

Trenco 818 Soundside Rd Edenton, NC 27932

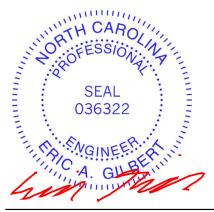
Re: J0823-4635 Lot 105 South Creek

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I60381511 thru I60381535

My license renewal date for the state of North Carolina is December 31, 2023.

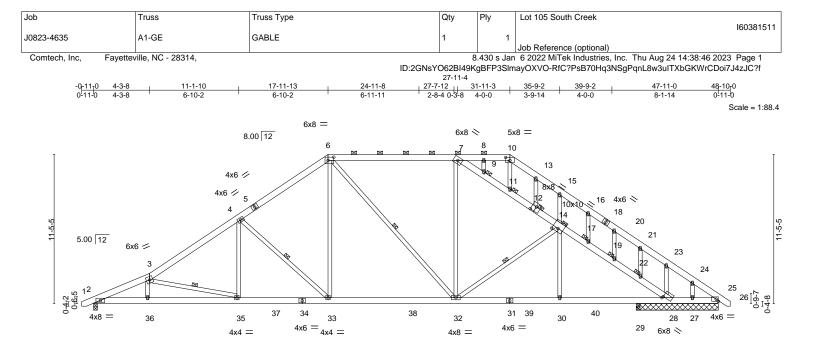
North Carolina COA: C-0844



August 24,2023

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



4	-3-8 6-10-2 6	-11-13 24-11-8 -10-2 6-11-11	2-8-4	<u>1-11-3 35-9-2</u> 4-3-8 3-9-14		<u>-7-8</u> 47-11-0 10-666-3-8	———————————————————————————————————————			
Plate Offsets (X,Y)	[6:0-5-4,0-3-0], [10:0-4-0,0-2-13], [12:0-	4-0,0-4-8], [14:0-5-0,0-3-0],	[28:0-0-9,0-1-10]							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.43 BC 0.56 WB 0.37 Matrix-S	Vert(LL) -0.20 Vert(CT) -0.33 Horz(CT) 0.10	0 32-33 >999 3 3 32-33 >999 3 0 25 n/a	L/d 360 240 n/a 240		GRIP 444/190 FT = 20%			
			Wind(EE) 0.11		210	Wolght: 127 lb	11 - 2070			
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x6 SF WEBS 2x4 SF OTHERS 2x4 SF	P No.1 P No.2		BRACING- TOP CHORD BOT CHORD WEBS	except 2-0-0 oc purlins (5- Rigid ceiling direct 8-5-0 oc bracing: 2 8-4-14 oc bracing: 1 Row at midpt	-1-1 max.): 6-10, ly applied or 10-(2-36 35-36. 3-35, 4	0-0 oc bracing, Exo	•			
(Ib) - Max H Max U	JOINTS 1 Brace at Jt(s): 9, 11, 12, 17, 19, 22, 14 REACTIONS. All bearings 6-3-8 except (jt=length) 2=0-3-8, 29=0-3-8. (lb) - Max Horz 2=345(LC 11) Max Uplift All uplift 100 lb or less at joint(s) except 2=-330(LC 12), 27=-120(LC 2), 28=-665(LC 13) Max Grav All reactions 250 lb or less at joint(s) 27 except 2=1854(LC 2), 25=379(LC 22), 28=1675(LC 2), 29=352(LC 2)									
TOP CHORD 2-3= 8-10 20-2 9-11	. Comp./Max. Ten All forces 250 (lb) oi -4065/1008, 3-4=-3064/793, 4-6=-2391/ =-302/141, 10-13=-368/137, 13-15=-364 1=-409/0, 21-23=-475/0, 23-24=-441/23 =-1918/629, 11-12=-1876/616, 12-14=-1 22=-2304/828, 22-28=-2301/839	725, 6-7=-1885/665, 7-8=-3 //85, 15-16=-335/0, 16-20=-3 6, 24-25=-528/195, 7-9=-190	369/0, 08/626,							
BOT CHORD 2-36 29-3 WEBS 3-35	==866/3708, 35-36==874/3705, 33-35=-4 30=-317/2146, 28-29=-317/2146, 27-28= =-1244/431, 4-35=-41/554, 4-33=-944/3 8=-574/361, 14-30=0/309, 14-32=-490/2	-192/433, 25-27=-192/433 93, 6-33=-159/1047, 7-32=-3				TH CA	BOLIN			
10750						JESST	di Vi			
 NOTES- Ubalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. Provide adequate drainage to prevent water ponding. All plates are 2x4 MT20 unless otherwise indicated. Gable studs spaced at 2-0-0 oc. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a lou of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 										
9) Provide mechanical	l connection (by others) of truss to bearing		ding 330 lb uplift at joi	nt 2, 120 lb uplift at		August	24,2023			
Corjainted and ages 20	uplift at joint 28.									
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the										

is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPII Quality Criteria and DSE-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

	Job	Truss	Truss Type	Qty	Ply	Lot 105 South Creek
						160381511
	J0823-4635	A1-GE	GABLE	1	1	
						Job Reference (optional)
Comtech, Inc, Fayetteville, NC - 28314,					.430 s Jan	6 2022 MiTek Industries, Inc. Thu Aug 24 14:38:46 2023 Page 2

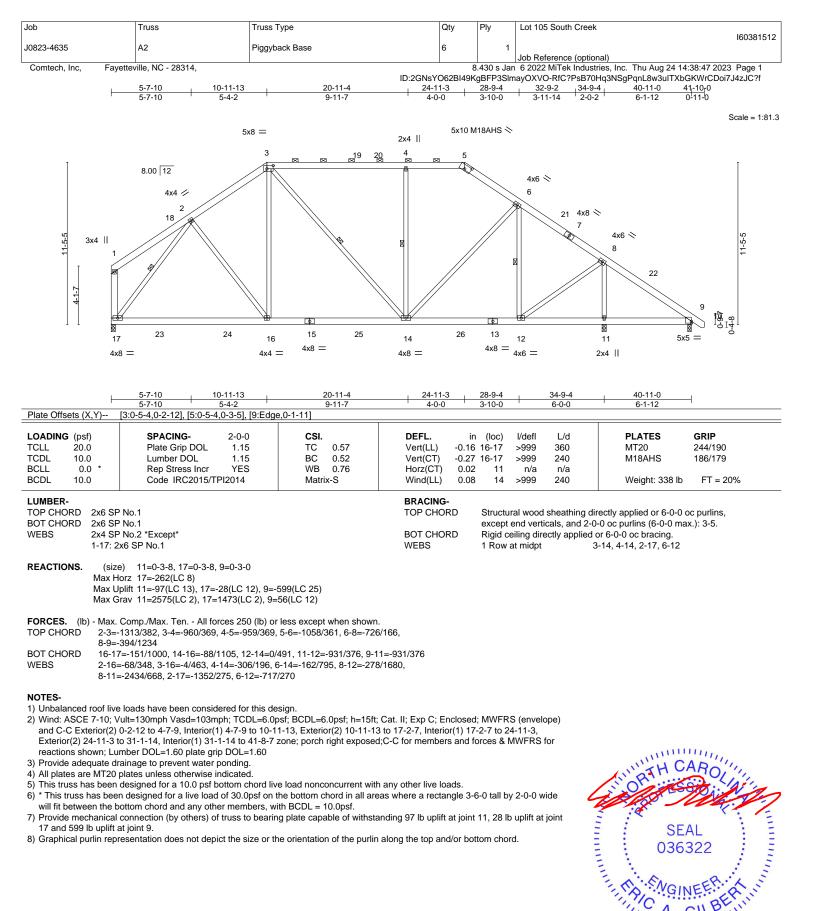
ID:2GNsYO62BI49KgBFP3SImayOXVO-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

NOTES-

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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A MITEK Affilia 818 Soundside Road

A. GILP.... August 24,2023

Edenton, NC 27932

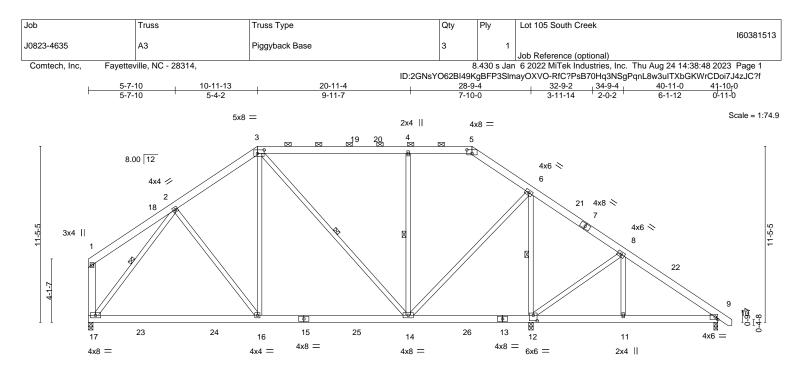


Plate Offsets (X,Y)	5-7-10 10-11-13 5-7-10 5-4-2 [3:0-5-4,0-2-12], [5:0-4-0,0-2-13], [12:0-13]	20-11-4 9-11-7 3-0.0-4-4]	<u>24-11-3</u> 4-0-0	28-9-4 34-9-4 3-10-1 6-0-0	40-11-0 6-1-12				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.45 BC 0.51 WB 0.68 Matrix-S	Vert(LL) -0.16 Vert(CT) -0.25 Horz(CT) 0.01	(loc) l/defi L/d 16-17 >999 360 16-17 >999 240 12 n/a n/a 14-16 >999 240	PLATES GRIP MT20 244/190 Weight: 338 lb FT = 20%				
LUMBER- BRACING- TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 *Except* 1-17: 2x6 SP No.1 BOT CHORD WEBS 1-17: 2x6 SP No.1									
Max U	ze) 12=0-3-8, 17=0-3-8, 9=0-3-0 Horz 17=-262(LC 8) Jplift 12=-178(LC 8), 17=-21(LC 12), 9=- Grav 12=2275(LC 2), 17=1187(LC 19), 9								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-967/282, 3-4=-440/222, 4-5=-439/223, 5-6=-531/216, 6-8=-216/641, 8-9=-132/439 BOT CHORD 16-17=-148/809, 14-16=-84/843, 12-14=-588/419, 11-12=-313/113, 9-11=-313/113 WEBS 3-16=-7/544, 3-14=-541/110, 4-14=-464/250, 6-14=-294/1359, 6-12=-1704/542, 2-17=-1001/179, 8-12=-549/464									
2) Wind: ASCE 7-10; '									

Exterior(2) 24-11-3 to 31-1-14, Interior(1) 31-1-14 to 41-8-7 zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 12, 21 lb uplift at joint 17 and 118 lb uplift at joint 9.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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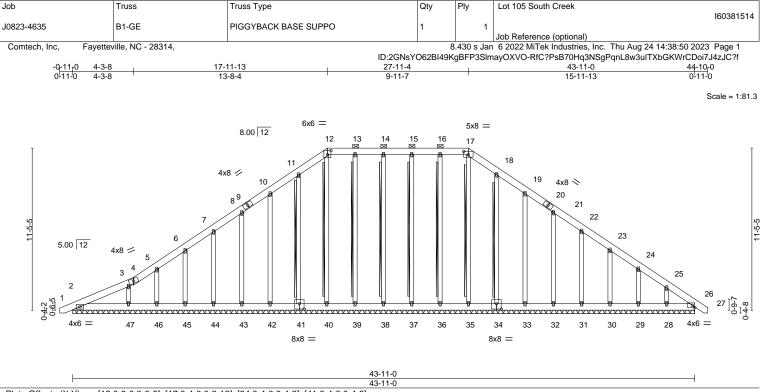


Plate Offsets (X,Y)	[12:0-3-0,0-3-8], [17:0-4-0,0-2-13], [34:0	0-4-0,0-4-8], [41:0-4-0,0-4-8	43-11-0 3]						
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL)	0.00	26	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.04	Vert(CT)	0.00	26	n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.18	Horz(CT)	0.01	26	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S						Weight: 428 lb	FT = 20%

LUMBER-		BRACING-		
TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood sheathing	directly applied or 6-0-0 oc purlins, except
BOT CHORD	2x6 SP No.1		2-0-0 oc purlins (6-0-0 ma:	x.): 12-17.
OTHERS	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applie	ed or 10-0-0 oc bracing.
		WEBS	T-Brace:	2x4 SPF No.2 - 17-35, 16-36, 15-37, 14-38
				, 13-39, 12-40, 11-41, 18-34
			Fasten (2X) T and I brace	es to narrow edge of web with 10d

REACTIONS. All bearings 43-11-0.

(lb) - Max Horz 2=345(LC 11)

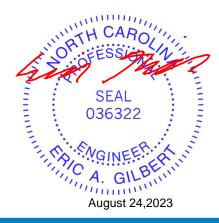
 Max Uplift
 All uplift 100 lb or less at joint(s) 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 34, 33, 32, 31, 30, 29, 26 except 2=-106(LC 8), 28=-122(LC 13)

 Max Grav
 All reactions 250 lb or less at joint(s) 2, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 34, 33, 32, 31, 30, 29, 28, 26 except 47=307(LC 1)

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-296/209, 10-11=-235/290, 11-12=-285/330, 12-13=-258/307, 13-14=-258/307, 14-15=-258/307, 15-16=-258/307, 16-17=-258/307, 17-18=-285/330, 18-19=-236/271

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
 - will fit between the bottom chord and any other members.
 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 34, 33, 32, 31, 30, 29, 26 except (jt=lb) 2=106, 28=122.
 - 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - 12) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

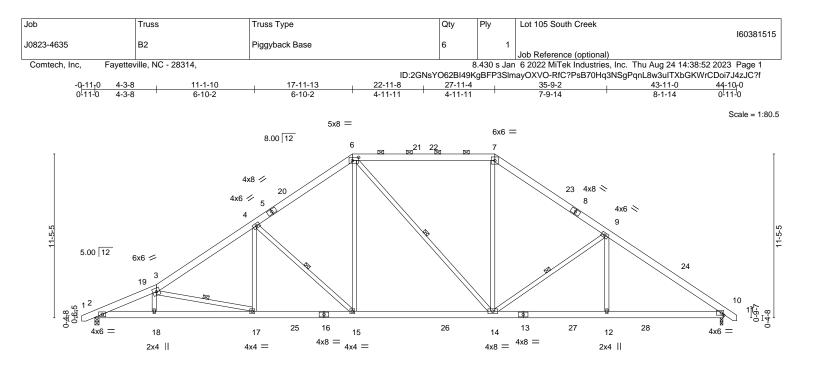


(0.131"x3") nails, 6in o.c., with 3in minimum end distance.

Brace must cover 90% of web length.

TRENCO A MiTek Affiliate

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L	4-3-8 11-1-	·10	17-11-13		27-11-4	1	3	5-9-2	43-10-7	43-11-0
	4-3-8 6-10	-2	6-10-2	1	9-11-7		7	-9-14	8-1-5	0-0 ⁴ 10
Plate Offsets (X,Y)	[6:0-5-4,0-2-12]									
LOADING (psf) TCLL 20.0	SPACING- Plate Grip DOL	2-0-0 1.15	CSI. TC	0.59	DEFL. Vert(LL)	in (loc) -0.20 14-15	l/defl >999	L/d 360	PLATES MT20	GRIP 244/190
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL Rep Stress Incr Code IRC2015	1.15 YES	BC	0.55 0.37	Vert(CT) Horz(CT) Wind(LL)	-0.34 14-15 0.10 10 0.08 17-18	>999 n/a >999	240 n/a 240	Weight: 335 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

WEBS

except

1 Row at midpt

2-0-0 oc purlins (4-10-3 max.): 6-7.

Rigid ceiling directly applied or 9-9-2 oc bracing.

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1

WEBS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 10=0-2-5 Max Horz 2=272(LC 11) Max Uplift 2=-92(LC 12), 10=-77(LC 13) Max Grav 2=1848(LC 2), 10=1951(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-4045/768, 3-4=-3046/625, 4-6=-2376/598, 6-7=-1810/553, 7-9=-2279/574,
- 9-10=-2903/555
- BOT CHORD 2-18=-641/3816, 17-18=-648/3813, 15-17=-338/2617, 14-15=-128/1944, 12-14=-321/2304, 10-12=-321/2304
- WEBS 3-17=-1242/328, 4-17=-12/557, 4-15=-930/286, 6-15=-88/1044, 6-14=-266/145, 7-14=-52/804, 9-14=-766/246, 9-12=0/414

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-4 to 3-7-9, Interior(1) 3-7-9 to 17-11-13, Exterior(2) 17-11-13 to 22-4-9, Interior(1) 22-4-9 to 27-11-4, Exterior(2) 27-11-4 to 32-4-0, Interior(1) 32-4-0 to 44-8-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 10.

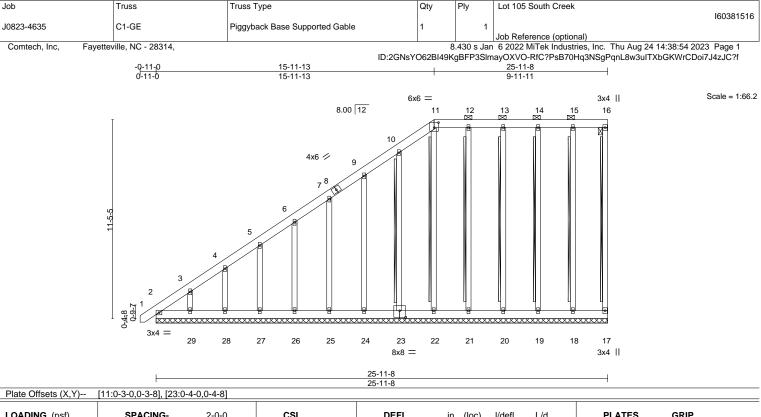
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 3-9-12 oc purlins,

3-17, 4-15, 6-14, 9-14

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LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.05	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.01	Vert(CT)	-0.00	1	n/r	120		
BCLL 0.0	Rep Stress Incr	YES	WB	0.17	Horz(CT)	-0.00	17	n/a	n/a		
BCDL 10.0	Code IRC2015/	PI2014	Matrix	k-S						Weight: 280 lb	FT = 20%
LUMBER-					BRACING-						
TOP CHORD 2	6 SP No.1				TOP CHOP	RD.	Structu	ral wood	sheathing dir	ectly applied or 6-0-0 o	c purlins,
BOT CHORD 2	6 SP No.1						except	end vertion	als, and 2-0-	0 oc purlins (6-0-0 max	k.): 11-16.
WEBS 22	4 SP No.2				BOT CHOF	RD	Rigid c	eiling dire	ctly applied o	or 10-0-0 oc bracing, E	Except:
OTHERS 2	4 SP No.2						6-0-0 o	c bracing	22-23.		

2x4 SP No.2 6-0-0 oc bracing: 22-23. WEBS 2x4 SPF No.2 - 16-17, 15-18, 14-19, 13-20 T-Brace: , 12-21, 11-22, 10-23

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 25-11-8.

(lb) -Max Horz 2=523(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 17, 2, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28 except 29=-161(LC 12) Max Grav All reactions 250 lb or less at joint(s) 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29 except 2=344(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-618/496, 3-4=-498/396, 4-5=-421/336, 5-6=-347/277, 6-7=-272/218

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 8) will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 2, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28 except (jt=lb) 29=161.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 49 lb down and 33 lb up at
- 24-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 12) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

LOAD CASE(S) Standard Continued on page 2

📣 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 105 South Creek			
J0823-4635	C1-GE	Piggyback Base Supported Gable	1	1	160381516			
JU623-4033	CI-GE	Figgyback base Supported Gable	1		Job Reference (optional)			
Comtech, Inc, Fayettev	rille, NC - 28314,		8	.430 s Jan	6 2022 MiTek Industries, Inc. Thu Aug 24 14:38:54 2023 Page 2			
		ID:2GNsY	ID:2GNsYO62BI49KgBFP3SImayOXVO-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f					

LOAD CASE(S) Standard

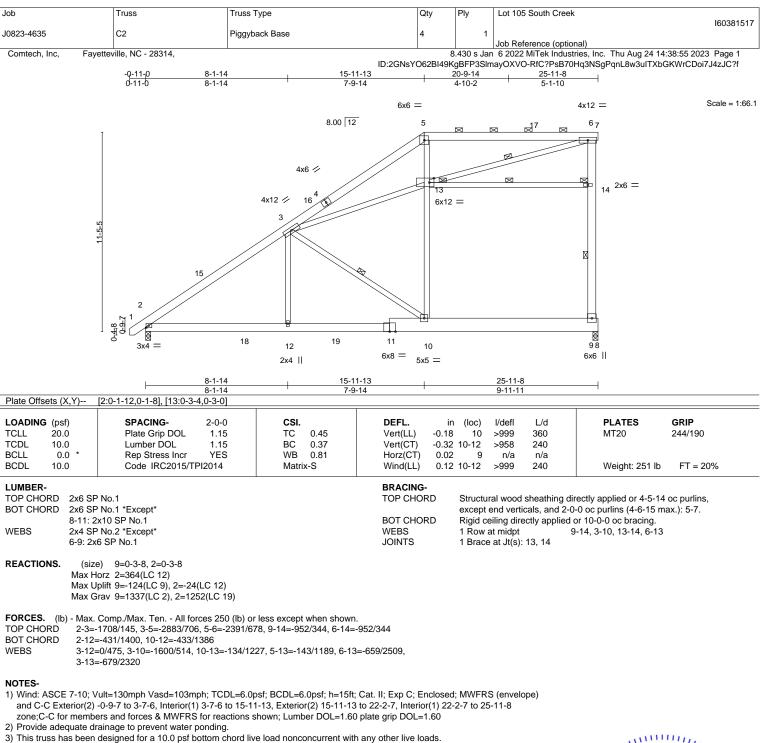
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-11=-60, 11-16=-60, 2-17=-20 Concentrated Loads (Ib)

Vert: 18=-49

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4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

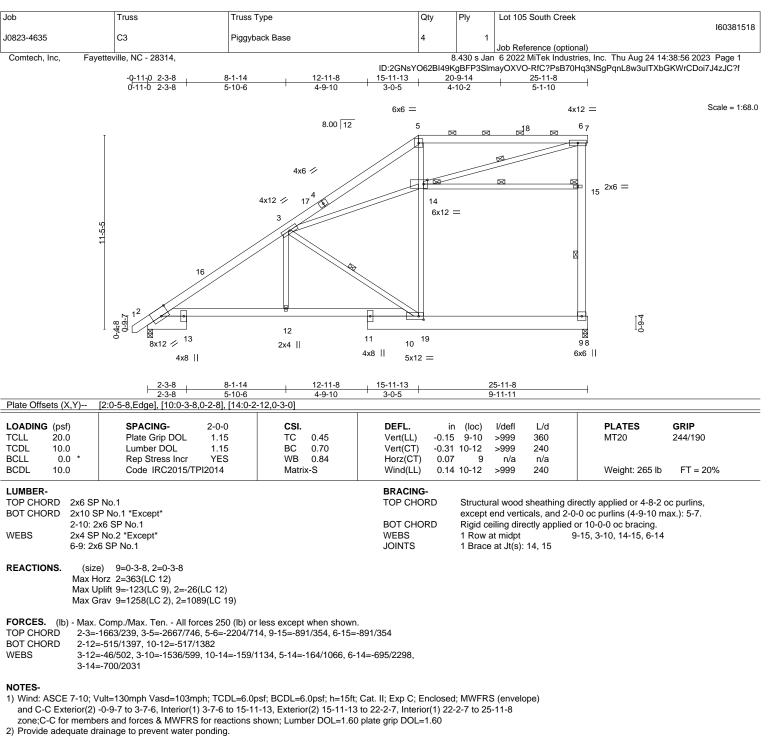
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 9=124

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

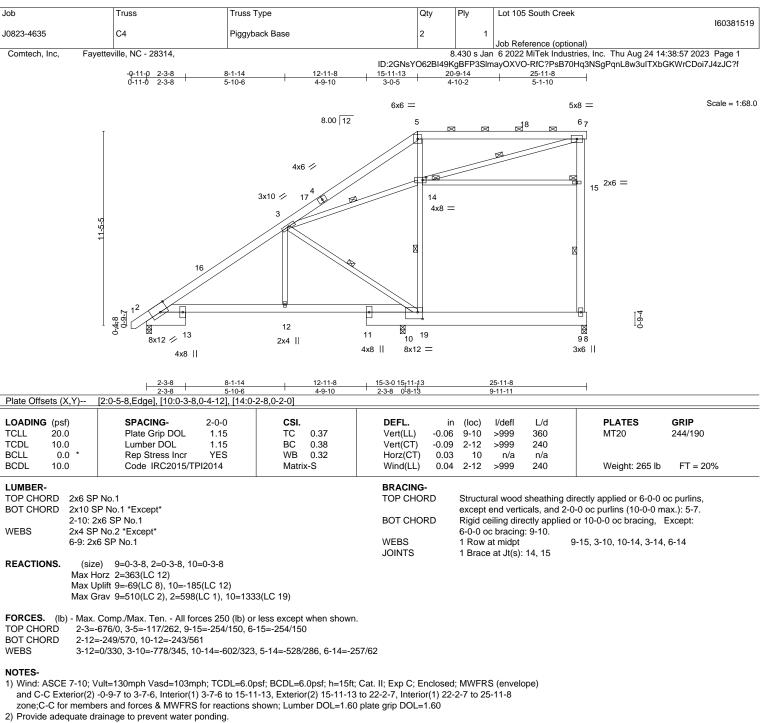
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 9=123.

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

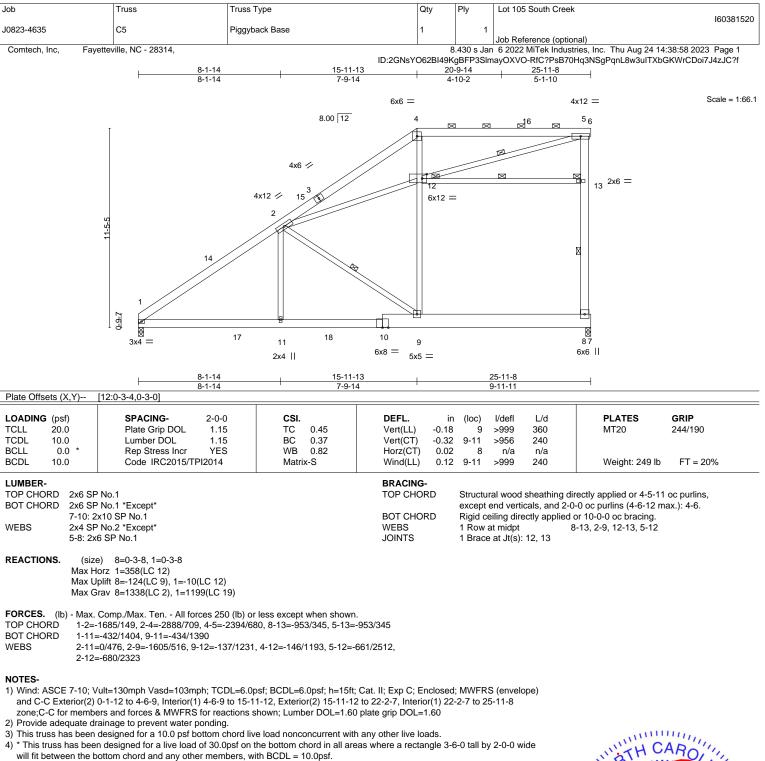
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 10=185.

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=124.

6) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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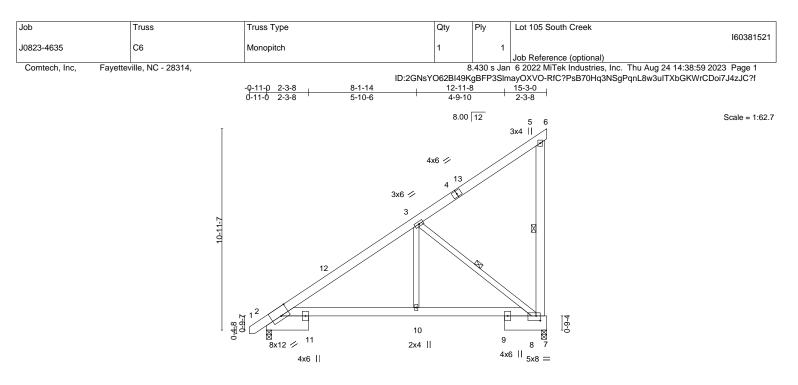


Plate Offsets (X,Y)	[2:0-5-8,Edge], [8:0-2-12,0-3-0]	2-3-8 8-1-14 2-3-8 5-10-6	12-11-1 4-9-10					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.24 BC 0.40 WB 0.26 Matrix-S	DEFL. in Vert(LL) -0.04 Vert(CT) -0.08 Horz(CT) 0.03 Wind(LL) 0.04	2-10 >999 360 2-10 >999 240 8 n/a n/a	PLATES GRIP MT20 244/190 Weight: 137 lb FT = 20%			
LUMBER- TOP CHORD 2x6 SP No.1 BOT CHORD 2x10 SP No.1 *Except* 2-8: 2x6 SP No.1 WEBS 2x4 SP No.2 *Except* 5-8: 2x6 SP No.1		BRACING- TOP CHORD BOT CHORD WEBS	except end verticals.	ng directly applied or 6-0-0 oc purlins, lied or 10-0-0 oc bracing. 5-8, 3-8				
REACTIONS. (size) 8=0-3-8, 2=0-3-8 Max Horz 2=344(LC 12) Max Uplit 8=-185(LC 12) Max Grav 8=680(LC 19), 2=630(LC 1)								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-753/0 BOT CHORD 2-10=-234/624, 8-10=-225/624 WEBS 3-10=0/404, 3-8=-809/287								

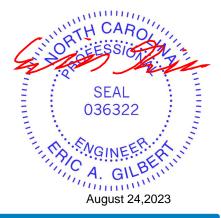
NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-7 to 3-7-6, Interior(1) 3-7-6 to 15-3-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

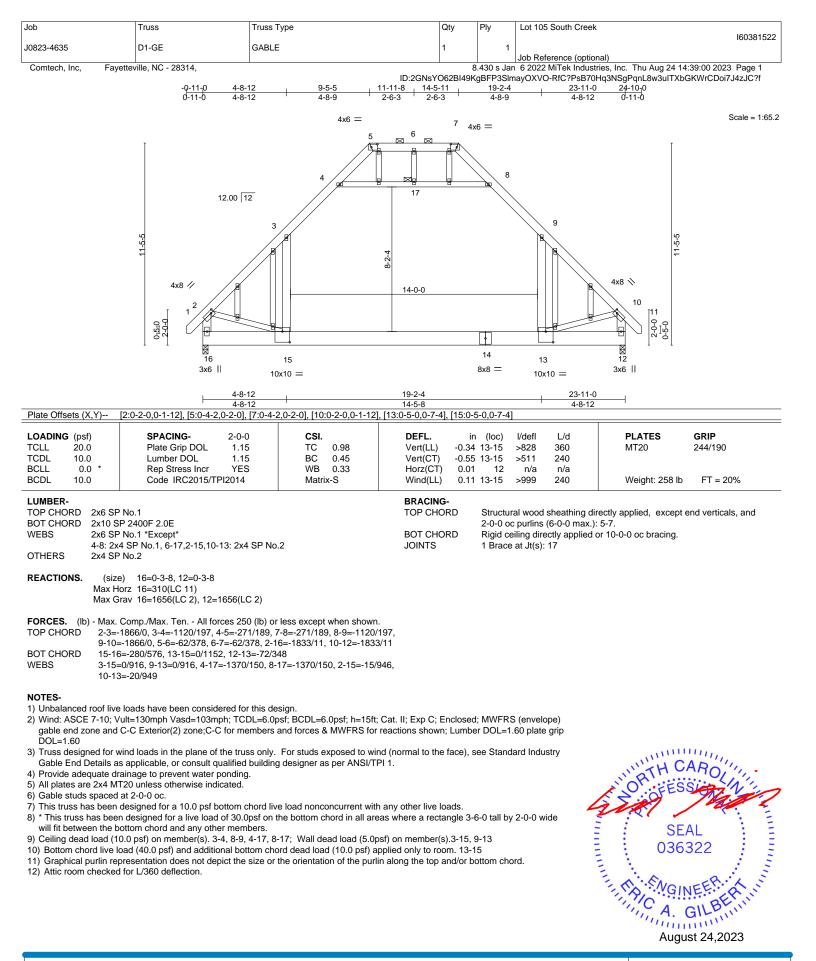
3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=185.



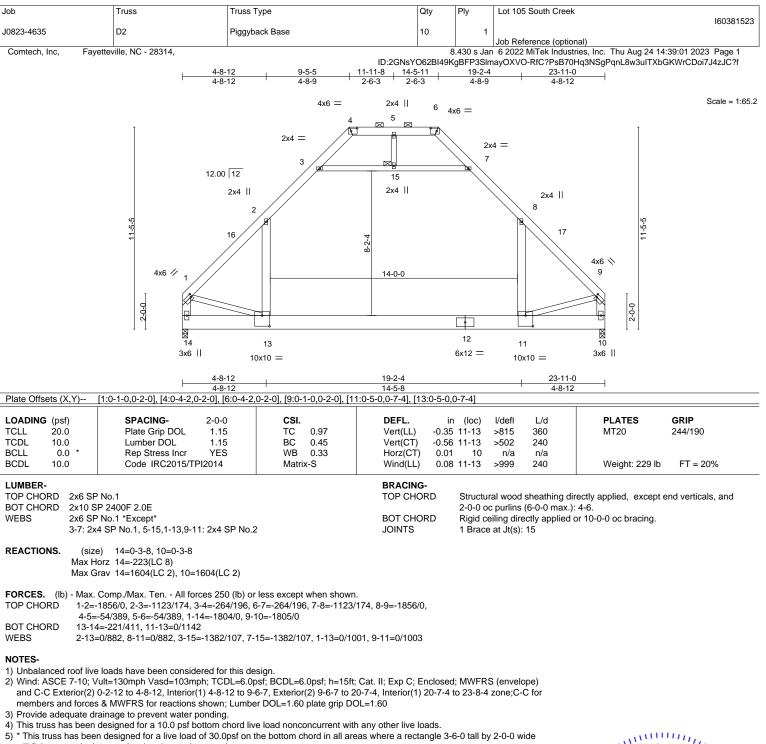
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will fit between the bottom chord and any other members.

6) Ceiling dead load (10.0 psf) on member(s). 2-3, 7-8, 3-15, 7-15; Wall dead load (5.0psf) on member(s).2-13, 8-11

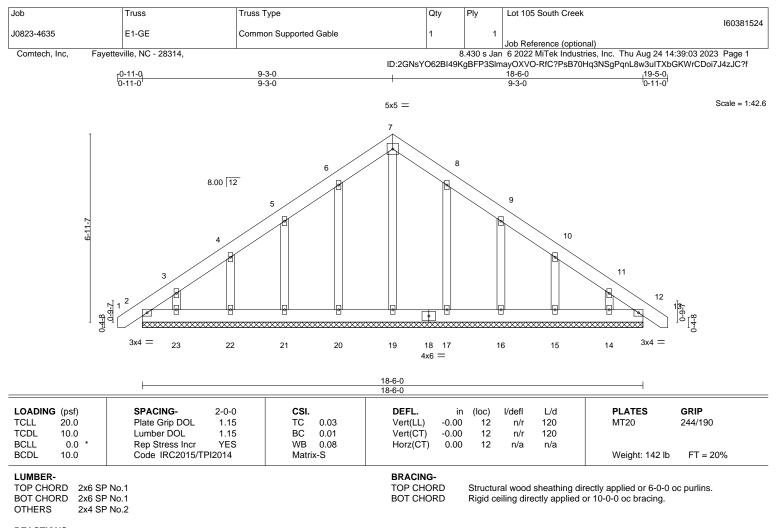
7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) Attic room checked for L/360 deflection.



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REACTIONS. All bearings 18-6-0.

(lb) - Max Horz 2=-200(LC 10)

 Max Uplift
 All uplift 100 lb or less at joint(s) 2, 20, 21, 22, 17, 16, 15, 14, 12 except 23=-106(LC 12)

 Max Grav
 All reactions 250 lb or less at joint(s) 2, 19, 20, 21, 22, 23, 17, 16, 15, 14, 12

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

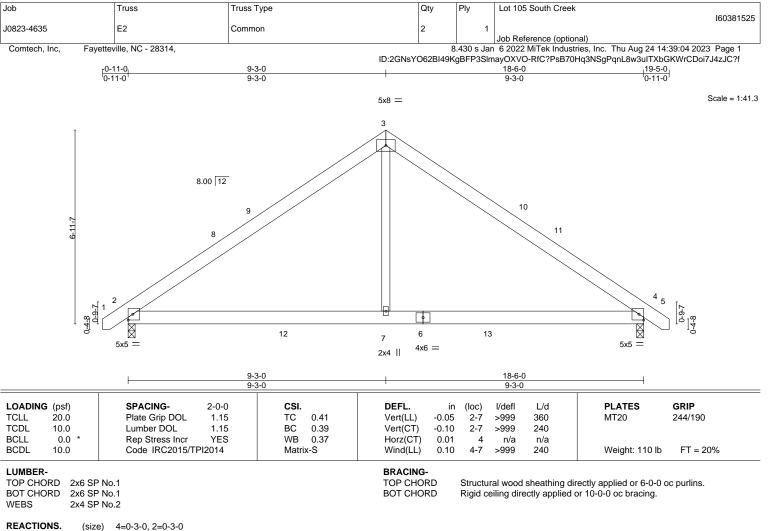
8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 21, 22, 17, 16, 15, 14, 12 except (jt=lb) 23=106.

10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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DNS. (size) 4=0-3-0, 2=0-3-0 Max Horz 2=160(LC 11) Max Uplift 4=-115(LC 8), 2=-115(LC 9) Max Grav 4=849(LC 2), 2=849(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1032/758, 3-4=-1032/758

BOT CHORD 2-7=-457/742, 4-7=-457/742

WEBS 3-7=-547/632

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-7 to 3-7-6, Interior(1) 3-7-6 to 9-3-0, Exterior(2) 9-3-0 to 13-7-13, Interior(1) 13-7-13 to 19-3-7 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=115, 2=115.

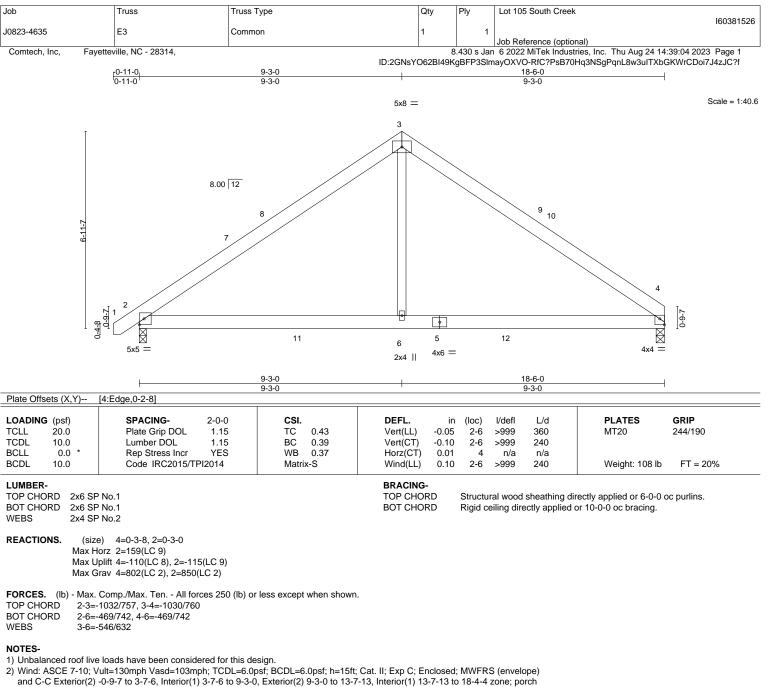


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left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

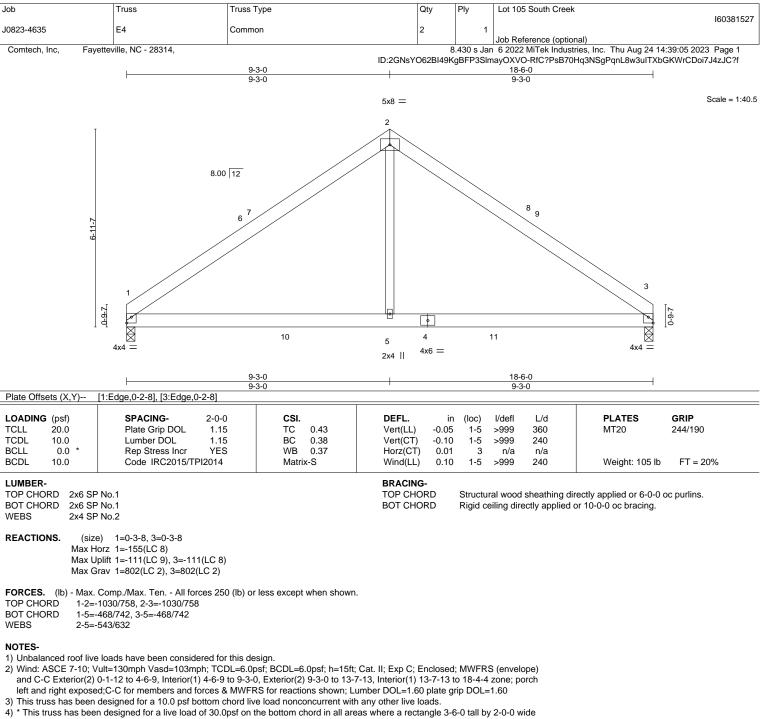
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=110, 2=115.



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will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)

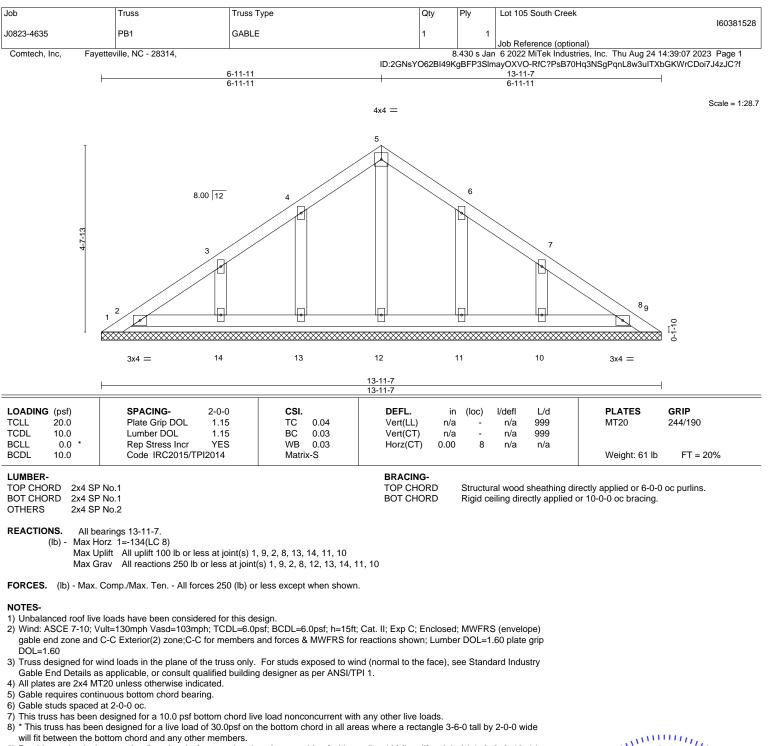
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j 1=111, 3=111.



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9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 2, 8, 13, 14, 11, 10.

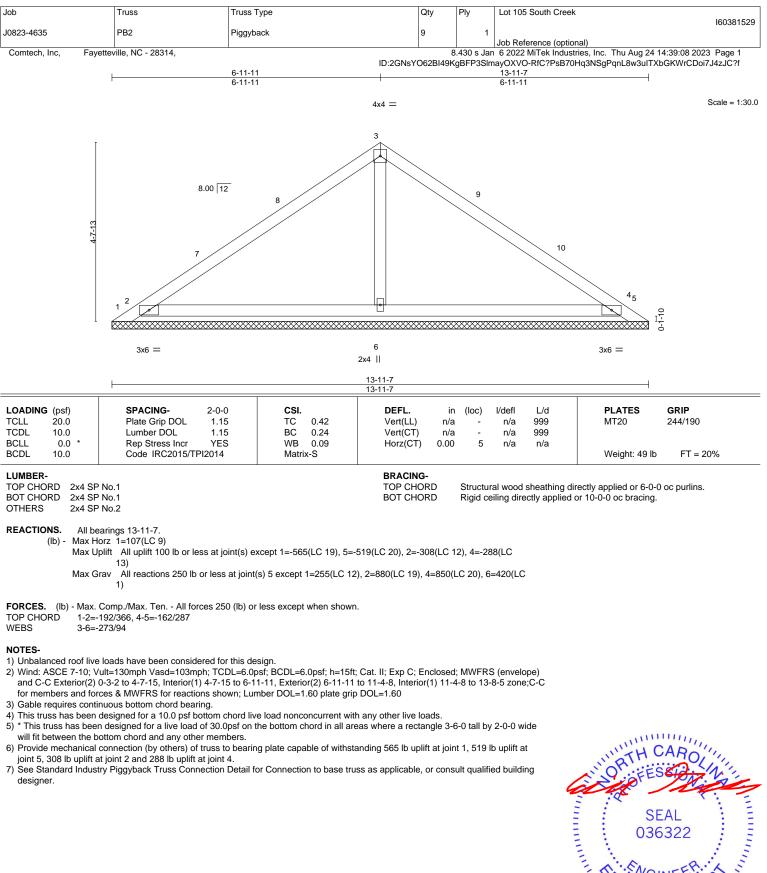
10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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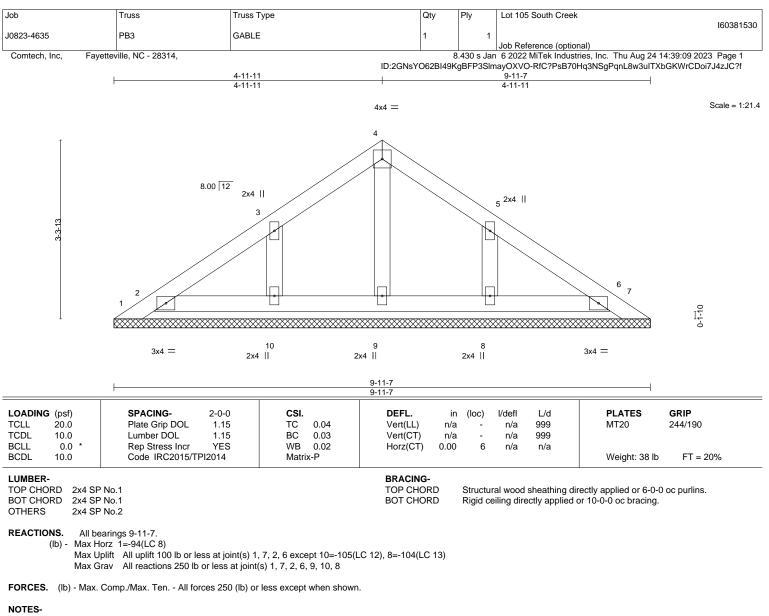
A MiTek Affi 818 Soundside Road

Edenton, NC 27932





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

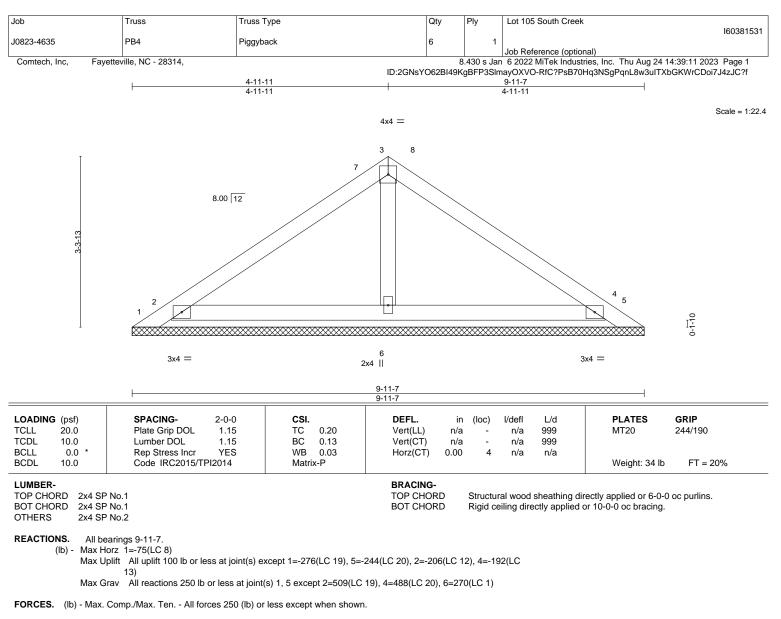
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 2, 6 except (jt=lb) 10=105, 8=104.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

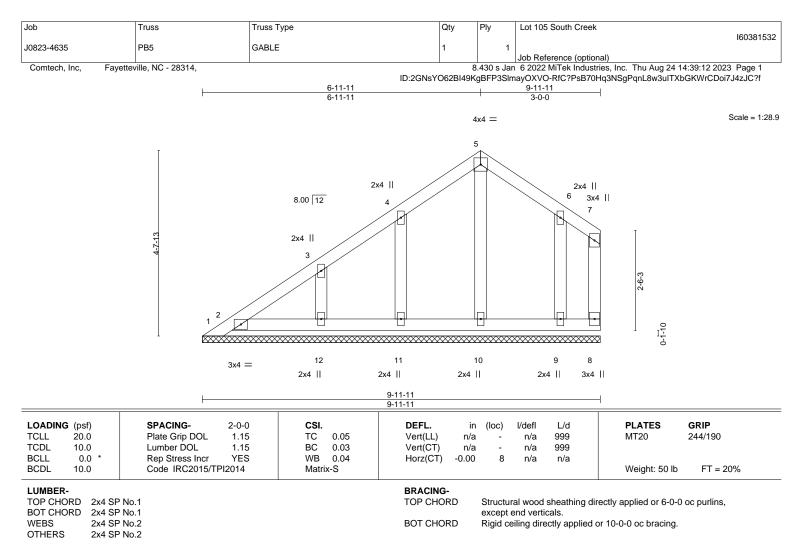
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 276 lb uplift at joint 1, 244 lb uplift at joint 5, 206 lb uplift at joint 2 and 192 lb uplift at joint 4.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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REACTIONS. All bearings 9-11-11.

(lb) - Max Horz 1=159(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 8, 2, 11, 12, 9

Max Grav All reactions 250 lb or less at joint(s) 1, 8, 2, 10, 11, 12, 9

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

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3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

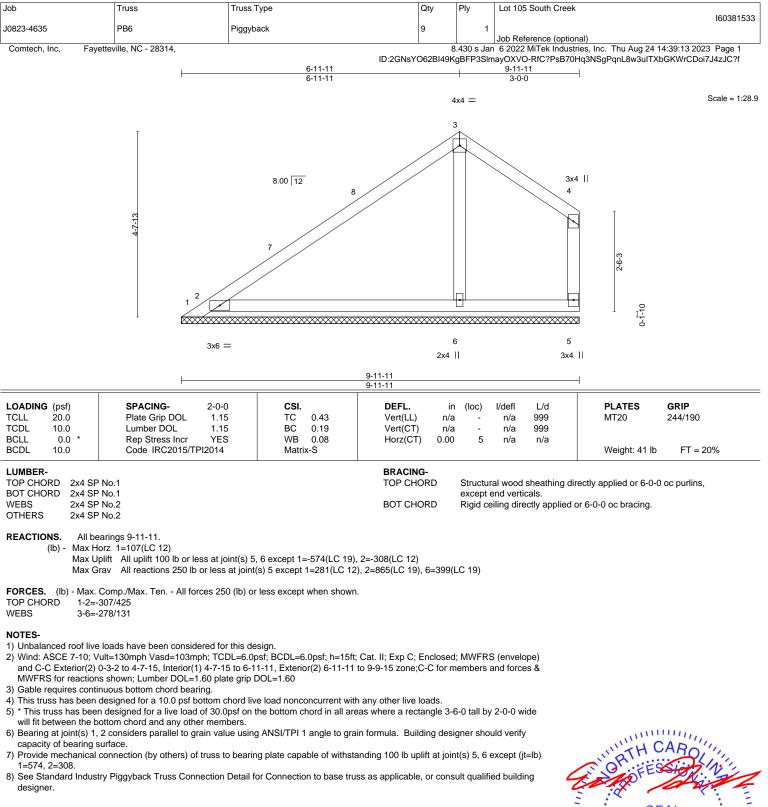
7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Bearing at joint(s) 1, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8, 2, 11, 12, 9.
 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

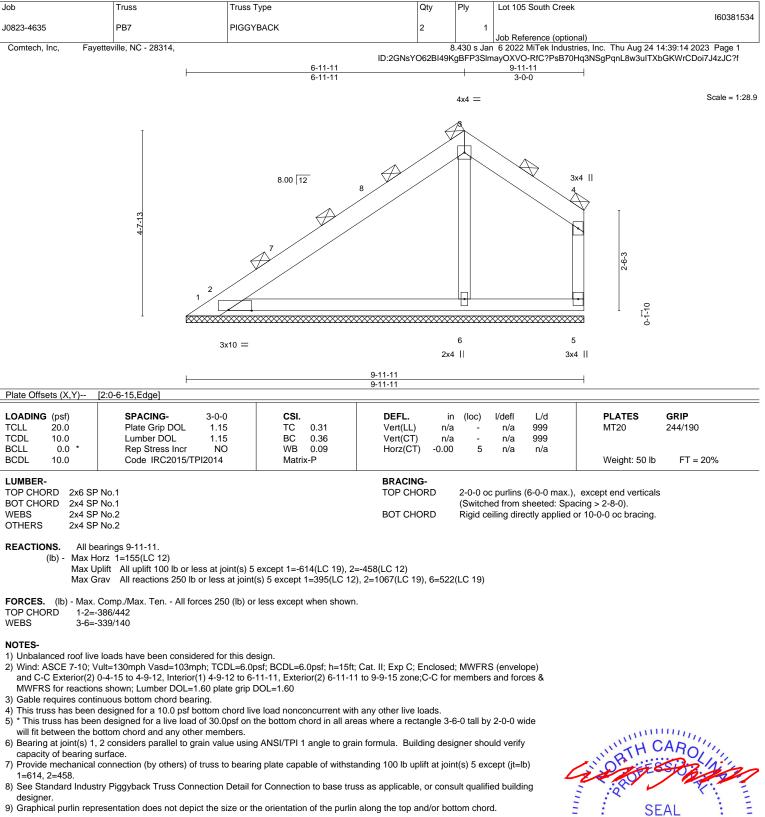


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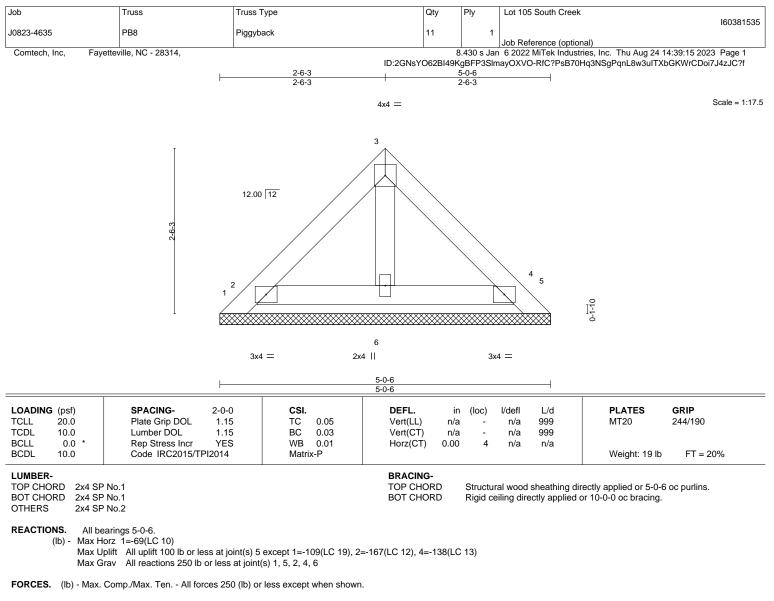


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- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 1, 5, 2, 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 1=109, 2=167, 4=138.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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