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in addition to all local codes and regulations. 3. Should these plans require structural calculations for permitting the contractor shall be required to obtain the

services of a structural engineer after notifying DRB DESIGN that such services are required.

4. Release of these plans requires further cooperation among the owner, his/her contractor, and DRB DESIGN.

5. Design and construction are complex and, although the designer performed his services with due care and

diligence, perfection is not a guarantee.

6. Communication is imperfect and every contingency cannot be anticipated. 7. Any ambiguity or discrepancy discovered by the use of these plans shall be reported immediately to DRB

DESIGN. Failure to notify the DRB DESIGN compounds misunderstandings and increases construction costs.

8. A failure to cooperate by a simple notice to DRB DESIGN shall relieve the designer from any and all

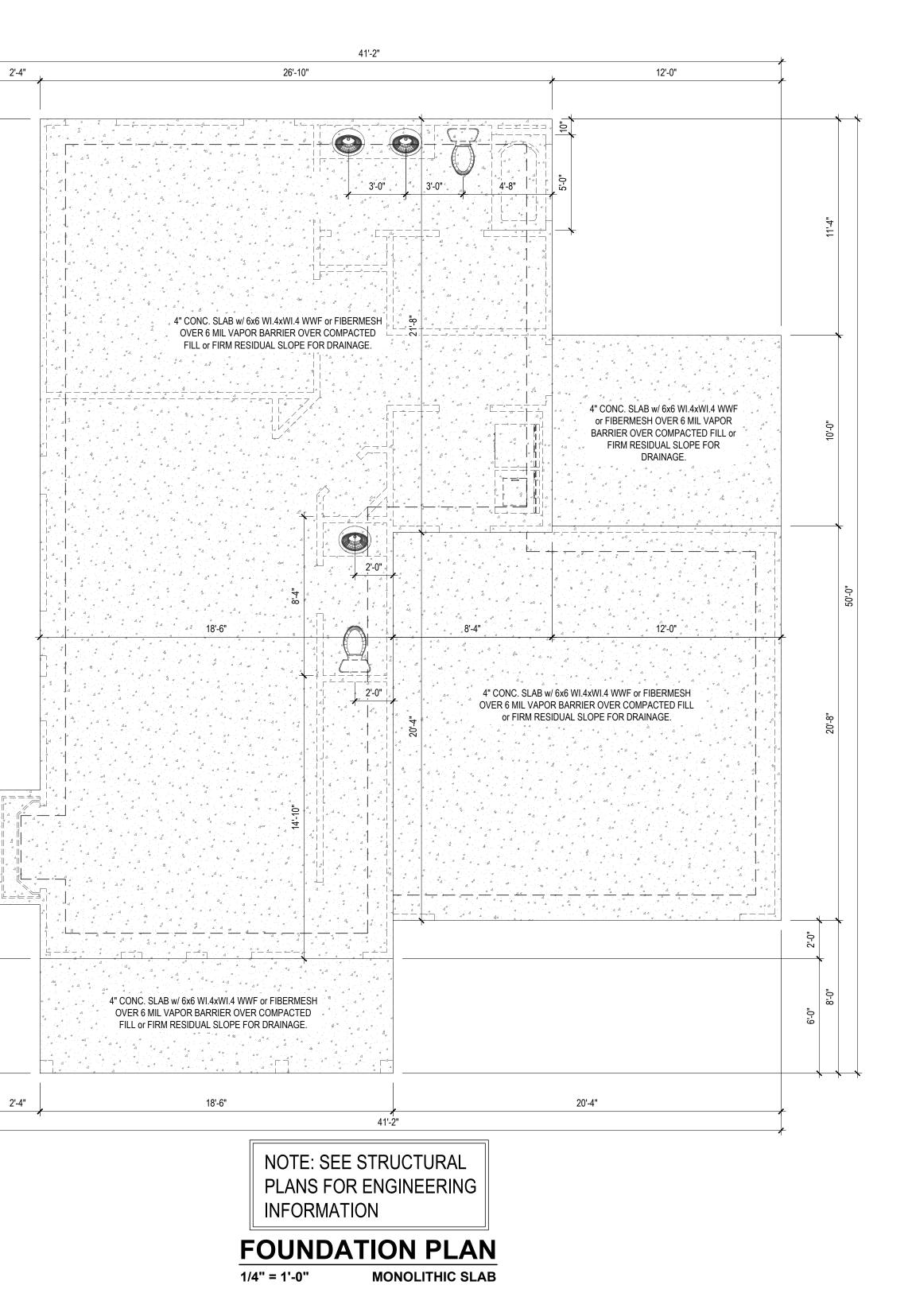
responsibilities for all consequences.

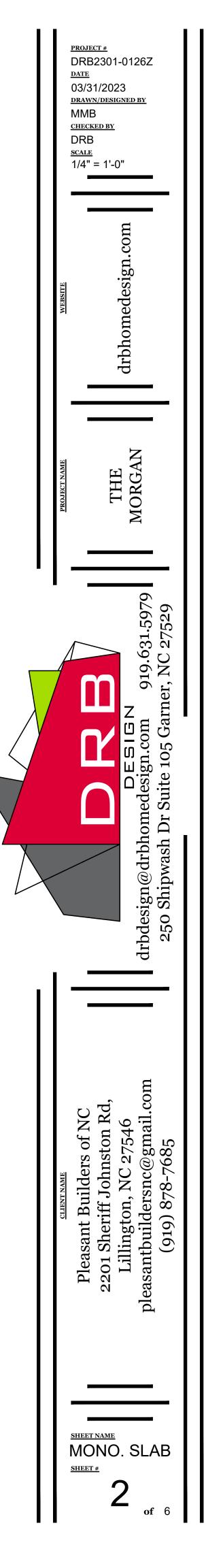
9. Changes made to these plans without the consent of the designer are unauthorized and shall relieve DRB

DESIGN of responsibility for any and all consequences arriving out of such changes. 10. Written dimensions on these plans always have precedence over scaled dimensions.

11. It is the contractors responsibility to verify and be responsible for all dimensions and square footage prior to construction, as well as conditions on the job site. DRB DESIGN is not responsible for dimension and square footage errors once construction has begun.

12. DRB DESIGN must be notified of any variations from the dimensions and conditions shown on these drawings.





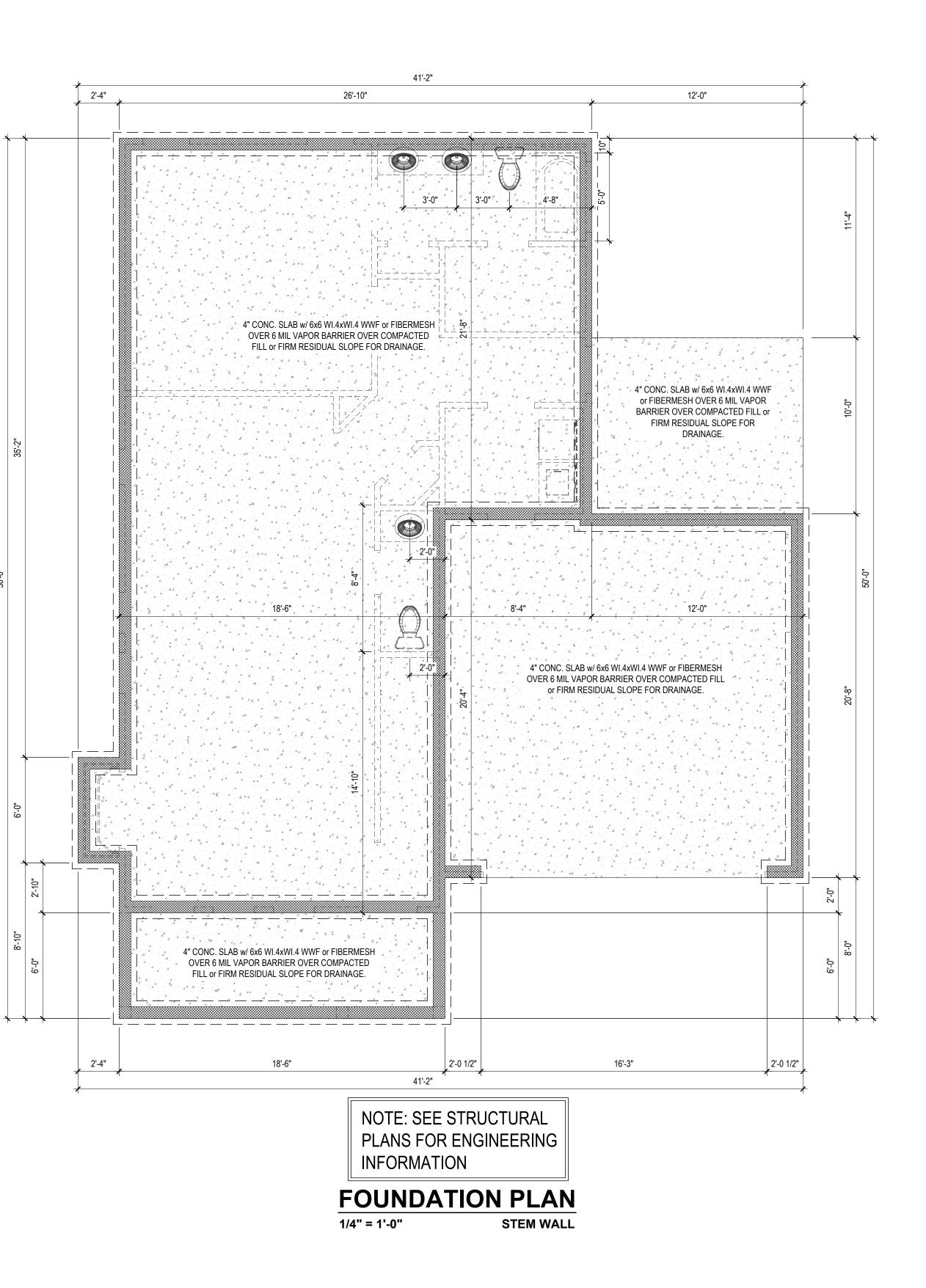
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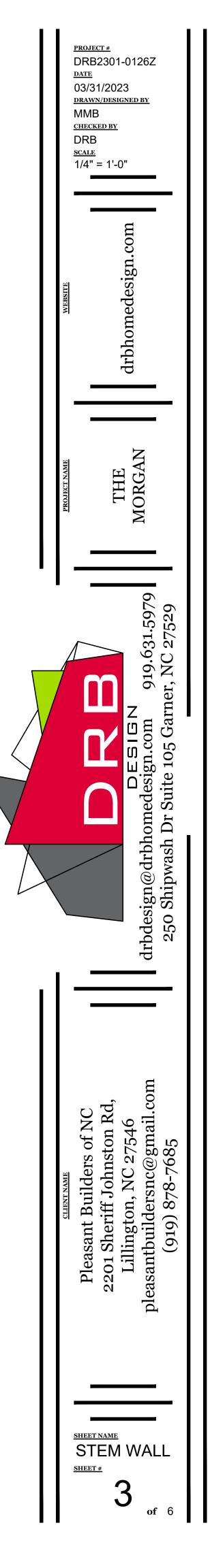
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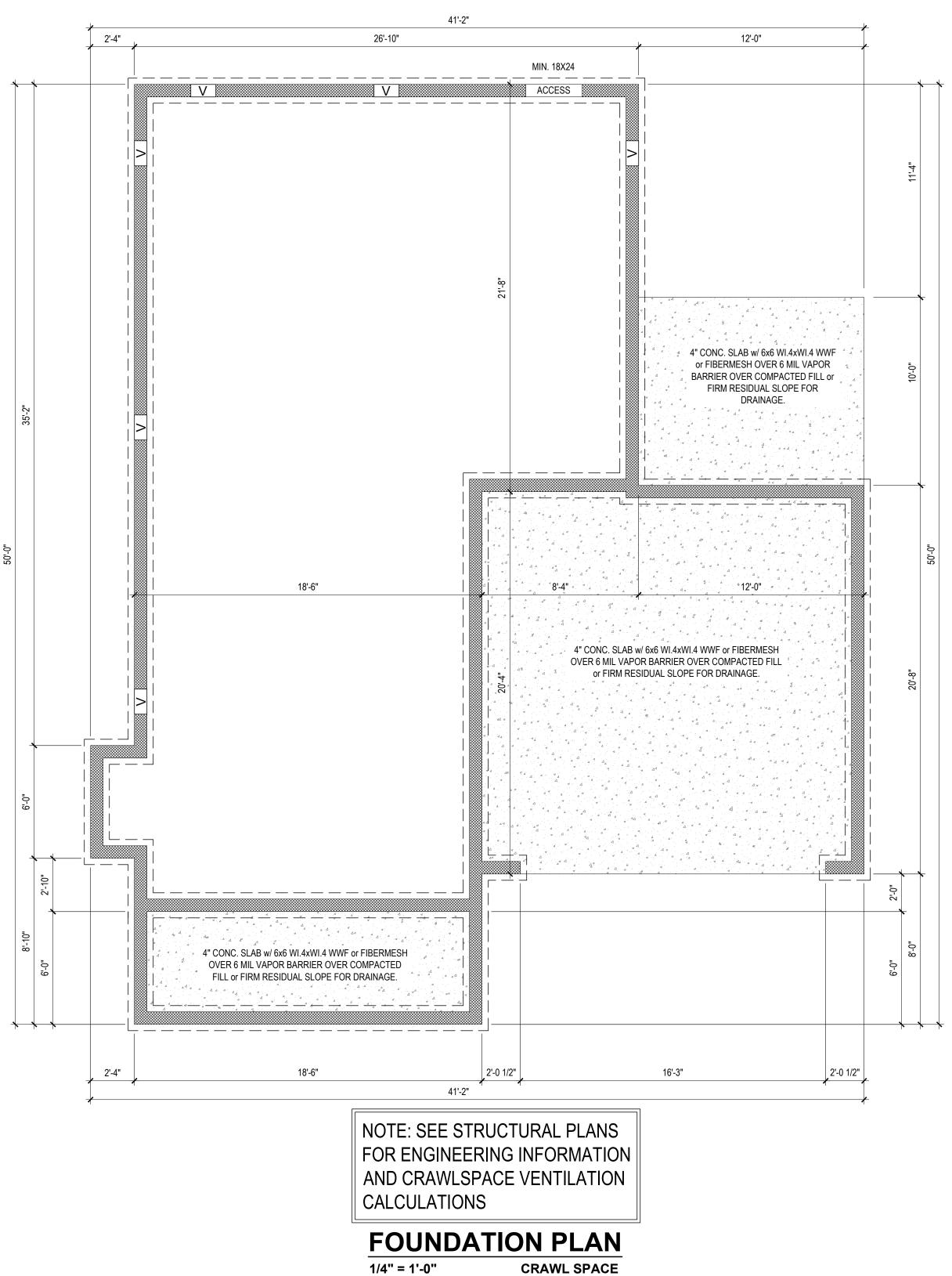
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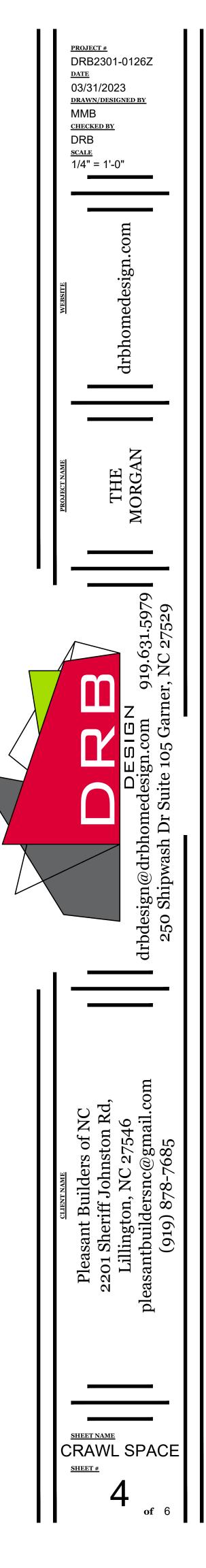
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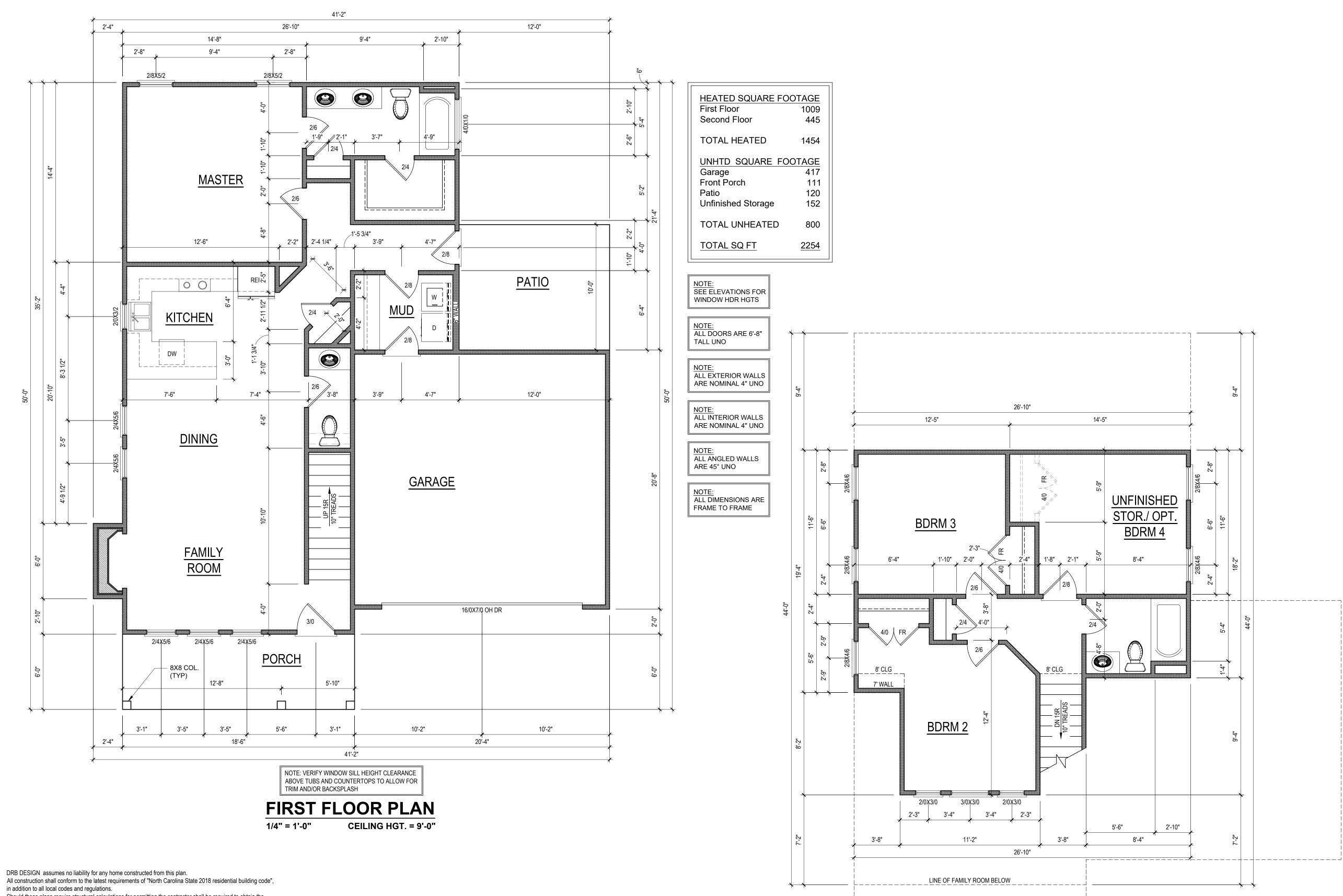
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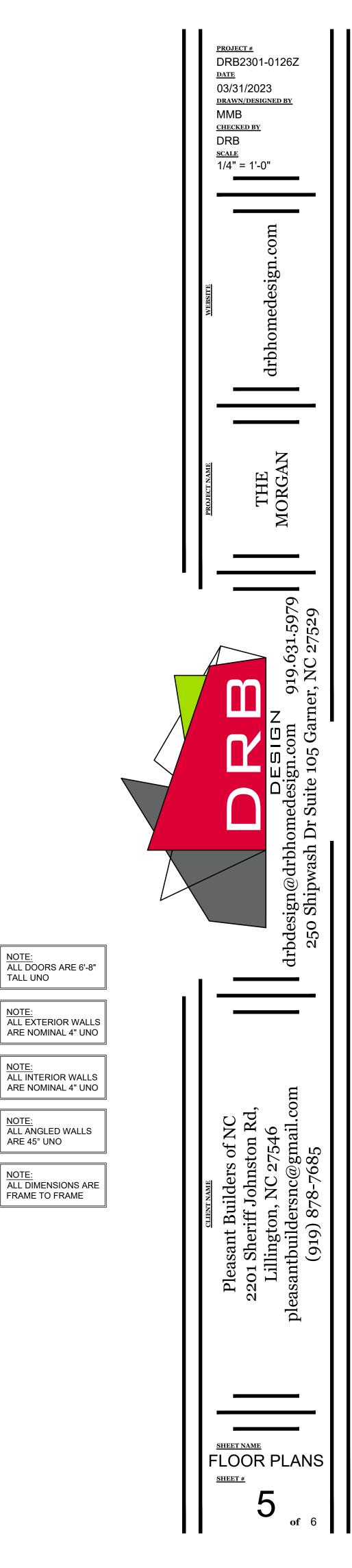
CRAWL SPACE





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NOTE: VERIFY WINDOW SILL HEIGHT CLEARANCE ABOVE TUBS AND COUNTERTOPS TO ALLOW FOR TRIM AND/OR BACKSPLASH **SECOND FLOOR PLAN** CEILING HGT. = 8'-0" 1/4" = 1'-0"



TALL UNO

NOTE:

NOTE:

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ARE 45° UNO

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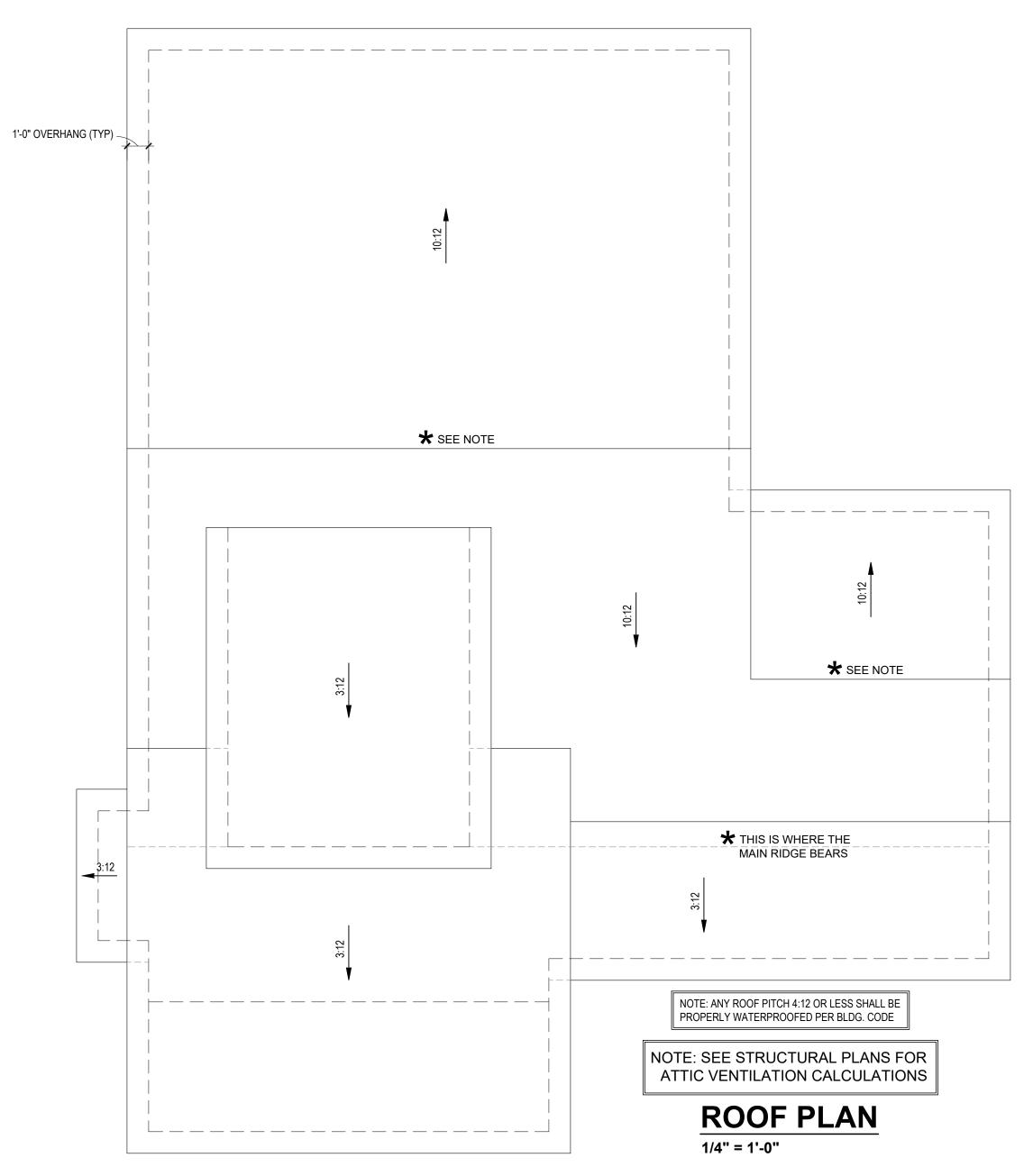
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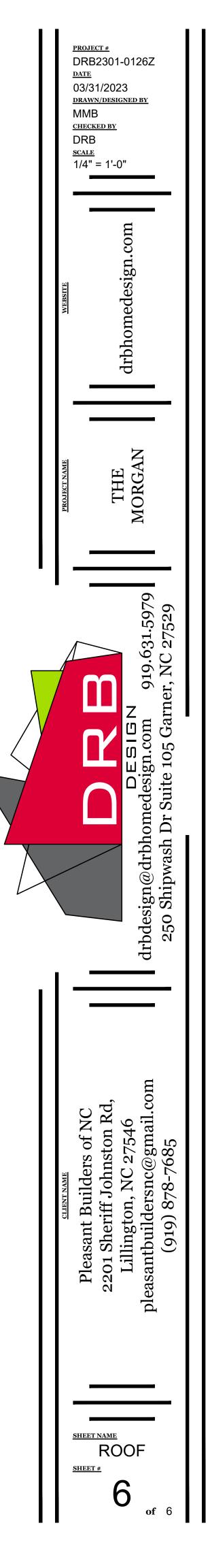
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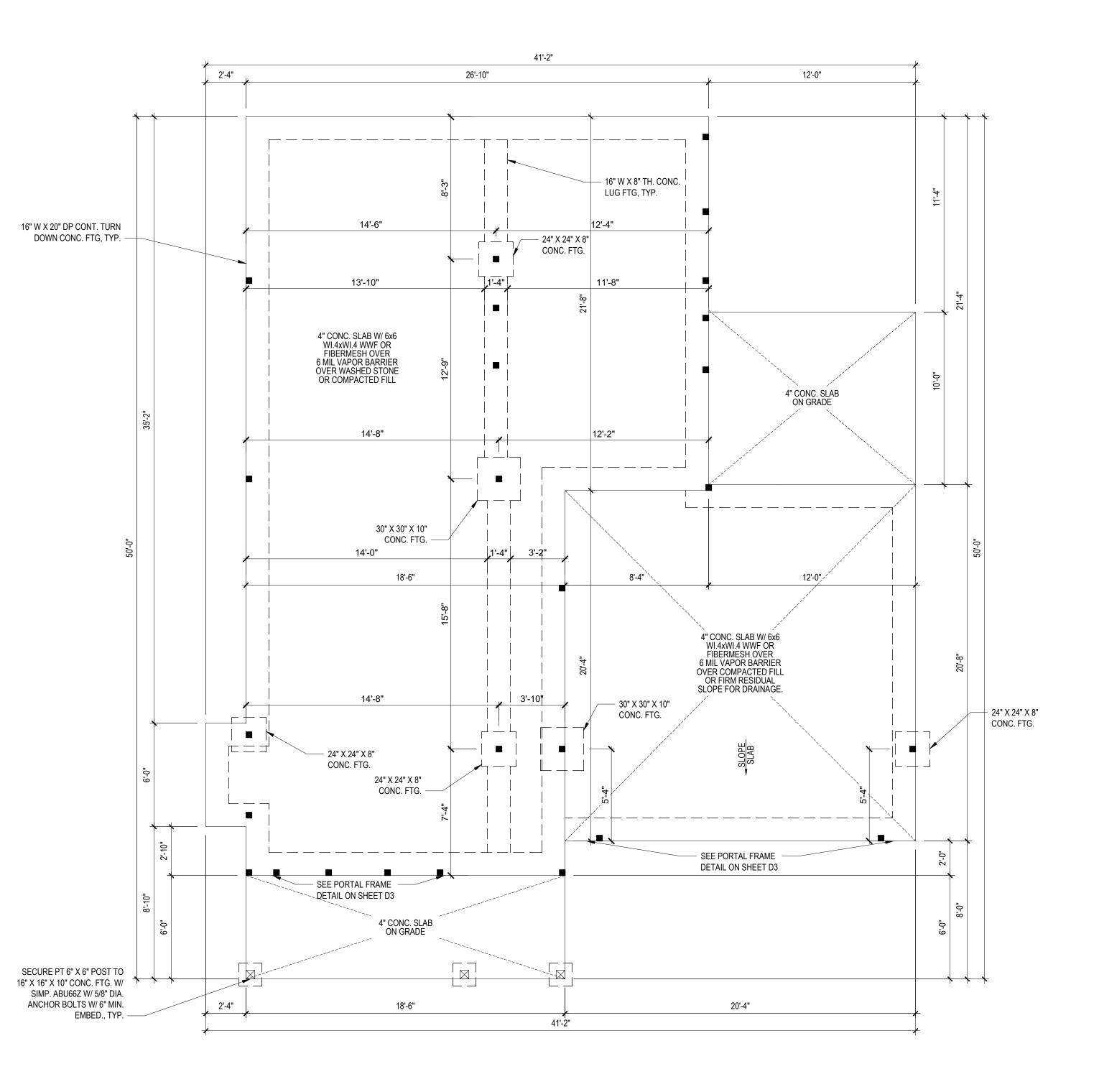




	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION	
	()	()	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BAS	SED ON 120 MPH (E	XPOSURE B)		
SEISMIC	BASED ON SEISMIC ZONES A, B & C				

- STRUCTURAL NOTES:

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- PSI, E = 1.9M PSI (I.E. iLEVEL MICROLAM) ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
- 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1)
- and R602.7(2).
 ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
 REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10-0" IN HEIGHT.
- 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, fc = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT
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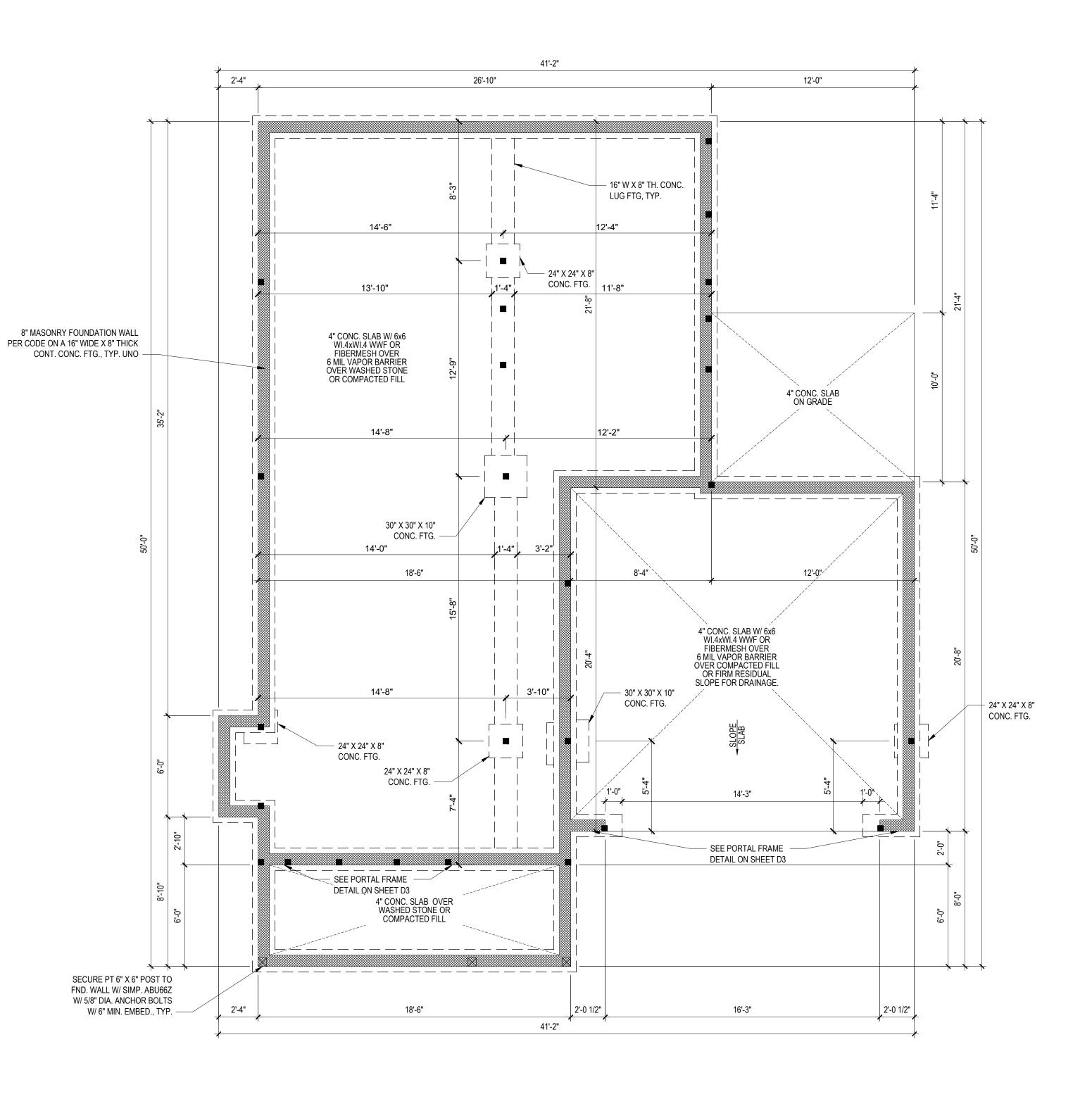
FOUNDATION PLAN

MONOLITHIC SLAB 1/4" = 1'-0"

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		250 Shipwash Crive = Garner = North Carolina = 278.29 Www.tyndallengineering.com
Client:	PLEASANT BUILDERS OF NC	Plan: THE MORGAN
	FOUNDATION PLAN	MONOSLAB OPT.
	Date: 4/20/20/ Engineered B EJM DWG. Check AWL SEE PL BEE PL Date: Date:	<u>y:</u> ed By:

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION		
	· · /	()	LL	TL		
FLOOR (primary)	40	10	L/360	L/240		
FLOOR (secondary)	40	10	L/360	L/240		
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FOUNDATION PLAN

1/4" = 1'-0"

STEM WALL

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TYNDALL ENGINEERING & DESIGN, P.A.	+ 919 778-1200 = # 919 778-9488 250 Shipwesh Orive = Gerner = North Ceroline = 27529 www.tyndellengineering.com
Client: PLEASANT BUILDERS OF NC	Plan: THE MORGAN
BASEMENT HEADER	STEMWALL OPT.
Date: 4/20/202 Engineered By EJM DWG. Checke AWL Scale: SEE PL <u>REVI</u> <u>No.</u> <u>Date:</u> <u>1</u> <u>2</u> <u>3</u> <u>4</u>	^{/:} <u>sd By:</u> AN <u>SIONS</u> <u>Remarks</u>
	Number B 9

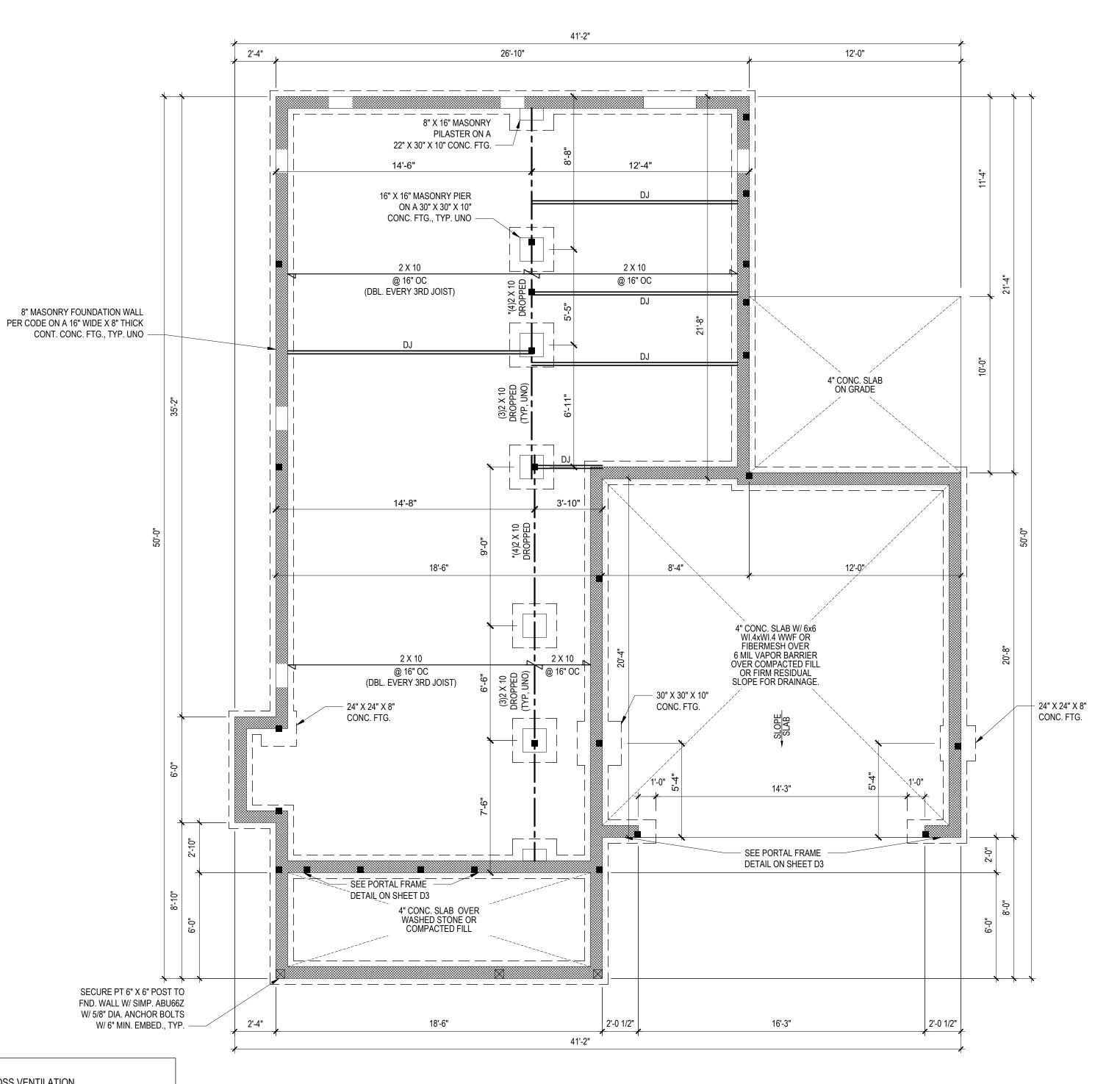


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WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BAS	ED ON SEISMIC ZO	NES A, B & C		

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*NOTE: SECURE 4-PLY W/ 1/2"Ø THRU-BOLTS @ 24" O.C. (OR EQUIV. STRUCTURAL SCREWS)



920 SQ. FT. OF CRAWL SPACE / 150 = 6.13 SQ. FT. OF REQ'D VENTILATION WITHOUT CROSS VENTILATION 6.13 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 7 VENTS REQ'D (BASED ON 8" X 16" VENTS) -OR-

920 SQ. FT. OF CRAWL SPACE / 1500 = 0.63 SQ. FT. OF REQ'D VENTILATION WITH CROSS VENTILATION 0.63 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 1 VENTS REQ'D (BASED ON 8" X 16" VENTS)2

- 1) VENT LOCATIONS MAY VARY FROM THOSE SHOWN ON PLAN, HOWEVER VENTS SHALL BE PLACED TO PROVIDE ADEQUATE VENTILATION AT ALL POINTS AND TO PREVENT DEAD AIR POCKETS.
- 2) THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 1/1500 OF THE CRAWL SPACE GROUND AREA WHERE THE REQUIRED OPENINGS ARE PLACED SO AS TO PROVIDE CROSS VENTILATION OF THE CRAWL SPACE. THE INSTALLATION OF OPERABLE LOUVERS SHALL NOT BE PROHIBITED. ONE FOUNDATION VENT SHALL BE WITHIN 3 FEET OF EACH CORNER OF THE BUILDING. TO PREVENT RAINWATER ENTRY WHEN THE CRAWL SPACE IS BUILT ON A SLOPED SITE, THE UPHILL FOUNDATION WALLS MAY BE CONSTRUCTED WITHOUT WALL VENT OPENINGS. VENT DAMS SHALL BE PROVIDED WHEN THE BOTTOM OF THE FOUNDATION VENT OPENING IS LESS THAN 4 INCHES ABOVE THE FINISHED EXTERIOR GRADE.

WALL VENTED CRAWL SPACES REQUIRE FULL COVERAGE GROUND VAPOR RETARDERS.

CRAWL SPACE VENTILATION CALCULATION

NO SCALE

LAST



1/4" = 1'-0"

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	do so will void Tyndal P.A. liability. *Please review these do Tyndall Engineering a interpret that all dime recommendations, etc. presented in these	niques, sequences, precaution. repancies on plans are nmediate attention of & Design, P.A. Failure to Il Engineering & Design, wouments carefully. & Design, P.A. will ensions, endocuments were ce construction begins.
	TYNDALL ENGINEERING & DESIGN, P.A.	+ 919 773-1200 = # 919 773-9458 250 Shipwash Drive = Garner = North Carolina = 27529 www.tyndallenginaering.com
	client: PLEASANT BUILDERS OF NC	Plan: THE MORGAN
	FOUNDATION PLAN	CRAWLSPACE OPT.
	Date: 4/20/202 Engineered By EJM DWG. Checke AWL Scale: SEE PL	v: ed By:
		Number LC 9

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BWL 1

BWL 2

2SC

BRACING PANEL LENGTHS REQUIRED: BWL A = 12.0 FT BWL B = 12.0 FT BWL 1 = 13.2 FT BWL 2 = 13.2 FT

BRACING PANEL LENGTHS PROVIDED: BWL A = 36.2 FT CS-WSP BWL B = 34.0 FT CS-WSP BWL 1 = 20.2 FT CS-WSP BWL 2 = 15.6 FT CS-WSP

*NOTE: SECURE 4-PLY W/ 1/2"Ø THRU-BOLTS @ 24" O.C. (OR EQUIV. STRUCTURAL SCREWS)

_____ _ _ _ _

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- 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
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- INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO) 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, fc = 3000 PSI MIN. PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM
- OF PORCH COLUMNS. (U.N.O.) 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC. MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST
- HORIZONTAL DIMENSION. 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE
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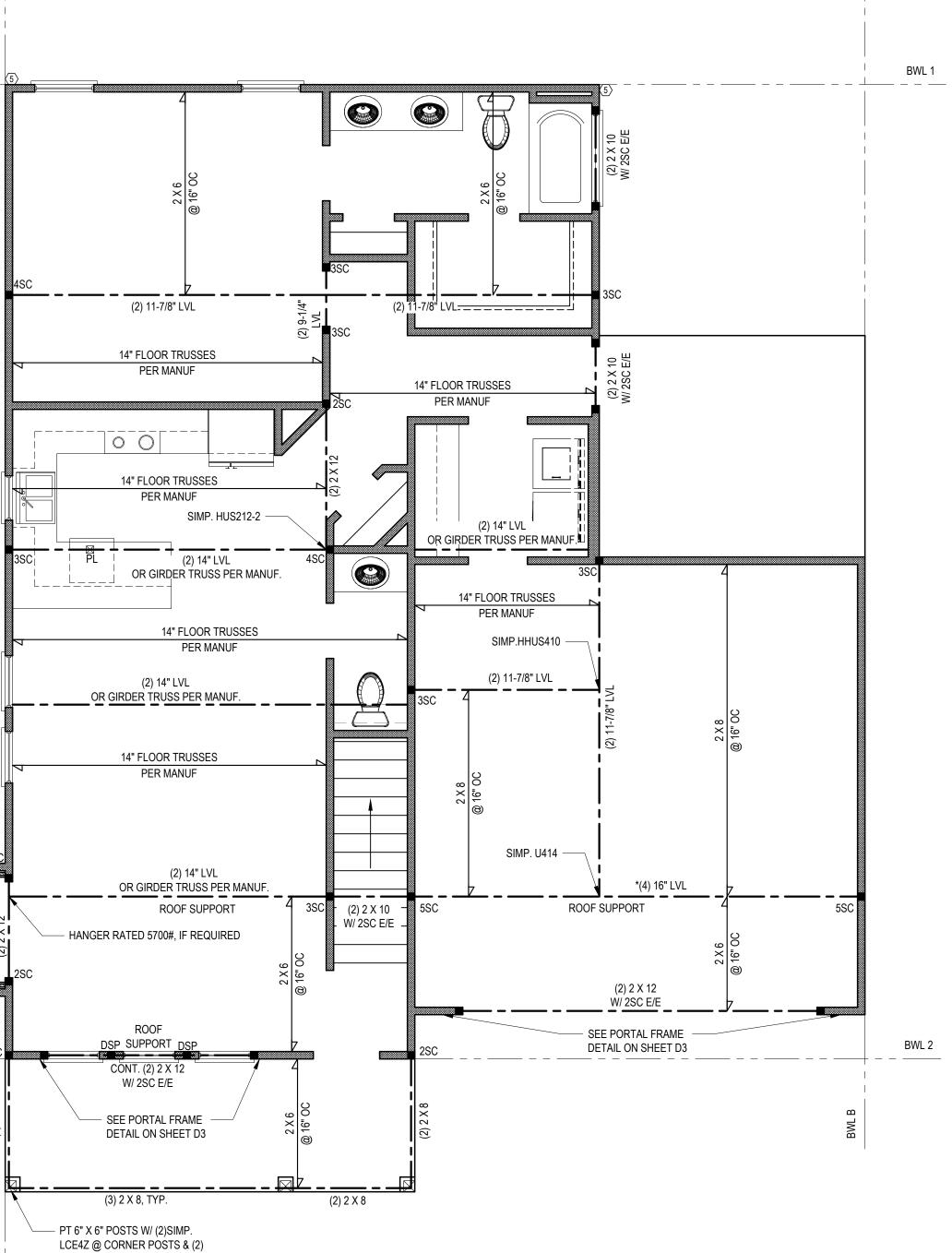
STRUCTURAL SHEATHING NOTES

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- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3.
 REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- $\langle 1 \rangle$ REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC. 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
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- 67% AND LESS THAN 85% OF WALL HEIGHT. - 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT $\langle 4 \rangle$ SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48° BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR

5 MINIMUM 800# HOLD-DOWN DEVICE

FRAMING BELOW.





SIMP. AC6Z @ INTERMEDIATE POSTS, TYP.



_		
/	means, methods, tech procedures or safety 1 *Any deviations or disk to be brought to the in Tyndall Engineering do so will void Tyndal P.A. liability. *Please review these do Tyndall Engineering interpret that all dim recommendations, etc. presented in these	precaution. crepancies on plans are mmediate attention of & Design, P.A. Failure to II Engineering & Design, ocuments carefully. & Design, P.A. will ensions, e documents were tee construction begins.
	TYNDALL ENGINEERING & DESIGN, P.A.	z50 Shipwash Oriva = (Samar = North Carolina = 27529 www.tyndallanginearing.com
	client: PLEASANT BUILDERS OF NC	Plan: THE MORGAN
	1ST FLOOR PLAN	2ND FLOOR HEADER
	Date: 4/20/20/2 Engineered B EJM DWG. Checke AWL Scale: SEE PL REVI <u>No.</u> Date: <u>1</u> <u>2</u> <u>3</u> <u>4</u>	<u>y:</u> ed By: JAN SIONS <u>Remarks</u>
		<u>Number</u> 2 9

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION		
	(* = * /	()	LL	TL		
FLOOR (primary)	40	10	L/360	L/240		
FLOOR (secondary)	40	10	L/360	L/240		
ATTIC (w/ storage)	20	10	L/240	L/180		
ATTIC (no access)	10	5	L/240	L/180		
EXTERNAL BALCONY	40	10	L/360	L/240		
ROOF	20	10	L/240	L/180		
ROOF TRUSS	20	20	L/240	L/180		
WIND LOAD	BAS	SED ON 120 MPH (E	XPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C					

- STRUCTURAL NOTES:

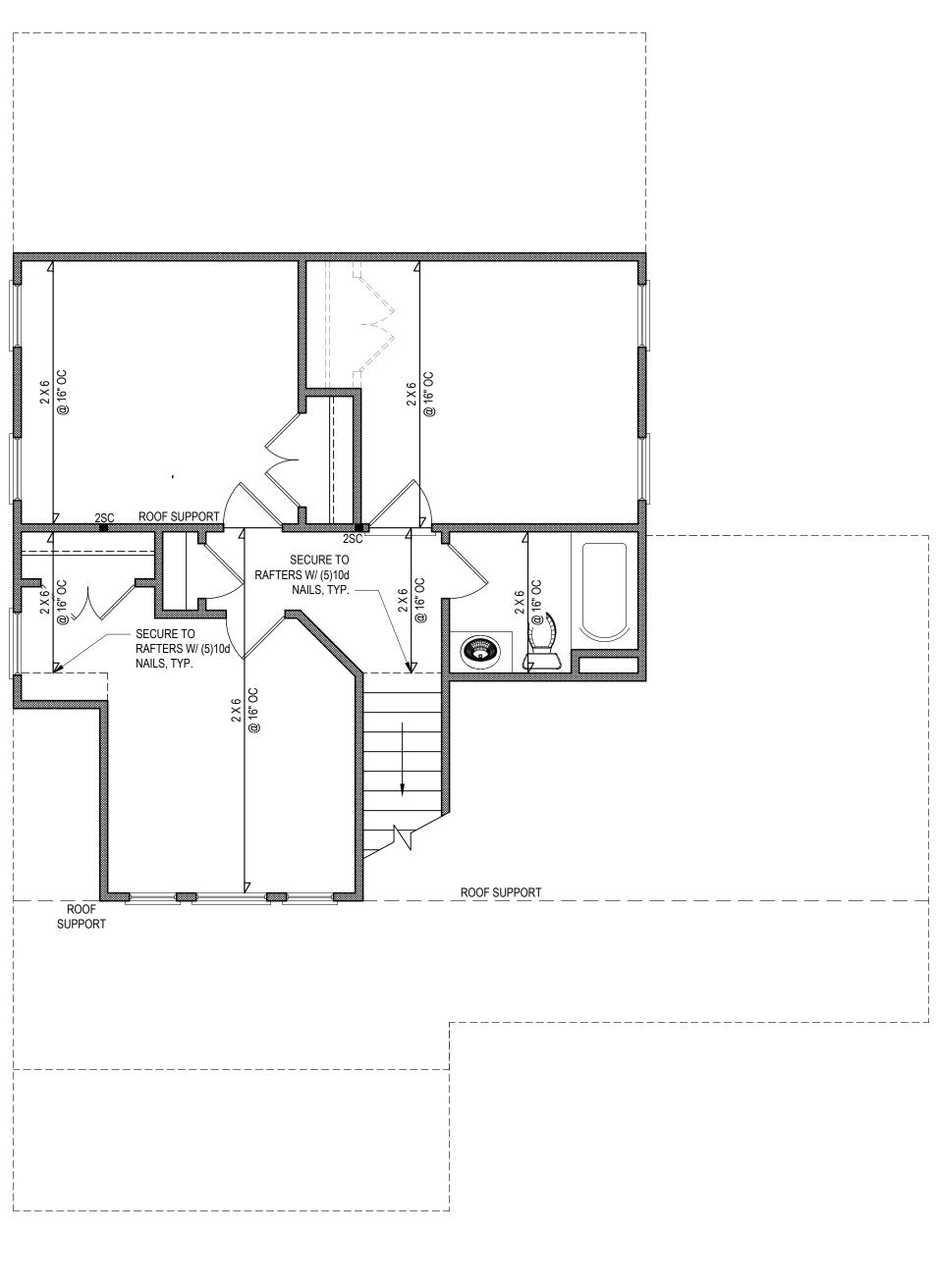
 1)
 ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL
 CODES AND REGULATIONS. 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE
- FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- 3) ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (I.E. iLEVEL MICROLAM)
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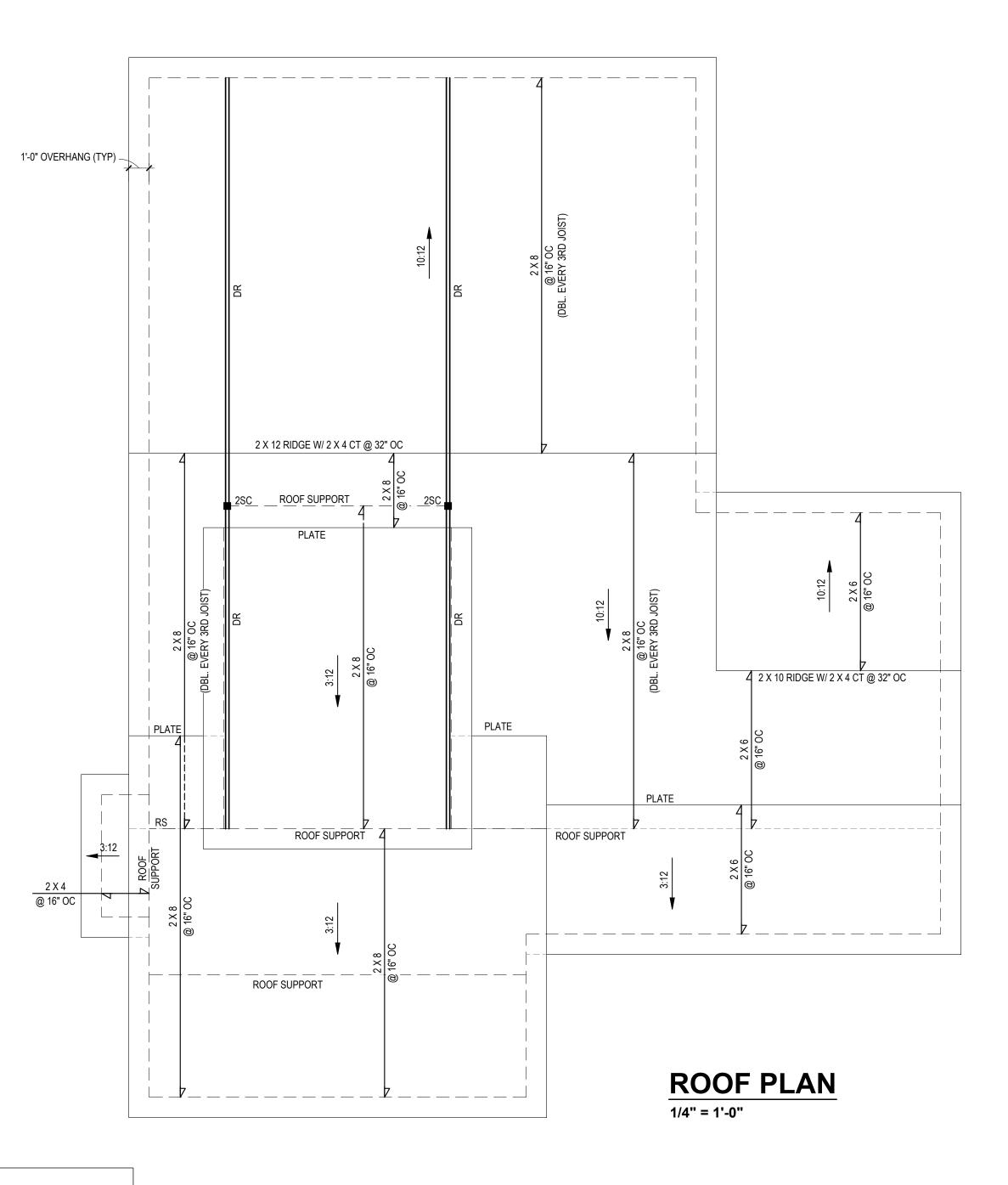
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5 MINIMUM 800# HOLD-DOWN DEVICE





do so will void Tyndal P.A. liability. *Please review these do Tyndall Engineering & interpret that all dime recommendations, etc. presented in these	tiques, sequences, recaution. repancies on plans are amediate attention of & Design, P.A. Failure to I Engineering & Design, cuments carefully. & Design, P.A. will nsions, documents were ce construction begins.
TYNDALL ENGINEERING & DESIGN, P.A.	+ 919 773-1200 = # 919 773-1200 = # 919 773-9645 250 Shipwesh Orive = Garner = North Garolina = 27529 www.tyndallanginaaring.com
cient: PLEASANT BUILDERS OF NC	Plan: THE MORGAN
Date: 4/20/202 Engineered By EJM DWG. Checke AWL Scale: SEE PL	<u>''</u> d By:
	Number 3



1530 SQ. FT. OF ATTIC / 300 = 6 SQ. FT. INLETS/OUTLETS REQUIRED

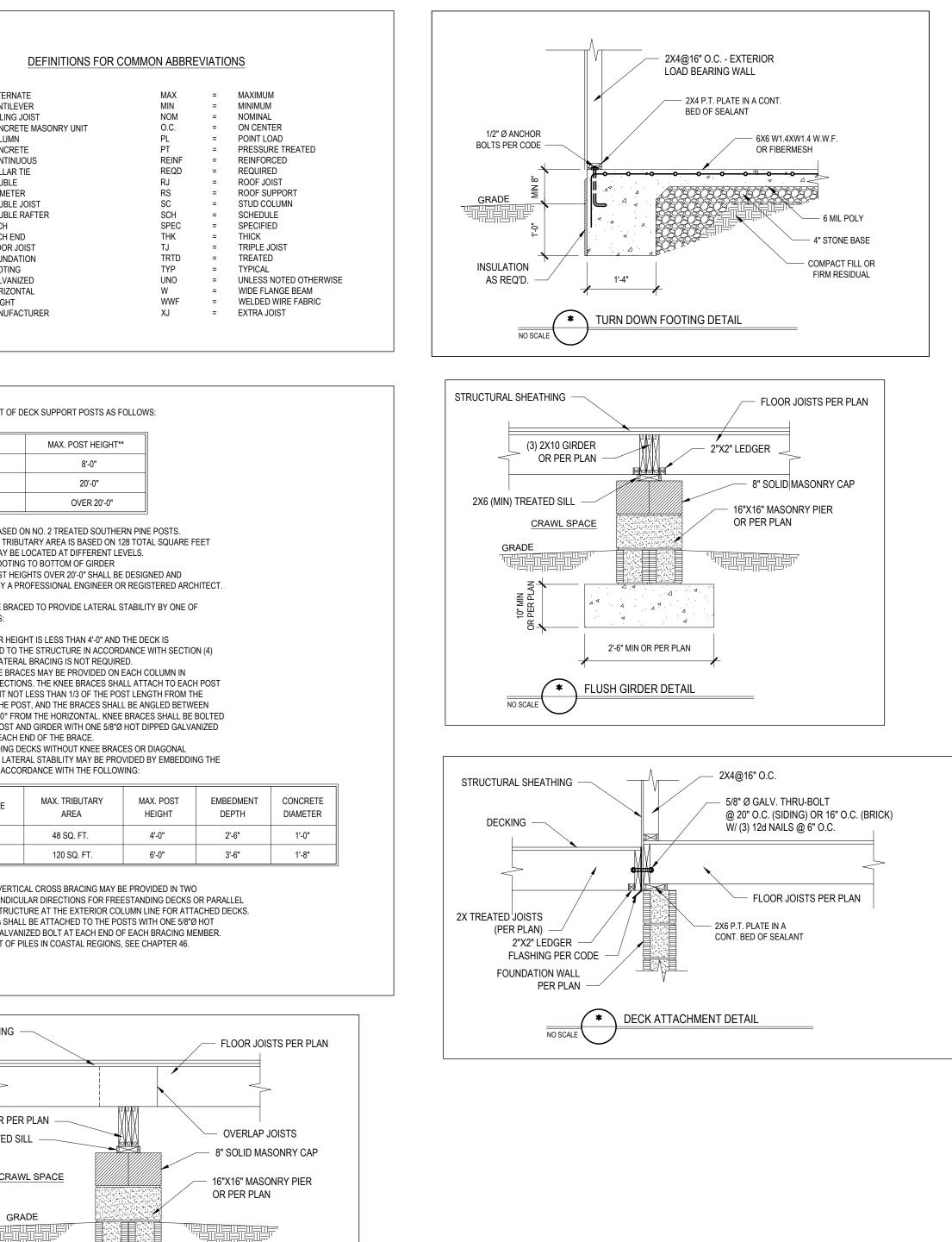
- CALCULATION BASED ON VENTILATORS USED AT LEAST 3'-0" ABOVE THE COMICE VENTS WITH THE BALANCE OF VENTILATION PROVIDED BY EAVE VENTS.
- 2) CATHEDRAL CEILINGS SHALL HAVE A 1" MINIMUM CLEARANCE BETWEEN THE BOTTOM OF THE ROOF DECK AND THE INSULATION.

NO SCALE *

ATTIC VENTILATION CALCULATION

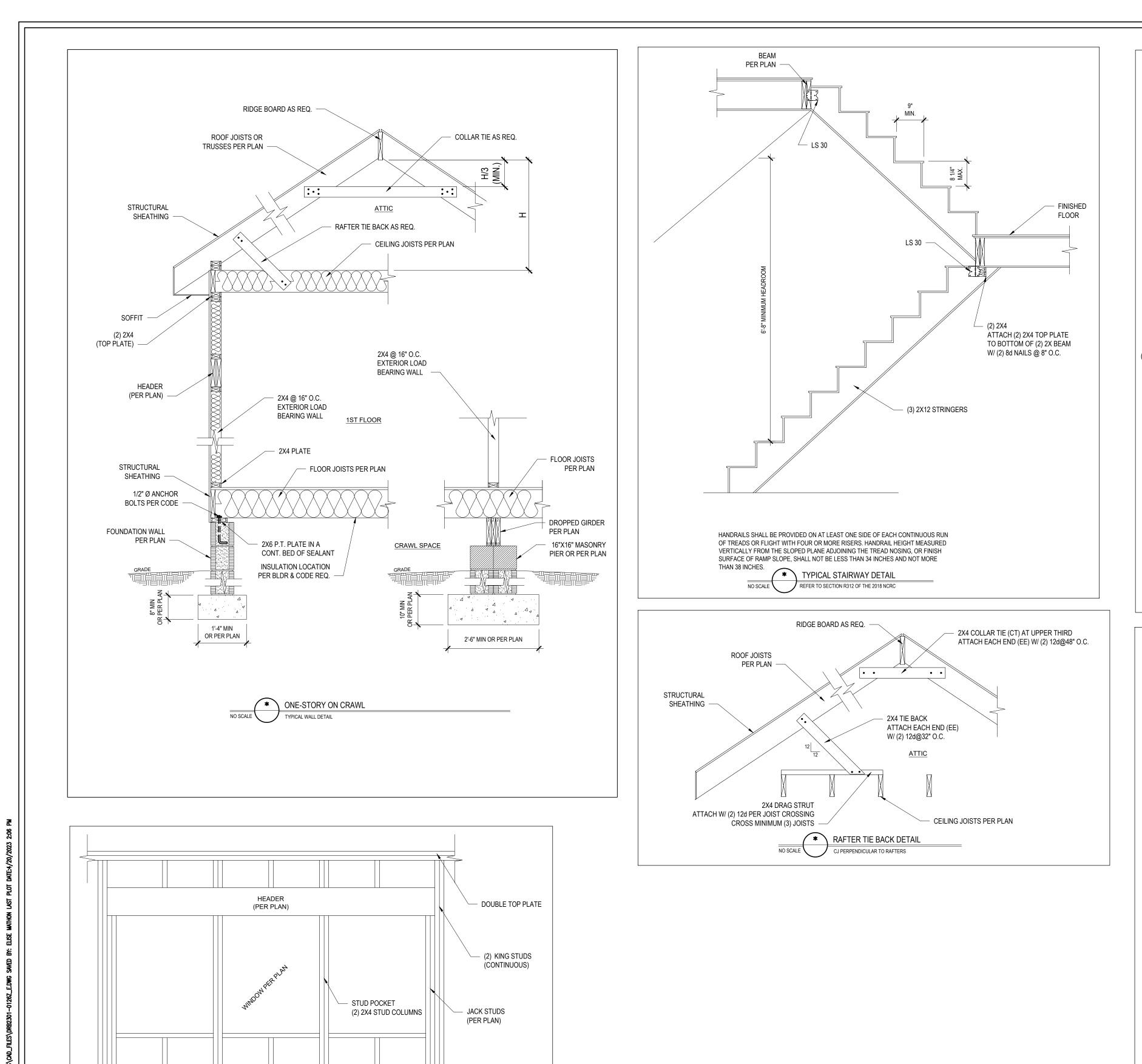
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	TYNDALL ENGINEERING & DESIGN, P.A.	+ 919 778-1200 = # 919 778-1200 = # 919 778-9668 250 Shipwesh Drive = Gerner = North Ceroline = 27529 www.tyndellengineering.com
	client: PLEASANT BUILDERS OF NC	Plan: THE MORGAN
	ROOF PLAN	
	Date: 4/20/202 Engineered By EJM DWG. Checke AWL Scale: SEE PL	<u>v:</u> ad By:

	ALL CONFORM TO THE LATEST REQU	JIREMENTS OF "NORTH CA	ROLINA STATE 2018 RESIDE	ENTIAL BUILDING						DEFINITIONS FOR COM
DESIGN LOADS:	ALL LOCAL CODES AND REGULATION								ALT	= ALTERNATE
		LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECT	TION	_			CANT CJ CMU	 CANTILEVER CEILING JOIST CONCRETE MASONRY UNIT
	ALL FLOORS ATTIC (w/ walk up stairs)	40 30	10 10	L/360 L/360	L/240 L/240				COL CONC CONT	= COLUMN = CONCRETE = CONTINUOUS
	ATTIC (pull down access) ATTIC (no access) EXTERNAL BALCONY	20 10 40	10 5 10	L/240 L/240 L/360	L/180 L/180 L/240	-			CT DBL DIA	= COLLAR TIE = DOUBLE = DIAMETER
	ROOF ROOF TRUSS	20 20	10 10 20	L/240 L/240	L/180 L/180	-			DJ DR EA	= DOUBLE JOIST = DOUBLE RAFTER = EACH
	WIND LOAD SEISMIC		BASED ON 120 MPH (EX SEISMIC ZONES A			_			EE FJ FND	= EACH END = FLOOR JOIST = FOUNDATION
	OIL BEARING PRESSURE = 2000 PSF								FTG GALV HORIZ	= FOOTING = GALVANIZED = HORIZONTAL
	A MINIMUM 28 DAY COMPRESSIVE S	STRENGTH OF 3000 PSI AND	D A MAXIMUM SLUMP OF FIV	/E INCHES					HT MANUF	= HEIGHT = MANUFACTURER
MAXIMUM DEPTH OF UN	NISE. (U.N.O.) BALANCED FILL AGAINST FOUNDATIO CTION R404 OF 2018 NC BUILDING CO									
ALL FRAMING LUMBER S ALL FRAMING LUMBER E ALL LVL LUMBER TO BE	AND UNBALANCED BACKFILL HEIGH SHALL BE SYP #2 (Fb = 800 PSI, BASEL EXPOSED TO THE ELEMENTS SHALL E 1.75" WIDE NOMINAL EACH SINGLE M	D ON 2x10) UNO. BE TREATED MATERIAL. IEMBER AND Fb = 2600 PSI,							1)	MAXIMUM HEIGHT OF DECK SUPPORT POSTS AS FOLLO
ALL PSL LUMBER TO BE	3.5" WIDE NOMINAL EACH SINGLE ME 3.5" WIDE NOMINAL EACH SINGLE ME ERIOR HEADERS SHALL BE AT (2) 2×1	EMBER AND Fb = 2400 PSI,	E = 1.8M PSI (U.N.O.)	חוודפ						POST SIZE MAX. POST HEIGHT**
REQUIREMENTS FOR HE	ERIOR HEADERS SHALL BE AT (2) 2x1 EADER SPANS FOR INTERIOR AND EX L W-SHAPES (I-BEAMS) SHALL BE AS	TERIOR LOAD CONDITIONS								4 x 4 8'-0" 6 x 6 20'-0"
	TES, AND C-CHANNELS SHALL BE AS									*** OVER 20'-0"
PROVIDE SOLID BEARIN	E SUPPORTED AT EACH END WITH A N G FROM BEAM SUPPORT TO FOUNDA LONG). LATERAL SUPPORT IS CONSI	ATION. BEAMS SHALL BE AT	TTACHED TO EACH SUPPOR	RT WITH TWO (2)						THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PI MAXIMUM TRIBUTARY AREA IS BASED ON 128 TO WHICH MAY BE LOCATED AT DIFFERENT LEVELS
SOLE PLATES, AND THE	LONG). LATERAL SUPPORT IS CONSI SOLE PLATES ARE NAILED OR BOLTI	ED TO THE BEAM FLANGES	S @ 48" O.C.							FROM TOP OF FOOTING TO BOTTOM OF GIRDER DECKS WITH POST HEIGHTS OVER 20'-0" SHALL BE DESI SEALED BY A PROFESSIONAL ENGINEER OR REC
THE END OF EACH PLAT EXTEND 7" INTO CONCR	PLACEMENT PER SECTION 403.1.6: 7 E SECTION. ANCHOR BOLTS SHALL E ETE OR MASONRY. THE BOLTS SHAL MUM TWO ANCHOR BOLTS PER PLAT	BE SPACED AT 3'-0" O.C. FO L BE LOCATED IN THE MIDI	R BASEMENTS. ANCHOR BC	OLT SHALL					· · ·	DECKS SHALL BE BRACED TO PROVIDE LATERAL STABI THESE METHODS:
	MUM TWO ANCHOR BOLTS PER PLAT		0 406 OF NC BUILDING CODE	Ξ.						THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE D ATTACHED TO THE STRUCTURE IN ACCORDANCI
	ING VALUES: BE DESIGNED FOR 28.0 POUNDS PEF SITIVE AND NEGATIVE SHALL BE AS I		T) OR GREATER POSITIVE A	ND NEGATIVE PRE	SSURE.				В.	ABOVE. LATERAL BRACING IS NOT REQUIRED. 4 x 4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH BOTH DIRECTIONS. THE KNEE BRACES SHALL AT
39.0 LBS/SQFT FOR ROC	OF PITCHES 0/12 TO 1.5/12 OF PITCHES 1.5/12 TO 6/12									AT A POINT NOT LESS THAN 1/3 OF THE POST LE TOP OF THE POST, AND THE BRACES SHALL BE / 45° AND 60° FROM THE HORIZONTAL. KNEE BRAC
**MEAN ROOF HEIGHT 3		NSTALL 2 LAYERS OF 15# F	ELT PAPER.						C.	TO THE POST AND GIRDER WITH ONE 5/8"Ø HOT BOLT AT EACH END OF THE BRACE. FOR FREESTANDING DECKS WITHOUT KNEE BRACES OI
	2.3 FOR FRAMING OF ALL WALLS OVI									BRACING, LATERAL STABILITY MAY BE PROVIDED POSTS IN ACCORDANCE WITH THE FOLLOWING:
	SHEATHING PER SECTION 602.10.3 O R THAN 500# SHALL BE CONTINUOUS		JNDATION							POST SIZE MAX. TRIBUTARY A AREA
	1 FOR PRESCRIPTIVE BUILDING ENV									4 x 4 48 SQ. FT.
	D WITH MAXIMUM HEIGHT OF 9'-0" (U 500# UPLIFT & LATERAL CONNECTIC	7)					D	6 x 6 120 SQ. FT.
MAXIMUM MASONRY PE										(2) PERPENDICULAR DIRECTIONS FOR FREESTAN
	& DESIGN, PA IS NOT RESPONSIBLE				N BEGINS.				E.	TO THE STRUCTURE AT THE EXTERIOR COLUMN THE 2 x 6s SHALL BE ATTACHED TO THE POSTS V DIPPED GALVANIZED BOLT AT EACH END OF EAC FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE
TYNDALL ENGINEERING		FOR DIMENSION OR SQUA	RE FOOTAGE ERRORS ONC		BEGINS. BASEMENT ^{c.Q} WALL R-VALUE	SLAB ^d R-VALUE AND DEPTH	CRAWL SPACE WALL R-VALUE			THE 2 x 6s SHALL BE ATTACHED TO THE POSTS V DIPPED GALVANIZED BOLT AT EACH END OF EAC FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE
TYNDALL ENGINEERING	& DESIGN, PA IS NOT RESPONSIBLE	FOR DIMENSION OR SQUA CEILING ^m R-VALUE R-VA <u>38 or 30</u> 15 <u>cont</u> 13 +	DOD MASS D WALL WALL ALUE R-VALUE 1 0 or 5/13 or 5/10 cont	E CONSTRUCTION	BASEMENT ^{C,Q} WALL	R-VALUE	WALL	C		THE 2 x 6s SHALL BE ATTACHED TO THE POSTS V DIPPED GALVANIZED BOLT AT EACH END OF EAC
TYNDALL ENGINEERING ATE FENESTRATION U-FACTOR 0.35 0.35	& DESIGN, PA IS NOT RESPONSIBLE SKYLIGHT b GLAZED FENESTRATION U-FACTOR B GLAZED FENESTRATION SHGC b.k	FOR DIMENSION OR SQUA CEILING ^m R-VALUE FRAME R-VALUE 13 + 38 or 30 15 cont 13 + 38 or 30 15 cont 13 +	ARE FOOTAGE ERRORS ONC DOD MASS D WALL WALL ALUE R-VALUE or 5/13 or 2.5 5/13 or 5/10 cont 5/10 cont	FLOOR R-VALUE 19 19	BASEMENT ^{C,Q} WALL R-VALUE	R-VALUE AND DEPTH	WALL R-VALUE	C		THE 2 x 6s SHALL BE ATTACHED TO THE POSTS V DIPPED GALVANIZED BOLT AT EACH END OF EAC FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE
TYNDALL ENGINEERING ATE FENESTRATION U-FACTOR b, j 0.35 0.35 0.35 0.35	& DESIGN, PA IS NOT RESPONSIBLE SKYLIGHT b GLAZED FENESTRATION 0.55 0.30 0.55 0.30 0.55 0.30 0.55 NR	FOR DIMENSION OR SQUA CEILING ^m R-VALUE R-VA <u>38 or 30</u> <u>15</u> <u>cont</u> 13 + <u>38 or 30</u> 15 <u>cont</u> 13 + <u>38 or 30</u> <u>15</u> <u>cont</u> <u>19, or</u> <u>or 19</u>	ARE FOOTAGE ERRORS ONC DOD MASS D WALL WALL ALUE R-VALUE or 5/13 or 2.5 5/13 or 5/10 cont 5/10 cont	FLOOR R-VALUE 19	BASEMENT ^{c.o} WALL R-VALUE <u>5/13</u> ^f	R-VALUE AND DEPTH 0	WALL R-VALUE 5/13		STRUCT	THE 2 x 6s SHALL BE ATTACHED TO THE POSTS V DIPPED GALVANIZED BOLT AT EACH END OF EAC FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE
TYNDALL ENGINEERING ATE FENESTRATION U-FACTOR b. j 0.35 0.35 0.35 0.35 0.35 0.35 0.35 0.35	& DESIGN, PA IS NOT RESPONSIBLE SKYLIGHT ^b GLAZED FENESTRATION U-FACTOR SHGC ^{b,k} 0.55 0.30 0.55 <u>0.30</u>	FOR DIMENSION OR SQUA CEILING ^m FRAME R-VALUE FRAME R-VALUE 15 <u>cont</u> 13+ 38 or 30 15 <u>cont</u> 13+ <u>38 or 30</u> 15 <u>cont</u> 13+ <u>38 or 30</u> 15 <u>cont</u> 19, or <u>or 19</u> , or <u>s. when insulation is installed in A</u>	RE FOOTAGE ERRORS ONC DOD MASS D WALL WALL ALUE R-VALUE ior $5/13 \text{ or}$ 2.5 $5/10 \text{ cont}$ ior $5/13 \text{ or}$ 2.5 $5/10 \text{ cont}$ $13+5$ h $13+5$ h $13/17 \text{ or}$ $13/12.5 \text{ cont}$	FLOOR R-VALUE 19 19 30 ⁹	BASEMENT ^{c,Q} WALL R-VALUE <u>5/13</u> ^f <u>10/15</u>	R-VALUE AND DEPTH 0 10	WALL R-VALUE 5/13 <u>10/15</u>		STRUCT (3) 2	THE 2 x 6s SHALL BE ATTACHED TO THE POSTS V DIPPED GALVANIZED BOLT AT EACH END OF EAC FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE
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2'-6" MIN OR PER PLAN

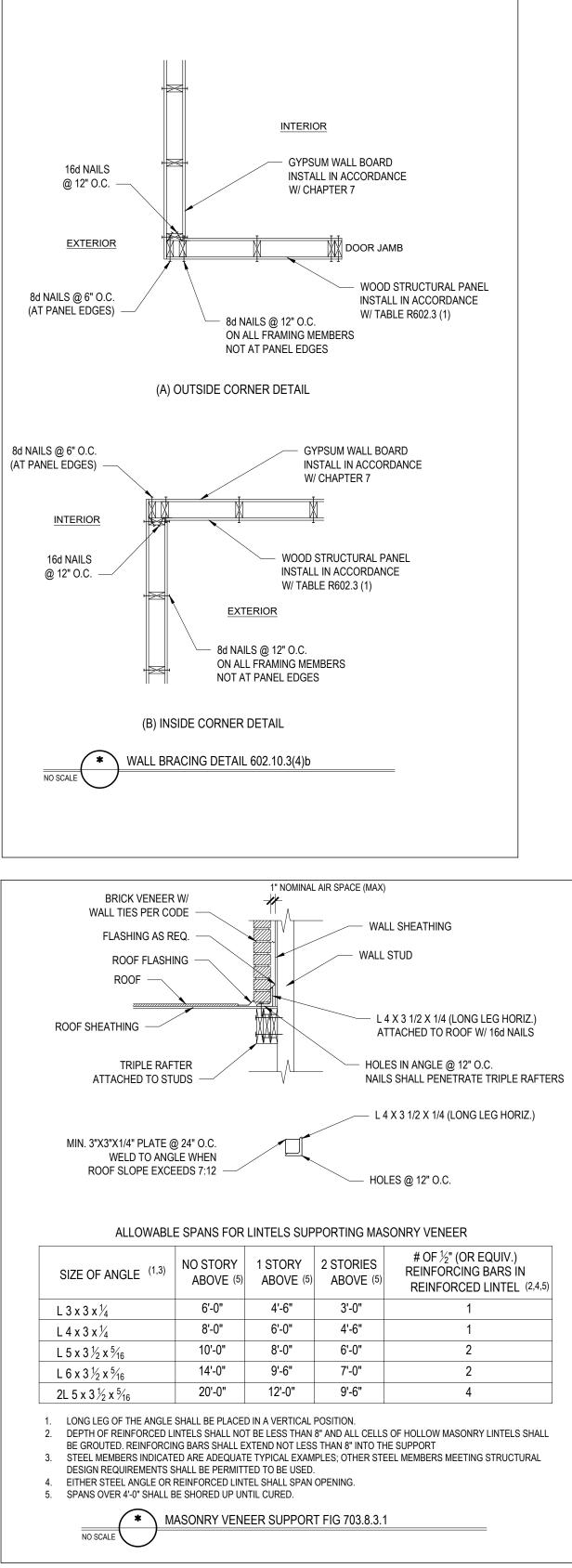
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	client: PLEASANT BUILDERS OF NC	Plan: THE MORGAN	
	Project #: DRB2301-0126Z Date: 4/20/2023 Engineered By: EJM DWG. Checked By: AWL SEE PLAN REVISIONS		
	$ \underline{No.} \underline{Date:} $ $ \underline{1} $ $ \underline{2} $ $ \underline{3} $ $ \underline{4} $	<u>Remarks</u>	
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- SOLE PLATE

***** TYPICAL WINDOW STUD POCKET DETAIL

NO SCALE



means procee * Any do to be t Tynda do so 0 P.A. li *Please Tynda interp recomm etc. pr deeme	*Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution. *Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure t do so will void Tyndall Engineering & Design P.A. liability. *Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.		
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Client:	PLEASANT BUILDERS OF NC	Plan: THE MORGAN	
	STANDARD DETAILS		
Project #: DRB2301-0126Z Date: 4/20/2023 Engineered By: EJM DWG. Checked By: AWL Scale: SEE PLAN <u>REVISIONS</u> No. Date: Remarks AWL Scale: SEE PLAN <u>REVISIONS</u> No. Date: Remarks AWL Scale: SEE PLAN <u>REVISIONS</u> No. Date: Remarks AWL Sheet Number			

REINFORCED LINTEL (2,4,5)

