PROJECT #: DRB2201-0065B

DATE: 07/13/2022 DRAWN BY:

CHECKED BY:

ELEVATION A

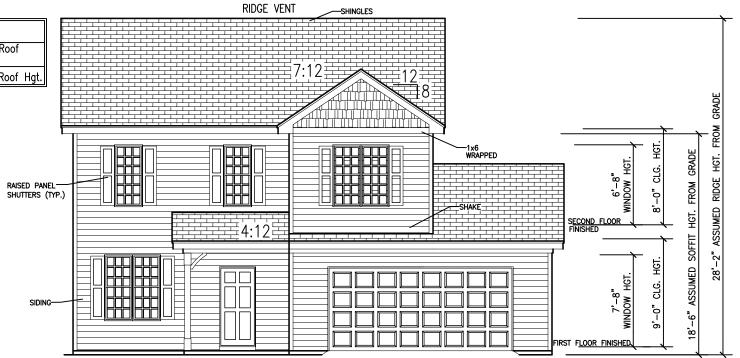
OF 9

ATTIC SPACE VENTILATION

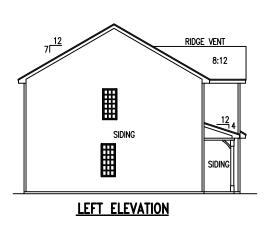
 $\underline{1016}$ SQ. FT. OF CLG. / 150 = $\underline{6.77}$ SQ. FT. REQUIRED

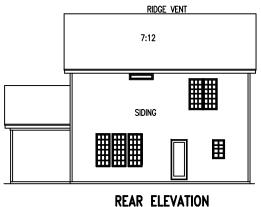
REFER TO SECTION R806 (ROOF VENTILATION) IN NORTH CAROLINA STATE 2018 INTERNATIONAL RESIDENTIAL BUILDING CODES.

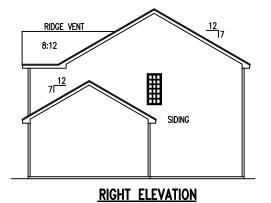
MEAN ROOF HGT.				
	+ Highest Ridge Hgt. From Assumed Grade	÷2		= Mean Roof Hgt.
18'-6"	+ 28'-2"	÷2	= 23'-4"	Mean Roof Hgt.



FRONT ELEVATION "A"







REAR ELEVATION

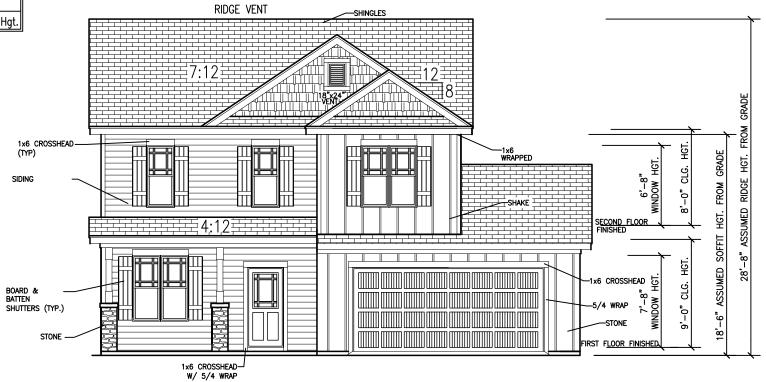
DRAWN BY:

ATTIC SPACE VENTILATION REQUIRED

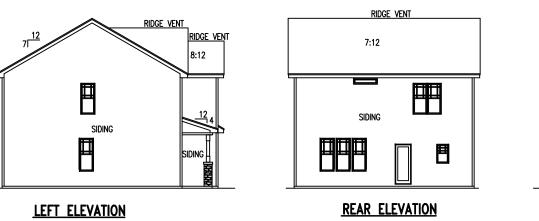
1062 SQ. FT. OF CLG. / 150 = 7.08 SQ. FT. REQUIRED

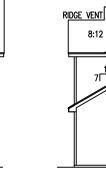
REFER TO SECTION R806 (ROOF VENTILATION) IN NORTH CAROLINA STATE 2018 INTERNATIONAL RESIDENTIAL BUILDING CODES.

MEAN ROOF HGT.						
Soffit Hgt. From Assumed Grade	+ Highest Ridge Hgt. From Assumed Grade	÷2		= Mean Hgt.	Roof	
18'-6"	+ 28'-2"	÷2	= 23'-4"	Mean	Roof	Hgt.



FRONT ELEVATION "B"





SIDING

RIGHT ELEVATION

= Mean Roof

Mean Roof Hgt.

ATTIC SPACE VENTILATION

1062 SQ. FT. OF CLG. / 150 = 7.08 SQ. FT. REQUIRED

REFER TO SECTION R806 (ROOF VENTILATION) IN NORTH CAROLINA STATE 2018 INTERNATIONAL RESIDENTIAL BUILDING CODES.

28'-2"

18'-6"

MEAN ROOF HGT.

÷2

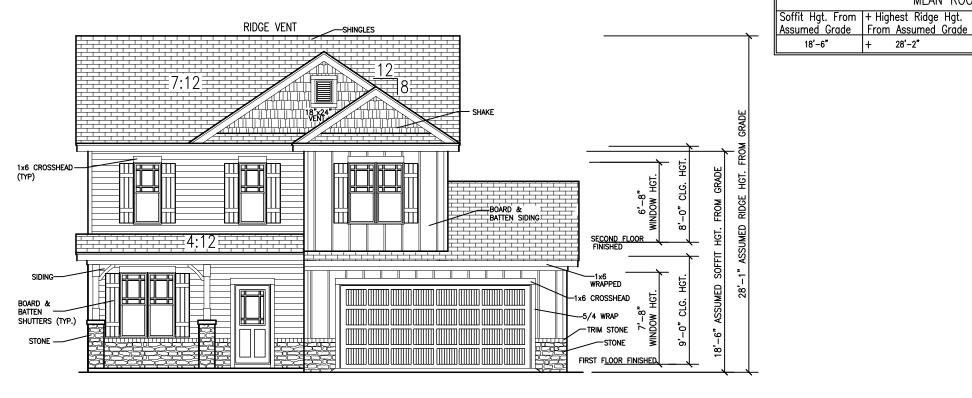
 $\div 2 = 23'-4"$

DATE: 07/13/2022

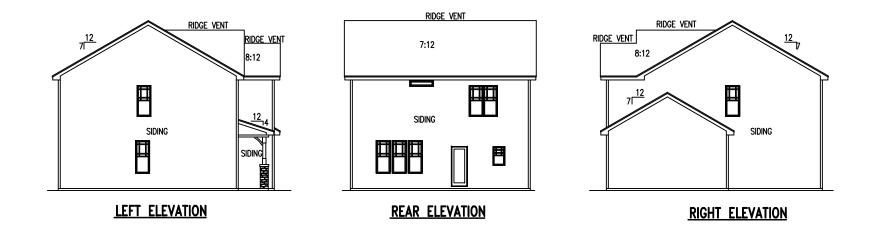
DRAWN BY: CHECKED BY:

ELEVATION C

OF 9



FRONT ELEVATION "C"

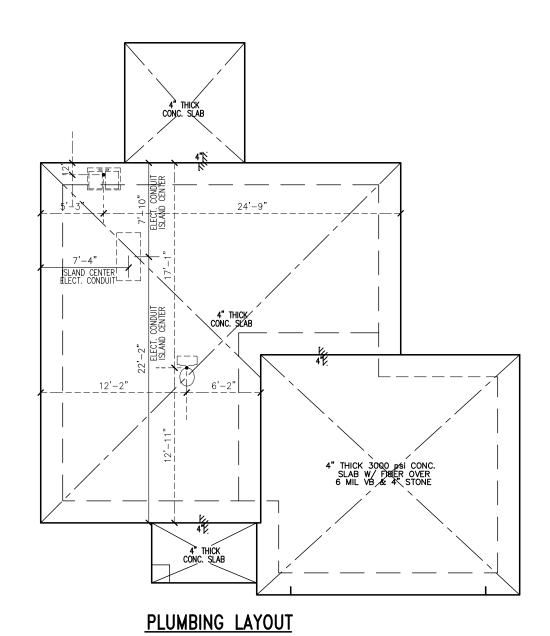


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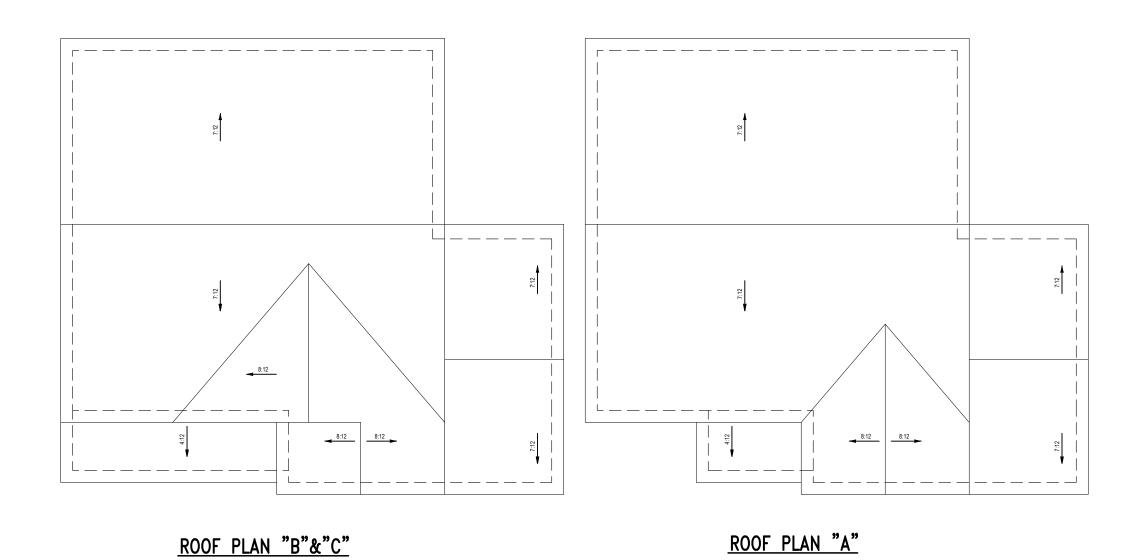
PLUMBING

CHECKED BY:





ROOF PLAN

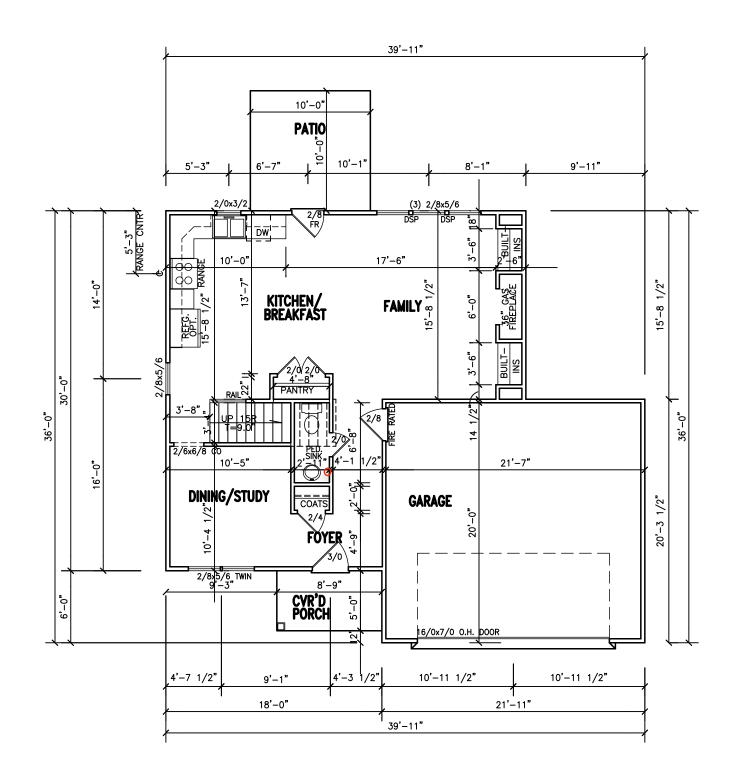


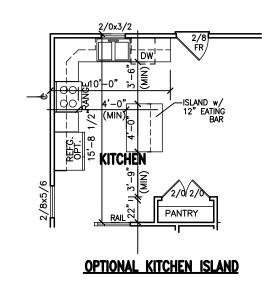
DATE: 07/13/2022 DRAWN BY:

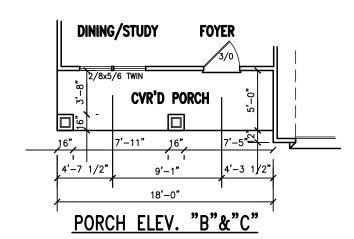
CHECKED BY:

FLOOR PLAN









SQUARE FOOTAGE	
1ST FLOOR HTD. 2ND FLOOR HTD.	737 942
FLOOR HTD.	1679
GARAGE PATIO ELV "A" PORCH ELV "B" PORCH ELV "C" PORCH	437 100 44 90 90
1ST FLOOR PLAN NOTE	· Q+

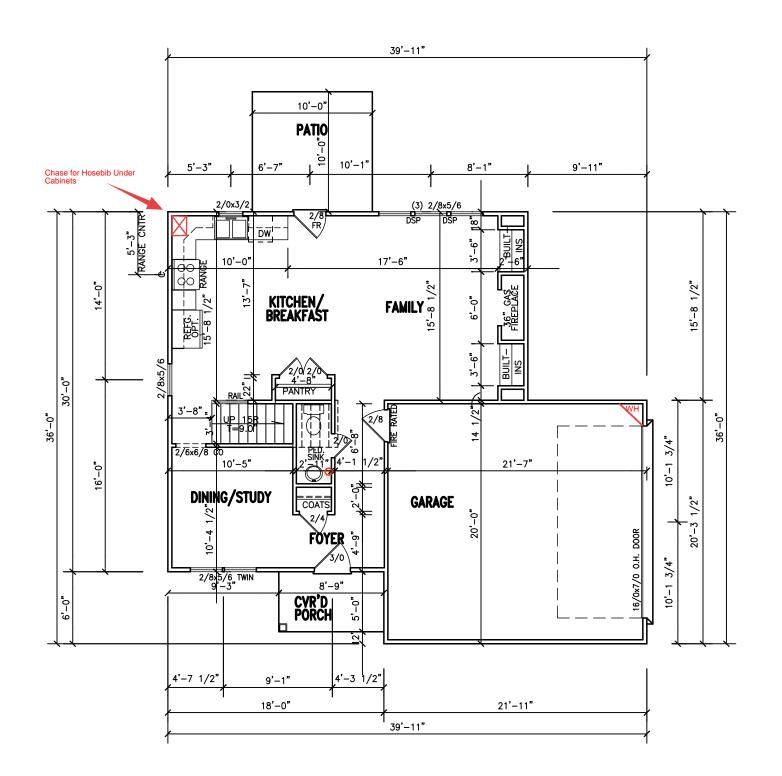
Ш	1ST FLOOR PLAN NOTES:
	1) 9'-0" CLG. HGT. (9'-1 1/2" PLT. HGT.) UNLESS OTHERWISE NOTED.
	 ALL EXTERIOR WALLS FIGURED AT 4"NOMINAL WIDTHS UNLESS OTHERWISE NOTED.
Ш	 ALL INTERIOR WALLS FIGURED AT 3 1/2" WIDTHS UNLESS OTHERWISE NOTED.
Ш	 SET WINDOWS AT 7'-8" A.F.F. UNLESS OTHERWISE NOTED.
	5) DIMENSIONS ARE TO FRAMING UNLESS OTHERWISE NOTED.

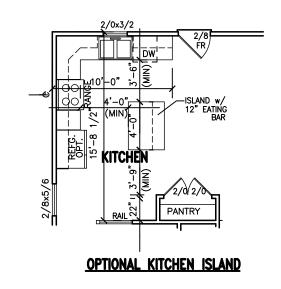
FIRST FLOOR PLAN

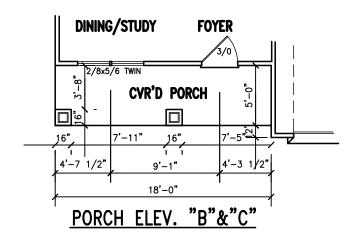
DRAWN BY:
CHECKED BY:

FLOOR PLAN









SQUARE FOOTAGE	
1ST FLOOR HTD. 2ND FLOOR HTD.	737 942
FLOOR HTD.	1679
GARAGE PATIO ELV "A" PORCH ELV "B" PORCH ELV "C" PORCH	437 100 44 90 90
1ST FLOOR PLAN NOTE	īs.

	1ST FLOOR PLAN NOTES:
l	1) 9'-0" CLG. HGT. (9'-1 1/2" PLT. HGT.) UNLESS OTHERWISE NOTED.
l	 ALL EXTERIOR WALLS FIGURED AT 4"NOMINAL WIDTHS UNLESS OTHERWISE NOTED.
	 ALL INTERIOR WALLS FIGURED AT 3 1/2" WIDTHS UNLESS OTHERWISE NOTED.
I	 SET WINDOWS AT 7'-8" A.F.F. UNLESS OTHERWISE NOTED.
I	5) DIMENSIONS ARE TO FRAMING UNLESS OTHERWISE NOTED.

FIRST FLOOR PLAN
SIDE LOAD GARAGE OPTION



CHECKED BY:

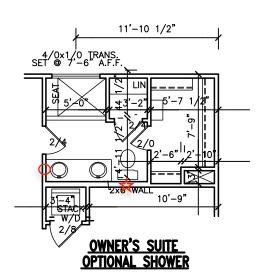
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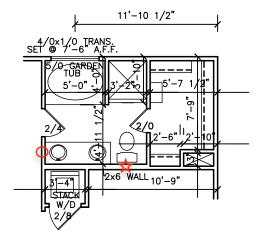
FLOOR PLAN

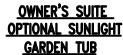
OF 9

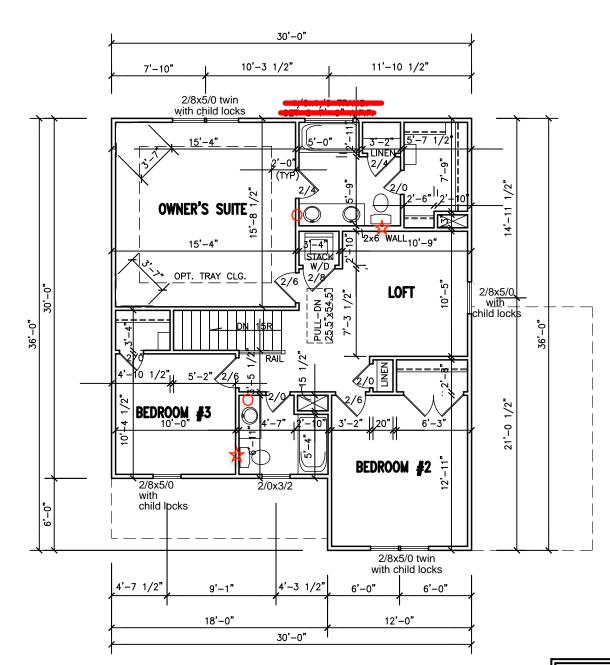
11'-10 1/2" 4/0x1/0 TRANS. SET @ 7'-6" A.F.F. 12x6 WALL10'-9" OWNER'S SUITE
OPT. GARDEN TUB/SHOWER











2ND FLOOR PLAN NOTES:

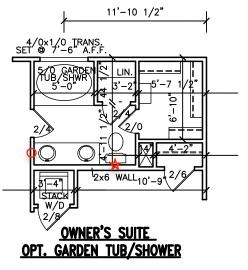
1) 8'-0" CLG. HGT. (8'-1 1/2" PLT. HGT.)
UNLESS OTHERWISE NOTED.
2) ALL EXTERIOR WALLS FIGURED AT 4"NOMINAL
WIDTHS UNLESS OTHERWISE NOTED.
3) ALL INTERIOR WALLS FIGURED AT 3 1/2"
WIDTHS UNLESS OTHERWISE NOTED.
4) SET WINDOWS AT 6'-8" A.F.F.
UNLESS OTHERWISE NOTED.
5) DIMENSIONS ARE TO FRAMING
UNLESS OTHERWISE NOTED.

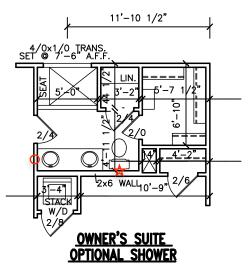
SECOND FLOOR PLAN

DRAWN BY: CHECKED BY:

FLOOR PLAN

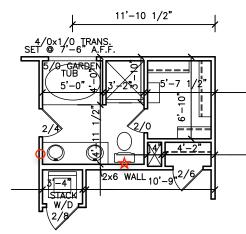




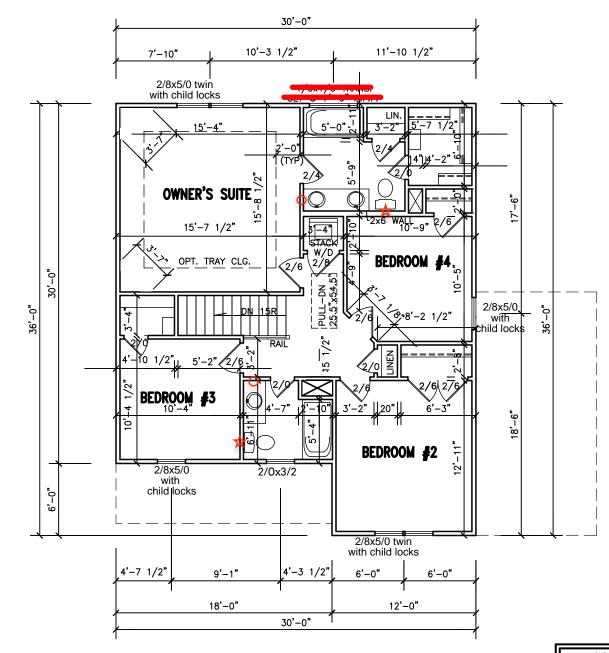


☆ Towel Bar

O Hand Towel Ring



OWNER'S SUITE OPTIONAL SUNLIGHT **GARDEN TUB**



2ND FLOOR PLAN NOTES:

1) 8'-0" CLG. HGT. (8'-1 1/2" PLT. HGT.)
UNLESS OTHERWISE NOTED.
2) ALL EXTERIOR WALLS FIGURED AT 4"NOMINAL
WIDTHS UNLESS OTHERWISE NOTED.
3) ALL INTERIOR WALLS FIGURED AT 3 1/2"
WIDTHS UNLESS OTHERWISE NOTED.
4) SET WINDOWS AT 6"-8" A.F.F.
UNLESS OTHERWISE NOTED.
5) DIMENSIONS ARE TO FRAMING
UNLESS OTHERWISE NOTED.

SECOND FLOOR PLAN

4TH BEDROOM OPTION

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION	
	(/	(1 01)	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BASED ON SEISMIC ZONES A, B & C				

- STRUCTURAL NOTES:

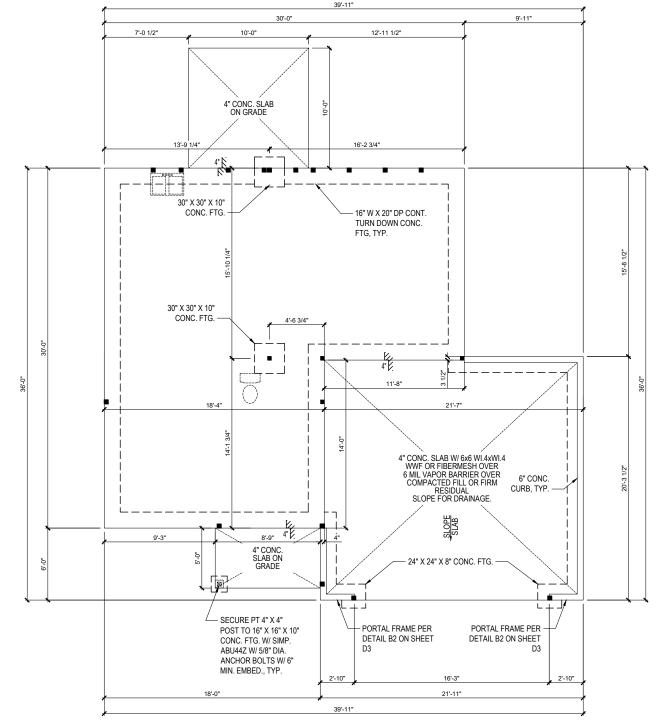
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
 OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN
- ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS. IT IS THE CONTRACTORS RESPONSIBILITY TO VENIFF ALL DIMENSION AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- Fb = 2600 PSI, E = 1.9M PSI
- (IE. ILEVEL MICROLAM)
 ALL LISL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
 ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R802.7.5. AND
- (1) 2x4 JACK S I UD (UN.O.) AND KING S I UDUS PER I ABLE. R602.7.5, AND TOGETHER N.C. [2] 10d NAILS @ 6" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6"5", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1"5". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFOR TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
 REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION

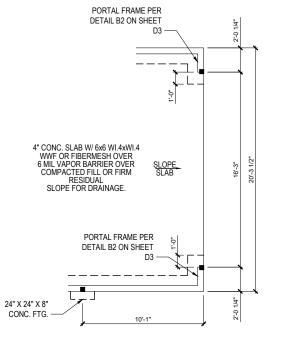
- REFER TO 2018 NO BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10.7° IN HEIGHT.

 ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
 Fy = 50 KSI MIN. (UNO)
 ALL EXTERIOR LUMBER TO BE #2 SYP PT
 ALL CONCRETE, fo = 3000 PSI MIN.
 PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/270 ANCHOR BOLTS SPACED AT MAXIMUM OF 6-0° O.C. AND NOT MORE
 THAN 12° FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS
 FOR PLATE SECTION. ANCHOR BOLT SHALL EXTEND 7° INTO CONCRETE OR
 MASONRY. MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLLIMNS (U.N.O.) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- INC.

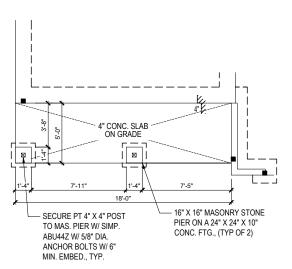
 MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION. METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.





SIDE LOAD GARAGE OPTION



PORCH ELEV. B & C

FOUNDATION PLAN (MONO SLAB)

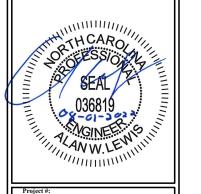
1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

PORCH ELEV. A

Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering







<u>Project #:</u> DRB2201-0065D
Date: 08/01/22
Engineered By: AM
DWG. Checked By: AWL
SEE PLAN
REVISIONS

Sheet Number

S₁A

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION	
	(/	(1 01)	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BASED ON SEISMIC ZONES A, B & C				

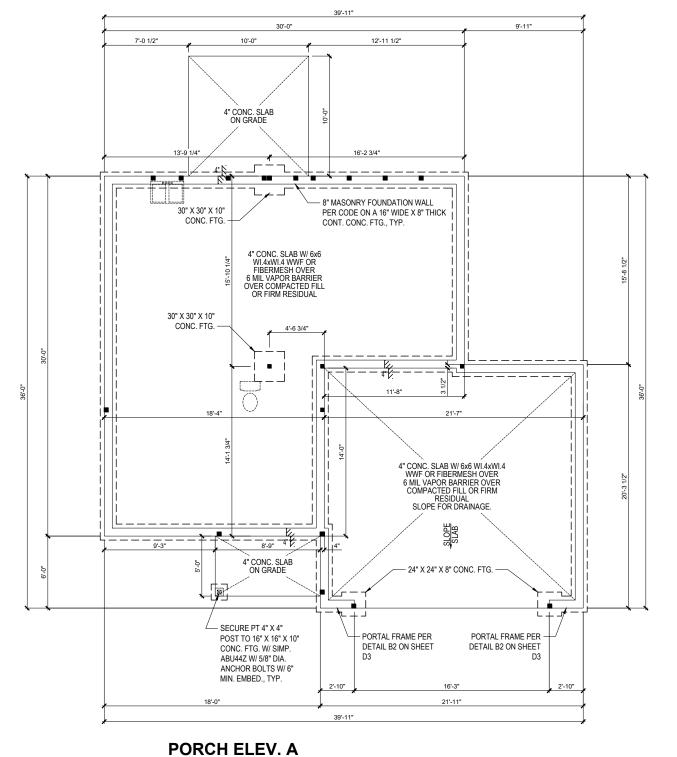
- STRUCTURAL NOTES:

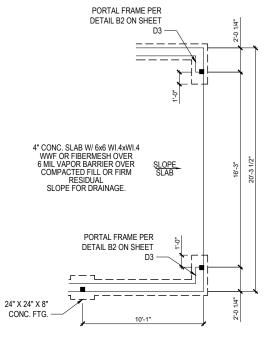
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 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS. IT IS THE CONTINCE TO THE STORY OF THE CONTINUE OF THE CONTIN
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- Fb = 2600 PSI, E = 1.9M PSI
- (IE. ILEVEL MICROLAM)
 ALL LISL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
 ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R802.7.5. AND
- (1) 2x4 JACK S I UD (UN.O.) AND KING S I UDUS PER I ABLE. R602.7.5, AND TOGETHER N.C. [2] 10d NAILS @ 6" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6"5", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1"5". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFOR TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION
- OF ALL WALLS OVER 10'-0" IN HEIGHT.

- OF ALL WALLS OVER 10-0" IN HEIGHT.
 ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
 Fy = 50 KSI MIN. (LINO)
 ALL EXTERIOR LUMBER TO BE #2 SVP PT
 ALL CONCRETE, 10 = 3000 PSI MIN.
 PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/2"O ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE
 THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS
 PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3"-0" O.C.
 FOR BASEMENTS. ANCHOR BOLT SHALL BE STACED AT 3"-0" O.C.
 FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
 MASONRY.
- MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLLIMNS (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- INC.

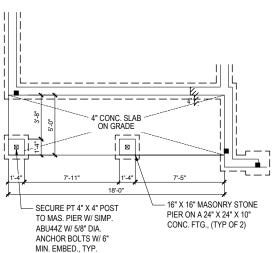
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- ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.





SIDE LOAD GARAGE OPTION



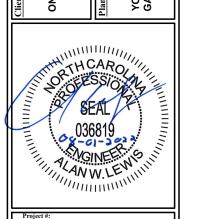
PORCH ELEV. B & C

FOUNDATION PLAN (STEM WALL)

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering





Project #: DRB2201-0065D	
Date: 08/01/22	
Engineered By: AM	
DWG. Checked By: AWL	
SEE PLAN	
REVIS	IONS
No. Date:	Remarks

S₁B 1 of 7

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEAD LOAD DEFLI	CTION
	(/	(/	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

- STRUCTURAL NOTES:
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- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND

- ALL LVI. LUMBER 10 BE 1.75' WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2800 PSI, E = 1 9M PSI

 (I.E. ILEVEL MICROLAM)
 ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)

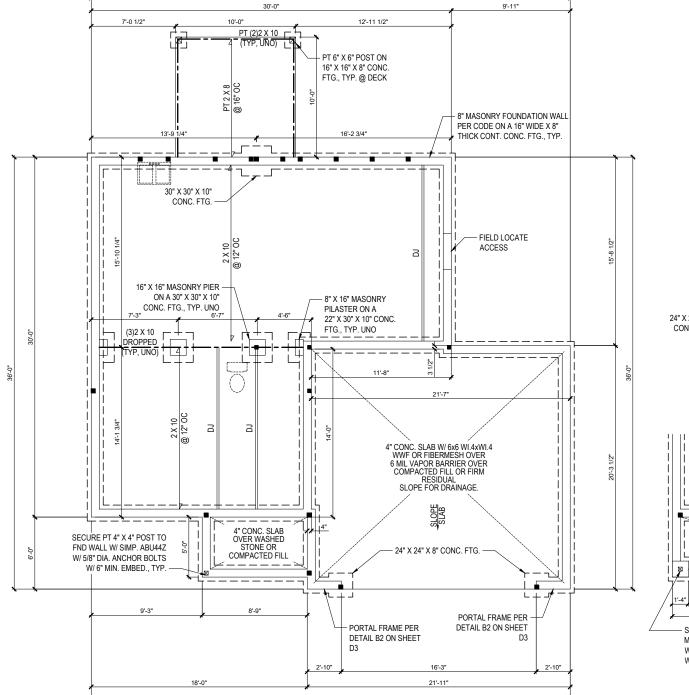
 ALL LODB BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w

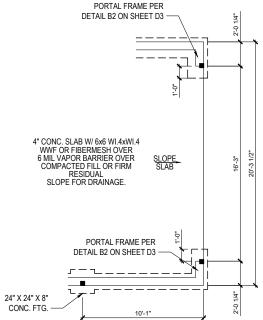
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND
- (1) 243 JACKS 100 (JANUC) AND KINGS 21 IOSP SEY READER REQUEZY, 3AND TOGETHER W. (2) 100 MAILS & 8° O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6°5', MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1°5', OTHERWISE REFER TO TABLES REGOZ! (A) AND REGOZ! (2) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFO. TO TABLES REGOZ! (7) BAND REGOZ! (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (1) AND STATEMENT OF THE STATEMENT OF TH
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.

- OF ALL WALLS OVER 10-0" IN HEIGHT.
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 Fy = 50 KSI MIN. (LINO)
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 ALL CONCRETE, 10 = 3000 PSI MIN.
 PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/2"O ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE
 THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS
 PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3"-0" O.C.
 FOR BASEMENTS. ANCHOR BOLT SHALL BE STACED AT 3"-0" O.C.
 FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
 MASONRY. MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
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- AND BOTTOM OF PORCH COLLIMNS (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- INC.

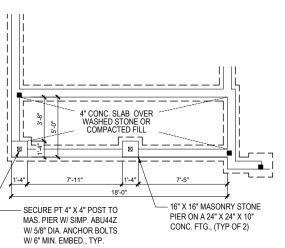
 MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

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- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.





SIDE LOAD GARAGE OPTION



PORCH ELEV. B & C

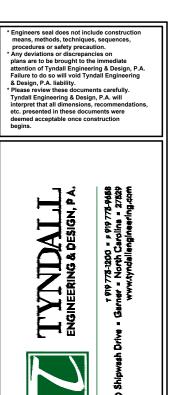
FOUNDATION PLAN (CRAWL SPACE)

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

660 SOLETIOF CRAWL SPACE / 150 = 4.4 SOLETIOF REGID VENTILATION WITHOUT CROSS VENTILATION 4.4 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 5 VENTS REQ'D (BASED ON 8" X 16" VENTS)

660 SQ. FT. OF CRAWL SPACE / 1500 = 0.44 SQ. FT. OF REQ'D VENTILATION WITH CROSS VENTILATION 0.44 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 1 VENTS REQ'D (BASED ON 8" X 16" VENTS)

- THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 11500 OF THE CRAWL SPACE. GROUND AREA WHERE THE REQUIRED OPENINGS ME PLACED SO AS TO REVOICE CROSS VENTILATE OF THE COMES. SHORT THE RESTALLATION OF OPENINGEL CONTRES SHALL ROTE REPORTING THE COMES OF THE PLACE THE PLACE OF THE PLACE SHALL THE PLACE SHALL ROTE SHALL ROTE OF THE PLACE SHALL THE PLACE SHALL THE CAMES SHALL TO MAIL OF THE SHALL THE LIFE AND AND AND AND THE PLACE SHALL THE CAMES SHALL SHALL TO MAIL SHALL SHA



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REVISIONS

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Project #: DRB2201-0065D

Date: 08/01/22

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION	
	(- /	(- /	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BASED ON SEISMIC ZONES A, B & C				

- STRUCTURAL NOTES:
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE". IN
- ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS IT IS THE CONTINCE TO THE STORY OF THE CONTINUE OF THE CONTIN
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND

- ALL LVI. LUMBER 10 BE 1.75' WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2800 PSI, E = 1 9M PSI

 (I.E. ILEVEL MICROLAM)
 ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)

 ALL LODB BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w

 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND
- (1) 243 JACKS 100 (JANUC) AND KINGS 21 IOSP SEY READER REQUEZY, 3AND TOGETHER W. (2) 100 MAILS & 8° O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6°5', MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1°5', OTHERWISE REFER TO TABLES REGOZ! (A) AND REGOZ! (2) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFO. TO TABLES REGOZ! (7) BAND REGOZ! (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (1) AND STATEMENT OF THE STATEMENT OF TH
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO)
 ALL EXTERIOR LUMBER TO BE #2 SYP PT
 ALL CONCRETE, f6 = 3000 PSI MIN.

- ALL CONCRETE, 16 3000 PSI MIN.
 PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/2"9 ANCHOR BOLTS SPACED AT MAXIMUM OF 6-0" O.C. AND NOT MORE
 THAN 12" FROM THE CORNET. THERE SHALL BE A MINIMUM OF (2) BOLTS
 PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3"0" O.C.
 FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
 MASCANIDY.
- MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLLIMNS (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

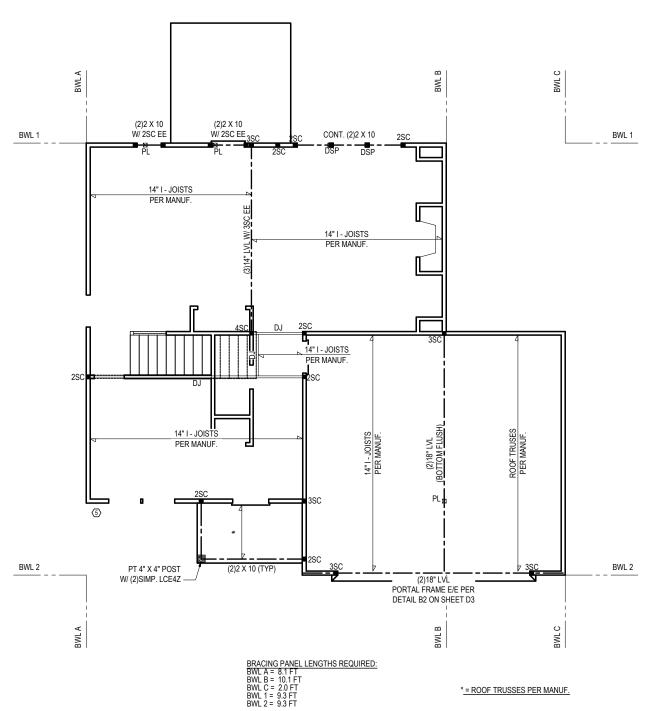
 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

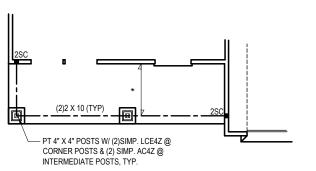
- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
 WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
- 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- 1 REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2) 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8"-0" (ISOLATED PANELS) OR 4"-0" (CONTINUOUS SHEATHING). SECURE W 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACEO @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION BOOM AS A WIND OF THE PROPERTY OF THE PROPERT
- R602-10.3 (UNO)

 ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS
 ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE
 CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP)
 SHEATHING WITH A MINIMUM THICKNESS OF 36". SHEATHING SHALL BE
 SECURED WITH MINIMUM 60 COMMON NAILS SPACED AT 6" O.C. AT
 PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.

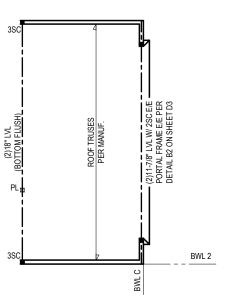
 MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL
 BE ASTEDIOLOGY.
- BE AS FOLLOWS:

 - S FOLLOWS:
 24" ADJACENT TO OPENINGS NOT MORE THAN
 67% OF WALL HEIGHT
 30" ADJACENT TO OPENINGS GREATER THAN
 67% AND LESS THAN 85% OF WALL HEIGHT.
 48" FOR OPENINGS GREATER THAN 85% OF
 WALL HEIGHT.
- 4 SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIFU OF A CORNER RETURN. ACCORDANCE WITH FIGURE R802.10 3.(4). IN LIEU OF A CORNER RETURE EITHER A MIN. 48° BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 8009 SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- 5 MINIMUM 800# HOLD-DOWN DEVICE





PORCH (ELEV. B & C)



SIDE LOAD GARAGE OPTION

BRACING PANEL LENGTHS REQUIRED: BWL C = 2.0 FT BWL 2 = 9.3 FT

BRACING PANEL LENGTHS PROVIDED: BWL C = 6.44 FT PF BWL 2 = 33.5 FT CS-WSP

FIRST FLOOR PLAN

BRACING PANEL LENGTHS PROVIDED: BWL A = 27.33 FT CS-WSP BWL B = 15.71 FT CS-WSP BWL C = 20.29 FT CS-WSP BWL 1 = 16.67 FT CS-WSP BWL 2 = 17.5 FT CS-WSP / PF

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

Engineers seal does not include cor procedures or safety precau 'Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engin

& Design, P.A. liability. Please review these doc Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendetc. presented in these documents were





Project #: DRB2201-0065D				
Date: 08/01/22				
Engineered By: AM				
DWG. Checked By: AWL				
Scale: SEE PLAN				
REVISIO	NS			
No. Date:	Remarks			

(2)2 X 10 (2)5-1/2" LVL BWL A BWL A PER MANUF ROOF TRUSSES PER MANUF. BWL B BWL B BRACING PANEL LENGTHS REQUIRED: BWL A = 3.2 FT BWL B = 3.2 FT BWL 1 = 3.8 FT BWL 2 = 3.8 FT

BRACING PANEL LENGTHS PROVIDED: BWL A = 30.0 FT BWL B = 33.33 FT BWL 1 = 20.5 FT BWL 2 = 20.5 FT

SECOND FLOOR PLAN

1/8" = 1'-0" (11"X17")

1/4" = 1'-0" (24"X36")

DESIGN LOADS

	LIVE LOAD DEAD LOAD (PSF) (PSF)		DEFLECTION				
	(/	(/	LL	TL			
FLOOR (primary)	40	10	L/360	L/240			
FLOOR (secondary)	40	10	L/360	L/240			
ATTIC (w/ storage)	20	10	L/240	L/180			
ATTIC (no access)	10	5	L/240	L/180			
EXTERNAL BALCONY	40	10	L/360	L/240			
ROOF	20	10	L/240	L/180			
ROOF TRUSS	20	20	L/240	L/180			
WIND LOAD	BAS	SED ON 120 MPH (E	XPOSURE B)				
SEISMIC	BAS	ED ON SEISMIC ZO	NES A, B & C				

- STRUCTURAL NOTES:
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
 OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN
- OF YOKEN CAROLINA SEATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYMPALL BUILDING SENSIONS AND SQUARE FOOTAGE PRIORS TO NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
 ALL LIMBER COLAID SECREM AND ADDITIONAL TO THE PROPERTY OF THE PRINCIPLE OF THE PRIN
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- ALL LVL LUMBER 10 be 1.7.6" WIDE NOMINAL EACH SINGLE MEMBER AND FD = 2800 PSI, E = 19 M PSI

 (I.E. ILEVEL MICROLAM)

 ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)

 ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R802.7.5, AND
- (1) 2x4 JACK S I UD (N.O.) AND KING S I UDS PEX 1ABLE 1602.7.5, AND TOGETHER N.E. (2) 10d NAILS @ 6" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6"5", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1"5". OTHERWISE REFER TO TABLES RE02.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFOR TO TABLES RE02.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS INDOC
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.

- OF ALL WALLS OVER 10-0" IN HEIGHT.

 ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50

 Fy = 50 KSI MIN. (UNO)

 ALL EXTERIOR LUMBER TO BE #2 SYP PT

 ALL CONCRETE, fo = 3000 PSI MIN.

 PRESUMPTIVE BERATING CAPACITY = 2000 PSF

 1/2" O ANCHOR BOLTS SPACED AT MAXIMUM OF 6-0" O.C. AND NOT MORE

 TAKEN AND CONCRETE WITH CONCRETE ATTERIOR SHALL BE A NUMBER OF 0" DO TO SEE 1/2"O ANCHOR BOLTS SPACED AT MAXIMUM OF 6:0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3:0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9:0" (UNO) PROVIDE A MINIMUM OF 500H UPILIT'S A LATERAL CONNECTION AT TOP AND ROTTOM OF EMPIREY COLUMNS. (MIN.)
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.)

 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC.

 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

 16) UPLIFT LOADS GREATER THAN 50# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL

STRUCTURAL SHEATHING NOTES

- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
 WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3.

 REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- 1 REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8"-0"
 (ISOLATED PANELS) OR 4"-0" (CONTINUOUS SHEATHING).
 SECURE W 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5)
 SPACED (@ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND
 BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- (3) 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- R602-10.3 (UNO)

 ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS
 ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE
 CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP)
 SHEATHING WITH A MININUM THICKNESS OF 3/6". SHEATHING SHALL BE
 SECURED WITH MINIMUM 64 COMMON NAILS SPACED AT 6" O.C. AT
 PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.

 MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL
 BE AS EQUI CIUS.
- MINIMOM BRACED WALL PANEL LENGTHS WITH CS-WSP
 BE AS FOLLOWS:
 24" ADJACENT TO OPENINGS NOT MORE THAN
 67% OF WALL HEIGHT
 3" ADJACENT TO OPENINGS GREATER THAN
 67% AND LESS THAN 85% OF WALL HEIGHT.
 48" FOR OPENINGS GREATER THAN 85% OF
 WALL HEIGHT
- 4 SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, ACCORDANCE WITH FIGURE R802.10 3.(4). IN LIEU OF A CORNER RETURE EITHER A MIN. 48° BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800% SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- 5 MINIMUM 800# HOLD-DOWN DEVICE

Engineers seal does not include cor means, methods, techniques, seque procedures or safety precaution

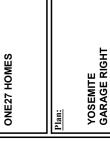
- Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Fallure to do so will void Tyndall Engineering
- **Please review these documents carefully Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommender, presented in these documents were depend executable accounts with the property of the produced the p





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Q-12



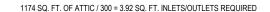


AWL Scale:
DWG. Checked By: AWL
Engineered By: AM
<u>Date:</u> 08/01/22
Project #: DRB2201-0065D

	REVISIONS						
No.	Date:	Remarks					
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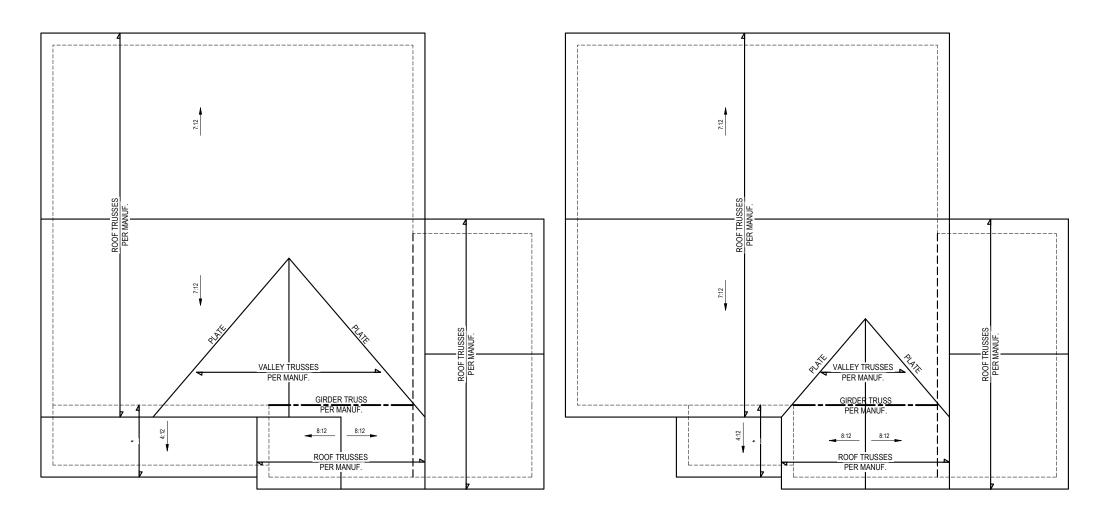
S3



- CALCULATION BASED ON VENTILATORS USED AT LEAST 3-0° ABOVE THE COMICE VENTS WITH THE BALANCE OF VENTILATION PROVIDED BY EAVE VENTS.
- CATHEDRAL CEILINGS SHALL HAVE A 1" MINIMUM CLEARANCE BETWEEN THE BOTTOM OF THE ROOF DECK AND THE INSULATION.



* ATTIC VENTILATION CALCULATION



* = ROOF TRUSSES PER MANUF

ROOF PLAN (ELEV. B & C)

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

ROOF PLAN (ELEV. A)

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

* Engineers seal does not include construction means, methods, techniques, sequences,

- means, methods, techniques, sequences, procedures or safety precaution.

 * Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Faliure to do so will void Tyndall Engineering & Design, P.A. liability.

 * Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.



YOSEMITE GARAGE RIGHT



Project #: DRB2201-0065D Date: 08/01/22 Engineered By: DWG. Checked By: AWL Scale: SEE PLAN

	REVISIO	NS
<u>.07</u>	Date:	Remarks
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Sheet Number

S4

STRUCTURAL NOTES

ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE". IN ADDITION TO ALL LOCAL CODES AND REGULATIONS

DESIGN LOADS:

	LIVE LOAD (PSF)			DEFLECTION		
	(, 0.)	(1.01)	LL	TL		
ALL FLOORS	40	10	L/360	L/240		
ATTIC (w/ walk up stairs)	30	10	L/360	L/240		
ATTIC (pull down access)	20	10	L/240	L/180		
ATTIC (no access)	10	5	L/240	L/180		
EXTERNAL BALCONY	40	10	L/360	L/240		
ROOF	20	10	L/240	L/180		
ROOF TRUSS	20	20	L/240	L/180		
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)					
SEISMIC	SEISMIC ZONES A, B & C					

- MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE, (U.N.O.)
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT. WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON 2x10) UNO.
 ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (U.N.O.)
 ALL SL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2235 PSI, E = 1.6M PSI (U.N.O.)
 ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10. (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50. ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2") at **LONG, LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE.

WALL AND ROOF CLADDING VALUES:
WALL CADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE.
ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:

39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1.5/12

36.0 LBS/SQFT FOR ROOF PITCHES 1.5/12 TO 6/12 18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12

**MEAN ROOF HEIGHT 30'-0" OR LESS

- 13) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- 18) PSI COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION.
 TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

CLIMATE ZONES	FENESTRATION U-FACTOR	SKYLIGHT ^b U-FACTOR	GLAZED FENESTRATION SHGC ^{b,k}	CEILING ^m R-VALUE	WOOD FRAMED WALL R-VALUE	MASS WALL R-VALUE	FLOOR R-VALUE	BASEMENT ^{C,Q} WALL R-VALUE	SLAB ^d R-VALUE AND DEPTH	CRAWL SPACE C WALL R-VALUE
3	0.35	0.55	0.30	38 or 30 cont	15 or 13 + 2.5 h	5/13 or 5/10 cont	19	<u>5/13</u> f	0	5/13
4	0.35	0.55	0.30	38 or 30 cont	15 or 13 + <u>2.5</u> h	5/13 or 5/10 cont	19	10/15	10	<u>10/15</u>
5	0.35	0.55	NR	38 or 30	n 19, or 13 + 5 or 15 + 3	13/17 <u>or</u> 13/12.5 cont	30 ^g	10/15	10	10/19

TABLE N1102.1 CLIMATE ZONES 3-5

- R-VALUES ARE MINIMUMS. U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS
 OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE.

- ASSIGNATION OF THE HOUSE SHARE OF REASTATON.

 "UNIS MEANS AND CONTINUOUS BUILD AND READMEND OF THE WITEROO OF EXTERIOR OF THE HOME OF THE CHARLES AND ASSIGNATION SHALL BE APPLIED FROM THE INSPECTION OF DOMINIMAND TO THE BOTTOM OF THE POST OF AND AND ASSIGNATION SHALL BE APPLIED FROM THE INSPECTION OF DESTROYAND AND AND ASSIGNATION SHALL BE APPLIED FROM THE INSPECTION OF LISTS FOR THE CHARLES AND AND AND ASSIGNATION OF THE CHARLES AND ASSIGNATION OF THE POST OF THE POST
- e. <u>Deleted</u> f. Basement wall insulation is not required in Warm-Humid Locations as defined by <u>Figure N1101.7</u> and <u>Table N1101.7</u>.
- a OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY IR-19 MINIMUM
- 5. OR INSTALLAND SHY-EXH I DUTL THE PROMINE CANTE. YES WINDOW.

 THE FRIST YALKE COANTY ROULDING THE SECOND VALUE SO CONTINUOUS RISULATION, SO "13-5" MEANS R-11 CANTY NIGULATION PULS R-8 INSLATED
 SHEATHNG. "15-7" MEANS R-15 CANTY ROULDING THE STATE COANTY ROULDING THE COTTENS OF THE COTTENS
- OR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WA

- I FOR MASS WALLS, THE SECTION WALLE APPLES WHEN LINGS: THE MINISTER MASS WALLS. THE STREET HAND AS SHALL BE PROMOTED THE DESCRIPTION OF SECTION THE STREET HAND AS SHALL BE PROMOTED THE SECTION OF SE
- OF THE ATTIC ROOF DECK.

 "THE LIVEL IR COURDED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF. THERE THE INSLIATION MUST FILL THE SPACE UP TO THE ARE BAFFLE.

 8. RI HERICALS BUTTS COMPRESSED AND INSTALLED AN ADMINAL 2 + 6 FRAMING CAVITY IS DEEMED TO COMPLY. PREPRICASS BUTTS RATTED R-19 OR HIGHER COMPRESSED

 MON INSTALLED AND AVENUEL IS 10TH DEPENDED TO COURTY.

 9. BASEMENT WHAL INSETING THE MINIMUM MASS WHAL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WHAL BY AULUE AS THE MINIMUM REQUIREMENT.

DEFINITIONS FOR COMMON ABBREVIATIONS

ALT	=	ALTERNATE	MAX	=	MAXIMUM
CANT	=	CANTILEVER	MIN	=	MINIMUM
CJ	=	CEILING JOIST	NOM	=	NOMINAL
CMU	=	CONCRETE MASONRY UNIT	O.C.	=	ON CENTER
COL	=	COLUMN	PL	=	POINT LOAD
CONC	=	CONCRETE	PT	=	PRESSURE TREATED
CONT	=	CONTINUOUS	REINF	=	REINFORCED
CT	=	COLLAR TIE	REQD	=	REQUIRED
DBL	=	DOUBLE	RJ	=	ROOF JOIST
DIA	=	DIAMETER	RS	=	ROOF SUPPORT
DJ	=	DOUBLE JOIST	SC	=	STUD COLUMN
DR	=	DOUBLE RAFTER	SCH	=	SCHEDULE
EA	=	EACH	SPEC	=	SPECIFIED
EE	=	EACH END	THK	=	THICK
FJ	=	FLOOR JOIST	TJ	=	TRIPLE JOIST
FND	=	FOUNDATION	TRTD	=	TREATED
FTG	=	FOOTING	TYP	=	TYPICAL
GALV	=	GALVANIZED	UNO	=	UNLESS NOTED OTHERWISE
HORIZ	=	HORIZONTAL	W	=	WIDE FLANGE BEAM
HT	=	HEIGHT	WWF	=	WELDED WIRE FABRIC
MANUF	=	MANUFACTURER	XJ	=	EXTRA JOIST

MAXIMUM HEIGHT OF DECK SUPPORT POSTS AS FOLLOWS:

POST SIZE	MAX. POST HEIGHT**
4 x 4	8'-0"
6 x 6	20'-0"
***	OVER 20'-0"

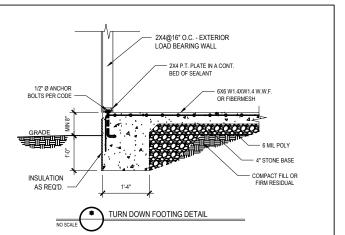
- THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS.

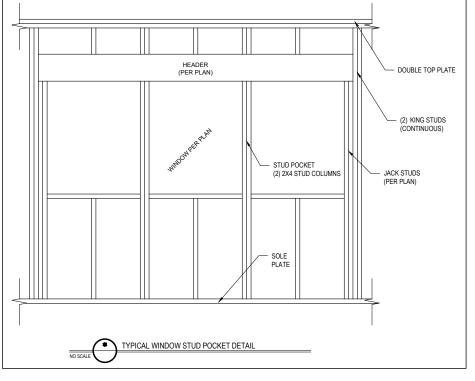
 MAXIMUM TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET
 WHICH MAY BE LOCATED AT DIFFERENT LEVELS.
- ROM TOP OF FOOTING TO BOTTOM OF GIRDER
- DECKS WITH POST HEIGHTS OVER 20'-0" SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT.
- DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF
- A. THE DECK FLOOR HEIGHT IS LESS THAN 4-0" AND THE DECK IS
 ATTACHED TO THE STRUCTURE IN ACCOPDANCE WITH SECTION (4)
 ABOVE LATERAL BRACING IS NOT REQUIRED.

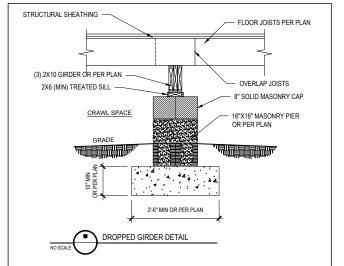
 B. 4-4 WOOD NIKE BRACES MAY BE PROVIDED ON EACH COLUMN IN
 BOTH DIRECTIONS. THE NIKEE BRACES SHALL ATTACH TO EACH POST
 AT A POINT NOT LESS THAN 13 OF THE POST LENGTH FROM THE
 TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL, KNEE BRACES SHALL BE BOLTED TO THE POST AND GIRDER WITH ONE 5/8"Ø HOT DIPPED GALVANIZED
- TO THE POST AND SIGNER WITH OWNERS OF HOT DIFFED GALVANIZED BOLT AT EACH END OF THE BRACE.
 REESTANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN ACCORDANCE WITH THE FOLLOWING:

POST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER
4 x 4	48 SQ. FT.	4'-0"	2'-6"	1'-0"
6 x 6	120 SQ. FT.	6'-0"	3'-6"	1'-8"

- D. 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO 2x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO
 (2) PERPENDICULAR DIRECTIONS FOR FRESTANDING DECKS OR PARALLEL.
 TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS.
 THE 2x 6s SHALL BE ATTACHED TO THE POSTS WITH ONE 50% DIOT
 DIPPED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER.
 FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.





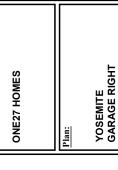


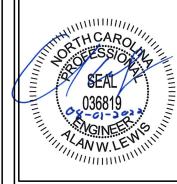
Engineers seal does not include construc procedures or safety precaution

Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. Please review these docu

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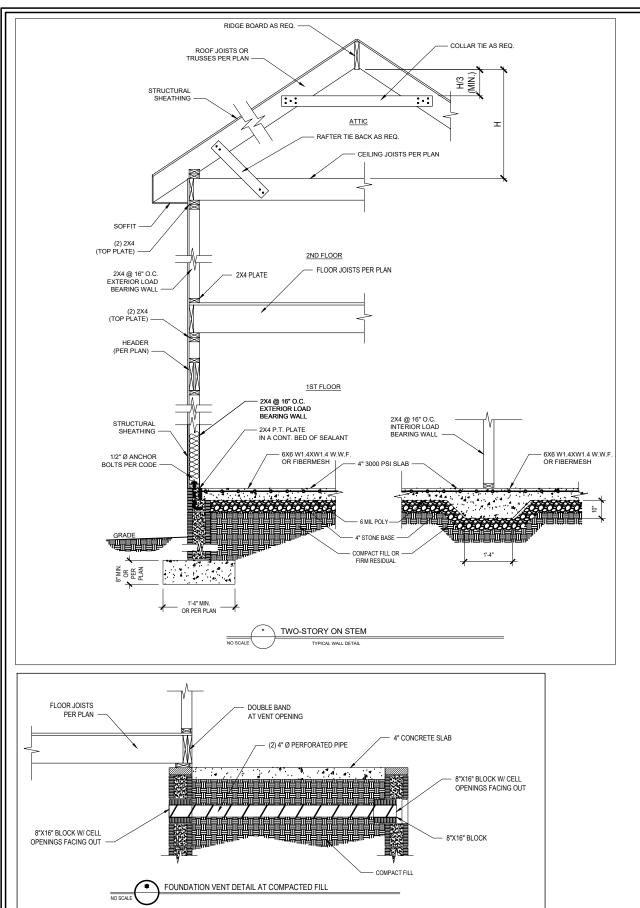


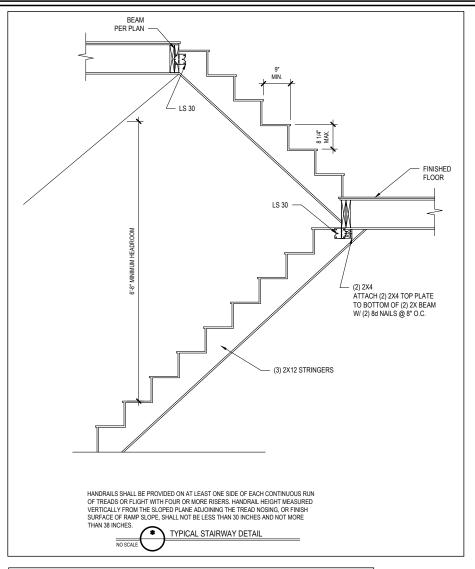


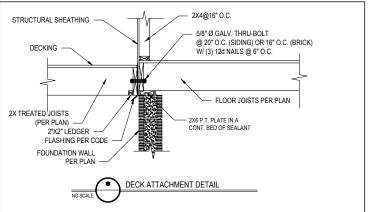
Project #: DRB2201-0065D						
<u>Date:</u> 08/01/22						
Engineered By: AM						
DWG. Checked By: AWL						
Scale: NOT TO SCALE						
REVISIONS						
No. <u>Date:</u>	Remarks					

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Raine to do so will votal syndan Engineeri & Design, P.A. Ilability.
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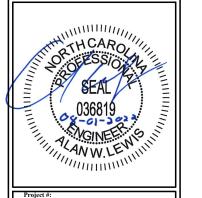




Dr.

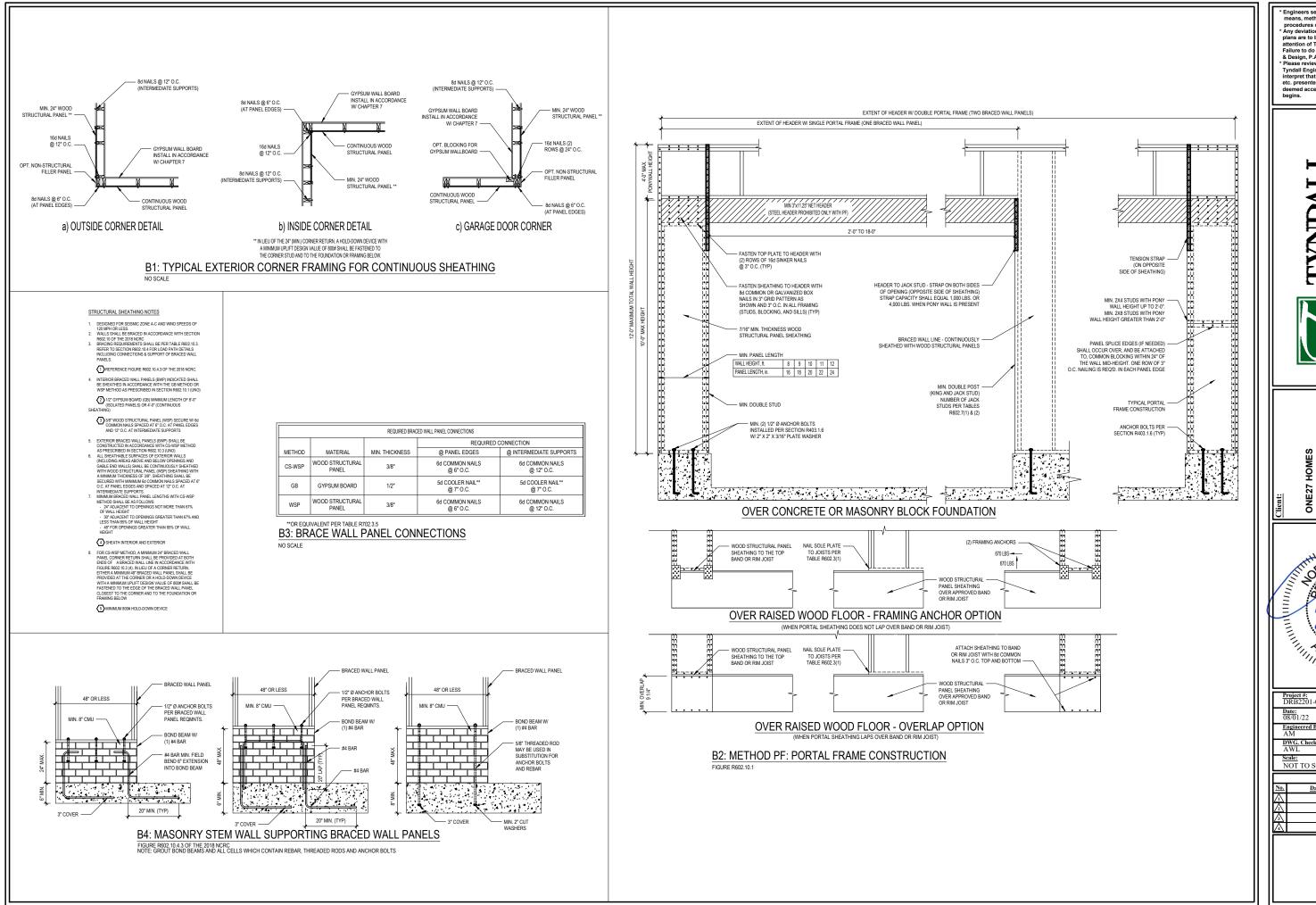
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| Plan: | YOSEMITE | GARAGE RIGHT |



Sheet Number

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Plan: YOSEMITE GARAGE RIGHT



o. Date: Remarks

Sheet Number

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