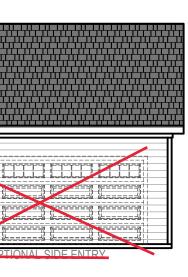


THE HAVILLAND

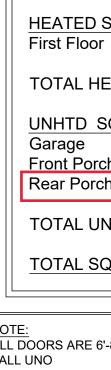
1/8" = 1'-0"

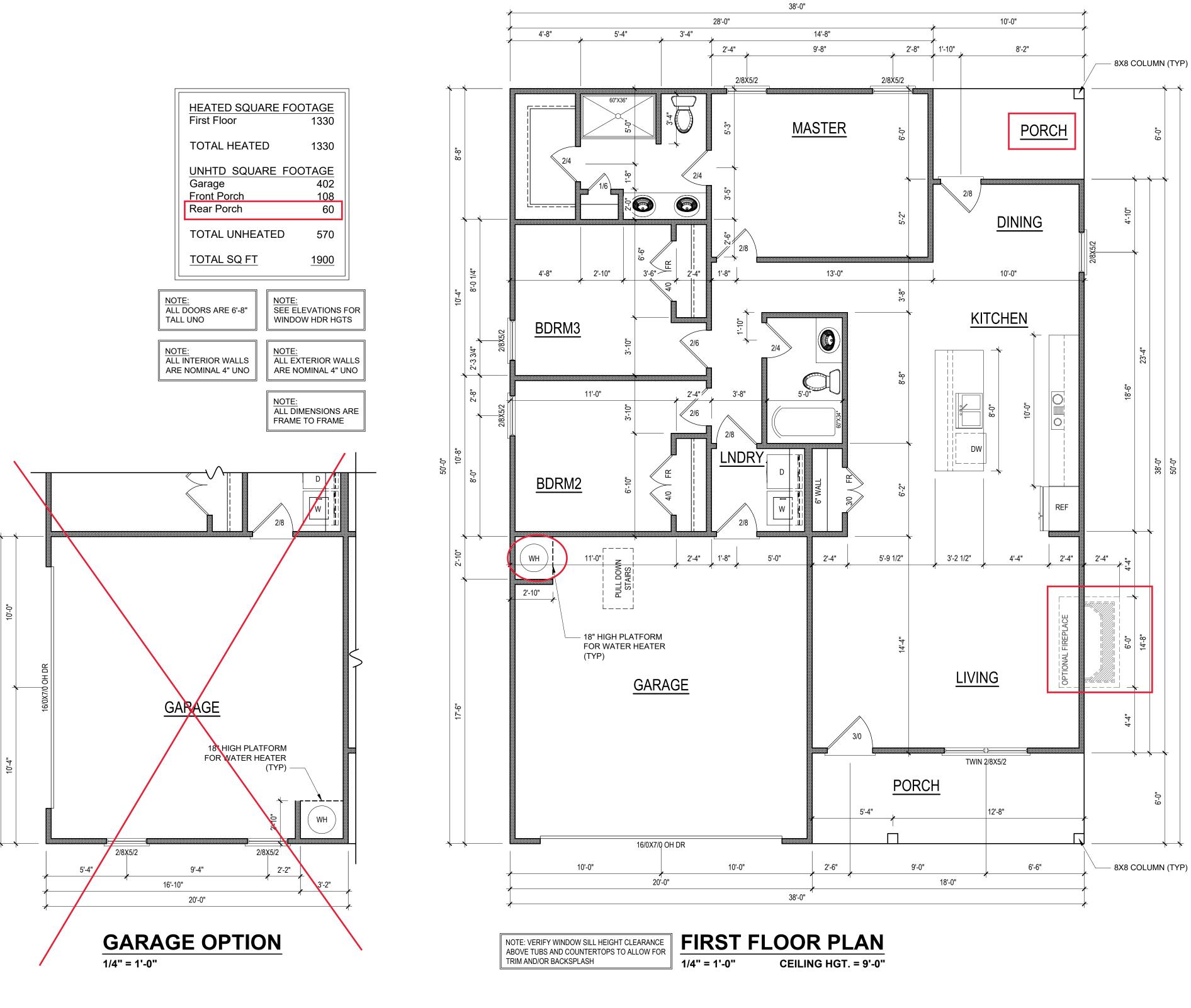
JNC-000-023 2x4 WALLS

- DRB DESIGN assumes no liability for any home constructed from this plan. 2. All construction shall conform to the latest requirements of "North Carolina State 2018 residential building code",
- in addition to all local codes and regulations. 3. Should these plans require structural calculations for permitting the contractor shall be required to obtain the services of a structural engineer after notifying DRB DESIGN that such services are required.
- 4. Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN. 5. Design and construction are complex and, although the designer performed his services with due care and diligence, perfection is not a guarantee.
- Communication is imperfect and every contingency cannot be anticipated.
- 7. Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB DESIGN. Failure to notify the DRB DESIGN compounds misunderstandings and increases construction costs. 8. A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all
- responsibilities for all consequences. 9. Changes made to these plans without the consent of the designer are unauthorized and shall relieve DRB DESIGN of responsibility for any and all consequences arriving out of such changes.
- 10. Written dimensions on these plans always have precedence over scaled dimensions.
- 11. It is the contractors responsibility to verify and be responsible for all dimensions and square footage prior to construction, as well as conditions on the job site. DRB DESIGN is not responsible for dimension and square
- footage errors once construction has begun. 12. DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.

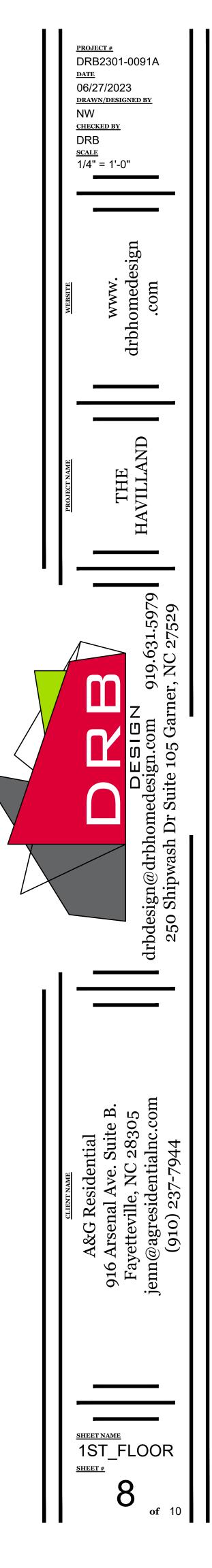




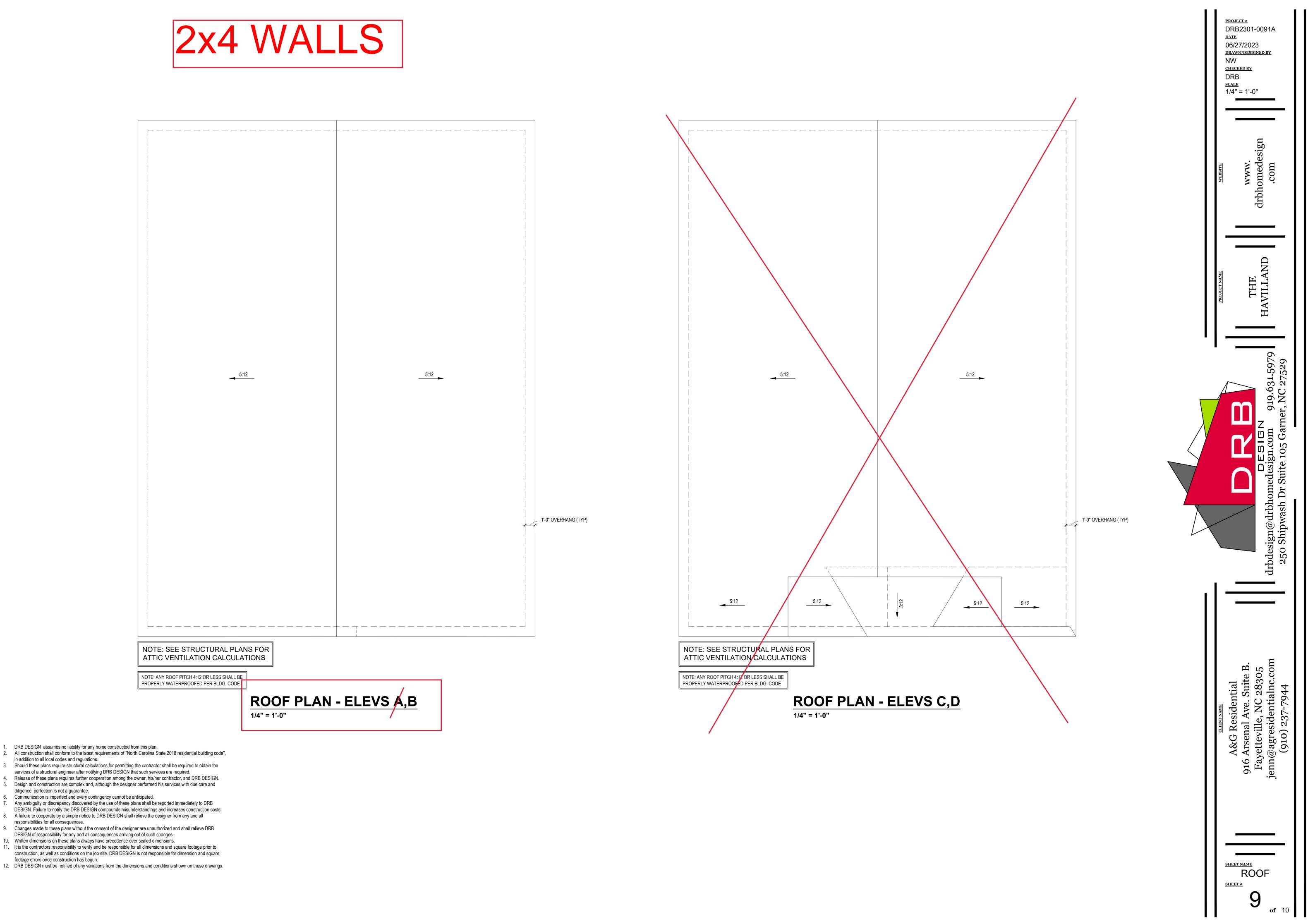




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3.

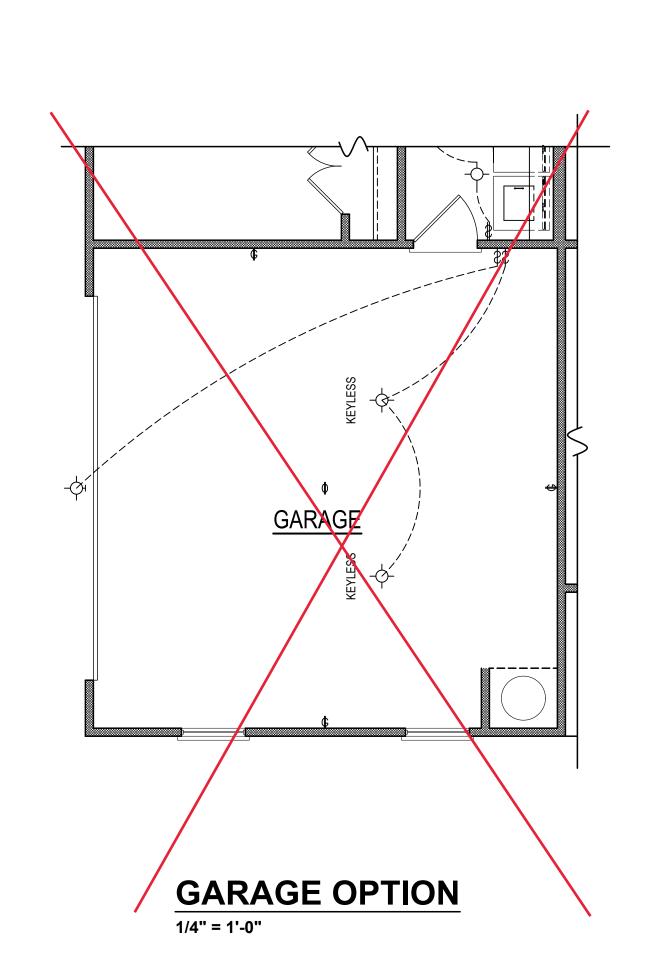
4.

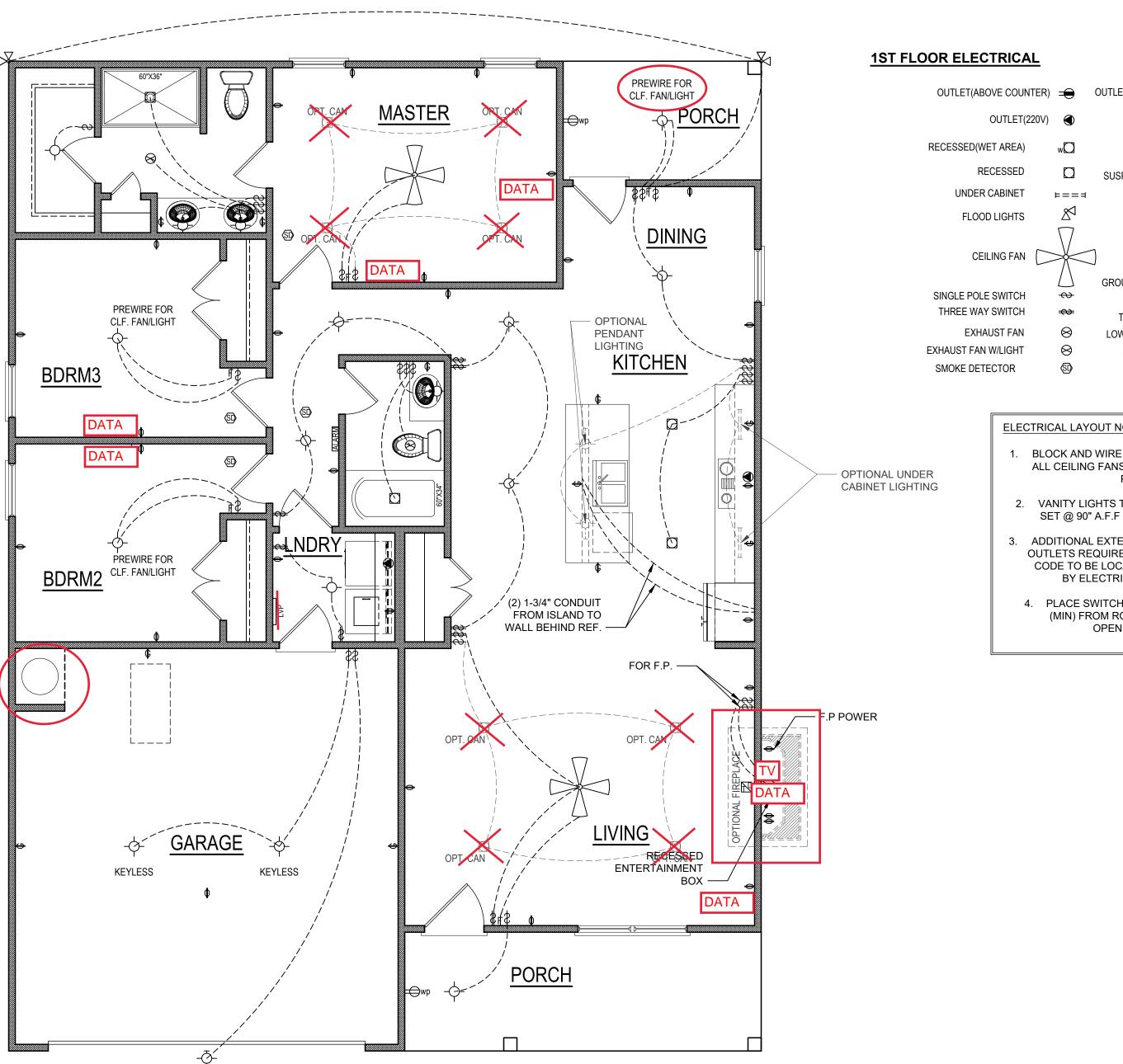
6.

7.

9.







DESIGN

OF DRB

CONSENT

WRITTEN

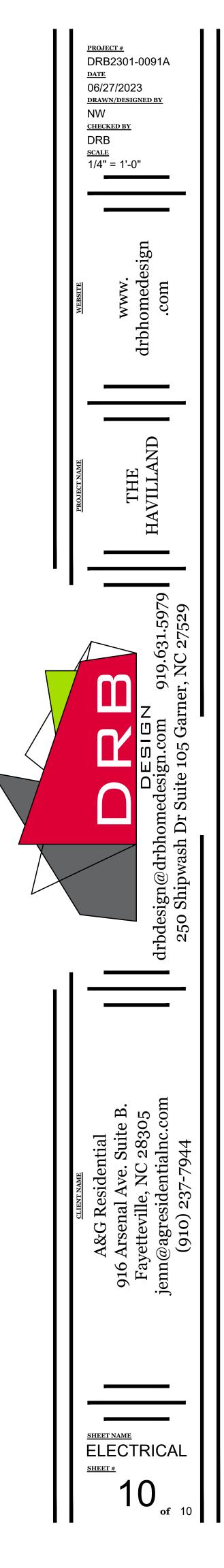
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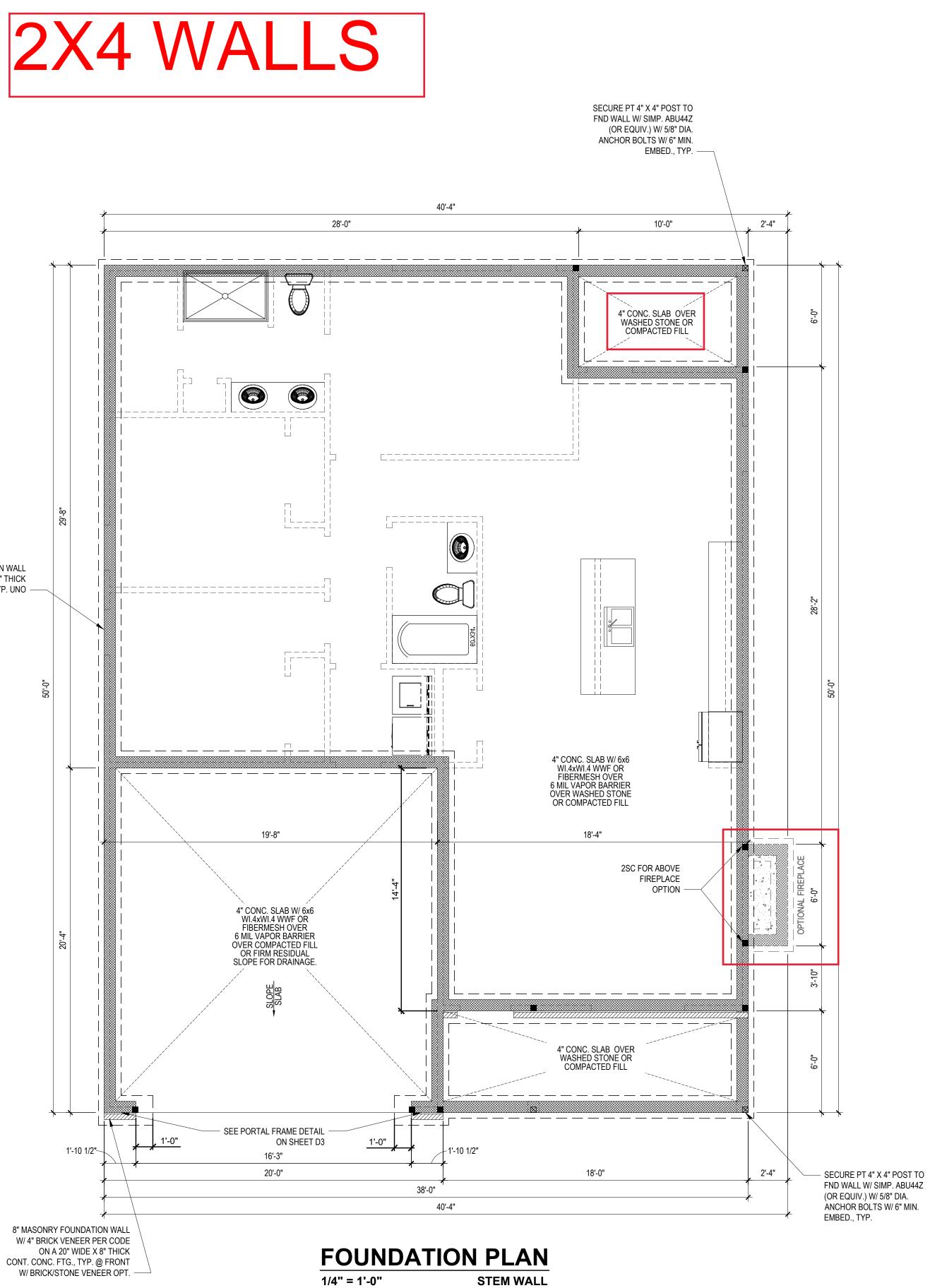
WITHOU

- DRB DESIGN assumes no liability for any home constructed from this plan. All construction shall conform to the latest requirements of "North Carolina State 2018 residential building code",
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FIRST FLOOR PLAN CEILING HGT. = 9'-0" 1/4" = 1'-0"

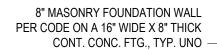
ET(WATERPROOF)	₩p
FLUSHMOUNT	-ф-
WALLMOUNT	ф-
PENDED FIXTURE	- \
FLOURESCENT	
FLOURESCENT	
110 OUTLET	Ф
OUND FAULT OUTLE	Г¢Г
FAN SWITCH	f
TV CONNECTION	TV-
W VOLTAGE PANEL	
ALARM PANEL	ALARM
TV/DATA	

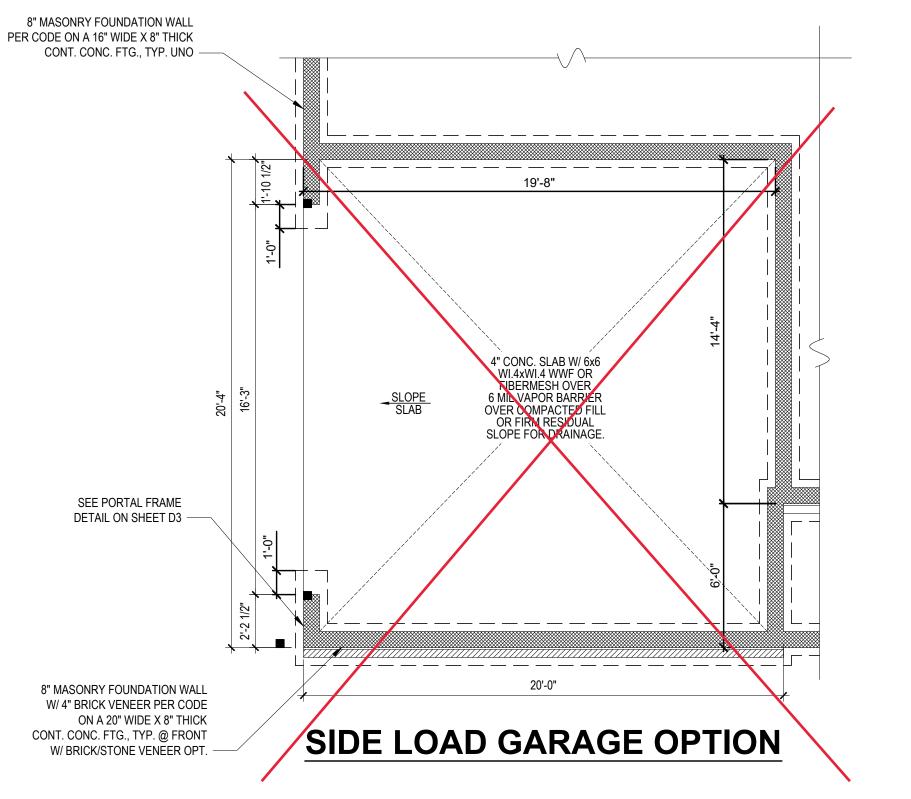




DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION		
	()	(,	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BASED ON SEISMIC ZONES A, B & C				





STRUCTURAL NOTES:

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS. 3) ALL LUMBER SHALL BE SYP #2 (UNO)
- ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
- 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- 5) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO)
- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, fc = 3000 PSI MIN. 9)
- 10) PRESUMPTIVE BEARING CAPACITY = 2000 PSF 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
- MASONRY. 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.) 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

means, methods, techn procedures or safety p *Any deviations or disc to be brought to the in Tyndall Engineering & do so will void Tyndal P.A. liability. *Please review these do Tyndall Engineering & interpret that all dime recommendations, etc. presented in these	*Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions,						
PREMI	SEAL F 92489923 K						
TYNDALL ENGINEERING & DESIGN, P.A.	7 919 772-1200 = # 919 772-1200 = # 919 772-9658 250 Shipwssh Drive = Garner = North Carolina = 27529 www.tyndellengineering.com						
THE C-2	AROUNT AR						
client: A&G RESIDENTIAL	Plan: THE HAVILLAND GARAGE LEFT						
FOUNDATION PLAN	STEM WALL OPT.						
Date: 7/6/2023 Engineered By HJS DWG. Checke PTII Scale: SEE PL SEE PL No. Date: 1 2 3 3	DRB2301-0091A Date: 7/6/2023 Engineered By: HJS DWG. Checked By: PTII Scale: SEE PLAN REVISIONS No. Date: A						
	<u>Sheet Number</u> Sheet Number S1B 1 of 5						



DESIGN LOADS

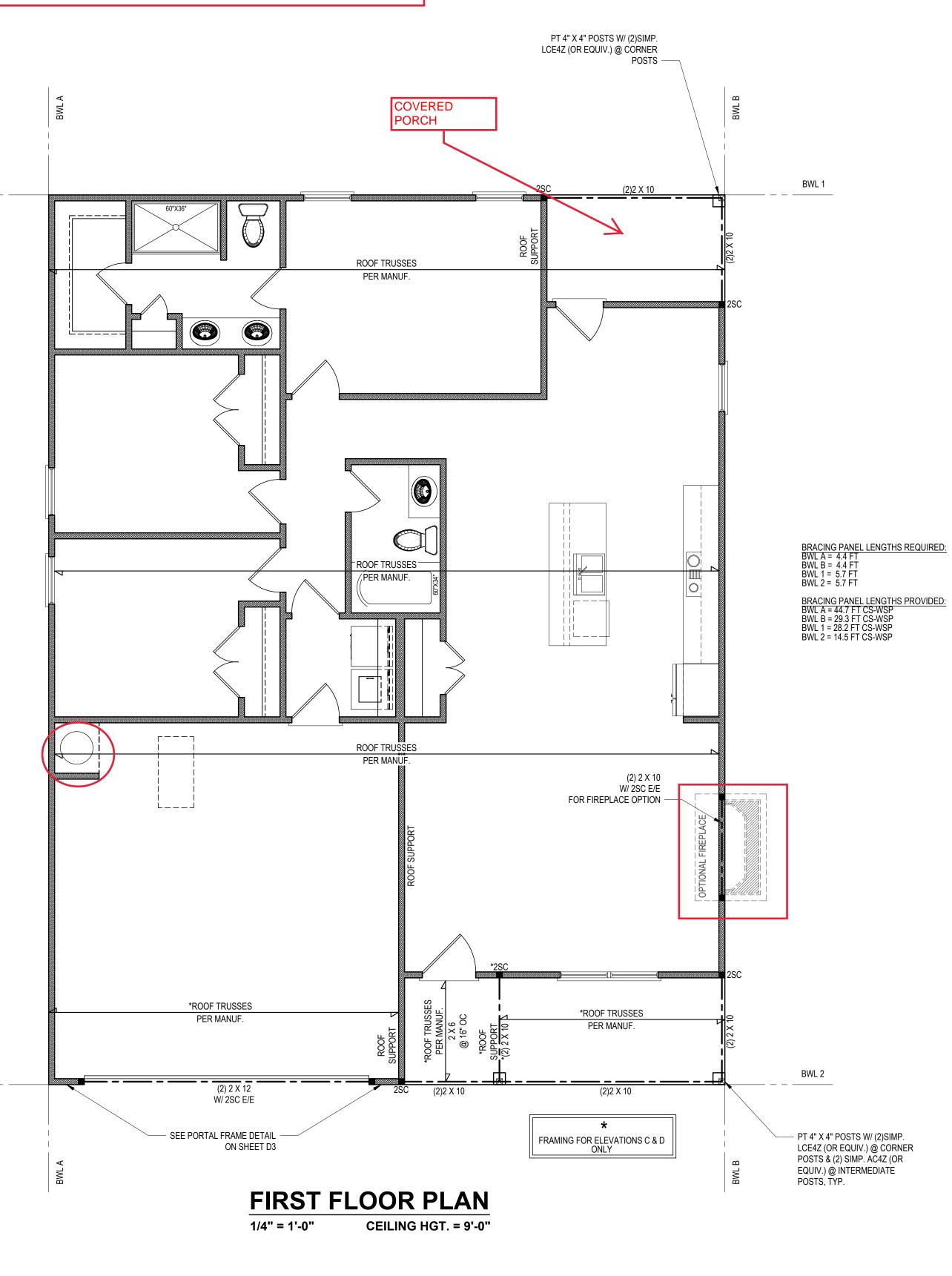
	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION	
	()	(* = *)	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
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WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BASED ON SEISMIC ZONES A, B & C				

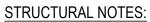
ROOF TRUSSES PER MANUF. ז א ב 50 SEE PORTAL FRAME DETAIL ON SHEET D3 -----BWL 2F **GARAGE OPTION** 1/4" = 1'-0"

BWL 1

BWL 2

2X4 WALLS



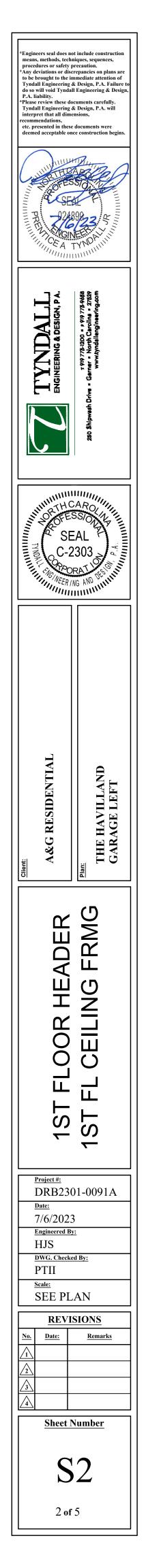


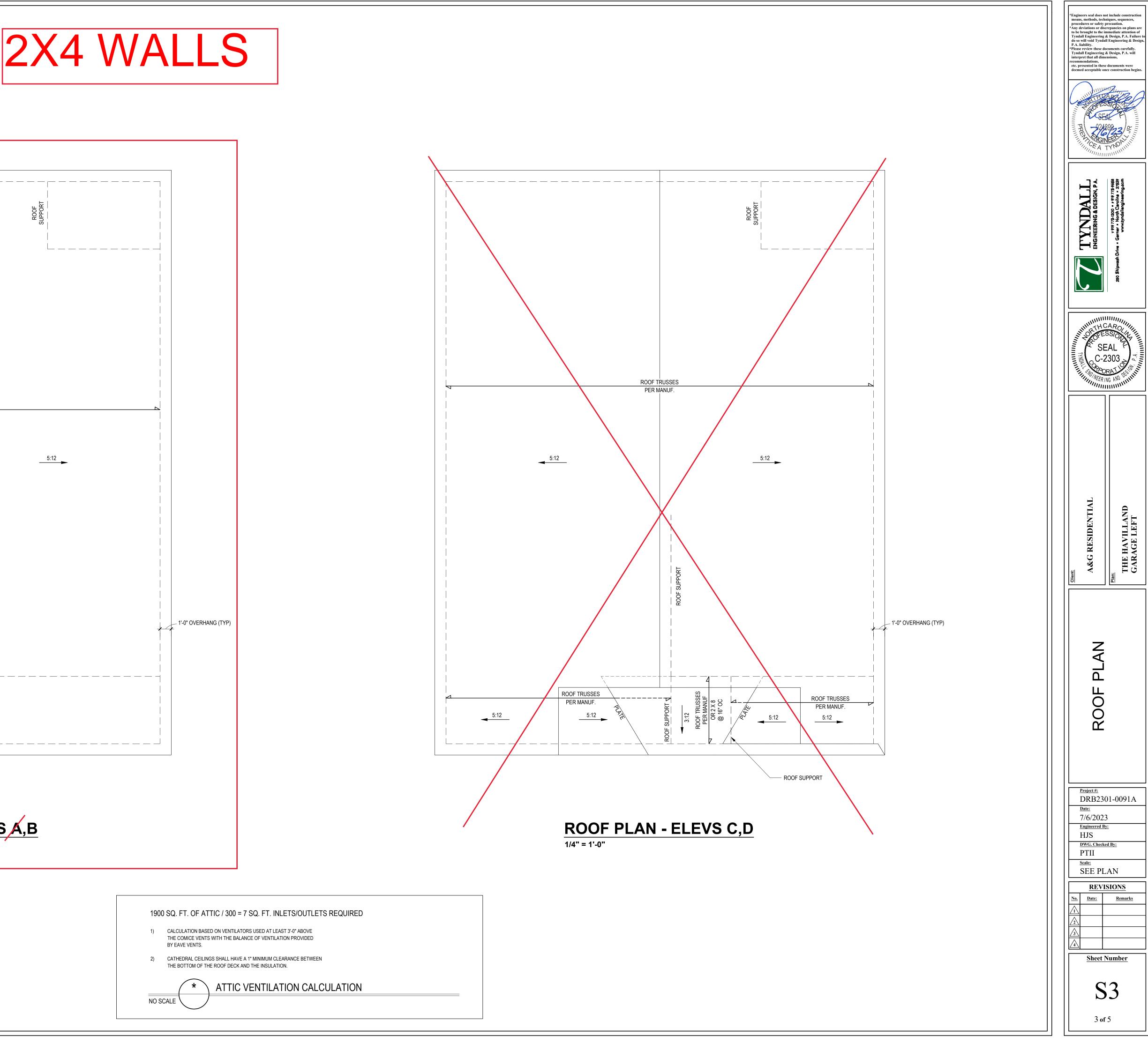
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- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- 9) ALL CONCRETE, fc = 3000 PSI MIN.
- 10) PRESUMPTIVE BEARING CAPACITY = 2000 PSF 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.) 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018
- NCRC. 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS
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- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

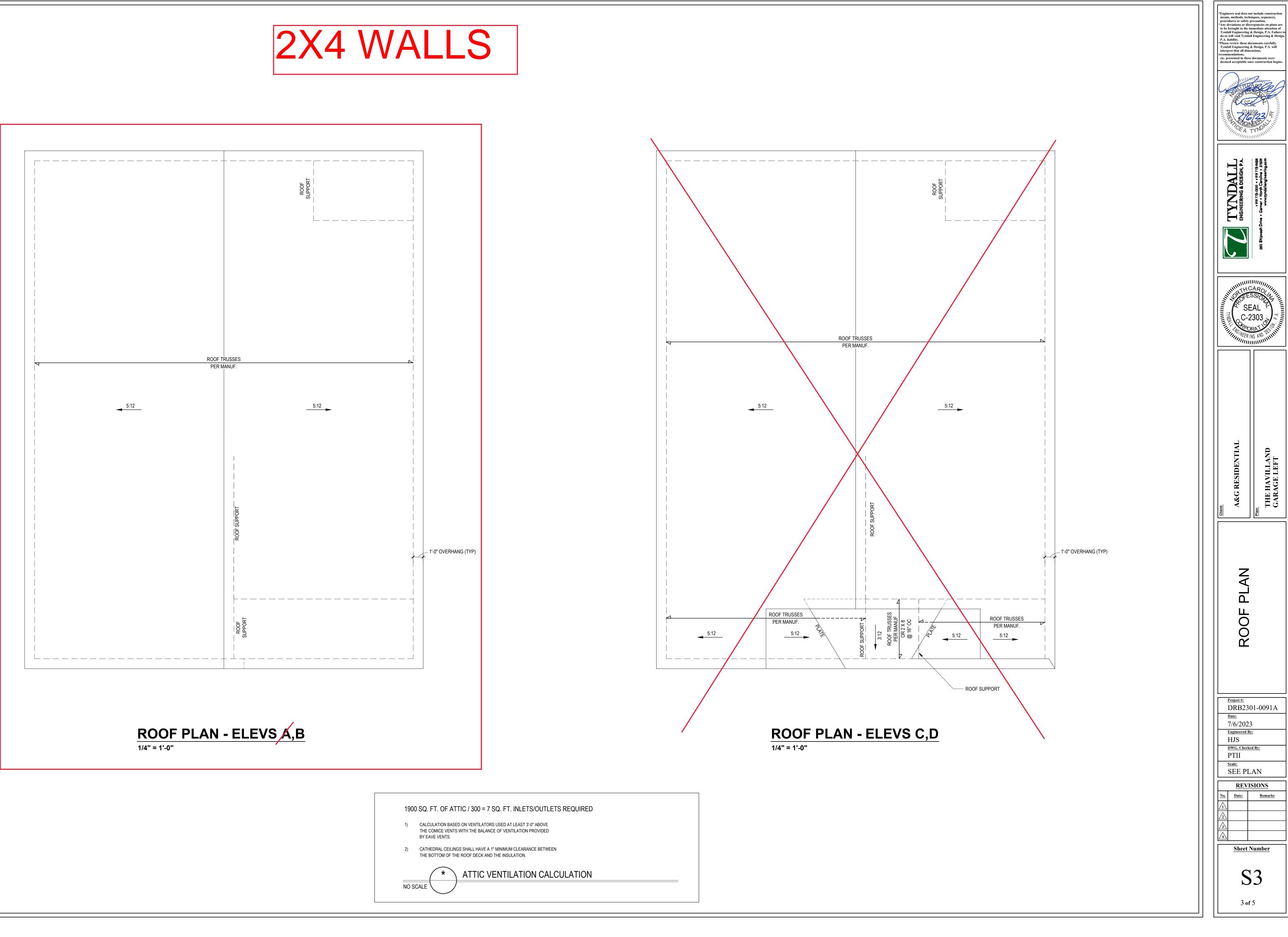
STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS
- 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC. 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3.
- REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- $\langle 1 \rangle$ REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- $\langle 2 \rangle$ 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON / NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION
- R602.10.3 (UNO) 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT
- PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS. MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS: - 24" ADJACENT TO OPENINGS NOT MORE THAN
 - 67% OF WALL HEIGHT
 - 30" ADJACENT TO OPENINGS GREATER THAN
 - 67% AND LESS THAN 85% OF WALL HEIGHT. - 48" FOR OPENINGS GREATER THAN 85% OF
- WALL HEIGHT $\langle 4 \rangle$ SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.

(5) MINIMUM 800# HOLD-DOWN DEVICE







	IGN LOADS:			LIVE L (PS	-	LOAD SF)	DEFLEC	TION			ALT = ALTEF CANT = CANT CJ = CEILIN
		ATTIC (w/	FLOORS / walk up stairs) // down access)	40 30 20		0 0 0 0	LL L/360 L/360 L/240	TL L/240 L/240 L/180	-		CMU = CONC COL = COLU CONC = CONC CONT = CONT CT = COLL
		ATTIC	(no access) IAL BALCONY	10	1	5 0	L/240 L/240 L/360	L/180 L/240	-		DBL = DOUB DIA = DIAME
			ROOF DF TRUSS	20		0	L/240 L/240	L/180 L/180			DJ = DOUB DR = DOUB EA = EACH
		WIN	ND LOAD		BASEI	O ON 120 MPH (EX	POSURE B)				EE = EACH FJ = FLOOI
		SI	EISMIC		\$	SEISMIC ZONES A,	B & C				FND = FOUN FTG = FOOT GALV = GALV
MINI	IMUM ALLOWABLE S	OIL BEARING PRE	SSURE = 2000 PSF								HORIZ = HORIZ HT = HEIGH
UNL	ESS NOTED OTHER	WISE. (U.N.O.)	AY COMPRESSIVE ST								MANUF = MANU
BRA THIC	CING. REFER TO SE CKNESS, SOIL TYPE,	CTION R404 OF 20 AND UNBALANCE	018 NC BUILDING COE ED BACKFILL HEIGHT. (Fb = 800 PSI, BASED	DE FOR BACKFILI							
ALL ALL ALL	FRAMING LUMBER E LVL LUMBER TO BE LSL LUMBER TO BE	EXPOSED TO THE 1.75" WIDE NOMIN 3.5" WIDE NOMIN	ELEMENTS SHALL BE NAL EACH SINGLE ME AL EACH SINGLE MEN AL EACH SINGLE MEN	E TREATED MATE MBER AND Fb = 1 IBER AND Fb = 2	2600 PSI, E = 1.9M P 325 PSI, E = 1.6M PS	l (Ù.N.O.)					1) MAXIMUM HEIGHT (
ALL REQ	LOAD BEARING EXT QUIREMENTS FOR HE	ERIOR HEADERS	SHALL BE AT (2) 2x10 R INTERIOR AND EXT	. (U.N.O.) REFER ERIOR LOAD CO	TO TABLE R602.7(1 NDITIONS UNLESS) & (2) FOR JACK S SPECIFICALLY NO	TUD TED ON PLANS.				4 x 4
) ALL	REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS. ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50. ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.										6 x 6 ***
PRO LAG	STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2"Ø x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE									* THIS TABLE IS BAS MAXIMUM T WHICH MAY ** FROM TOP OF FOO	
)) PRO THE EXTE	SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C. PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE.									*** DECKS WITH POST SEALED BY 2) DECKS SHALL BE E THESE METHODS:	
			OR BOLTS PER PLATE G OR WATERPROOFII		1 405 AND 406 OF N	C BUILDING CODE					A. THE DECK FLOOR
2) WAL	L AND ROOF CLADE	DING VALUES:									ATTACHED ABOVE. LAT B. 4 x 4 WOOD KNEE B
ROO 39.0 36.0 18.0		SITIVE AND NEGA OF PITCHES 0/12 T OF PITCHES 1.5/12 OF PITCHES 6/12 T	TO 6/12	,	LBS/SQFT) OR GRE/	ATER POSITIVE AN	D NEGATIVE PR	ESSURE.			BOTH DIREC AT A POINT TOP OF THE 45° AND 60° TO THE POS
			4/12, BUILDER TO IN	STALL 2 LAYERS	OF 15# FELT PAPEF	R.					BOLT AT EA C. FOR FREESTANDIN BRACING. L
) REFI	ER TO SECTION R60	2.3 FOR FRAMING	G OF ALL WALLS OVER	R 10'-0" IN HEIGH	Т.						POSTS IN A
,			SECTION 602.10.3 OF								POST SIZE
/			L BE CONTINUOUSLY			RIA					4 x 4
,			NHEIGHT OF 9'-0" (U.N								6 x 6
) PRO	VIDE A MINIMUM OF	500# UPLIFT & LA	TERAL CONNECTION	I AT TOP AND BC	TTOM OF PORCH C	OLUMNS. (U.N.O.)					D. 2 x 6 DIAGONAL VE
)) MAX	(IMUM MASONRY PE	IR HEIGHT SHALL	NOT EXCEED FOUR	TIMES ITS LEAST	HORIZONTAL DIME	NSION.					(2) PERPEN TO THE STR
, -			Y TO VERIFY ALL DIM NOT RESPONSIBLE F					N BEGINS.			THE 2 x 6s 5 DIPPED GA E. FOR EMBEDMENT
	FENESTRATION	SKYLIGHT ^b	GLAZED FENESTRATION		WOOD FRAMED WALL	MASS	FLOOR	BASEMENT ^{C, C} WALL	SLAB ^d R-VALUE	CRAWL SPACE [°] WALL]
ONES 3	0.35	U-FACTOR 0.55	0.30	R-VALUE <u>38 or 30</u>	R-VALUE <u>15</u> or 13 + 2.5 ^h	R-VALUE <u>5/13 or</u> 5/10 cont	R-VALUE 19	R-VALUE <u>5/13</u> f	AND DEPTH 0	R-VALUE 5/13	STRUCTURAL SHEATHIN
	0.35	0.55	0.30	<u>cont</u> 38 or 30	15 or	<u>5/13 or</u>	19	10/15	10	10/15	
4		0.55	<u>0.30</u> NR	cont 3 38 or 30	13 + 2.5 " <u>19, or 13 + 5</u>	<u>5/10 cont</u> 13/17 <u>or</u>	30 ^g	10/15	10	10/19	
4	0.35		111.1	cont j	or 15 + 3	13/12.5 cont					

