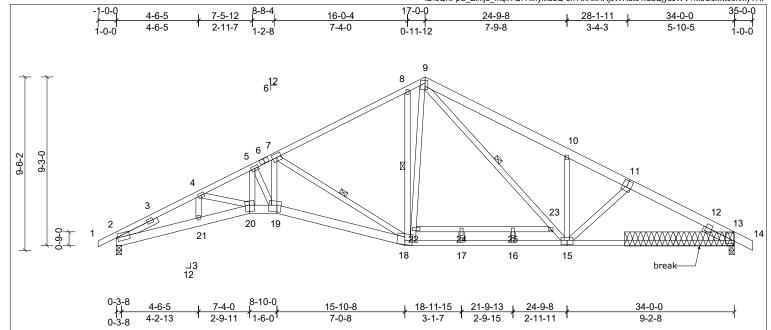


UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, clm

Run: 8.62 S Sep 22 2022 Print: 8.620 S Sep 22 2022 MiTek Industries, Inc. Fri Sep 29 15:21:46

Page: 1 $ID:6QKFpC_ZImjd_mqkYQ7RIhyMauQ-5hYthRkHRj0WRut64ISaQjy8JWV?M0OsMwJ6hmyYl4rndquares and the state of the sta$



Repair for a break in the bottom chord where indicated.

Attach 2x10 x 6' SP or SPF No.2 scab to each face of truss as shown with 2 rows of 10d (.131" x 3") nails spaced 4" oc in the bottom chord and three staggered 10d nails in the top chord and slider.

2-0-0 oc purlins (2-2-3 max.) (Switched from sheeted: Spacing > 2-0-0). Rigid ceiling directly applied or 8-7-3 oc bracing.

1 Row at midpt

Plate Offsets (X, Y): [2.0-0-14,0-2-11], [9.0-1-12,0-1-12], [11:0-3-0,0-4-8], [13:Edge,0-2-4], [15:0-4-0,0-3-0], [19:0-2-12,0-4-8], [20:0-3-0,0-4-4]												
Loading	(psf)	Spacing	2-3-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.94	Vert(LL)	-0.23	18-19	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.95	Vert(CT)	-0.47	18-19	>871	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.94	Horz(CT)	0.22	13	n/a	n/a		
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MSH							Weight: 246 lb	FT = 20%

BRACING

WEBS

TOP CHORD

2x6 SP No.2 *Except* T2:2x4 SP No.2, T1:2x4 SP No.1 BOT CHORE 2x6 SP No.2 *Except* B1:2x6 SP No.1, B4,B5:2x4 SP No.2 WEBS

2x4 SP No.3 *Except* W8,W10,W9:2x4 SP No.2 Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 1-8-13

> (lb/size) 2=1591/0-3-8, (min. 0-1-13), 13=1601/0-3-8, (min. 0-1-14)

Max Horiz 2=-180 (LC 11)

2=-232 (LC 10), 13=-235 (LC 11)

FORCES1 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD $2.3 - .2216/468, \ 3.4 - .4002/996, \ 4.5 - .4281/1053, \ 5.6 - .3687/942, \ 6.7 - .3654/945, \ 7.8 - .1856/610, \ 8.9 - .1727/715, \ 9.10 - .2494/891, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \ 10.11 - .2358/679, \ 11.12 - .2503/685, \ 12.13 - .1005/116, \$ BOT CHORD

2-21=-768/3556, 20-21=-787/3641, 19-20=-721/3747, 18-19=-710/3595, 17-18=-55/1265, 16-17=-55/1265, 15-16=-55/1265, 13-15=-457/2133 WEBS

 $5-20=-219/935,\ 5-19=-722/109,\ 7-19=-189/1626,\ 7-18=-2295/612,\ 8-18=-266/231,\ 18-22=-302/835,\ 9-22=-277/836,\ 9-23=-391/1011,\ 15-23=-460/1119,\ 4-20=0/320,\ 4-21=-294/120,\ 10-15=-526/362$

NOTES (9)

LUMBER

SLIDE

REACTIONS

TOP CHORE

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior 2)
- (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3)
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

 Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 2 and 235 lb uplift at joint 13. 6)
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 7)
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) This repair has been prepared based on information and use conditions supplied by client. Designer has made a good faith effort to outline damage and repair conditions as reported by client. When actual field conditions do not approximate those indicated on this drawing, client shall immediately inform the engineer and refrain from applying the repair



7-18, 8-18, 9-15

This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.

