MATERIALS

. FRAMING LUMBER SHALL BE #2 SPRUCE PINE FIR (SPF) WITH THE FOLLOWING DESIGN PROPERTIES.

Fb = 875 PSI Fv = 70 PSI E = 1.4E6 PS

COUTHERN YELLOW PINE (SYP) TREATED IN ACCORDANCE WITH THE GROUND, CONCRETE OR MASONRY SHALL BE #2. SOUTHERN YELLOW PINE (SYP) TREATED IN ACCORDANCE WITH AWPA C22 WITH THE FOLLOWING DESIGN PROPERTIES: Fb = 1050 P31 | Fv = 56 P31 | E = 1.656 P51 2. FRAMING LUMBER EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE #2

3. ENGINEERED WOOD BEAMS SHALL BE LAMINATED VENEER LUMBER (LVL) OR PARALLEL STRAND LUMBER (PSL) WITH THE FOLLOWING MINIMUM DESIGN PROPERTIES:

Fb = 2900 PSI Fv = 285 PSI E = 1.9E6 PSI

4. STRUCTURAL STEEL SHALL CONFORM TO ASTM A-36 MINIMUM GRADE.

5. BOLTS SHALL CONFORM TO A307 MINIMUM GRADE.

6. REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615 GRADE 60.

7 POLIRED CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3000 PSLAT 28 DAYS. MATERIALS. JSED TO PRODUCE CONCRETE SHALL COMPLY WITH THE APPLICABLE STANDARDS LISTED IN ACI 318 OR ASTM C 1157.

8. CONCRETE LOCATED PER <u>TABLE R402.2</u> SHALL BE AIR ENTRAINED WITH THE TOTAL AIR CONTENT NOT LESS THAN 5 PERCENT OR MORE THAN 7 PERCNET.

9. MASONRY UNITS SHALL CONFORM TO ACI 530/ASCE 5/TMS 402 AND MORTAR SHALL COMPLY WITH ASTM C 270.

10. ALLOWARI E SOIL BEARING PRESSURE 2000 PSE

ENGINEER'S SEAL APPLIES TO STRUCTURAL COMPONENTS ONLY AND DOES NOT CERTIFY ARCHITECTURAL LAYOUT OR DIMENSIONAL ACCURACY. ENGINEER IS NOT RESPONSIBLE FOR CONSTRUCTION METHODS OR ANY DEVIATION FROM THE

ALL CONSTRUCTION WORKMANSHIP MATERIAL QUALITY AND SELECTION SHALL BE IN ACCORDANCE WITH THE NORT CAROLINA STATE BUILDING CODE - RESIDENTIAL CODE 2012 EDITION FROM THE INTERNATIONAL RESIDENTIAL CODE 2012

(IRC), AND LOCAL CODES AND REGULATIONS. DIMENSIONS SHALL GOVERN OVER SCALE AND CODE SHALL GOVERN OVER

ADDITIONAL LOADS

FIGURE R301.2(4) - BASIC DESIGN WIND SPEED 100 MPH

FIGURE R301.2(2) - SEISMIC DESIGN CATEGORY B

TABLE R301.2(4) - DESIGN POSITIVE AND NEGATIVE PRESSURE FOR DOORS AND WINDOW FOR A MEAN ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSF

ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE DESIGNED BASED ON ROOF PITCHES AS FOLLOWS: 45.4 PSF FOR 0:12 TO 2.25:12, 34.8 PSF FOR 2.25:12 TO 7:12 AND 21 PSF FOR 7:12 TO 12:12 WALL CLADDING IS DESIGNED FOR A 24.1 PSF POSITIVE AND NEGATIVE PRESSURE

TABLE N1102.1 - REFER TO TABLE N1101.1 TO DETERMINE THE CLIMATE ZONE BY COUNTY AND REFER TO TABLE N1102.1 FOR R VALUE INSULATION REQUIREMENTS LISTED BY ZONE.

 $\begin{array}{l} \underline{\text{TABLE N1102.1-ZONE 7}} - \text{MAX. GLAZING U FACTOR: } \underline{0.40}. & \text{MIN. INSULATION R VALUES: CEILING } \underline{\text{R-30}}, \text{WALLS } \underline{\text{R-13}}, \\ \underline{\text{FLOORS }} \underline{\text{R-19}}, \text{BASEMENT WALLS } \underline{\text{R-7}}, \text{SLAB PERIMETER } \underline{\text{R-0}}, \text{CRAWL SPACE WALLS } \underline{\text{R-7}}. \\ \end{array}$

 $\begin{array}{l} \underline{\text{TABLE N1102.1-ZONE 8-MAX. GLAZING U FACTOR: 0.40.}} & \underline{\text{MIN. INSULATION R VALUES: CEILING R-30, WALLS R-13,}} \\ \underline{\text{FLOORS R-19, BASEMENT WALLS R-8, SLAB PERIMETER R-6 (2 FT DEEP), CRAWL SPACE WALLS R-10.}} \\ \end{array}$

1. STEEL FLITCH BEAMS SHALL BE FASTENED TOGETHER WITH 1/2" DIAMETER BOLTS WITH WASHERS PLACED UNDER THE THREADED END OF THE BOLT. BOLTS SHALL BE SPACED AT MAXIMUM 2/4" o.c. STAGGERED TOP AND BOTTOM OF BEAM WITH A MINIMUM 2" EDGE DISTANCE. TWO BOLTS SHALL BE LOCATED AT 6" FROM EACH END OF FLITCH BEAM.

2. STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3 1/2" AND FULL FLANGE WIDTH. BEAMS MUST BE ANCHORED AT EACH END WITH A MINIMUM OF FOUR 16d NAILS OR TWO 1/2" x 4" LAG SCREWS

3. ENGINEERED WOOD BEAMS SHALL BE INSTALLED WITH ALL CONNECTIONS PER MANUFACTURER'S INSTRUCTIONS.

4. ALL BEAMS SHALL BE CONTINUOUSLY SUPPORTED LATERALLY AND SHALL BEAR FULL WIDTH ON THE SUPPORTING WALLS OR COLUMNS INDICATED WITH A MINIMUM OF THREE STUDS.

5. SOLID BLOCKING SHALL BE PROVIDED AT ALL POINT LOADS TO TRANSFER LOADS THROUGH FLOOR LEVELS. COLUMNS SHALL BE CONTINUOUS TO THE FOUNDATION OR TO OTHER STRUCTURAL ELEMENTS.

6 ENGINEERED WOOD FLOOR SYSTEMS AND ROOF TRUSS SYSTEMS SHALL BE PROVIDED FOR REVIEW AND COORDINATED

. WALL BRACING REQUIREMENTS SHALL BE IN ACCORDANCE WITH SECTION R602.10 OF THE NORTH CAROLINA RESIDENTIAL

8. BRICK LINTELS SHALL BE 3 1/2 x 3 1/2 x 1/4 STEEL ANGLE FOR UP TO 6'0" MAXIMUM SPAN AND 6 x 4 x 5/16 FOR SPANS

9. BRICK LINTELS AT SLOPED AREAS SHALL BE 4 x 3 1/2 x 1/4 STEEL ANGLE WITH 16d NAILS IN 3/16" HOLES IN 4" ANGLE LEG AT 12" o.c. TO DOUBLE RAFTER. WHEN THE SLOPE EXCEEDS 4:12 A MINIMUM OF 3 x 3 x 1/4 PLATES SHALL BE WELDED AT 24" o.c. ALONG THE STEEL ANGLE.

Lot 16 Cotton Farms

SQUARE FOOTAGE							
	HEATED S.F.	UNHEATED S.F.					
FIRST FLOOR	1904						
SECOND FLOOR	350						
GARAGE		519					
FRONT PORCH		125					
DECK		186					
TOTAL	2254	830					
OPT. SCREENED PORCH		186					
OPT. 3RD CAR GARAGE		278					

	REVISION LOG						
Rev	Description	Drawn By	Date	Sheets Affected	Brochure Required	Engineering Required	
1							
2							
3							
4							
5							
6							
7							
8							
9							
10							
11							
12							
13							
14							

CLADDING POSITIVE & NEGATIVE PRESSURE = 21 PSF

1 1/2 STORY = 19'-0" CLADDING POSITIVE & NEGATIVE PRESSURE = 34.8 PSF

CLADDING POSITIVE & NEGATIVE PRESSURE = 34.8 PSF

INSTALL ANCHOR BOLTS, NUTS, AND WASHERS PER CODE AT ALL EXTERIOR WALL TREATED PLATES AND AT INTERIOR BEARING WALL TREATED PLATES ON SLAB FOUNDATIONS. TO BE A MINIMUM OF 6' O.C. AND WITHIN 12" FROM THE ENDS OF EACH PLATE.

DESIGN PRESSURES MINIMUM RATING: 25 PSF

TABLE N1102.1 CLIMATE ZONES 3-5

CONCRETE CONT
DBL
DJ
DSP
EA
FL PT
FTG
HGR
LVL
NTS
OC
PSL
PT
SC
SP CONTINUOUS DOUBLE JOIST
DOUBLE STUD POCKET
EACH
FLAT PLATE FOOTING HANGER I AMINATED VENEER I UMBER NOT TO SCALE STUD POCKET TRIPLE JOIST UNLESS NOTED OTHERWISE

CLIMATE ZONES	FENESTRATION U-FACTOR b	SKYLIGHT ^b U-FACTOR	GLAZED FENESTRATION SHGC b,e	CEILING ^k R-VALUE	WOOD FRAMED WALL R-VALUE	MASS WALL R-VALUE	FLOOR R-VALUE	BASEMENT C WALL R-VALUE	SLAB ^d R-VALUE AND DEPTH	CRAWL SPACE WALL R-VALUE
3	0.35	0.65	0.30	30	13	5/10	19	10/13 ^f	0	5/13
4	0.35	0.60	0.30	38 OR 30 CONT j	15 OR 13+2.5 ^h	5/10	19	10/13	10 ^d	10/13
5	0.35	0.60	NR	38 OR 30 CONT j	19 OR 13+5 OR 15+3e,h	13/17	30 9	10/13	10 ^d	10/13
TALLIES ADE MINIMILIANS IL EACTORS AND SINCE ADE MAYBUINS										

- b. THE FENESTRATION LEFACTOR COLUMN EXCLUDED SKYLIGHTS. THE SHCC COLUMN APPLIES TO ALL GLAZED FENESTRATION.

- THE PENSTRATION LYACTOR COLUMN EXCLUDED SYNCHOLYS. THE SHIGC COLUMN APPLIES TO ALL GLAZED FENESTRATION.
 "1015 MEANS R-10 CORT. INSULATED SHEATHING ON THE INTERIOR OF EXTERIOR OF THE HONGE OR R-13 CAVITY INSULATION AT THE INTERIOR OF THE BASEMENT WALL OR CRAWL SPACE WALL.
 F. CRINGOLOTHIC SLABS. INSULATION SHALL BE APPLIED FROM THE INSPECTION AGD POINWARD TO THE BOTTOM OF THE FOOTING OR A MAXIMUM OF 18 INCHES BELOW GRADE. WHICHCHER IS LESS. FOR FIGHTING SLABS. INSULATION SHALL EXTEND TO THE BOTTOM OF THE FOOTING ON A MAXIMUM OF 18 INCHES BELOW GRADE. WHICHCHER IS LESS. FOR FIGHTING SLABS.
 R. 791 BERCIALS SCHATE COMPRESSED AND INSTALLED IN A NOMINAL 266 CAVITY IS DEEMED TO COMPLY, FIBERGLASS BATTS RATED R-19 OR HIGHER COMPRESSED AND INSTALLED IN A 24 WALL IS NOT DEEMED TO COMPLY.
 BASEMENT WALL. INSULATION IS NOT REQUIRED IN WARM-HUMID LOCATIONS AS DEFINED BY FIGURE 11/10.2 (1 AND 2) AND TABLE BIT101.
 I. 1935 MEANS R-13 CAVITY INSULATION FULL SE INSULATED SHEATHING. 154 MEANS R-15 CAVITY INSULATION FULLS SHEATHING COVERS 25 PERCENT ON LEW FRANKE SHATHING SHOT SHEATHING. IS STRUCTURAL SHEATHING COVERS 25 PERCENT ON LEW FRANKE SHATHING IS NOT RECURRED WITH INSULATION FULL SHEATHING. SHOT SHATHING SHOT SHATHING SHOT SHATHING SHOT SHATHING SHOT SHATHING. SHOT SHATHING SHATHING SHOT SHATHING SHA

MI WINDOWS 3500 SERIES LOW E-GLASS WINDOWS

	ATTIC VENT SCHEDULE								
	Cotton Farms Lot 16								
MAIN HOUSE SQ FTG 2488					AT	/ NEAR RID	GE	AT / NE	AR EAVE
VENT TYPE	SQ. FT. REQUIRED		SQ. FT.	PERCENT OF TOTAL	POT LARGE (SQ. FT. EACH)	POT SMALL (SQ. FT. EACH)	RIDGE VENT (SQ. FT. PER LF)	EAVE VENT (SQ. IN. EACH)	CONT. VENT (SQ. IN. PER LF)
	RAN		SUPPLIED	SUPPLIED	0.4236	0.2778	0.125	0.1944	0.0625
RIDGE VENT	3.32	4.15	6.00	49.48	0	0	48.00		
SOFFIT VENTS	4.98	4.15	6.13	50.52				0	98.00
TOTAL (MIN)	8.29	8.29	12.13	100.00	POT VENTS MAY BE REQUIRED IF THERE IS INSUFFICIENT RIDGE AVAILABLE				

* SCHEDULE HAS BEEN CALCULATED ASSUMING EAVE VENTILATION AT 50-60% OF TOTAL AND RIDGE AT 40-50% OF TOTAL REQUIRED VENTILATION

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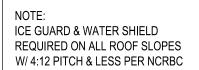
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A

Triple A Homes

FRONT ELEVATION

Plan No.

Sheet No. EL-1



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LEFT &
RIGHT SIDE
ELEVATION

Plan No.

 $\underline{EL-2}$

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8 :12





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REAR ELEVATION

Plan No.

EL-3



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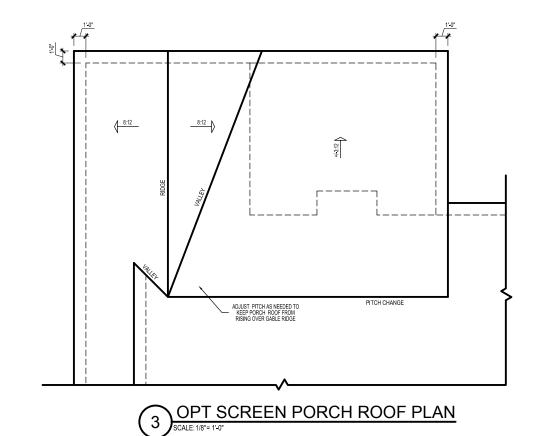
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OPT 3 CAR GARAGE **ELEVS**

Plan No.

EL-4

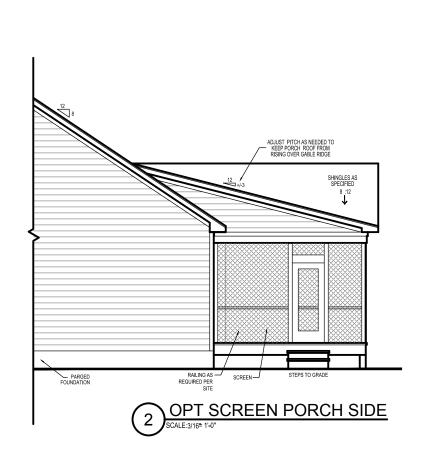
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SHINGLES AS SPECIFIED — HORIZONTAL SIDING PER SPECS.

OPT SCREEN PORCH REAR

SCALE:3/16= 1-0"





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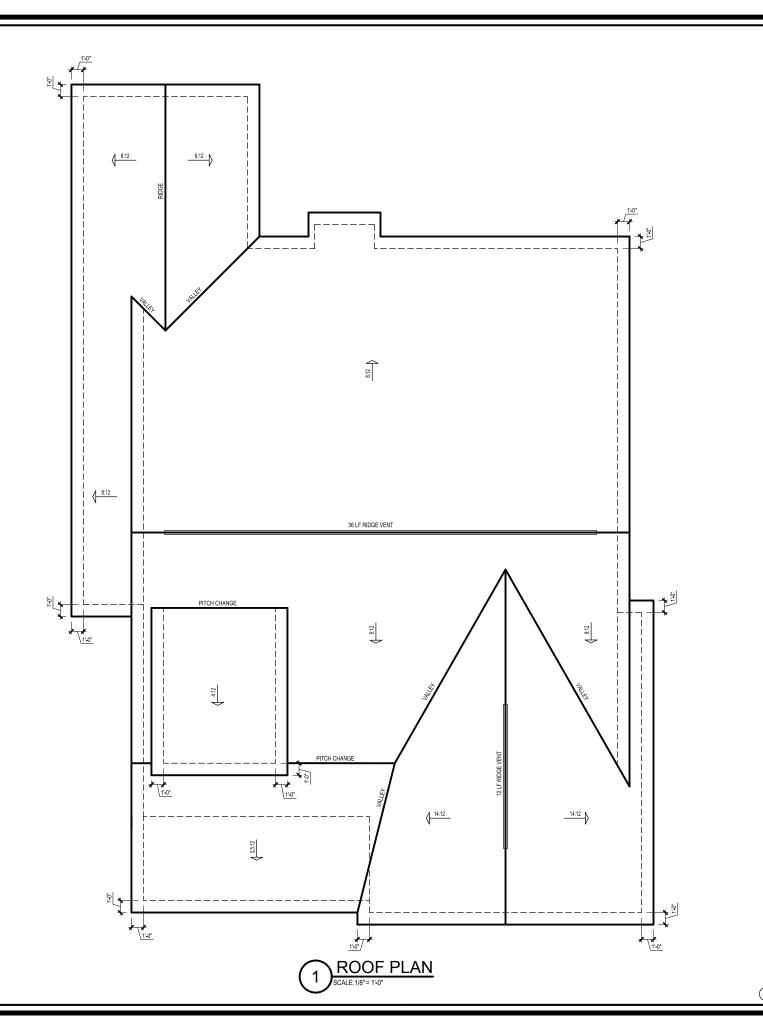
OPT SCREEN PORCH **ELEVS**

Plan No.

EL-5

	ATTIC VENT SCHEDULE																			
	Cotton Farms Lot 16																			
MAIN	MAIN HOUSE SQ FTG 2488					/ NEAR RID	GE	AT / NE	AR EAVE											
VENT TYPE	SQ. FT. REQUIRED		SQ. FT.	PERCENT OF TOTAL	POT LARGE (SQ. FT. EACH)	POT SMALL (SQ. FT. EACH)	RIDGE VENT (SQ. FT. PER LF)	EAVE VENT (SQ. IN. EACH)	CONT. VENT (SQ. IN. PER LF)											
	RAN		SUPPLIED	SUPPLIED	SUPPLIED	SUPPLIED	SUPPLIED	SUPPLIED	SUPPLIED	SUPPLIED	SUPPLIED	SUPPLIED	SUPPLIED	SUPPLIED	SUPPLIED	0.4236	0.2778	0.125	0.1944	0.0625
0																				
RIDGE VENT	3.32	4.15	6.00	49.48	0	0	48.00													
SOFFIT VENTS	4.98	4.15	6.13	50.52				0	98.00											
TOTAL (MIN)	8.29	8.29	12.13	100.00	POT VENTS MAY BE REQUIRED IF THERE IS INSUFFICIENT RIDGE AVAILABLE															

* SCHEDULE HAS BEEN CALCULATED ASSUMING EAVE VENTILATION AT 50-60% OF TOTAL AND RIDGE AT 40-50% OF TOTAL REQUIRED VENTILATION





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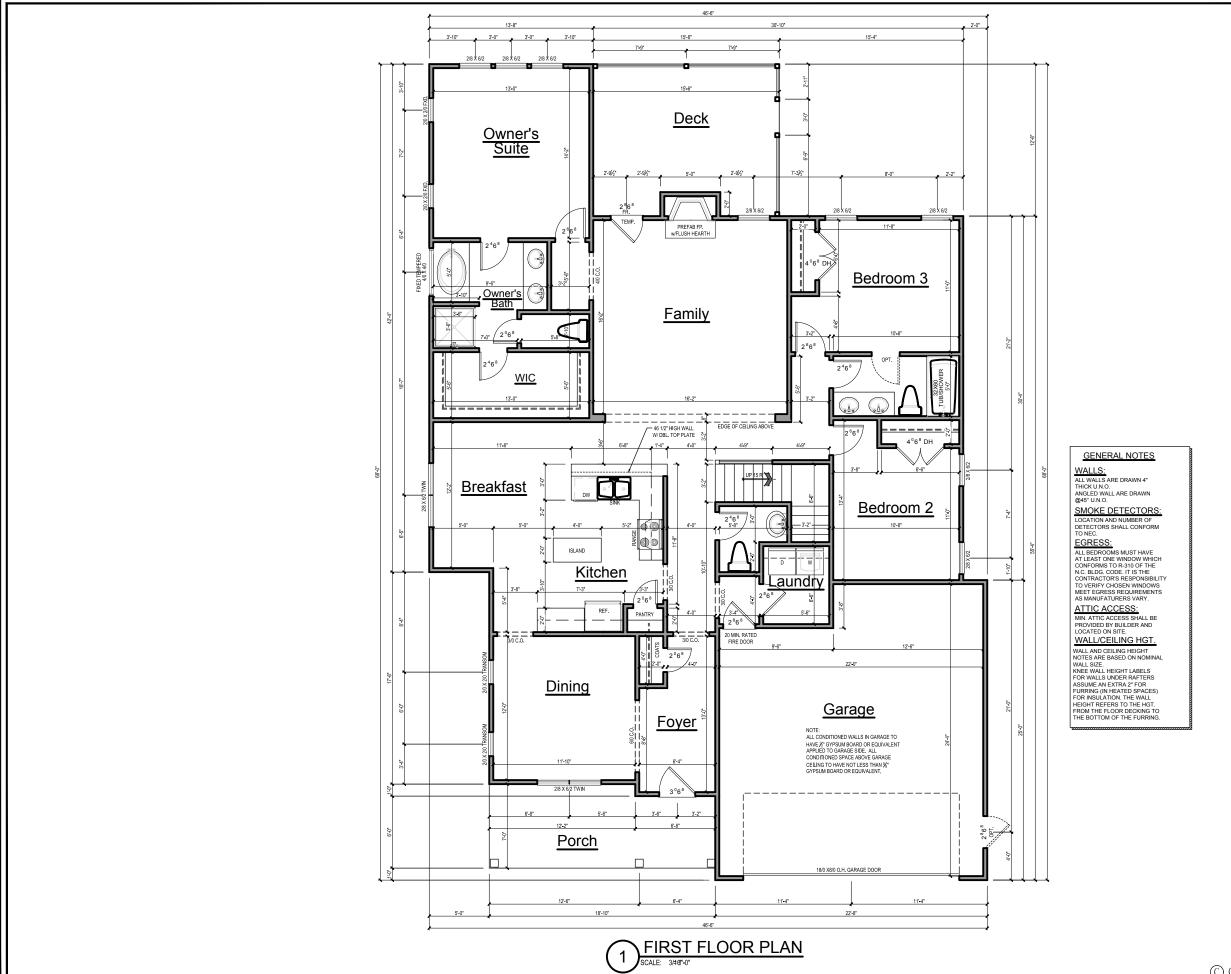
Triple A

ROOF PLAN

Plan No.

 $\mathop{EL}_{\mathsf{Sheet}\,\mathsf{No.}}$

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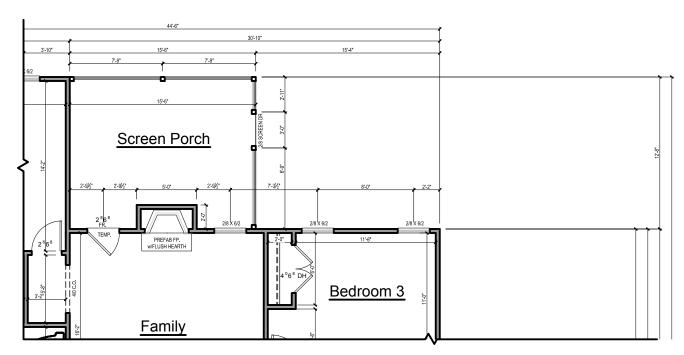
Triple A Homes

FIRST FLOOR PLAN

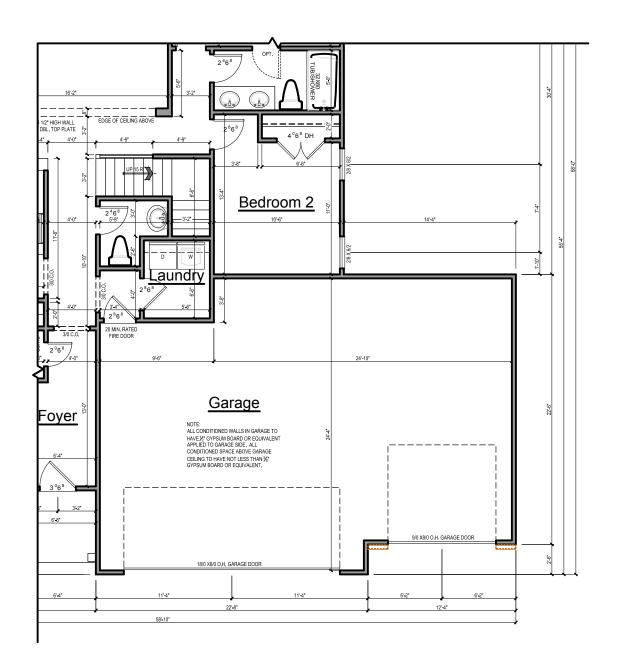
Plan No.

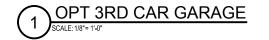
 $_{\text{Sheet No.}}$ A-1

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OPT SCREEN PORCH
SCALE: 1/8"= 1'-0"







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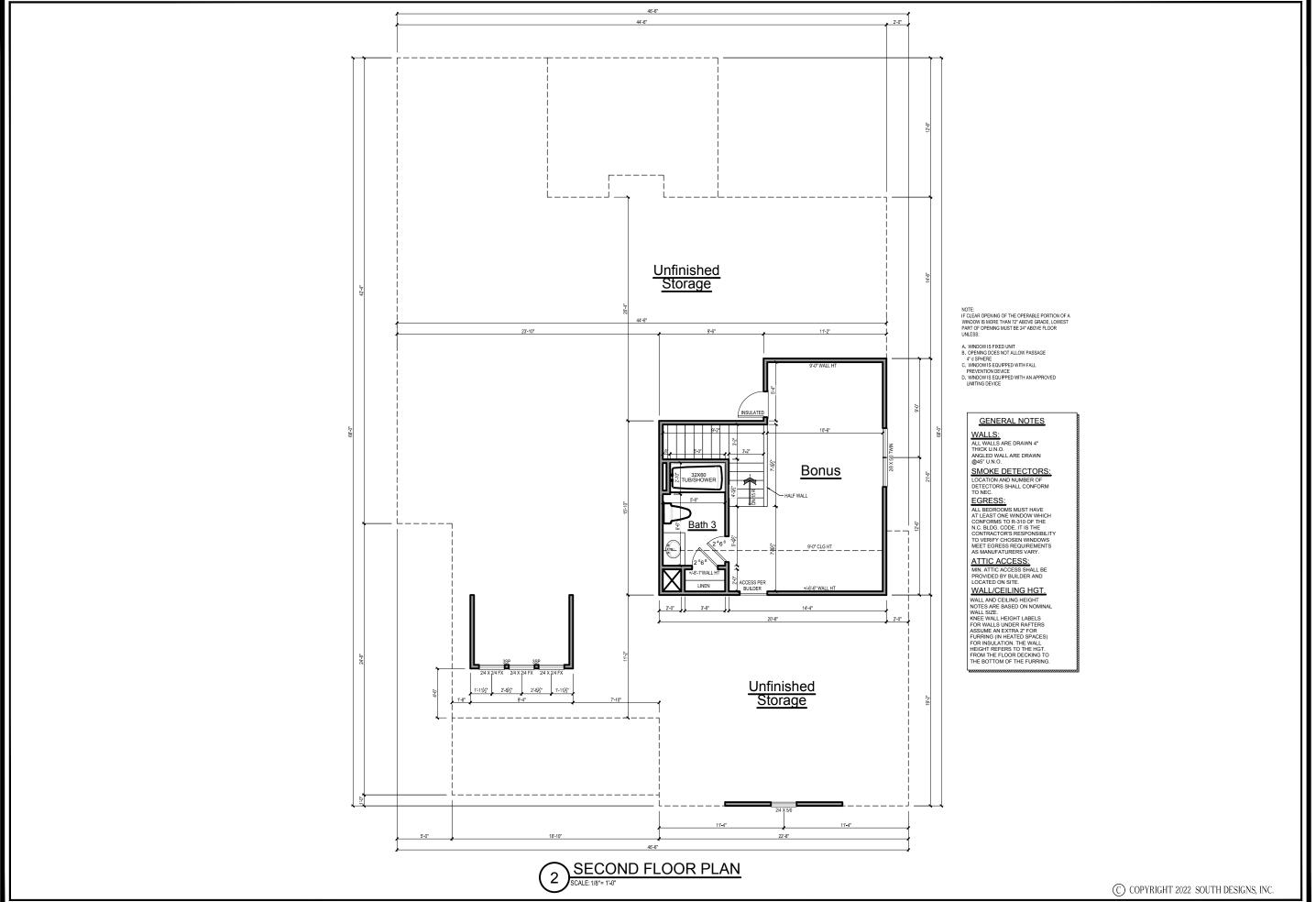
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Triple A Homes

FIRST **FLOOR OPTIONS**

Plan No.

 $\underline{\mathbf{A}} - \mathbf{1.1}$



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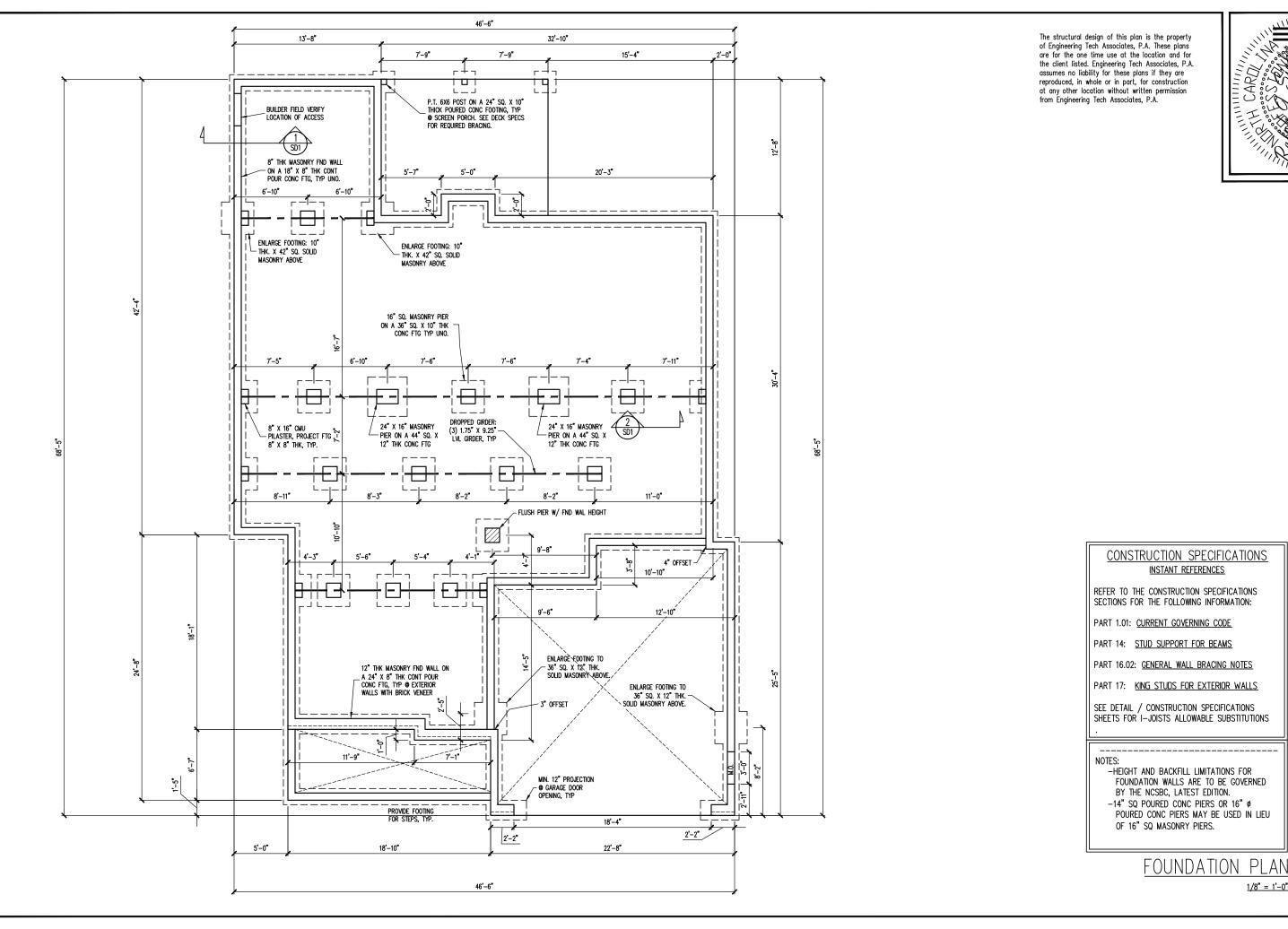
Client

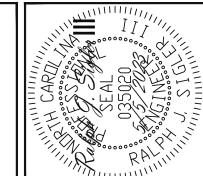
Triple A Homes

FIRST FLOOR PLAN

Plan No.

Sheet No. A-2





318 W / Raleigh,

STRUCTURAL ADDENDUM

- 1 23-66-779 5 9

ENG: RJS/CR

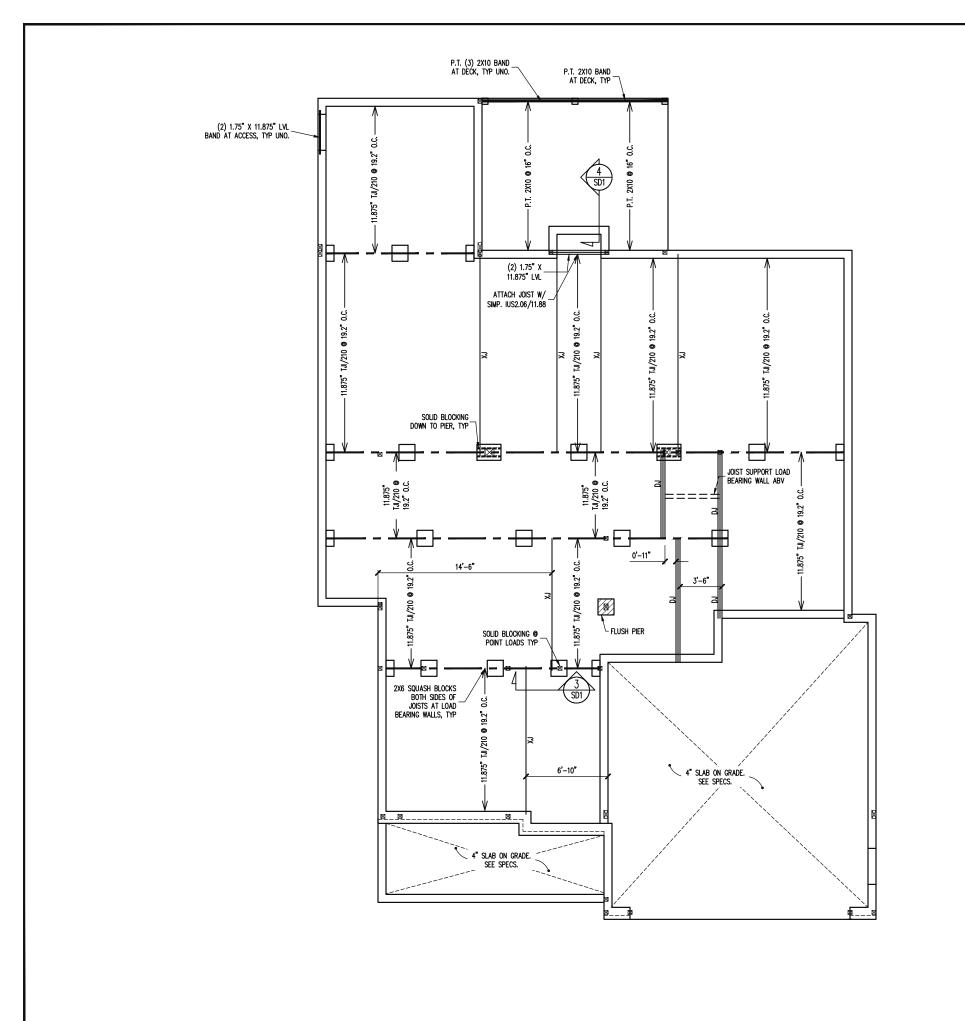
1/8" = 1'-0"

DATE: 5/5/2023

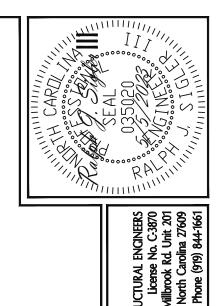
PROJECT NO. 23-28-008

SHEET NO.

S1 1 of 10



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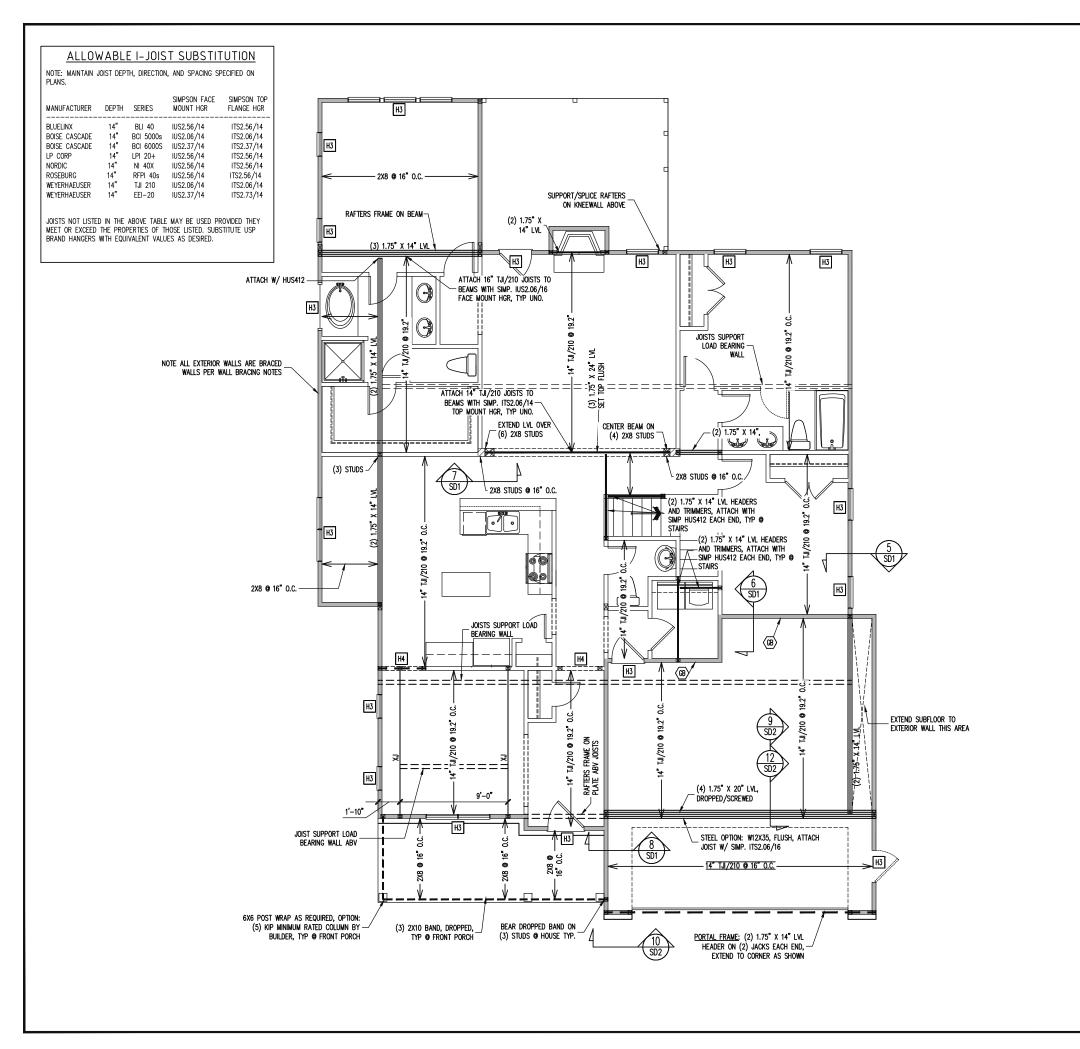
		STRU		318 W M	/ ech Ralaigh	- (1.0)-MI		٠,
]	/ech		4 018	ASSOCIATES, P.A.
			DATE	5-17-2023				
			REV # REF PROJ # DATE	23-66-179 5-17-2023				
	(ES	MDQN	REV #	-				
	TRIPLE A HOMES	STRUCTURAL ADDENDUM	16 COTTON FAPAS	IO COLLOIN LANNED				
		SCOPE	٢	ا ا				

CRAWLSPACE FRAMING PLAN

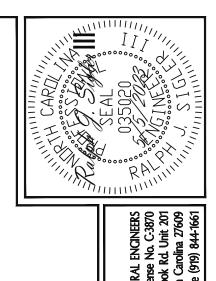
<u>1/8" = 1'-0"</u>

PROJECT NO. 23-28-008

ENG: RJS/CR DATE: 5/5/2023



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318 W / Raleigh,

TRIPLE A HOMES
STRUCTURAL ADDENDUM

- 10145

ENG:

9

RJS/CR

DATE: 5/5/2023

WALL BRACING

SHADED WALLS:

ALL EXTERIOR STUD WALLS, EXTERIOR SIDE, ARE TO BE CONTINUOUSLY SHEATHED WITH 7/16 APA RATED OSB NAILED TO STUDS WITH 8d NAILS © 6" O.C. AT PANEL EDGES, 12" O.C. IN PANEL FIELD.

NOTES:

PROVIDED CONTINUOUS SHEATHING = 229' MIN.

REFERENCE PART 16.02 OF CONSTRUCTION SPECIFICATIONS FOR GENERAL WIND BRACING INFORMATION.

HEADER SCHEDULE

- H1 SINGLE 2X4 TURNED FLAT (A)
- H2 (2) 2X4'S ON SINGLE JACKS (B)
- H3 (2) 2X10'S ON SINGLE JACKS (C)
- H4 (2) 1.75" X 9.25" LVL'S ON DBL JACKS
- H5 (3) 2X10'S ON SINGLE JACKS
- (A) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPENING 38" MAX.
- (B) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPNG 38" TO 74" MAX.
- (C) TYPICAL FOR ALL CONDITIONS NOT LISTED IN (A) OR (B) UNO.

NOTES:

-HEADERS IN NON LOAD BEARING INTERIOR WALLS ARE NOT LABELED.

1ST FLOOR FRAMING PLAN

WALLS AND CEILING $\frac{1/8"}{1} = 1'-0"$

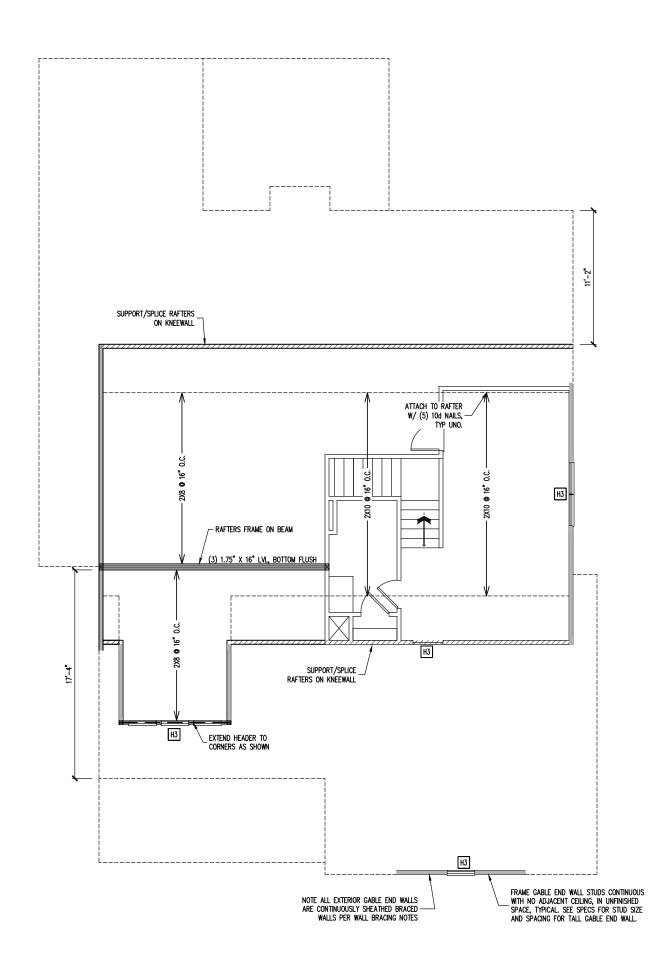
SHEET NO.

\$3

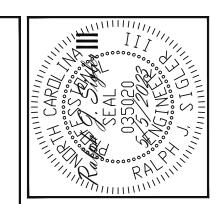
3 of 10

PROJECT NO.

23-28-008



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WALL BRACING

SHADED WALLS:

ALL EXTERIOR STUD WALLS, EXTERIOR SIDE, ARE TO BE CONTINUOUSLY SHEATHED WITH 7/16 APA RATED OSB NAILED TO STUDS WITH 8d NAILS @ 6" O.C. AT PANEL EDGES, 12" O.C. IN PANEL FIELD.

NOTES

PROVIDED CONTINUOUS SHEATHING = 30' MIN.

REFERENCE PART 16.02 OF CONSTRUCTION SPECIFICATIONS FOR GENERAL WIND BRACING INFORMATION.

HEADER SCHEDULE

- H1 SINGLE 2X4 TURNED FLAT (A)
- H2 (2) 2X4'S ON SINGLE JACKS (B)
- H3 (2) 2X10'S ON SINGLE JACKS (C)
- H4 (2) 1.75" X 9.25" LVL'S ON DBL JACKS
- H5 (3) 2X10'S ON SINGLE JACKS
- (A) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPENING 38" MAX.
- (B) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPNG 38" TO 74" MAX.
- (C) TYPICAL FOR ALL CONDITIONS NOT LISTED IN (A) OR (B) UNO.

|| NOTES

-HEADERS IN NON LOAD BEARING INTERIOR WALLS ARE NOT LABELED.

2ND FLOOR FRAMING PLAN

WALLS AND CEILING $\frac{1/8"}{1} = 1'-0"$

TRUCE TO STRUCE TO STRUCE

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TRIPLE A HOMES	SCOPE STRUCTURAL ADDENDUM	I OC 16 COTTON EARMS				
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PROJECT NO.

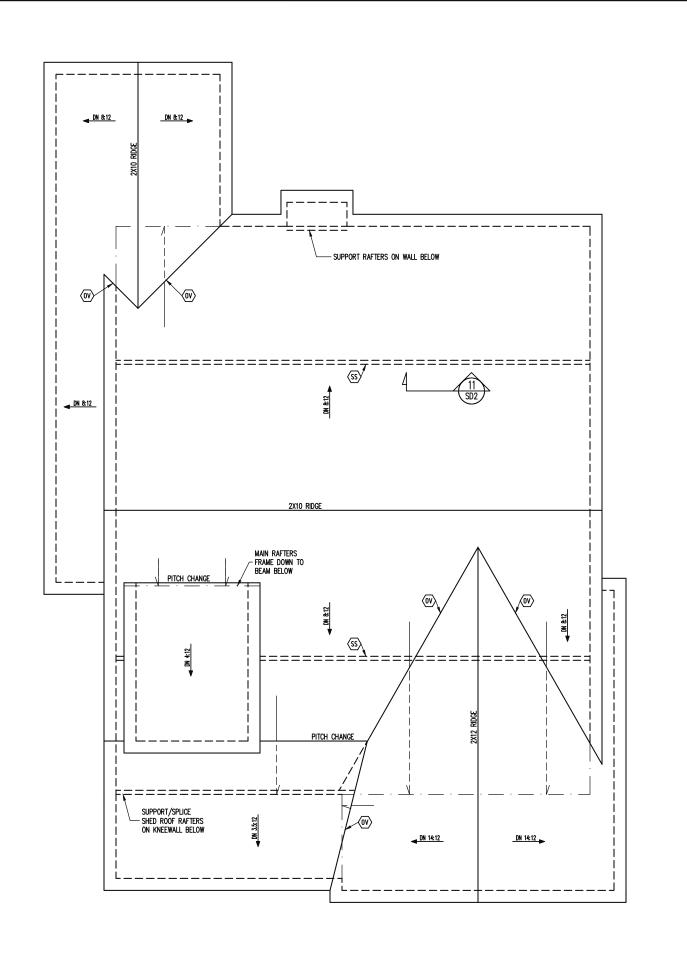
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DATE: 5/5/2023

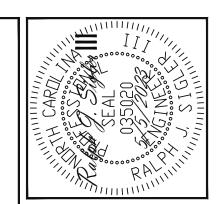
SHEET NO.

S4

4 of 10



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STRUCTURAL ADDENDUM

16 COTTON FARMS

1 23-66-79 5

16

ENG: RJS/CR

DATE: 5/5/2023

FRAMING NOTES ROOF ONLY

-COMMON RAFTERS 2X8 @ 16" O.C. TYP U.N.O. -COLLAR TIES 2X4 EVERY 3RD SET OF RAFTERS TYP U.N.O.

-verify roof pitches, overhang lengths, and kneewall framing hgts with architectural drawings, typical.

FRAMING SCHEDULE ROOF ONLY

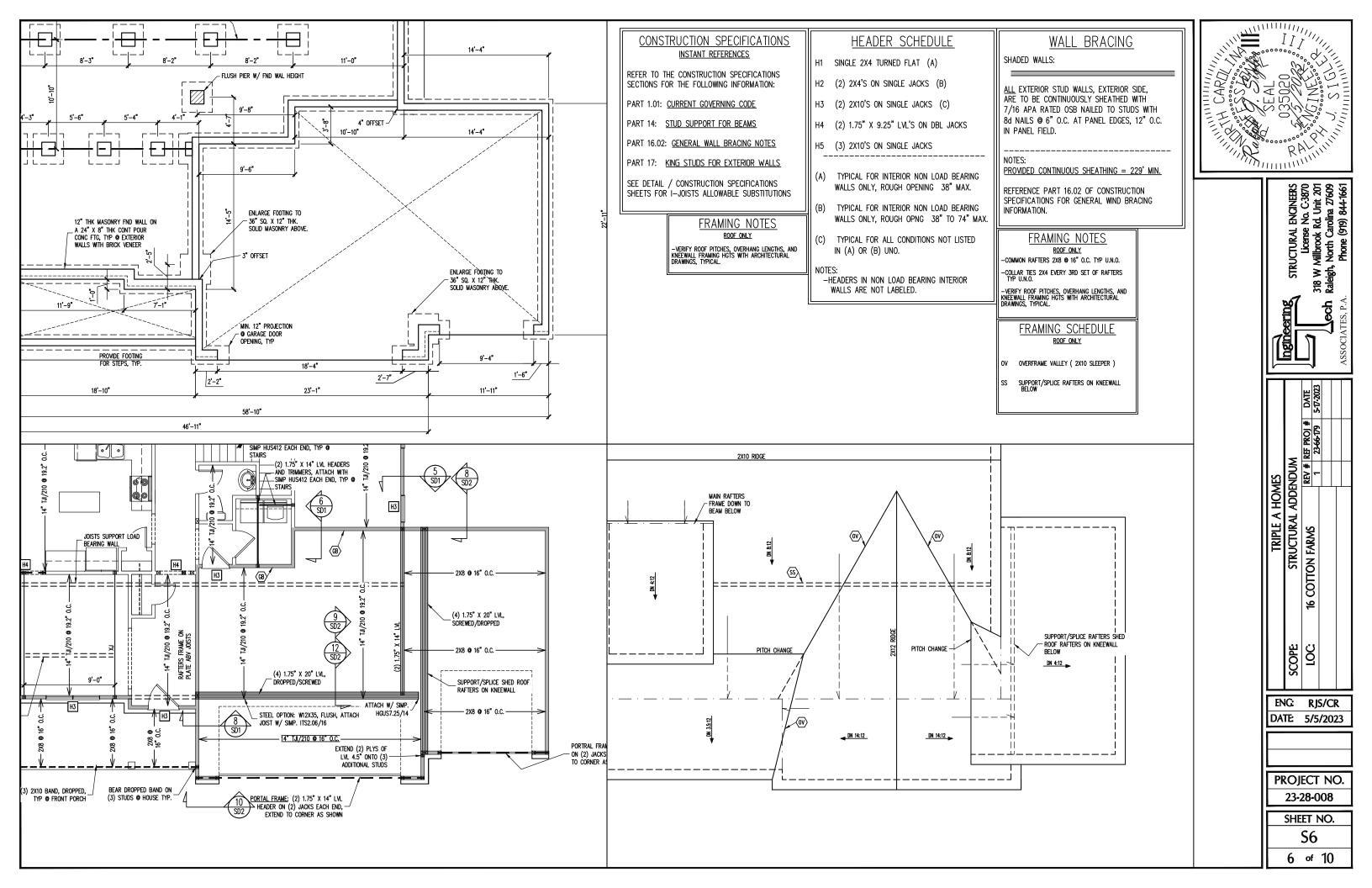
OVERFRAME VALLEY (2X10 SLEEPER)

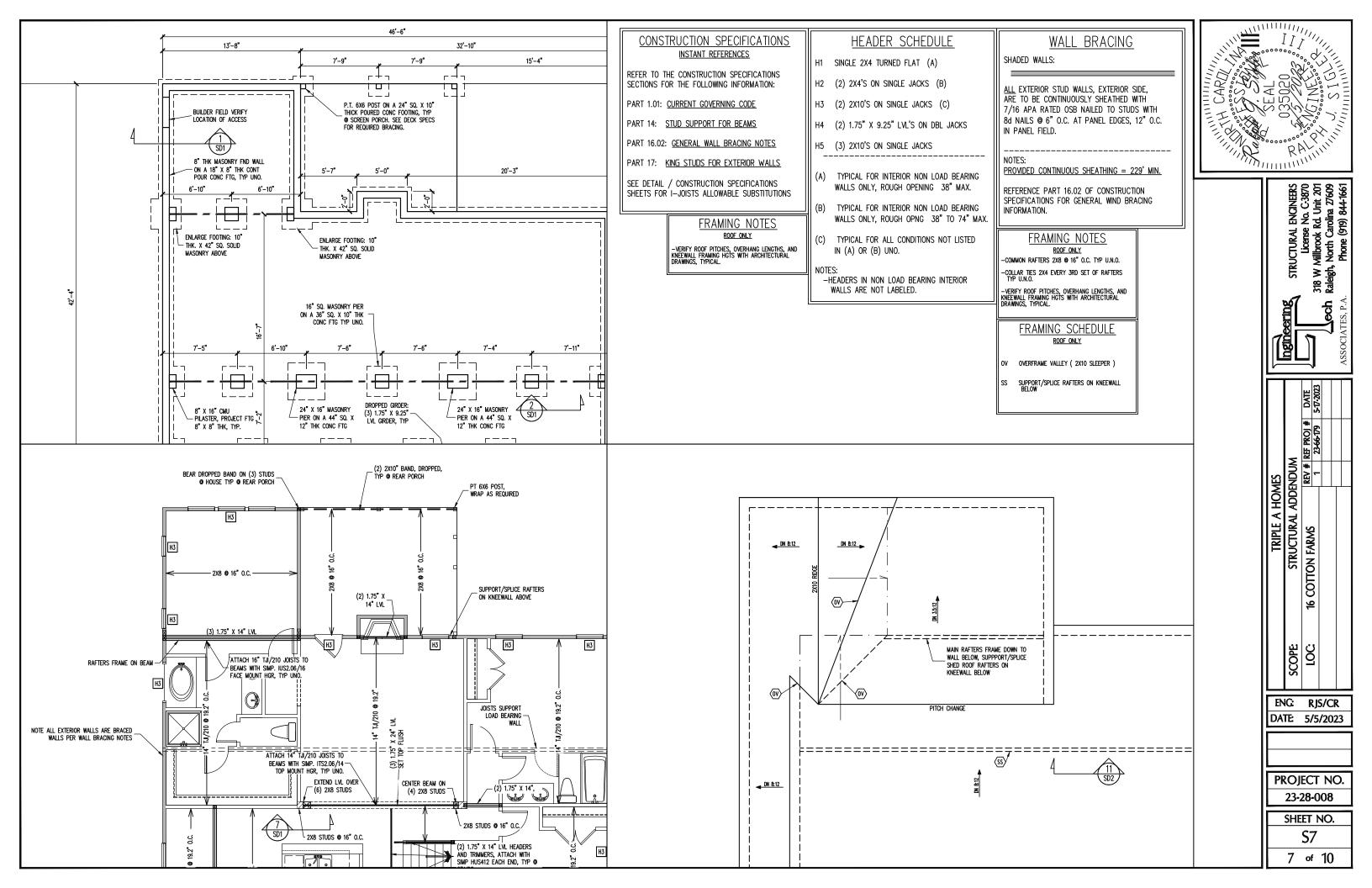
SUPPORT/SPLICE RAFTERS ON KNEEWALL BELOW

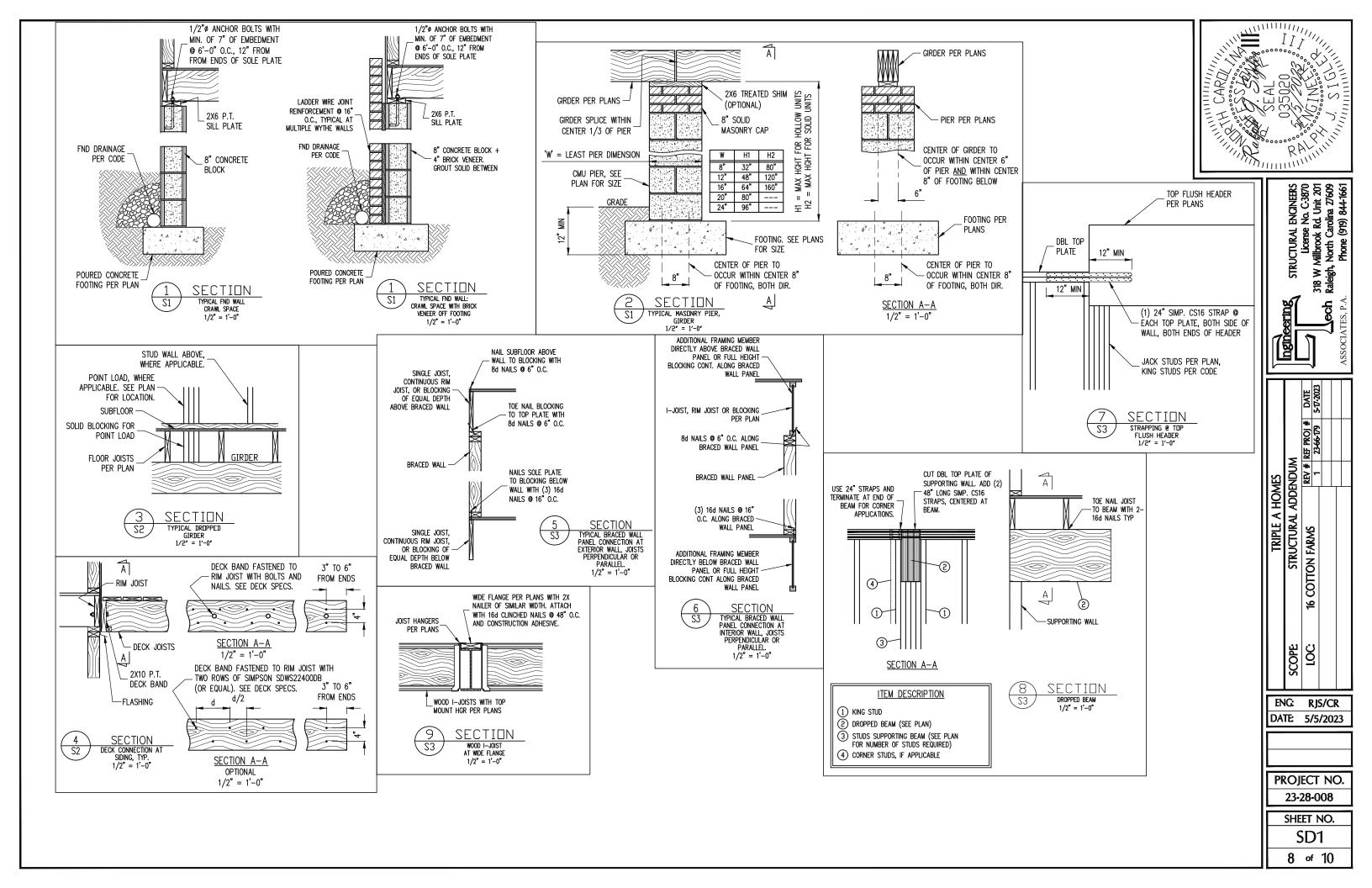
ROOF FRAMING PLAN

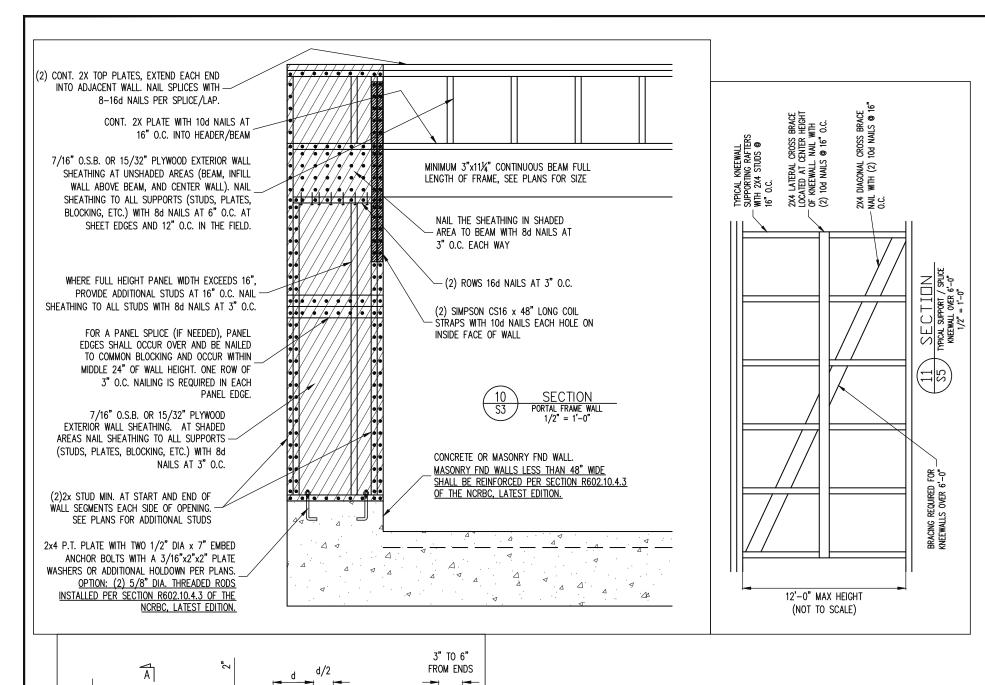
23-28-008 SHEET NO. **S5** 5 of 10

PROJECT NO.









__ (4) PLY LVL

SIMPSON SDW22634 TOP

SECTION

TYPICAL FASTENING (4)

PLY LVL 1/2" = 1'-0"

S3

AND BOTTOM. d = 16"

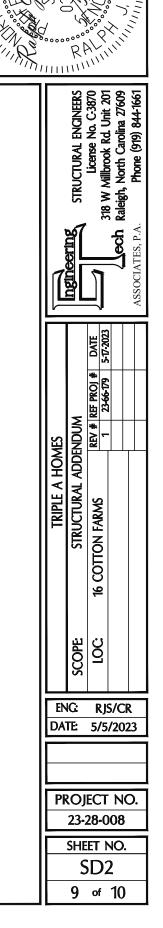
SIMPSON SDW22634

@ EQUAL SPACING.

 $\frac{\text{SECTION } A - A}{1/2" = 1' - 0"}$

*BUILDER PERMITTED TO SUBSTITUTE 1/2" Ø

THROUGH BOLTS FOR SIMP. SDW SCREWS



CONSTRUCTION SPECIFICATIONS

PART 1: GENERAL

- CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.
- 1.02 DIMENSIONS SHOWN SHALL GOVERN OVER SCALE ON THESE DRAWINGS.
- 1.05 METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR, WHO SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND INSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.

 7.04

PART 2: DESIGN LOADS

2.01 DESIGN LOADS SHALL CONFORM WITH THE TABLE BELOW:

BALCONIES, DECKS, ATTICS WITH FIXED STAIR ACCESS, DWELLING UNITS INCLUDING ATTICS WITH FIXED STAIR ACCESS, STAIRS, FIRE ESCAPES	40	10

LIVE LOAD (PSF) DEAD LOAD (PSF)

GARAGES (PASSENGER CARS ONLY) 50 ATTICS (NO STORAGE, LESS THAN 5' HEADROOM' 10 10 ATTICS (WITH STORAGE) 10 20 ROOF 20 10 (15 FOR VAULTS)

- INDIVIDUAL STAIR TREADS ARE TO BE DESIGNED FOR THE UNIFORMLY DISTRIBUTED LIVE LOAD OF 40 PSF OR A 300 LB. CONCENTRATED LOAD ACTING OVER AN AREA OF 4 SQ. WHICHEVER PRODUCES THE GREATER STRESS. NOTES:
 - OF 4 36. MINUTE ATT PRODUCES THE OREALEX STRESS.

 BUILDER TO VERIFY DEAD LOAD DOES NOT EXCEED 10 PSF WHEN HEAVY FLOOR OR ROOF FINISHES SUCH AS TILE OR SLATE ARE UTILIZED. NOTIFY ENGINEERING UNDER TRUSCE CAMPITATION.
- 2.02 INTERIOR WALLS: 5 PSF LATERAL.
- 2.03 BASIC WIND DESIGN VELOCITY OF 120 MPH.
- 2.04 SOIL BEARING CAPACITY 2000 PSF (PRESUMPTIVE).

PART 3: STRUCTURAL STEEL

- 3.01 WIDE_FLANGE BEAMS AND TEE SECTIONS SHALL CONFORM TO ASTM A992 MINIMUM
- 3.02 SQUARE AND RECTANGULAR TUBING SHALL CONFORM TO ASTM A500 GRADE B MINIMUM GRADE.
- 3.03 STEEL PIPE SHALL CONFORM TO ASTM A53 GRADE B, TYPE S, MINIMUM GRADE
- 3.04 ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 MINIMUM GRADE
- 3.05 STRUCTURAL STEEL CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.

PART 4: WELDING

4.01 WELDING ELECTRODES SHALL BE E70XX AND ALL WELDING SHALL BE PERFORMED BY AN AWS CERTIFIED WELDER

PART 5: CONCRETE AND SLABS ON GRADE

- CAST IN PLACE CONCRETE SHALL BE OF NORMAL WEIGHT, 4-6% AIR ENTRAINMENT, FOR EXTERIOR CONCRETE AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS TYP UNO. ALL ITEMS NOTED AS 'CONCRETE' ARE TO BE CAST IN PLACE,
- 5.02 REINFORCED CAST IN PLACE CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACED IN ACCORDANCE WITH THE SPECIFICATIONS OF ACI 318, LATEST EDITION.
- 5.03 SLABS ON GRADE, IF ANY, SHALL BE CAST IN PLACE, CONTAIN SYNTHETIC POLYPROPYLENE FIBRILLATED MICRO FIBERS, FIBER LENGTH 1 1/2", DOSAGE RATE 1 1/2 LBS/CU YD. SLAB TO BE PLACED ON A 6 MIL VAPOR BARRIER ON 4" MIN GRANULAR FILL ON SOIL WITH 90% MIN STANDARD PROCTOR DENSITY. VAPOR BARRIER MAY BE OMITTED FOR SLABS NOT IN ENCLOSED AREAS

PART 6: REBAR AND WIRE REINFORCEMENT

- REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615 GRADE 60 TYP UNO 6.01
- LAP SPLICES SHALL BE CLASS B AS DEFINED BY ACI 318, TYP UNO
- 6.03 WIRE REINFORCEMENT SHALL BE 9 GA AND SHALL CONFORM TO ASTM A1064.

PART 7: MASONRY

7.01 CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND C55, NORMAL WEIGHT,

fM = 1.500 PSI MIN

- 7.02 CLAY MASONRY UNITS SHALL CONFORM TO ASTM C62-17 GRADE SW
- MORTAR SHALL BE TYPE S. MORTAR AND GROUT SHALL CONFORM TO ASTM C476, MIN COMPRESSIVE STRENGTH OF 2000 PSI.
- MASONRY CONSTRUCTION SHALL CONFORM TO THE SPECIFICATIONS OF ACI 530
- LADDER WIRE REINFORCEMENT SHALL CONFORM TO ASTM A951. 6" MIN LAPS FOR CONTINUOUS WALL APPLICATIONS

PART 8: BOLTS AND LAG SCREWS

- BOLTS SHALL CONFORM TO ASTM A307 MINIMUM GRADE TYP UNO. INSTALL STANDARD STEEL WASHERS (ASTM F844-07a) FOR THE NUT / BOLT HEAD WHEN BOLTING WOOD MEMBERS. HOLES FOR BOLTS SHALL BE AISC STANDARD HOLES UNO
- 8.02 LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1-1981. PILOT HOLES SHALL BE USED FOR LAG SCREW INSTALLATION AND SHALL BE BORED ACCORDING TO NDS SPECIFICATIONS. INSTALL STANDARD STEEL WASHERS (ASTM F844-070) FOR
- 8.03 ANCHOR RODS AND BOLTS SHALL CONFORM TO ASTM F1554-15 GRADE 36 UNO. BENT ANCHOR BOLTS SHALL HAVE A 2" MIN HOOK UNO

PART 9: DRIVEN FASTENERS

NAILS, SPIKES AND STAPLES SHALL CONFORM TO ASTM F 1667- 05. NAILS ARE TO BE COMMON WIRE OR BOX

PART 10: DIMENSIONAL LUMBER

10.01 Solid sawn wood framing design is based on no. 2 spruce pine fir \underline{or} syp #2 for joists, rafters, girders, beams, studs, etc.

PART 11: ENGINEERED LUMBER

- LVL OR PSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS: E= 1.9 X 10E6 PSI, Fb = 2600 PSI, Fv = 285 PSI, Fc = 750 PSI LSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS: E= 1.3 X 10E6 PSI, Fb = 1700 PSI, Fv = 400 PSI, Fc = 680 PSI
- LVL OR PSL MEMBERS MAY BE RIPPED FROM DEEPER MEMBERS TO MATCH THE MEMBER DEPTH SPECIFIED IN THE PLANS

PART 12: PRESSURE TREATED LUMBER

LUMBER IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA STANDARD C-15. ALL OTHER EXPOSED LUMBER SHALL BE TREATED IN ACCORDANCE WITH AWPA STANDARD C-2 OR BY ANY METHOD GIVING EQUAL PROTECTION. THE BUILDING CODE OFFICE MAY ALSO APPROVE A NATURAL DECAY RESISTANT WOOD PER SECTION 19-6(A)

PART 13: STEEL FLITCH PLATE BEAMS

FLITCH PLATE BEAMS SHALL CONSIST OF A CONTINUOUS STEEL PLATE BOLTED BETWEEN TWO PIECES OF CONTINUOUS LUMBER AS SIZED ON THE PLANS. BOLT PIECES TOGETHER USING 1/2" @ BOLTS SPACED AT 16" O.C. STAGGERED TOP TO BOTTOM OF THE BEAM. 13.01 MAINTAIN A 2" EDGE DISTANCE. PLACE TWO BOLTS, ONE ABOVE THE OTHER, 16" MAX FROM EACH END OF THE BEAM TYP UNO

PART 14: STUD SUPPORTS FOR BEAMS

- 14.01 STEEL, ENGINEERED LUMBER, AND FLITCH PLATE BEAMS BEARING ON A STUD WALL
- 1-WHEN THE REAM IS PERPENDICULAR TO OR SKEWED RELATIVE TO THE WALL. THE REAM SHALL BEAR SUL WITH ON THE SUPPORTING WALL INDICATED AND SHALL BE SUPPORTED BY A MINIMUM OF THREE GANGED STUDS, OR A GANGED STUD COLUMN WITH A NUMBER OF STUDS SUCH THAT THE STUD COLUMN IS AT LEAST AS WIDE AS THE TRUE WIDTH OF THE BEAM BEING SUPPORTED, WHICHEVER IS GREATER, TYP UND. FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON THE BEAM
- 2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 4 1/2" ONTO THE WALL AND BE SUPPORTED BY A TRPL STUD GANGED
- 4.02 DIMENSIONAL LUMBER BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:
- 1-when the beam is perpendicular to, or skewed relative to the wall, the beam shall bear <u>full width</u> on the supporting wall indicated (less 1 1/2" to allow for a continuous rim joist where applicable) and shall be supported by a GANGED STUD COLUMN THE SAME WIDTH AS THE BEAM TYP UNO. (E.G. A TRIPLE 2X10 IS TO BE SUPPORTED BY (3) STUDS), FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON THE BEAM 2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR

MINIMUM OF 3" ONTO THE WALL AND BE SUPPORTED BY A DBL STUD GANGED COLUMN

- 14.03 EXTRA JOISTS BEARING ON A STUD WALL PERPENDICULAR TO OR SKEWED RELATIVE TO THE BEAM SHALL BE SUPPORTED BY ONE ADDITIONAL STUD.
- STUDS THAT ARE CANCED TO FORM A COLLIMN SHALL HAVE ADJACENT STUDS WITHIN STUDIS THAT ARE GANGED TO FORM A COLUMN SHALL HAVE ADJACENT STUDS MITHIN THE COLUMN NALLED TOCETHER WITH ONE ROW OF 10d NAILS AT 8" O.C., (TWO ROWS OF 10d NAILS @ 8" O.C., 3" APART, FOR 2X8 OR 2X10 STUDS) ALL COLUMNS SHALL BE CONTINUOUS DOWN TO THE FOUNDATION OR OTHER PROPERLY DESIGNED STRUCTURAL ELEMENT SUCH AS A BEAM. COLUMNS TRANSFERRING LOADS THROUGH FLOOR LEYELS SHALL BE SOLIDLY BLOCKED FOR THE FULL WIDTH OF THE STUD COLUMN MATCH THE CANTY CONTINUES BY THE STUD COLUMN. THE CAVITY FORMED BY THE

PART 15: NAILING OF MULTI PLY WOOD BEAMS

- SOLID SAWN LUMBER JOISTS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM NAILED TOGETHER WITH THREE ROWS OF 10d NAILS 15.01 9 16" O.C. FOR 2X10 OR LARGER, TWO ROWS OF 10d NAILS @ 16" O.C. FOR 2X8, ONE ROW OF 10d NAILS @ 16" O.C. FOR 2X6 OR SMALLER, STAGGER ROWS 5" MIN.
- LVL MEMBERS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM FASTENED TOGETHER PER MANUFACTURERS RECOMMENDATIONS, TYP 15.02

PART 16: WALL FRAMING AND BRACING

STUD WALLS SHALL CONSIST OF 2X4 STUDS SPACED AT 16" O.C. UNO. STUDS SHALL BE CONTINUOUS FROM SOLE PLATE AT FLOOR TO DOUBLE TOP PLATE AT THE CEILING OR ROOF. NO INTERMEDIATE BANDS OR PLATES SHALL CAUSE DISCONTINUITES IN A STUD WALL EXCEPT AS REQUIRED FOR DOOR OR WINDOW OPENINGS. THE KING STUDS FOR SUCH OPENINGS SHALL BE CONTINUOUS, TYP UNO.

MAX ALLOWABLE WALL HEIGHTS FOR EXTERIOR STUD WALLS, INCLUSIVE OF SOLE IN A TABLE OF THE PLATE AND DOES TO BE AT THE ART OF THE PLATE AND DOES TO BE AT THE ART OF THE PLATE AND DOES TO BE AT THE ART OF THE PLATE AND DOES TO BE AT THE ART OF THE PLATE AND DOES TO BE AT THE PLA 16.01

MAX ALLOWABLE WALL HEIGHTS FOR EXTENDOR STOLD WALLS, INCLUSIVE OF SOLE PLATE AND DEL TOP PLATE AND 7/16" OSB EXTERIOR BRACING AND ROW OF 2X4 2X6 PURLINS AT 8" HEIGHT (AND AT 16" HEIGHT FOR TALL WALLS), TYP UNO: 2X4 @ 16" O.C.: 11"-11/2" 2X6 @ 16" O.C.: 17"-0" 2X4 @ 12" O.C.: 12"-11/2" 2X6 @ 16" O.C.: 18"-8" DBL 2X4 @ 16" O.C.: 13"-4" DBL 2X6 @ 16" O.C.: 21"-0"

16.02 FOR WALL BRACING THE FOLLOWING SHALL APPLY:

-BLOCKING AT UNSUPPORTED PANEL EDGES IS REQUIRED TYP UNO.

-WALL BRACING IS BY ENGINEERED DESIGN AND NOT PRESCRIPTIVE PER SECTION
602.10 OF THE 2018 NCRC. CONTINUOUS SHEATHING HAS BEEN PROVIDED, ALONG
WITH ALTERNATIVE METHODS TO INSURE THE MINIMUM INTENT OF SECTION 602.10
OF THE 2018 NCRC HAS BEEN MET AND EXCEEDED.

-BRACED WALL PANELS SHALL BE FASTENED IN ACCORDANCE WITH TABLE 602.3(1) TO
PROVIDE CONTINUOUS PANEL INJUST OF SECTIANCE AND COMBINANCE WITH MORE OF THE 10.10 PROVIDED CONTINUOUS PANEL INJUST OF SECTIANCE AND COMBINANCE WITH MORE OF THE 10.10 PROVIDED CONTINUOUS PANEL INJUST OF SECTIANCE AND COMBINANCE WITH MORE OF THE PROVIDED CONTINUOUS PANEL INJUST OF SECTIANCE AND COMBINANCE WITH MORE OF THE PROVIDED CONTINUOUS PANEL INJUST OF SECTIANCE AND COMBINANCE WITH MORE OF THE PROVIDED CONTINUOUS PANEL INJUST OF SECTIANCE AND COMBINANCE WITH MORE OF THE PROVIDED CONTINUOUS PANEL INJUST OF SECTIANCE AND COMBINANCE WITH MORE OF THE PROVIDED CONTINUOUS PANEL INJUST OF SECTIANCE AND COMBINANCE WITH MORE OF THE PROVIDED CONTINUOUS PANEL INJUST OF SECTIANCE AND COMBINANCE WITH MORE OF THE PROVIDED CONTINUOUS PANEL INJUST OF SECTION CONTINUOUS PANEL INJUST OF SECTION CONTINUOUS PANEL TO SECTIO

PROVIDE CONTINUOUS PANEL UPLIT RESISTANCE AND COMPLIANCE WITH NCRBC R602.3.5 AND R802.11 UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS.

-MAY SUBSTITUTE WSP FOR GB
-SINGLE JOIST, CONTINUOUS RIM JOIST, OR BLOCKING OF EQUAL DEPTH IS REQUIRED
ABOVE AND BELOW ALL BRACED WALLS. NAIL BLOCKING ABOVE WALL TO TOP PLATE
WITH 16d TOE NAILS @ 6" O.C. NAIL SOLE PLATE OF BRACED WALL TO BLOCKING BELOW WITH (3) 16d NAILS @ 16" O.C. BLOCKING AT HORIZONTAL JOINTS IN BRACED WALL LINES ONLY REQUIRED AT SHADED WALLS, UNO.

17.01 KING STUDS FOR OPENINGS IN EXTERIOR WALLS SHALL BE AS FOLLOWS:

			NOWRF	R OF KIN	IG STUDS	
MAX OPENIN	G WIDTH	5'-0"	9'-0"	13'-0"	17'-0"	21'-0
	2X4	1	2	3	4	5
Stud Size	2X6	1	1	2	2	2
	2X8	1	1	1	1	2

PART 18: SUBSTITUTIONS

18.01 MATERIAL OR MEMBER SIZE SUBSTITUTIONS OR PLAN DEVIATIONS REQUIRE THE WRITTEN AUTHORIZATION OF THE DESIGNERS. UNAUTHORIZED DEVIATIONS ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

PART 19: OWNERSHIP OF STRUCTURAL DESIGN

19.01 THE STRUCTURAL DESIGN OF THIS PLAN IS THE PROPERTY OF ENGINEERING TECH ASSOCIATES (ETA). THESE PLANS ARE FOR THE ONE TIME USE AT THE LOCATION INDICATED AND FOR THE CLIENT LISTED. ETA ASSUMES NO LIABILITY FOR THESE PLANS IF THEY ARE REPRODUCED, IN WHOLE OR IN PART, FOR CONSTRUCTION AT ANY OTHER LOCATION WITHOUT WRITTEN PERMISSION FROM ETA

DECK SPECIFICATIONS

- A DECK IS AN EXPOSED EXTERIOR WOOD FLOOR STRUCTURE WHICH MAY BE ATTACHED TO A STRUCTURE OR BE FREE STANDING. ROOFED PORCHES, OPEN OR SCREENED IN, MAY BE CONSTRUCTED LISING THESE PROVISIONS
- SUPPORT POSTS SHALL BE SUPPORTED BY A FOOTING.
- WHEN ATTACHED TO A STRUCTURE, THE STRUCTURE TO WHICH ATTACHED SHALL HAVE A TREATED WOOD BAND FOR THE LENGTH OF THE DECK, OR CORROSION RESISTANT FLASHING SHALL BE USED TO PREVENT MOISTURE FROM COMING IN CONTACT WITH THE UNTREATED FRAMING OF THE STRUCTURE. THE DECK BAND AND THE STRUCTURE BAND SHALL BE CONSTRUCTED IN CONTACT WITH FACH OTHER EXCEPT AT BRICK VENEER AND WHERE PLYWOOD SHEATHING IS REQUIRED AND PROPERLY FLASHED. SIDING SHALL NOT BE INSTALLED BETWEEN THE STRUCTURE AND THE DECK BAND. IF ATTACHED TO A BRICK STRUCTURE, NEITHER FLASHING NOR A TREATED BAND FOR THE BRICK STRUCTURE IS REQUIRED. IN ADDITION, THE TREATED DECK BAND SHALL BE CONSTRUCTED IN CONTACT
- WHEN THE DECK IS SUPPORTED AT THE STRUCTURE BY ATTACHING THE DECK TO THE STRUCTURE. THE FOLLOWING ATTACHMENT SCHEDULES SHALL APPLY FOR ATTACHING THE

A. ALL STRUCTURES EXCEPT BRICK STRUCTURES

	JOIST LENGTH			
	UP TO 8' MAX.	UP TO 16' MAX.		
REQUIRED FASTENERS	ONE- 5/8" Ø BOLT @ 42" O.C. AND (2) ROWS OF 12d NAILS @ 8" O.C. OR TWO ROWS OF SIMPSON SDWS22400DB @ d = 32" O.C. STAGGERED	ONE- 5/8" # BOLT @ 20" O.C. AND (3) ROWS OF 12d NAILS @ 6" O.C. OR TWO ROWS OF SIMPSON SDWS22400DB @ d = 16" O.C. STAGGERED	ATIONS	
A . BRICK VENEER STRUCTURES				

A . BRICK VENEER STRUCTURES

	JOIST LENGTH		
	UP TO 8' MAX.	UP TO 16' MAX.	
REQUIRED FASTENERS	ONE- 5/8" Ø BOLT ② 28" O.C.	ONE- 5/8" Ø BOLT @ 16" O.C.	

- IF THE DECK BAND IS SUPPORTED BY A 1/2" MINIMUM MASONRY LEDGE ALONG THE FOUNDATION WALL, 5/8" Ø BOLTS SPACED @ 48" O.C. MAY BE USED FOR SUPPORT.
- OTHER MEANS OF SUPPORT, SUCH AS JOIST HANGERS, MAY BE USED TO CONNECT DECK JOISTS TO A TREATED STRUCTURE BAND
- GIRDERS SHALL BEAR DIRECTLY ON POSTS OR BE BE CONNECTED TO THE SIDES OF POSTS WITH 2- 5/8" Ø BOLTS
- FLOOR DECKING SHALL BE NO. 2 GRADE TREATED SOUTHERN PINE OR EQUIVALENT. THE MINIMUM FLOOR DECKING THICKNESS SHALL BE AS FOLLOWS:

JOIST SPAN	DECKING
12" O.C.	1" S4S
16" O.C.	1" T&G
24" O.C.	1 1/4" S4S
32" O.C.	2" S4S

MAXIMUM HEIGHT OF DECK SUPPORT POSTS IS AS FOLLOWS:

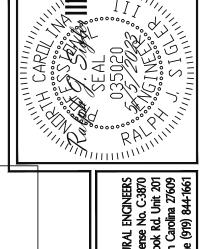
POST SIZE	MAX POST HEIGHT
4X4	8'
6X6	20'
ENGINEERED	20' +

- NOTES: 1) THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. 2) THIS TABLE IS BASED ON A MAXIMUM TRIBUTARY AREA OF 128 SQ. FT. 3) POST HEIGHT IS FROM TOP OF FOOTING TO BOTTOM OF GIRDER.
- 10. DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THE FOLLOWING
- A. WHEN THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION 4, LATERAL BRACING IS NOT REQUIRED.
- B. 4X4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL KNEE BRACES SHALL BE ATTACHED A THE ENDS TO THE GIRDER AND THE POST WITH ONE - 5/8" BOLT
- C. FOR FREE STANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN CONCRETE IN ACCORDANCE

1	POST SIZE	TRIBUT. AREA	POST HEIGHT	EMB. DEPTH	CONC. DIAM.
	4X4	48 SQ. FT.	4'-0"	2'-6"	1'-0"
	6X6	120 SQ. FT.	6'-0"	3'-6"	1'-8"

D. 2X6 DIAGONAL VERTICAL CROSS BRACING SHALL BE PROVIDED IN TWO PERPENDICULAR DIRECTIONS FOR FREE STANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE BRACES SHALL BE ATTACHED TO THE POSTS WITH ONE - 5/8" Ø BOLT AT EACH END OF THE BRACE.

- NOTES: 1) ALL NAILS AND BOLTS ARE TO BE HOT DIPPED GALVANIZED.
 - 2) MINIMUM EDGE DISTANCE FOR BOLTS IS 2 1/2".
 - 3) nails must penetrate the supporting structure band a minimum of 1 1/2".



STUD NOTE ISE JOIST

FOUNDATION
FOUNDE
HOT DIPPED
GALVANIZED
HANGER
LAMINATED
LAMINATED
ON TO SCALE
ON TO SCALE
ON CENTER
PARALLEL STRAND
LAMBER
PRESSURE TREATED
QUAD JOIST
SUNGE STUD POCKET
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WORKING PLANS C

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318 W / Raleigh,

ADDENDUM REV # RI TRIPLE A UCTURAL 8 9

ENG: RJS/CR DATE: 5/5/2023

PROJECT NO. 23-28-008

SHEET NO. SD3

10 of 10