30 in

BEAM DATA

Center

Span Length 9 ft

Unbraced Length-Top 0 ft

Unbraced Length-Bottom 9 ft

Beam End Elevation Difference 5.67 ft

Live Load Duration Factor 1.00

Notch Depth 0.00

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir (South)

Base Values

Adjusted

Bending Stress: Fb = 775 psi Fb' = 775 psi

Cd=1.00 CF=1.00

Shear Stress: Fv = 135 psi Fv' = 135 psi

Cd=1.00

Modulus of Elasticity: E = 1100 ksi E' = 1100 ksi

Comp. $\perp$ to Grain: Fc - $\perp$ = 335 psi Fc - $\perp$ ' = 335 psi

Controlling Moment: 3577 ft-lb

4.137 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: 1568 lb

At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

Req'd

Provided

Section Modulus: 55.38 in3 63.28 in3

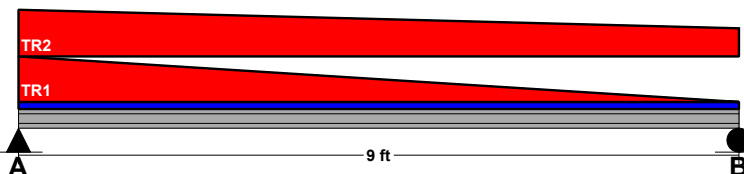
Area (Shear): 17.43 in2 33.75 in2

Moment of Inertia (deflection): 123.64 in4 355.96 in4

Moment: 3577 ft-lb 4087 ft-lb

Shear: 1568 lb 3038 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf

Uniform Dead Load 0 plf

Beam Self Weight 5 plf

Total Uniform Load 5 plf

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number	One	Two
Left Live Load	138 plf	142 plf
Left Dead Load	104 plf	106 plf
Right Live Load	0 plf	85 plf
Right Dead Load	0 plf	64 plf
Load Start	0 ft	0 ft
Load End	9 ft	9 ft
Load Length	9 ft	9 ft

Project: 250304B Fletcher

Location: ROOF-Valley at Playroom

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(2) 1.75 IN x 11.875 IN x 19.08 FT (4.3 + 14.8) (Actual 22.7 FT)

1.9E Microllam - iLevel Trus Joist

Section Adequate By: 5.5%

Controlling Factor: Moment



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* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

	Left	Center
Live Load	-0.02 IN L/2559	0.35 IN L/609
Dead Load	-0.02 in	0.32 in
Total Load	-0.05 IN L/1345	0.66 IN L/317
Live Load Deflection Criteria: L/240 Total Load Deflection Criteria: L/180		

REACTIONS

	A	B	C
Live Load	90 lb	4791 lb	2767 lb
Dead Load	-1532 lb	4509 lb	2553 lb
Total Load	-1442 lb	9300 lb	5320 lb
Uplift (1.5 F.S)	-3293 lb	0 lb	0 lb
Bearing Length	0.00 in	3.54 in	2.03 in

BEAM DATA

	Left	Center
Span Length	4.33 ft	14.75 ft
Unbraced Length-Top	0 ft	0 ft
Unbraced Length-Bottom	4.33 ft	14.75 ft
Beam End Elevation Difference	12.25 ft	
Live Load Duration Factor	1.00	
Notch Depth	0.00	

MATERIAL PROPERTIES

1.9E Microllam - iLevel Trus Joist

	Base Values	Adjusted
Bending Stress:	Fb = 2600 psi Cd=1.00 Cl=0.88 CF=1.00	Fb' = 2281 psi
Shear Stress:	Fv = 285 psi Cd=1.00	Fv' = 285 psi
Modulus of Elasticity:	E = 1900 ksi	E' = 1900 ksi
Comp. ⊥ to Grain:	Fc - ⊥ = 750 psi	Fc - ⊥' = 750 psi

Controlling Moment:

-14825 ft-lb

Over left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 1, 2

Controlling Shear:

4550 lb

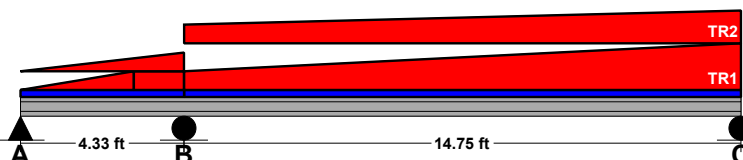
At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 1, 2

Comparisons with required sections:

	Req'd	Provided
Section Modulus:	77.98 in3	82.26 in3
Area (Shear):	23.95 in2	41.56 in2
Moment of Inertia (deflection):	277.7 in4	488.41 in4
Moment:	-14825 ft-lb	15639 ft-lb
Shear:	4550 lb	7897 lb

LOADING DIAGRAM



UNIFORM LOADS

	Left	Center
Uniform Live Load	0 plf	0 plf
Uniform Dead Load	0 plf	0 plf
Beam Self Weight	13 plf	13 plf
Total Uniform Load	13 plf	13 plf

TRAPEZOIDAL LOADS - LEFT SPAN

Load Number	One	Two	Three
Left Live Load	0 plf	93 plf	0 plf
Left Dead Load	0 plf	70 plf	0 plf
Right Live Load	47 plf	113 plf	57 plf
Right Dead Load	35 plf	85 plf	42 plf
Load Start	0 ft	3 ft	0 ft
Load End	3 ft	4.33 ft	4.33 ft
Load Length	3 ft	1.33 ft	4.33 ft

CENTER SPAN

Load Number	One	Two
Left Live Load	113 plf	57 plf
Left Dead Load	85 plf	42 plf
Right Live Load	343 plf	240 plf
Right Dead Load	257 plf	180 plf
Load Start	0 ft	0 ft
Load End	14.75 ft	14.75 ft
Load Length	14.75 ft	14.75 ft

Project: 250304B Fletcher

Location: FL2- Roof beam at Foyer
Combination Roof And Floor Beam
[2015 International Building Code(2015 NDS)]
(3) 1.5 IN x 9.25 IN x 11.0 FT
#2 - Spruce-Pine-Fir (South) - Dry Use
Section Adequate By: 17.1%
Controlling Factor: Moment



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.19 IN L/698
Dead Load 0.15 in
Total Load 0.34 IN L/384
Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 1031 lb 1031 lb
Dead Load 841 lb 841 lb
Total Load 1872 lb 1872 lb
Bearing Length 1.24 in 1.24 in

BEAM DATA

Center

Span Length 11 ft
Unbraced Length-Top 0 ft
Roof Pitch 10 :12
Floor Duration Factor 1.00
Roof Duration Factor 1.15
Notch Depth 0.00

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir (South)

Base Values

Adjusted

Bending Stress: $F_b = 775$ psi $F_b' = 1127$ psi
 $C_d = 1.15$ $C_F = 1.10$ $C_r = 1.15$
Shear Stress: $F_v = 135$ psi $F_v' = 155$ psi
 $C_d = 1.15$
Modulus of Elasticity: $E = 1100$ ksi $E' = 1100$ ksi
Comp. $\perp$ to Grain: $F_c - \perp = 335$ psi $F_c - \perp' = 335$ psi

Controlling Moment: 5149 ft-lb

5.5 ft from left support

Created by combining all dead and live loads.

Controlling Shear: 1872 lb

At support.

Created by combining all dead and live loads.

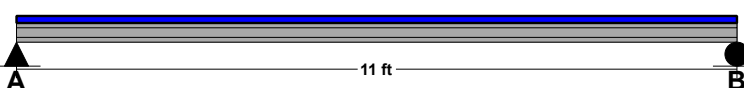
Comparisons with required sections:

Req'd

Provided

Section Modulus: 54.81 in³ 64.17 in³
Area (Shear): 18.09 in² 41.63 in²
Moment of Inertia (deflection): 185.34 in⁴ 296.79 in⁴
Moment: 5149 ft-lb 6029 ft-lb
Shear: 1872 lb 4308 lb

LOADING DIAGRAM



ROOF LOADING

		Side 1	Side 2
Roof Live Load	RLL =	25 psf	0 psf
Roof Dead Load	RDL =	15 psf	0 psf
Roof Tributary Width	RTW =	7.5 ft	0 ft

FLOOR LOADING

		Side 1	Side 2
Floor Live Load	FLL =	0 psf	0 psf
Floor Dead Load	FDL =	0 psf	0 psf
Floor Tributary Width	FTW =	0 ft	0 ft

Wall Load WALL = 0 plf

BEAM LOADING

Roof Uniform Live Load:	wL-roof =	188 plf
Roof Uniform Dead Load:	wD-roof =	146 plf
Floor Uniform Live Load:	wL-floor =	0 plf
Floor Uniform Dead Load:	wD-floor =	0 plf
Beam Self Weight:	BSW =	6 plf
Combined Uniform Live Load:	wL =	188 plf
Combined Uniform Dead Load:	wD =	153 plf
Combined Uniform Total Load:	wT =	340 plf

Project: 250304B Fletcher

Location: FL1-Beam at Rear of garage

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(2) 1.75 IN x 9.25 IN x 12.0 FT

1.9E Microllam - iLevel Trus Joist

Section Adequate By: 44.7%

Controlling Factor: Deflection



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.28 IN L/521

Dead Load 0.08 in

Total Load 0.36 IN L/404

Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 1694 lb 1457 lb

Dead Load 486 lb 426 lb

Total Load 2180 lb 1883 lb

Bearing Length 0.83 in 0.72 in

BEAM DATA

Center

Span Length 12 ft

Unbraced Length-Top 0 ft

Unbraced Length-Bottom 12 ft

Live Load Duration Factor 1.00

Notch Depth 0.00

MATERIAL PROPERTIES

1.9E Microllam - iLevel Trus Joist

Base Values

Adjusted

Bending Stress: Fb = 2600 psi Fb' = 2694 psi

Cd=1.00 CF=1.04

Shear Stress: Fv = 285 psi Fv' = 285 psi

Cd=1.00

Modulus of Elasticity: E = 1900 ksi E' = 1900 ksi

Comp. $\perp$ to Grain: Fc - $\perp$ = 750 psi Fc - $\perp$ ' = 750 psi

Controlling Moment: 6028 ft-lb

5.64 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: 2179 lb

At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

Req'd

Provided

Section Modulus: 26.85 in3 49.91 in3

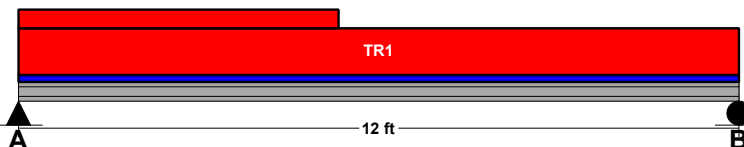
Area (Shear): 11.47 in2 32.38 in2

Moment of Inertia (deflection): 159.54 in4 230.84 in4

Moment: 6028 ft-lb 11204 ft-lb

Shear: 2179 lb 6151 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf

Uniform Dead Load 0 plf

Beam Self Weight 10 plf

Total Uniform Load 10 plf

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number	One	Two
Left Live Load	227 plf	80 plf
Left Dead Load	57 plf	20 plf
Right Live Load	227 plf	80 plf
Right Dead Load	57 plf	20 plf
Load Start	0 ft	0 ft
Load End	12 ft	5.33 ft
Load Length	12 ft	5.33 ft

Project: 250304B Fletcher

Location: FL1-Beam between Dining and Family
Uniformly Loaded Floor Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 9.25 IN x 10.0 FT
1.9E Microllam - iLevel Trus Joist
Section Adequate By: 19.6%
Controlling Factor: Shear



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.12 IN L/979

Dead Load 0.10 in

Total Load 0.22 IN L/534

Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 502 lb 2843 lb

Dead Load 447 lb 2300 lb

Total Load 949 lb 5143 lb

Bearing Length 0.36 in 1.96 in

BEAM DATA

Center

Span Length 10 ft

Unbraced Length-Top 0 ft

Floor Duration Factor 1.00

Notch Depth 0.00

MATERIAL PROPERTIES

1.9E Microllam - iLevel Trus Joist

Base Values

Adjusted

Bending Stress: Fb = 2600 psi Fb' = 2694 psi

Cd=1.00 CF=1.04

Shear Stress: Fv = 285 psi Fv' = 285 psi

Cd=1.00

Modulus of Elasticity: E = 1900 ksi E' = 1900 ksi

Comp. $\perp$ to Grain: Fc - $\perp$ = 750 psi Fc - $\perp$ ' = 750 psi

Controlling Moment: 7703 ft-lb

5.0 ft from left support

Created by combining all dead and live loads.

Controlling Shear: -5143 lb

At support.

Created by combining all dead and live loads.

Comparisons with required sections:

Req'd

Provided

Section Modulus: 34.32 in3 49.91 in3

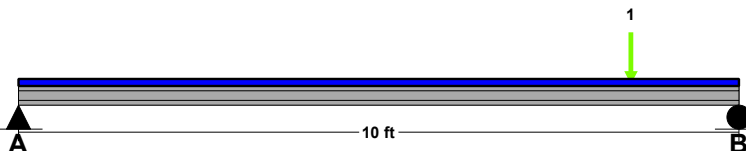
Area (Shear): 27.07 in2 32.38 in2

Moment of Inertia (deflection): 103.67 in4 230.84 in4

Moment: 7703 ft-lb 11204 ft-lb

Shear: -5143 lb 6151 lb

LOADING DIAGRAM



FLOOR LOADING

Side 1

Side 2

Floor Live Load FLL = 0 psf 0 psf

Floor Dead Load FDL = 0 psf 0 psf

Floor Tributary Width FTW = 0 ft 0 ft

Wall Load WALL = 0 plf

BEAM LOADING

Beam Total Live Load: wL = 0 plf

Beam Total Dead Load: wD = 0 plf

Beam Self Weight: BSW = 10 plf

Total Maximum Load: wT = 10 plf

POINT LOADS - CENTER SPAN

Load Number One *

Live Load 3345 lb

Dead Load 2646 lb

Location 8.5 ft

* Load obtained from Load Tracker. See Summary Report for details.

Project: 250304B Fletcher

Location: FL1- Beam at Dining
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
1.75 IN x 16.0 IN x 10.33 FT
1.9E Microllam - iLevel Trus Joist
Section Adequate By: 6.1%
Controlling Factor: Shear



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DEFLECTIONS

Center

Live Load 0.12 IN L/1066
Dead Load 0.07 in
Total Load 0.19 IN L/656
Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 3037 lb 2469 lb
Dead Load 1977 lb 1504 lb
Total Load 5014 lb 3973 lb
Bearing Length 3.82 in 3.03 in

BEAM DATA

Center

Span Length 10.33 ft
Unbraced Length-Top 0 ft
Unbraced Length-Bottom 10.33 ft
Live Load Duration Factor 1.00
Notch Depth 0.00

MATERIAL PROPERTIES

1.9E Microllam - iLevel Trus Joist

Base Values

Adjusted

Bending Stress: $F_b = 2600$ psi $F_b' = 2500$ psi
 $C_d = 1.00$ $CF = 0.96$
Shear Stress: $F_v = 285$ psi $F_v' = 285$ psi
 $C_d = 1.00$
Modulus of Elasticity: $E = 1900$ ksi $E' = 1900$ ksi
Comp. $\perp$ to Grain: $F_c - \perp = 750$ psi $F_c - \perp' = 750$ psi

Controlling Moment: 10987 ft-lb

4.75 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: 5015 lb

At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

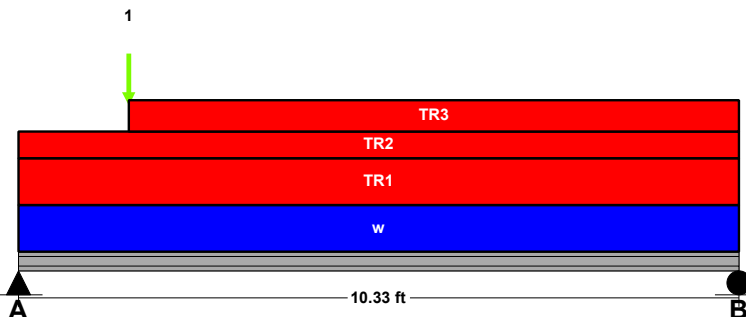
Comparisons with required sections:

Req'd

Provided

Section Modulus: 52.73 in³ 74.67 in³
Area (Shear): 26.39 in² 28 in²
Moment of Inertia (deflection): 218.44 in⁴ 597.33 in⁴
Moment: 10987 ft-lb 15557 ft-lb
Shear: 5015 lb 5320 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf
Uniform Dead Load 64 plf
Beam Self Weight 9 plf
Total Uniform Load 73 plf

POINT LOADS - CENTER SPAN

Load Number One *

Live Load 1031 lb
Dead Load 841 lb
Location 1.58 ft

* Load obtained from Load Tracker. See Summary Report for details.

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number	One	Two	Three
Left Live Load	230 plf	110 plf	110 plf
Left Dead Load	58 plf	55 plf	82.5 plf
Right Live Load	230 plf	110 plf	110 plf
Right Dead Load	58 plf	55 plf	82.5 plf
Load Start	0 ft	0 ft	1.58 ft
Load End	10.33 ft	10.33 ft	10.33 ft
Load Length	10.33 ft	10.33 ft	8.75 ft

Project: 250304B Fletcher

Location: FL1-Front porch header
Combination Roof And Floor Beam
[2015 International Building Code(2015 NDS)]
(2) 1.5 IN x 9.25 IN x 11.0 FT
#2 - Spruce-Pine-Fir (South) - Dry Use
Section Adequate By: 28.0%
Controlling Factor: Moment



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.17 IN L/775
Dead Load 0.10 in
Total Load 0.27 IN L/483
Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 619 lb 619 lb
Dead Load 374 lb 374 lb
Total Load 993 lb 993 lb
Bearing Length 0.99 in 0.99 in

BEAM DATA

Center

Span Length 11 ft
Unbraced Length-Top 0 ft
Roof Pitch 3 :12
Floor Duration Factor 1.00
Roof Duration Factor 1.15
Notch Depth 0.00

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir (South)

Base Values

Adjusted

Bending Stress: Fb = 775 psi Fb' = 980 psi
Cd=1.15 CF=1.10
Shear Stress: Fv = 135 psi Fv' = 155 psi
Cd=1.15
Modulus of Elasticity: E = 1100 ksi E' = 1100 ksi
Comp. $\perp$ to Grain: Fc - $\perp$ = 335 psi Fc - $\perp$ = 335 psi

Controlling Moment:

2730 ft-lb

5.5 ft from left support

Created by combining all dead and live loads.

Controlling Shear:

-993 lb

At support.

Created by combining all dead and live loads.

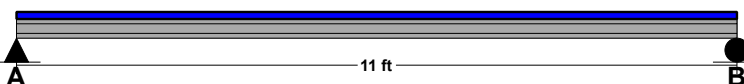
Comparisons with required sections:

Req'd

Provided

Section Modulus: 33.41 in3 42.78 in3
Area (Shear): 9.59 in2 27.75 in2
Moment of Inertia (deflection): 98.26 in4 197.86 in4
Moment: 2730 ft-lb 3495 ft-lb
Shear: -993 lb 2872 lb

LOADING DIAGRAM



ROOF LOADING

		Side 1	Side 2
Roof Live Load	RLL =	25 psf	0 psf
Roof Dead Load	RDL =	15 psf	0 psf
Roof Tributary Width	RTW =	2.5 ft	0 ft

FLOOR LOADING

		Side 1	Side 2
Floor Live Load	FLL =	20 psf	0 psf
Floor Dead Load	FDL =	10 psf	0 psf
Floor Tributary Width	FTW =	2.5 ft	0 ft

Wall Load

WALL = 0 plf

BEAM LOADING

Roof Uniform Live Load:	wL-roof =	63 plf
Roof Uniform Dead Load:	wD-roof =	39 plf
Floor Uniform Live Load:	wL-floor =	50 plf
Floor Uniform Dead Load:	wD-floor =	25 plf
Beam Self Weight:	BSW =	4 plf
Combined Uniform Live Load:	wL =	113 plf
Combined Uniform Dead Load:	wD =	68 plf
Combined Uniform Total Load:	wT =	180 plf

Project: 250304B Fletcher

Location: FL1-Garage beam

Uniformly Loaded Floor Beam

[2015 International Building Code(2015 NDS)]

(3) 1.75 IN x 24.0 IN x 24.67 FT

Versa-Lam 2800 Fb DF - Boise Cascade

Section Adequate By: 116.7%

Controlling Factor: Moment



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.31 IN L/947

Dead Load 0.14 in

Total Load 0.45 IN L/651

Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 5595 lb 5595 lb

Dead Load 2551 lb 2551 lb

Total Load 8146 lb 8146 lb

Bearing Length 2.07 in 2.07 in

BEAM DATA

Center

Span Length 24.67 ft

Unbraced Length-Top 0 ft

Floor Duration Factor 1.00

Notch Depth 0.00

MATERIAL PROPERTIES

Versa-Lam 2800 Fb DF - Boise Cascade

Base Values

Adjusted

Bending Stress: Fb = 2800 psi Fb' = 2592 psi

Cd=1.00 CF=0.93

Shear Stress: Fv = 285 psi Fv' = 285 psi

Cd=1.00

Modulus of Elasticity: E = 2000 ksi E' = 2000 ksi

Comp. $\perp$ to Grain: Fc - $\perp$ = 750 psi Fc - $\perp$ ' = 750 psi

Controlling Moment: 50244 ft-lb

12.335 ft from left support

Created by combining all dead and live loads.

Controlling Shear: -8147 lb

At support.

Created by combining all dead and live loads.

Comparisons with required sections:

Req'd

Provided

Section Modulus: 232.57 in3 504 in3

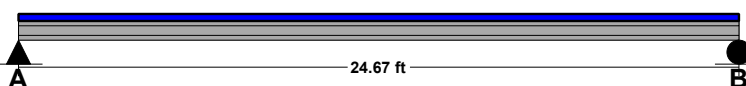
Area (Shear): 42.88 in2 126 in2

Moment of Inertia (deflection): 2298.18 in4 6048 in4

Moment: 50244 ft-lb 108883 ft-lb

Shear: -8147 lb 23940 lb

LOADING DIAGRAM



FLOOR LOADING

Side 1

Side 2

Floor Live Load FLL = 40 psf 40 psf

Floor Dead Load FDL = 15 psf 15 psf

Floor Tributary Width FTW = 5.7 ft 5.7 ft

Wall Load WALL = 0 plf

BEAM LOADING

Beam Total Live Load: wL = 454 plf

Beam Total Dead Load: wD = 170 plf

Beam Self Weight: BSW = 37 plf

Total Maximum Load: wT = 660 plf

Project: 250304B Fletcher

Location: FL1- header at side load garage
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 24.0 IN x 18.67 FT
Versa-Lam 2800 Fb DF - Boise Cascade
Section Adequate By: 19.2%
Controlling Factor: Moment



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.28 IN L/811
Dead Load 0.14 in
Total Load 0.42 IN L/539
Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 6143 lb 5871 lb
Dead Load 3180 lb 3045 lb
Total Load 9323 lb 8916 lb
Bearing Length 3.55 in 3.40 in

BEAM DATA

Center

Span Length 18.67 ft
Unbraced Length-Top 0 ft
Unbraced Length-Bottom 18.67 ft
Live Load Duration Factor 1.00
Notch Depth 0.00

MATERIAL PROPERTIES

Versa-Lam 2800 Fb DF - Boise Cascade

	Base Values	Adjusted
Bending Stress:	Fb = 2800 psi Cd=1.00 CF=0.93	Fb' = 2592 psi
Shear Stress:	Fv = 285 psi Cd=1.00	Fv' = 285 psi
Modulus of Elasticity:	E = 2000 ksi	E' = 2000 ksi
Comp. $\perp$ to Grain:	Fc - $\perp$ = 750 psi	Fc - $\perp$ = 750 psi

Controlling Moment:

60896 ft-lb

9.34 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear:

9323 lb

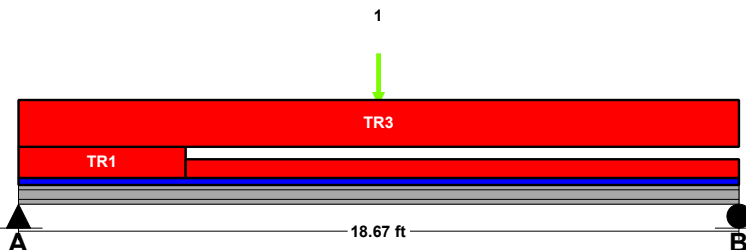
At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

	Req'd	Provided
Section Modulus:	281.88 in3	336 in3
Area (Shear):	49.07 in2	84 in2
Moment of Inertia (deflection):	1794.34 in4	4032 in4
Moment:	60896 ft-lb	72589 ft-lb
Shear:	9323 lb	15960 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf
Uniform Dead Load 0 plf
Beam Self Weight 25 plf
Total Uniform Load 25 plf

POINT LOADS - CENTER SPAN

Load Number One *
Live Load 5595 lb
Dead Load 2551 lb
Location 9.33 ft

* Load obtained from Load Tracker. See Summary Report for details.

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number	<u>One</u>	<u>Two</u>	<u>Three</u>
Left Live Load	163 plf	82 plf	243 plf
Left Dead Load	81 plf	41 plf	122 plf
Right Live Load	163 plf	82 plf	243 plf
Right Dead Load	81 plf	41 plf	122 plf
Load Start	0 ft	4.33 ft	0 ft
Load End	4.33 ft	18.67 ft	18.67 ft
Load Length	4.33 ft	14.34 ft	18.67 ft

Project: 250304B Fletcher

Location: FL1- Ceiling joists at breakfast room

Floor Joist

[2015 International Building Code(2015 NDS)]

(2) 1.5 IN x 9.25 IN x 15.0 FT @ 16 O.C.

#2 - Southern Pine - Dry Use

Section Adequate By: 22.9%

Controlling Factor: Moment



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CAUTIONS

* Properly connect sheathing to double joists/rafters or fully laminate to transfer diaphragm forces.

DEFLECTIONS

Center

Live Load 0.29 IN L/629

Dead Load 0.12 in

Total Load 0.40 IN L/447

Live Load Deflection Criteria: L/480 Total Load Deflection Criteria: L/360

REACTIONS

A

B

Live Load 468 lb 493 lb

Dead Load 208 lb 218 lb

Total Load 676 lb 711 lb

Bearing Length 0.40 in 0.42 in

SUPPORT LOADS

A

B

Live Load 351 plf 370 plf

Dead Load 156 plf 164 plf

Total Load 507 plf 533 plf

MATERIAL PROPERTIES

#2 - Southern Pine

Base Values

Adjusted

Bending Stress: Fb = 800 psi Fb' = 920 psi

Cd=1.00 CF=1.00 Cr=1.15

Shear Stress: Fv = 175 psi Fv' = 175 psi

Cd=1.00

Modulus of Elasticity: E = 1400 ksi E' = 1400 ksi

Comp. $\perp$ to Grain: Fc - $\perp$ = 565 psi Fc - $\perp$ ' = 565 psi

Controlling Moment: 2668 ft-lb

7.35 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: -711 lb

At right support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

Req'd

Provided

Section Modulus: 34.81 in³ 42.78 in³

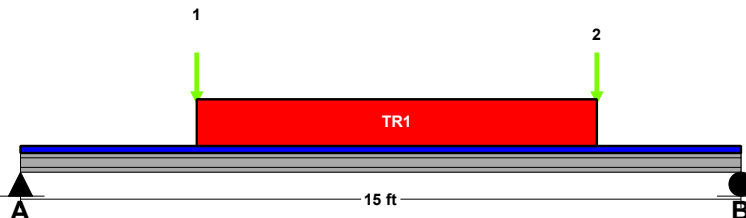
Area (Shear): 6.1 in² 27.75 in²

Moment of Inertia (deflection): 159.42 in⁴ 197.86 in⁴

Moment: 2668 ft-lb 3280 ft-lb

Shear: -711 lb 3238 lb

LOADING DIAGRAM



JOIST DATA

Center

Span Length 15 ft

Unbraced Length-Top 0 ft

Unbraced Length-Bottom 0 ft

Floor sheathing applied to top of joists-top of joists fully braced.

Floor Duration Factor 1.00

JOIST LOADING

Uniform Floor Loading

Center

Live Load LL = 20 psf

Dead Load DL = 10 psf

Total Load TL = 30 psf

TL Adj. For Joist Spacing wT = 40 plf

Wall Loading

Wall One

Live Load ($\perp$ to Joists): L1 = 127 plf

Dead Load ($\perp$ to Joists): D1 = 85 plf

Load Location X1 = 3.67 ft

Wall Two

Live Load ($\perp$ to Joists): L2 = 127 plf

Dead Load ($\perp$ to Joists): D2 = 85 plf

Load Location X2 = 12 ft

Partially Distributed Loading

Live Load LL = 20 psf

Dead Load DL = 0 psf

Load Start A = 3.67 ft

Load End B = 12 ft

Load Length C = 8.33 ft

Project: 250304B Fletcher

Location: FL2- Roof beam at Bedroom 4

Roof Beam

[2015 International Building Code(2015 NDS)]

(2) 1.75 IN x 9.25 IN x 13.33 FT

Versa-Lam 2800 Fb DF - Boise Cascade

Section Adequate By: 120.5%

Controlling Factor: Deflection



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.22 IN L/733

Dead Load 0.18 in

Total Load 0.40 IN L/397

Live Load Deflection Criteria: L/240 Total Load Deflection Criteria: L/180

REACTIONS

A

B

Live Load 945 lb 945 lb

Dead Load 801 lb 801 lb

Total Load 1746 lb 1746 lb

Bearing Length 0.66 in 0.66 in

BEAM DATA

Span Length 13.3 ft

Unbraced Length-Top 0 ft

Unbraced Length-Bottom 0 ft

Roof Pitch 10 :12

Roof Duration Factor 1.15

MATERIAL PROPERTIES

Versa-Lam 2800 Fb DF - Boise Cascade

Base Values

Adjusted

Bending Stress: Fb = 2800 psi Fb' = 3314 psi
Cd=1.15 CF=1.03

Shear Stress: Fv = 285 psi Fv' = 328 psi
Cd=1.15

Modulus of Elasticity: E = 2000 ksi E' = 2000 ksi

Comp. $\perp$ to Grain: Fc - $\perp$ = 750 psi Fc - $\perp$ ' = 750 psi

Controlling Moment: 5817 ft-lb

6.665 ft from left support

Created by combining all dead and live loads.

Controlling Shear: 1746 lb

At support.

Created by combining all dead and live loads.

Comparisons with required sections:

Req'd

Provided

Section Modulus: 21.06 in3 49.91 in3

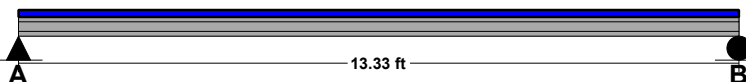
Area (Shear): 7.99 in2 32.38 in2

Moment of Inertia (deflection): 104.67 in4 230.84 in4

Moment: 5817 ft-lb 13786 ft-lb

Shear: 1746 lb 7074 lb

LOADING DIAGRAM



ROOF LOADING

Side One:

Roof Live Load: LL = 25 psf

Roof Dead Load: DL = 15 psf

Tributary Width: TW = 5.7 ft

Side Two:

Roof Live Load: LL = 0 psf

Roof Dead Load: DL = 0 psf

Tributary Width: TW = 0 ft

Wall Load: WALL = 0 plf

SLOPE/PITCH ADJUSTED LENGTHS AND LOADS

Adjusted Beam Length: Ladj = 13.33 ft

Beam Self Weight: BSW = 9 plf

Beam Uniform Live Load: wL = 142 plf

Beam Uniform Dead Load: wD_adj = 120 plf

Total Uniform Load: wT = 262 plf

Project: 250304B Fletcher

Location: F11- Ceiling joists above Family room

Floor Joist

[2015 International Building Code(2015 NDS)]

1.5 IN x 9.25 IN x 17.83 FT @ 12 O.C.

#2 - Spruce-Pine-Fir - Dry Use

Section Adequate By: 29.7%

Controlling Factor: Moment



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DEFLECTIONS

Center

Live Load 0.55 IN L/390

Dead Load 0.16 in

Total Load 0.71 IN L/300

Live Load Deflection Criteria: L/240 Total Load Deflection Criteria: L/180

REACTIONS

A

B

Live Load 244 lb 329 lb

Dead Load 89 lb 89 lb

Total Load 333 lb 418 lb

Bearing Length 0.52 in 0.66 in

SUPPORT LOADS

A

B

Live Load 244 plf 329 plf

Dead Load 89 plf 89 plf

Total Load 333 plf 418 plf

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir

Base Values

Adjusted

Bending Stress: Fb = 875 psi Fb' = 1273 psi

Cd=1.15 CF=1.10 Cr=1.15

Shear Stress: Fv = 135 psi Fv' = 155 psi

Cd=1.15

Modulus of Elasticity: E = 1400 ksi E' = 1400 ksi

Comp. $\perp$ to Grain: Fc $\perp$ = 425 psi Fc $\perp$ ' = 425 psi

Controlling Moment: 1749 ft-lb

9.45 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: -418 lb

18.0 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

Req'd

Provided

Section Modulus: 16.49 in³ 21.39 in³

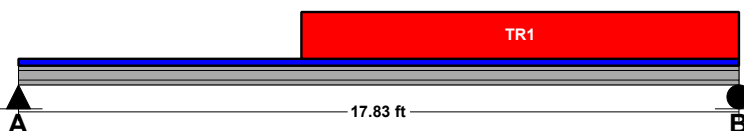
Area (Shear): 4.04 in² 13.88 in²

Moment of Inertia (deflection): 60.81 in⁴ 98.93 in⁴

Moment: 1749 ft-lb 2269 ft-lb

Shear: -418 lb 1436 lb

LOADING DIAGRAM



JOIST DATA

Center

Span Length 17.83 ft

Unbraced Length-Top 0 ft

Unbraced Length-Bottom 0 ft

Floor sheathing applied to top of joists-top of joists fully braced.

Floor Duration Factor 1.15

JOIST LOADING

Uniform Floor Loading

Center

Live Load LL = 20 psf

Dead Load DL = 10 psf

Total Load TL = 30 psf

TL Adj. For Joist Spacing wT = 30 plf

Partially Distributed Loading

Live Load LL = 20 psf

Dead Load DL = 0 psf

Load Start A = 7 ft

Load End B = 17.83 ft

Load Length C = 10.83 ft

Project: 250304B Fletcher

Location: F11- Beam at family room ceiling
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 16.0 IN x 18.67 FT
Versa-Lam 2800 Fb DF - Boise Cascade
Section Adequate By: 25.8%
Controlling Factor: Deflection



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.49 IN L/453
Dead Load 0.16 in
Total Load 0.65 IN L/344
Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 3919 lb 4231 lb
Dead Load 1220 lb 1376 lb
Total Load 5139 lb 5607 lb
Bearing Length 1.96 in 2.14 in

BEAM DATA

Center

Span Length 18.67 ft
Unbraced Length-Top 0 ft
Unbraced Length-Bottom 18.67 ft
Live Load Duration Factor 1.00
Notch Depth 0.00

MATERIAL PROPERTIES

Versa-Lam 2800 Fb DF - Boise Cascade

	Base Values	Adjusted
Bending Stress:	Fb = 2800 psi Cd=1.00 CF=0.97	Fb' = 2712 psi
Shear Stress:	Fv = 285 psi Cd=1.00	Fv' = 285 psi
Modulus of Elasticity:	E = 2000 ksi	E' = 2000 ksi
Comp. $\perp$ to Grain:	Fc - $\perp$ = 750 psi	Fc - $\perp$ = 750 psi

Controlling Moment:

24715 ft-lb

9.71 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear:

-5607 lb

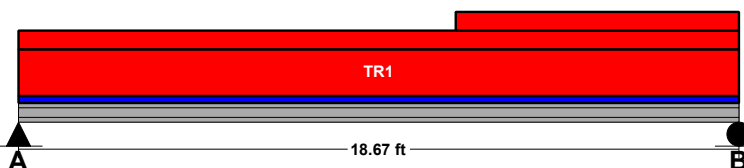
19.0 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

	Req'd	Provided
Section Modulus:	109.36 in3	149.33 in3
Area (Shear):	29.51 in2	56 in2
Moment of Inertia (deflection):	949.32 in4	1194.67 in4
Moment:	24715 ft-lb	33748 ft-lb
Shear:	-5607 lb	10640 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf
Uniform Dead Load 0 plf
Beam Self Weight 16 plf
Total Uniform Load 16 plf

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number	One *	Two *	Three *
Left Live Load	329 plf	80 plf	70 plf
Left Dead Load	89 plf	20 plf	35 plf
Right Live Load	329 plf	80 plf	70 plf
Right Dead Load	89 plf	20 plf	35 plf
Load Start	0 ft	0 ft	11.33 ft
Load End	18.67 ft	18.67 ft	18.67 ft
Load Length	18.67 ft	18.67 ft	7.34 ft

* Load obtained from Load Tracker. See Summary Report for details.

Project: 250304B Fletcher

Location: FL1- Beam at breakfast room

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(3) 1.75 IN x 9.25 IN x 15.0 FT

Versa-Lam 2800 Fb DF - Boise Cascade

Section Adequate By: 38.7%

Controlling Factor: Deflection



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.36 IN L/499

Dead Load 0.13 in

Total Load 0.49 IN L/366

Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 2759 lb 730 lb

Dead Load 920 lb 327 lb

Total Load 3679 lb 1057 lb

Bearing Length 0.93 in 0.27 in

BEAM DATA

Center

Span Length 15 ft

Unbraced Length-Top 0 ft

Unbraced Length-Bottom 15 ft

Live Load Duration Factor 1.00

Notch Depth 0.00

MATERIAL PROPERTIES

Versa-Lam 2800 Fb DF - Boise Cascade

Base Values

Adjusted

Bending Stress: Fb = 2800 psi Fb' = 2882 psi

Cd=1.00 CF=1.03

Shear Stress: Fv = 285 psi Fv' = 285 psi

Cd=1.00

Modulus of Elasticity: E = 2000 ksi E' = 2000 ksi

Comp. $\perp$ to Grain: Fc - $\perp$ = 750 psi Fc - $\perp$ ' = 750 psi

Controlling Moment: 10858 ft-lb

3.9 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: 3679 lb

At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

Req'd

Provided

Section Modulus: 45.21 in3 74.87 in3

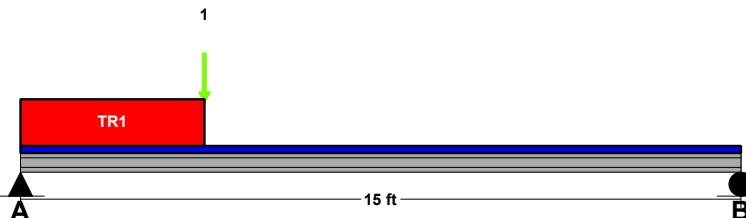
Area (Shear): 19.37 in2 48.56 in2

Moment of Inertia (deflection): 249.6 in4 346.26 in4

Moment: 10858 ft-lb 17982 ft-lb

Shear: 3679 lb 9227 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf

Uniform Dead Load 0 plf

Beam Self Weight 14 plf

Total Uniform Load 14 plf

POINT LOADS - CENTER SPAN

Load Number One *

Live Load 2229 lb

Dead Load 694 lb

Location 3.83 ft

* Load obtained from Load Tracker. See Summary Report for details.

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number One *

Left Live Load 329 plf

Left Dead Load 89 plf

Right Live Load 329 plf

Right Dead Load 89 plf

Load Start 0 ft

Load End 3.83 ft

Load Length 3.83 ft

* Load obtained from Load Tracker. See Summary Report for details.

Project: 250304B Fletcher

Location: FL1- Header at bedroom hallway
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 9.25 IN x 4.25 FT
Versa-Lam 2800 Fb DF - Boise Cascade
Section Adequate By: 97.9%
Controlling Factor: Shear



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.03 IN L/2026
Dead Load 0.01 in
Total Load 0.03 IN L/1522
Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A B

Live Load 2319 lb 2089 lb
Dead Load 789 lb 717 lb
Total Load 3108 lb 2806 lb
Bearing Length 1.18 in 1.07 in

BEAM DATA

Center

Span Length 4.25 ft
Unbraced Length-Top 0 ft
Unbraced Length-Bottom 4.25 ft
Live Load Duration Factor 1.00
Notch Depth 0.00

MATERIAL PROPERTIES

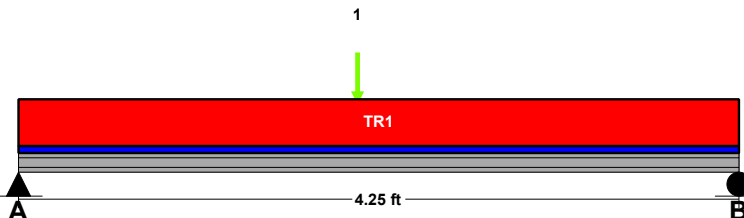
Versa-Lam 2800 Fb DF - Boise Cascade

	Base Values	Adjusted
Bending Stress:	Fb = 2800 psi Cd=1.00 CF=1.03	Fb' = 2882 psi
Shear Stress:	Fv = 285 psi Cd=1.00	Fv' = 285 psi
Modulus of Elasticity:	E = 2000 ksi	E' = 2000 ksi
Comp. $\perp$ to Grain:	Fc $\perp$ = 750 psi	Fc $\perp$ ' = 750 psi

Controlling Moment: 5845 ft-lb
2.0 Ft from left support of span 2 (Center Span)
Created by combining all dead loads and live loads on span(s) 2
Controlling Shear: 3108 lb
At left support of span 2 (Center Span)
Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:	Req'd	Provided
Section Modulus:	24.34 in3	49.91 in3
Area (Shear):	16.36 in2	32.38 in2
Moment of Inertia (deflection):	41.02 in4	230.84 in4
Moment:	5845 ft-lb	11988 ft-lb
Shear:	3108 lb	6151 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf
Uniform Dead Load 0 plf
Beam Self Weight 9 plf
Total Uniform Load 9 plf

POINT LOADS - CENTER SPAN

Load Number One *
Live Load 3919 lb
Dead Load 1220 lb
Location 2 ft

* Load obtained from Load Tracker. See Summary Report for details.

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number One
Left Live Load 115 plf
Left Dead Load 58 plf
Right Live Load 115 plf
Right Dead Load 58 plf
Load Start 0 ft
Load End 4.25 ft
Load Length 4.25 ft

Project: 250304B Fletcher

Location: FL1-Beam between Kitchen and Family
Uniformly Loaded Floor Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 16.0 IN x 17.83 FT
1.9E Microllam - iLevel Trus Joist
Section Adequate By: 11.5%
Controlling Factor: Shear



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.33 IN L/642
Dead Load 0.15 in
Total Load 0.48 IN L/443
Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 2635 lb 6998 lb
Dead Load 1058 lb 2546 lb
Total Load 3693 lb 9544 lb
Bearing Length 1.41 in 3.64 in

BEAM DATA

Center

Span Length 17.83 ft
Unbraced Length-Top 0 ft
Floor Duration Factor 1.00
Notch Depth 0.00

MATERIAL PROPERTIES

1.9E Microllam - iLevel Trus Joist

Base Values

Adjusted

Bending Stress: $F_b = 2600$ psi $F_b' = 2500$ psi
 $C_d = 1.00$ $CF = 0.96$
Shear Stress: $F_v = 285$ psi $F_v' = 285$ psi
 $C_d = 1.00$
Modulus of Elasticity: $E = 1900$ ksi $E' = 1900$ ksi
Comp. $\perp$ to Grain: $F_c - \perp = 750$ psi $F_c - \perp' = 750$ psi

Controlling Moment: 20485 ft-lb

8.915 ft from left support

Created by combining all dead and live loads.

Controlling Shear: -9544 lb

At support.

Created by combining all dead and live loads.

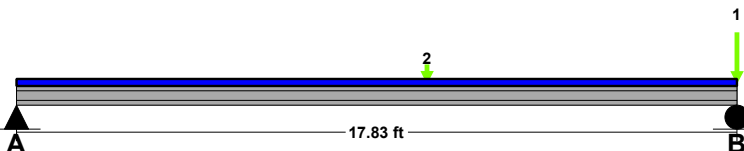
Comparisons with required sections:

Req'd

Provided

Section Modulus: 98.32 in³ 149.33 in³
Area (Shear): 50.23 in² 56 in²
Moment of Inertia (deflection): 669.39 in⁴ 1194.67 in⁴
Moment: 20485 ft-lb 31114 ft-lb
Shear: -9544 lb 10640 lb

LOADING DIAGRAM



FLOOR LOADING

		Side 1	Side 2
Floor Live Load	FLL =	0 psf	40 psf
Floor Dead Load	FDL =	0 psf	10 psf
Floor Tributary Width	FTW =	0 ft	6.3 ft
Wall Load	WALL =	0 plf	

BEAM LOADING

Beam Total Live Load: $w_L = 250$ plf
Beam Total Dead Load: $w_D = 63$ plf
Beam Self Weight: $BSW = 18$ plf
Total Maximum Load: $w_T = 330$ plf

POINT LOADS - CENTER SPAN

Load Number	One *	Two *
Live Load	4231 lb	945 lb
Dead Load	1376 lb	801 lb
Location	17.83 ft	10.16 ft

* Load obtained from Load Tracker. See Summary Report for details.

Project: 250304B Fletcher

Location: FL2- Beam at fl2 bath
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.5 IN x 9.25 IN x 9.67 FT
#2 - Spruce-Pine-Fir - Dry Use
Section Adequate By: 3.3%
Controlling Factor: Moment



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CAUTIONS

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DEFLECTIONS

Center

Live Load 0.13 IN L/881
Dead Load 0.07 in
Total Load 0.20 IN L/578
Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 927 lb 848 lb
Dead Load 485 lb 446 lb
Total Load 1412 lb 1294 lb
Bearing Length 1.11 in 1.01 in

BEAM DATA

Center

Span Length 9.67 ft
Unbraced Length-Top 0 ft
Unbraced Length-Bottom 9.67 ft
Live Load Duration Factor 1.00
Notch Depth 0.00

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir

Base Values

Adjusted

Bending Stress: $F_b = 875$ psi $F_b' = 963$ psi
 $C_d = 1.00$ $CF = 1.10$
Shear Stress: $F_v = 135$ psi $F_v' = 135$ psi
 $C_d = 1.00$
Modulus of Elasticity: $E = 1400$ ksi $E' = 1400$ ksi
Comp. $\perp$ to Grain: $F_c - \perp = 425$ psi $F_c - \perp' = 425$ psi

Controlling Moment:

3322 ft-lb

4.74 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear:

1412 lb

At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

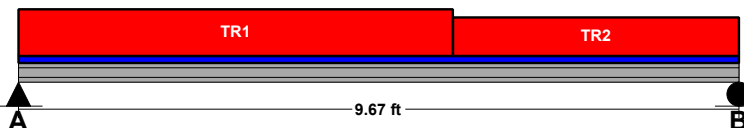
Comparisons with required sections:

Req'd

Provided

Section Modulus: 41.41 in³ 42.78 in³
Area (Shear): 15.69 in² 27.75 in²
Moment of Inertia (deflection): 82.17 in⁴ 197.86 in⁴
Moment: 3322 ft-lb 3431 ft-lb
Shear: 1412 lb 2498 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf
Uniform Dead Load 0 plf
Beam Self Weight 5 plf
Total Uniform Load 5 plf

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number	One	Two
Left Live Load	197 plf	163 plf
Left Dead Load	98 plf	81 plf
Right Live Load	197 plf	163 plf
Right Dead Load	98 plf	81 plf
Load Start	0 ft	5.83 ft
Load End	5.83 ft	9.67 ft
Load Length	5.83 ft	3.84 ft

Project: 250304B Fletcher

Location: FL1- beam at master entry

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(3) 1.5 IN x 9.25 IN x 5.0 FT

#2 - Spruce-Pine-Fir - Dry Use

Section Adequate By: 35.8%

Controlling Factor: Shear



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.03 IN L/2152

Dead Load 0.01 in

Total Load 0.04 IN L/1607

Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 2060 lb 2060 lb

Dead Load 699 lb 699 lb

Total Load 2759 lb 2759 lb

Bearing Length 1.44 in 1.44 in

BEAM DATA

Center

Span Length 5 ft

Unbraced Length-Top 0 ft

Unbraced Length-Bottom 5 ft

Live Load Duration Factor 1.00

Notch Depth 0.00

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir

Base Values

Adjusted

Bending Stress: Fb = 875 psi Fb' = 1107 psi

Cd=1.00 CF=1.10 Cr=1.15

Shear Stress: Fv = 135 psi Fv' = 135 psi

Cd=1.00

Modulus of Elasticity: E = 1400 ksi E' = 1400 ksi

Comp. $\perp$ to Grain: Fc - $\perp$ = 425 psi Fc - $\perp$ ' = 425 psi

Controlling Moment: 3449 ft-lb

2.5 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: 2759 lb

At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

Req'd

Provided

Section Modulus: 37.39 in3 64.17 in3

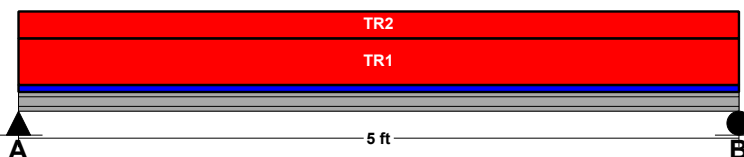
Area (Shear): 30.65 in2 41.63 in2

Moment of Inertia (deflection): 49.65 in4 296.79 in4

Moment: 3449 ft-lb 5919 ft-lb

Shear: 2759 lb 3746 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf

Uniform Dead Load 0 plf

Beam Self Weight 8 plf

Total Uniform Load 8 plf

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number	One	Two
Left Live Load	557 plf	267 plf
Left Dead Load	139 plf	133 plf
Right Live Load	557 plf	267 plf
Right Dead Load	139 plf	133 plf
Load Start	0 ft	0 ft
Load End	5 ft	5 ft
Load Length	5 ft	5 ft

Project: 250304B Fletcher

Location: FL1- Header at Breakfast room door

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(2) 1.5 IN x 9.25 IN x 6.0 FT

#2 - Southern Pine - Dry Use

Section Adequate By: 23.4%

Controlling Factor: Moment



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.04 IN L/1949

Dead Load 0.02 in

Total Load 0.05 IN L/1332

Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 1053 lb 1053 lb

Dead Load 488 lb 488 lb

Total Load 1541 lb 1541 lb

Bearing Length 0.91 in 0.91 in

BEAM DATA

Center

Span Length 6 ft

Unbraced Length-Top 0 ft

Unbraced Length-Bottom 6 ft

Live Load Duration Factor 1.00

Notch Depth 0.00

MATERIAL PROPERTIES

#2 - Southern Pine

Base Values

Adjusted

Bending Stress: $F_b = 800$ psi $F_b' = 800$ psi

$C_d = 1.00$ $CF = 1.00$

Shear Stress: $F_v = 175$ psi $F_v' = 175$ psi

$C_d = 1.00$

Modulus of Elasticity: $E = 1400$ ksi $E' = 1400$ ksi

Comp. $\perp$ to Grain: $F_c - \perp = 565$ psi $F_c - \perp' = 565$ psi

Controlling Moment: 2311 ft-lb

3.0 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: 1541 lb

At left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

Req'd

Provided

Section Modulus: 34.67 in³ 42.78 in³

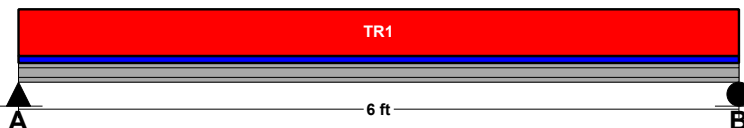
Area (Shear): 13.21 in² 27.75 in²

Moment of Inertia (deflection): 36.55 in⁴ 197.86 in⁴

Moment: 2311 ft-lb 2852 ft-lb

Shear: 1541 lb 3238 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf

Uniform Dead Load 0 plf

Beam Self Weight 7 plf

Total Uniform Load 7 plf

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number One *

Left Live Load 351 plf

Left Dead Load 156 plf

Right Live Load 351 plf

Right Dead Load 156 plf

Load Start 0 ft

Load End 6 ft

Load Length 6 ft

* Load obtained from Load Tracker. See Summary Report for details.

Project: 250304B Fletcher

Location: FL1- Beam at stairs in garage
Multi-Loaded Multi-Span Beam
[2015 International Building Code(2015 NDS)]
(2) 1.75 IN x 9.25 IN x 11.67 FT
Versa-Lam 2800 Fb DF - Boise Cascade
Section Adequate By: 68.1%
Controlling Factor: Deflection



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.23 IN L/605
Dead Load 0.07 in
Total Load 0.30 IN L/469
Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 1406 lb 1615 lb
Dead Load 409 lb 463 lb
Total Load 1815 lb 2078 lb
Bearing Length 0.69 in 0.79 in

BEAM DATA

Center

Span Length 11.67 ft
Unbraced Length-Top 0 ft
Unbraced Length-Bottom 11.67 ft
Live Load Duration Factor 1.00
Notch Depth 0.00

MATERIAL PROPERTIES

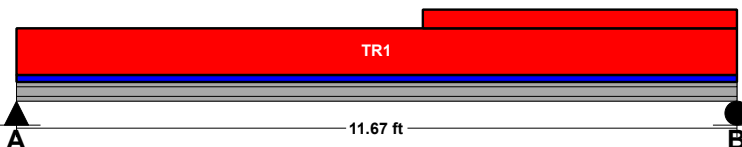
Versa-Lam 2800 Fb DF - Boise Cascade

	Base Values	Adjusted
Bending Stress:	Fb = 2800 psi Cd=1.00 CF=1.03	Fb' = 2882 psi
Shear Stress:	Fv = 285 psi Cd=1.00	Fv' = 285 psi
Modulus of Elasticity:	E = 2000 ksi	E' = 2000 ksi
Comp. $\perp$ to Grain:	Fc - $\perp$ = 750 psi	Fc - $\perp$ = 750 psi

Controlling Moment: 5609 ft-lb
6.19 Ft from left support of span 2 (Center Span)
Created by combining all dead loads and live loads on span(s) 2
Controlling Shear: -2078 lb
12.0 Ft from left support of span 2 (Center Span)
Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:	Req'd	Provided
Section Modulus:	23.35 in3	49.91 in3
Area (Shear):	10.94 in2	32.38 in2
Moment of Inertia (deflection):	137.36 in4	230.84 in4
Moment:	5609 ft-lb	11988 ft-lb
Shear:	-2078 lb	6151 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf
Uniform Dead Load 0 plf
Beam Self Weight 9 plf
Total Uniform Load 9 plf

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number	One	Two
Left Live Load	227 plf	73 plf
Left Dead Load	57 plf	19 plf
Right Live Load	227 plf	73 plf
Right Dead Load	57 plf	19 plf
Load Start	0 ft	6.58 ft
Load End	11.67 ft	11.67 ft
Load Length	11.67 ft	5.09 ft

Project: 250304B Fletcher

Location: F11- Beam between garages

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(2) 1.5 IN x 9.25 IN x 3.67 FT

#2 - Southern Pine - Dry Use

Section Adequate By: 36.1%

Controlling Factor: Shear



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS

Center

Live Load 0.00 IN L/MAX

Dead Load 0.00 in

Total Load 0.01 IN L/8709

Live Load Deflection Criteria: L/360 Total Load Deflection Criteria: L/240

REACTIONS

A

B

Live Load 332 lb 1797 lb

Dead Load 162 lb 582 lb

Total Load 494 lb 2379 lb

Bearing Length 0.29 in 1.40 in

BEAM DATA

Center

Span Length 3.67 ft

Unbraced Length-Top 0 ft

Unbraced Length-Bottom 3.67 ft

Live Load Duration Factor 1.00

Notch Depth 0.00

MATERIAL PROPERTIES

#2 - Southern Pine

Base Values

Adjusted

Bending Stress: $F_b = 800$ psi $F_b' = 800$ psi

$C_d = 1.00$ $CF = 1.00$

Shear Stress: $F_v = 175$ psi $F_v' = 175$ psi

$C_d = 1.00$

Modulus of Elasticity: $E = 1400$ ksi $E' = 1400$ ksi

Comp. $\perp$ to Grain: $F_c - \perp = 565$ psi $F_c - \perp' = 565$ psi

Controlling Moment: 563 ft-lb

2.28 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: -2379 lb

4.0 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:

Req'd

Provided

Section Modulus: 8.44 in³ 42.78 in³

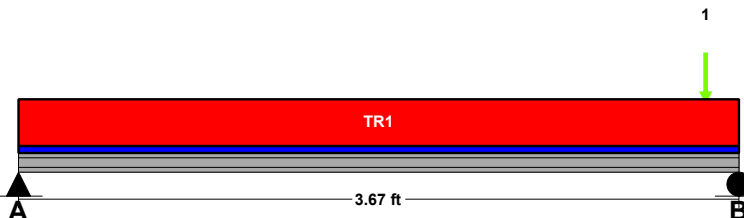
Area (Shear): 20.39 in² 27.75 in²

Moment of Inertia (deflection): 5.68 in⁴ 197.86 in⁴

Moment: 563 ft-lb 2852 ft-lb

Shear: -2379 lb 3238 lb

LOADING DIAGRAM



UNIFORM LOADS

Center

Uniform Live Load 0 plf

Uniform Dead Load 0 plf

Beam Self Weight 7 plf

Total Uniform Load 7 plf

POINT LOADS - CENTER SPAN

Load Number One *

Live Load 1615 lb

Dead Load 463 lb

Location 3.5 ft

* Load obtained from Load Tracker. See Summary Report for details.

TRAPEZOIDAL LOADS - CENTER SPAN

Load Number One

Left Live Load 140 plf

Left Dead Load 70 plf

Right Live Load 140 plf

Right Dead Load 70 plf

Load Start 0 ft

Load End 3.67 ft

Load Length 3.67 ft