

WEST PRESERVE LOT - 25 TBD OLEANDER LANE SANFORD, NC 27332 **COVERED PORCH**

GENERAL NOTES

- 1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AND REGULATIONS.
- 2. CONTRACTOR SHALL THOROUGHLY REVIEW ALL SHEETS IN PLAN SET AND VERIFY ALL DETAILS AND DIMENSIONS BEFORE BEGINNING CONSTRUCTION. DESIGN, INC. FOR JUSTIFICATION AND/OR CORRECTION BEFORE PROCEEDING WITH WORK. CONTRACTORS SHALL ASSUME RESPONSIBILITY FOR ERRORS THAT ARE NOT REPORTED PRIOR TO CONSTRUCTION.
- 3. ALL DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED. . CONTRACTOR SHALL ENSURE COMPATIBILITY OF THE BUILDING WITH ALL

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DATE: MAY 23, 2022

SCALE: 1/4" = 1'-0"

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REVIEWED BY:

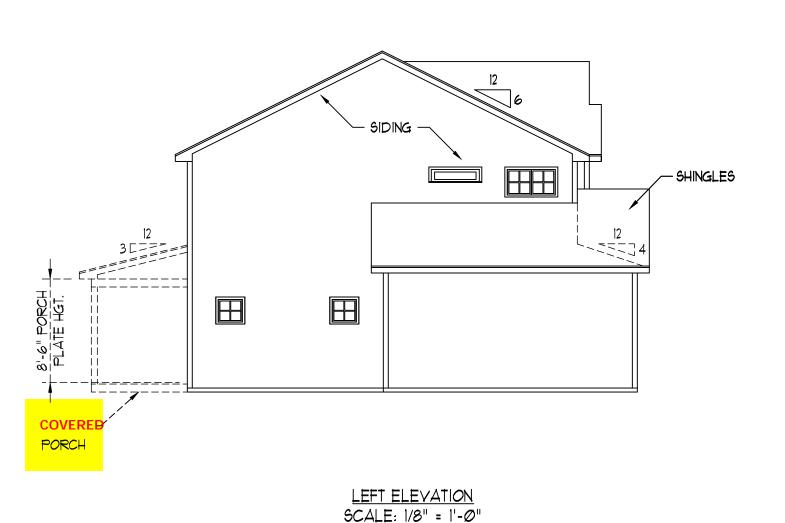
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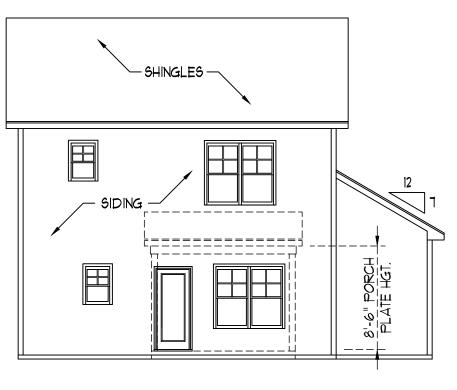
A-3

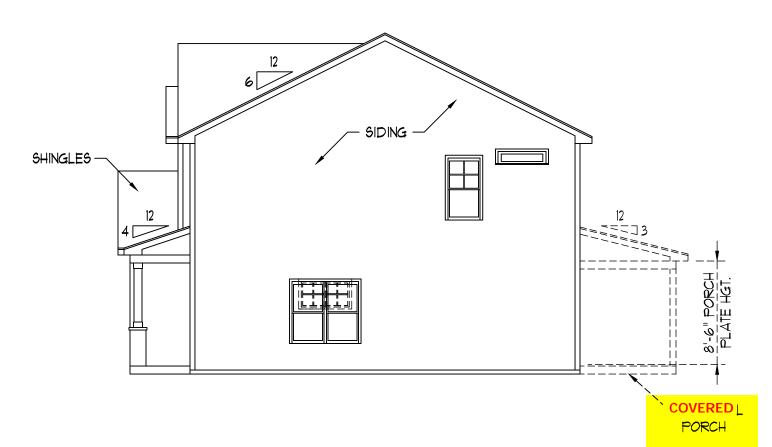


FRONT ELEVATION-C SCALE: 1/4" = 1'-0"

NUIE: MONO SLAB - STONE TO RUN TO THE BOTTOM OF WINDOW STEM WALL - STONE TO FOUNDATION HEIGHT ONLY



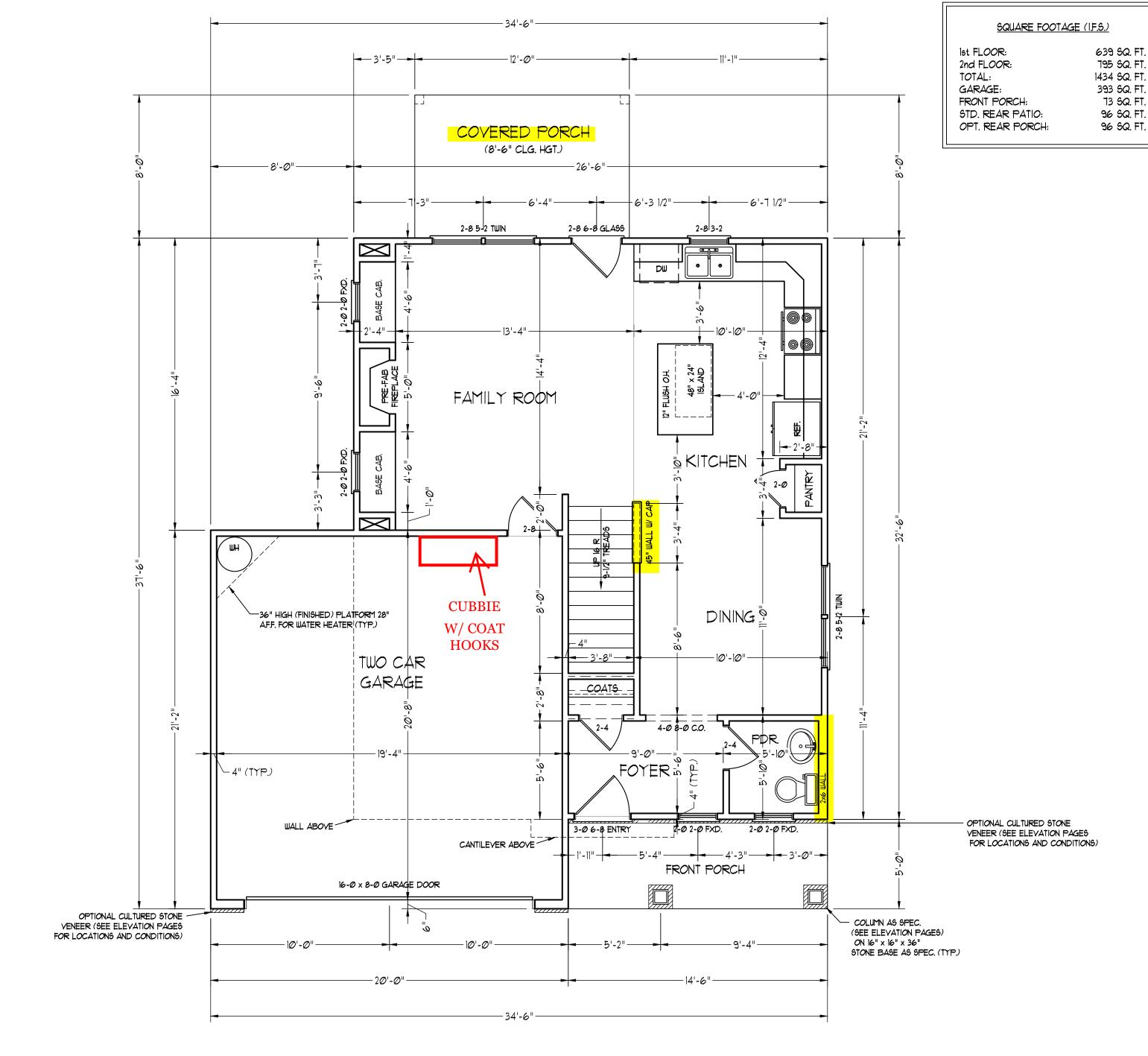




<u>RIGHT ELEVATION</u>

SCALE: 1/8" = 1'-0"

REAR ELEVATION SCALE: 1/8" = 1'-0"



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WEAVER HOMES CAROLINA COLLECTION HICKORY-II DRIVE LEFT

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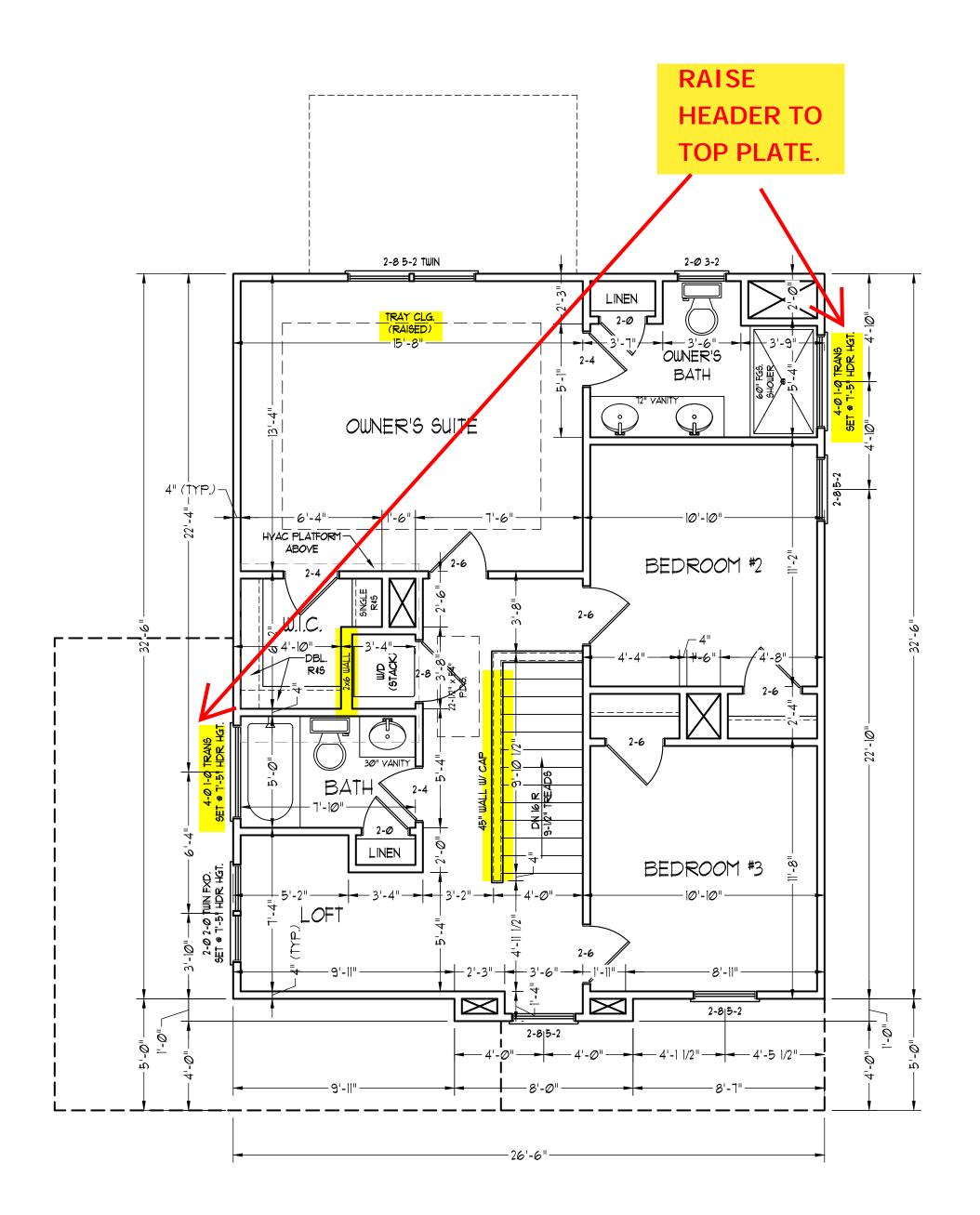
FIRST FLOOR PLAN

A-4

SCALE NOTE: 18x24 PRINTS ARE

TO SCALE AS NOTED.

11x17 PRINTS ARE NOT TO SCALE





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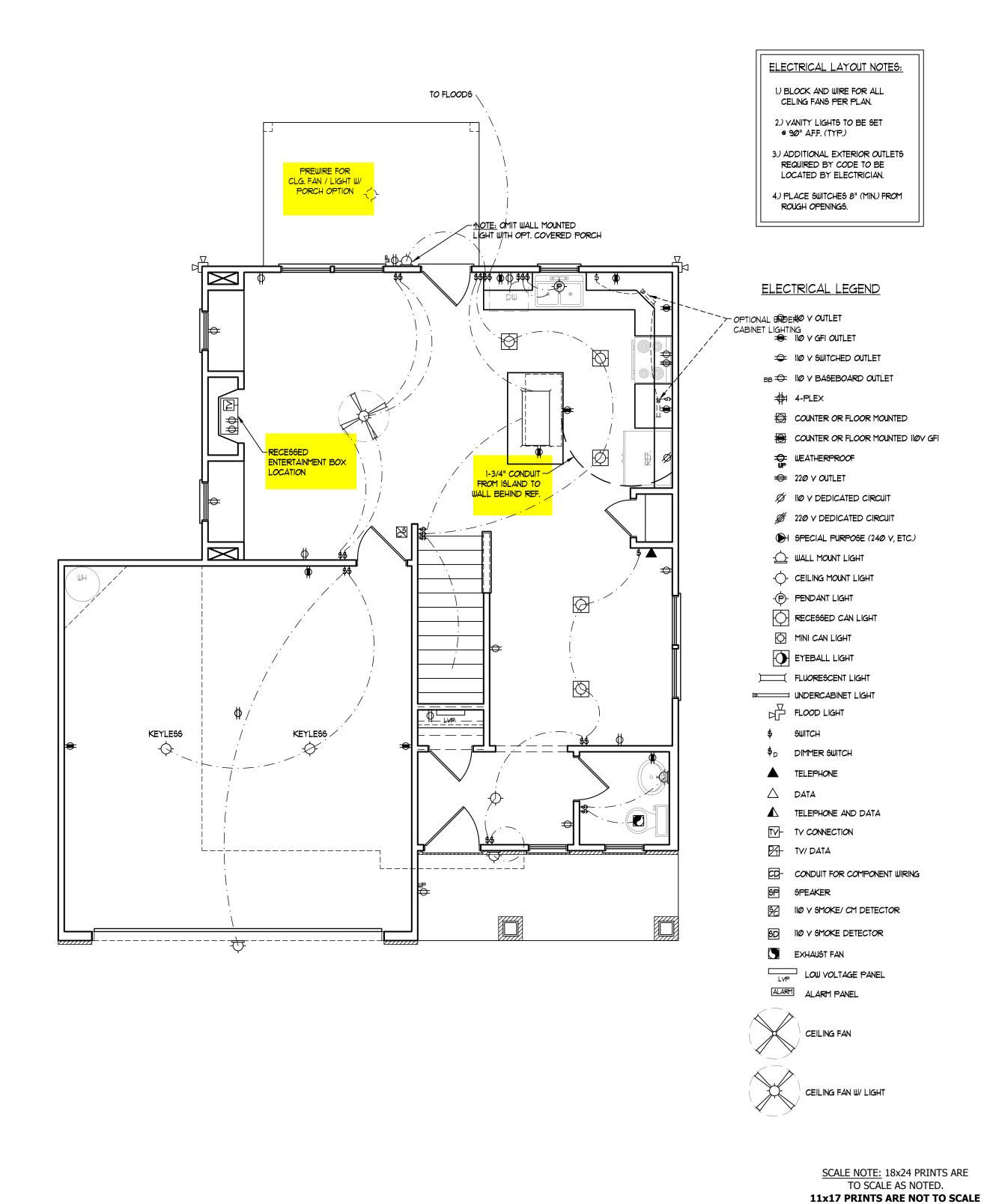
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SECOND FLOOR

PLAN

A-5

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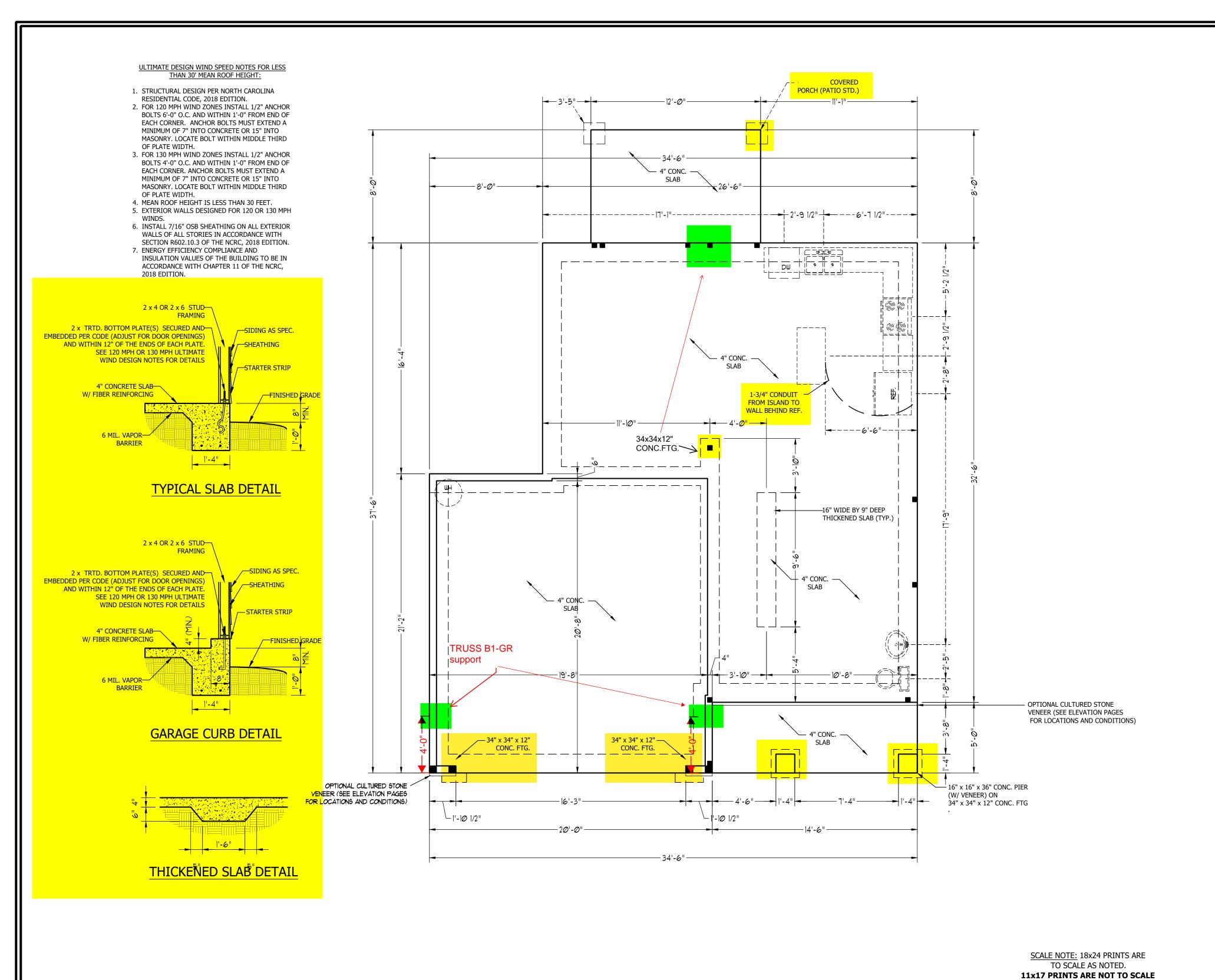
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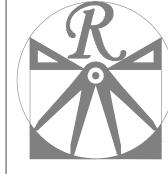
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FIRST FLOOR ELECTRICAL PLAN

E-1





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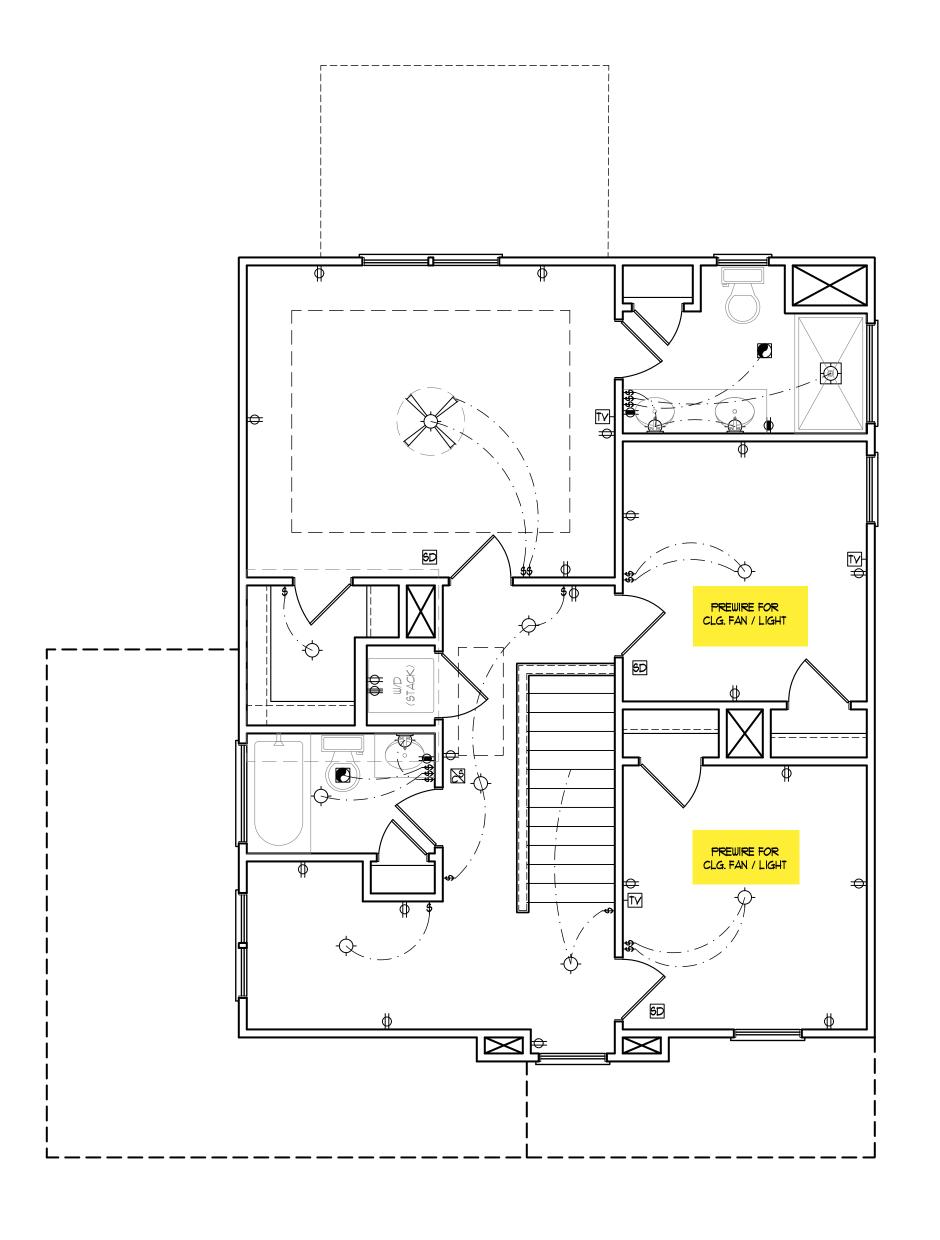
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ENGINEERED BY:

REVIEWED BY:

MONO SLAB FOUNDATION PLAN

S-1



ELECTRICAL LAYOUT NOTES:

- 1.) BLOCK AND WIRE FOR ALL CELING FANS PER PLAN.
- 2.) VANITY LIGHTS TO BE SET 9 90" AFF. (TYP.)
- 3.) ADDITIONAL EXTERIOR OUTLETS REQUIRED BY CODE TO BE LOCATED BY ELECTRICIAN.
- 4.) PLACE SWITCHES 8" (MIN.) FROM ROUGH OPENINGS.

ELECTRICAL LEGEND

- →
 I/Ø
 V
 OUTLET
- ₩ IIØ V GFI OUTLET
- → 110 Y SWITCHED OUTLET
- BB 110 Y BASEBOARD OUTLET
- 4-PLEX
- COUNTER OR FLOOR MOUNTED
- COUNTER OR FLOOR MOUNTED 110/ GFI
- ₩EATHERPROOF
- **⇒** 22Ø ∨ OUTLET
- Ø 110 V DEDICATED CIRCUIT
- # 220 Y DEDICATED CIRCUIT
- ●H SPECIAL PURPOSE (240 V, ETC.)
- WALL MOUNT LIGHT
- CEILING MOUNT LIGHT
- -P- PENDANT LIGHT
- RECESSED CAN LIGHT
- MINI CAN LIGHT
- EYEBALL LIGHT

FLUORESCENT LIGHT

UNDERCABINET LIGHT

FLOOD LIGHT

SWITCH

\$D DIMMER SWITCH

TELEPHONE

△ DATA

TELEPHONE AND DATA

TV- TV CONNECTION

TV/ DATA

CD- CONDUIT FOR COMPONENT WIRING

SP SPEAKER

MOKE/ CO DETECTOR

6D 110 V SMOKE DETECTOR

EXHAUST FAN

LOW VOLTAGE PANEL

ALARM PANEL



CEILING FAN III/ L

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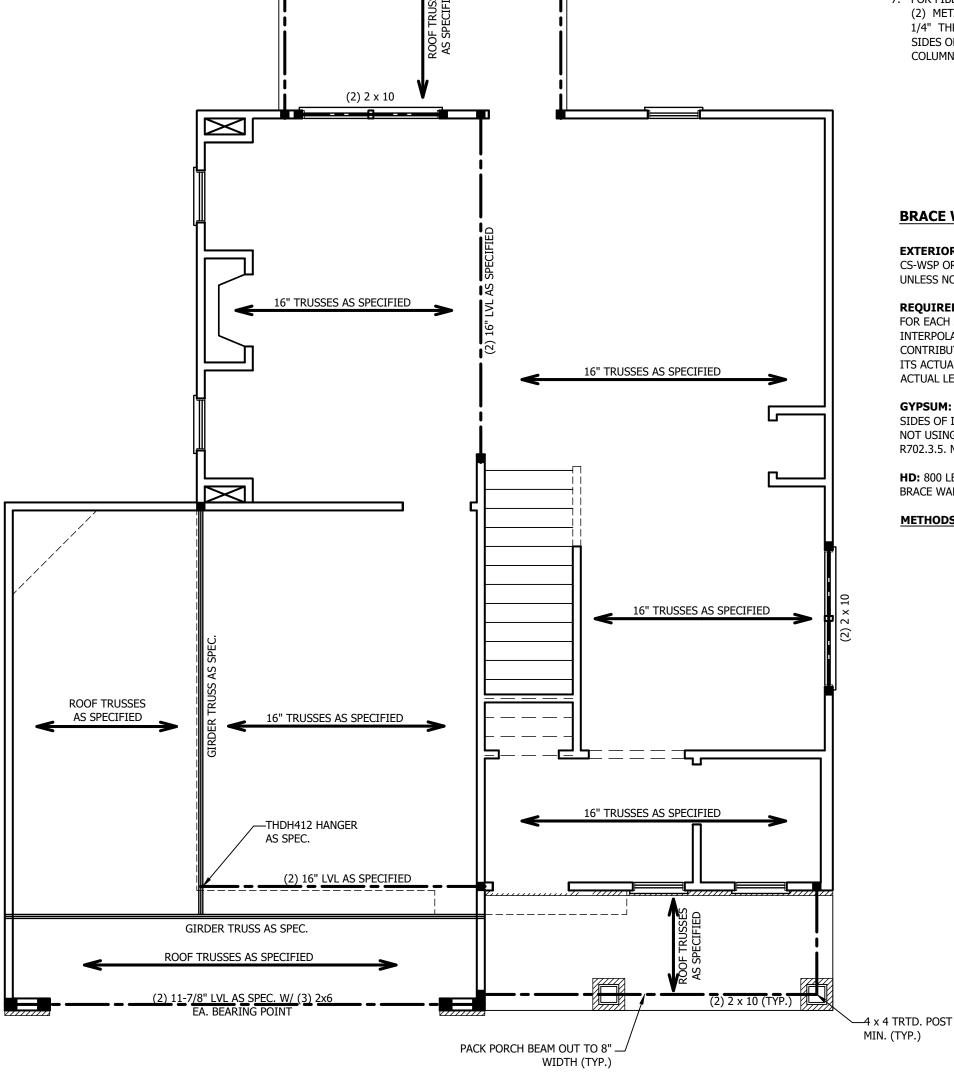
DRAWN BY: WG

ENGINEERED BY:

REVIEWED BY:
SECOND FLOOR

ELCTRICAL

PLAN E-2



STRUCTURAL NOTES:

-4 x 4 TRTD. POST

MIN. (TYP.)

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
- 4. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 5. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
- 6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS
- FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/
 (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/
 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE
 SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING
 COLUMN.

BRACE WALL PANEL NOTES:

EXTERIOR WALLS: ALL EXTERIOR WALLS TO BE SHEALTHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

REQUIRED LENGTH OF BRACING: REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THIER ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

GYPSUM: ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSUM INSTALLED. WHEN NOT USING METHOD GB GYPSUM TO BE FASTENED PER TABLE R702.3.5. METHOD GB TO BE FASTENED PER TABLE R602.10.1.

HD: 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

METHODS: PER TABLE R602.10.1



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REV.:

 $\frac{\text{SCALE: } 1/4" = 1' \cdot 0"}{\text{DRAWN BY: WG}}$

ENGINEERED BY:
REVIEWED BY:

SECOND FLOOR FRAMING PLAN

S-2

FASTEN TOP
PLATE TO
PLATE TO
PLATE TO
HEADER WITH
TWO ROWS OF
SID SIRNER NAILS
AT 3 O.C. TYP

MIN. 7x4" WOOD
STRUCTURAL
PANEL
SHEATHONG

MIN. 2X4 STUDS WITH
PONY WALL HEIGHT UP TO
2" MIN 2X6 STUDS WITH
PONY WALL HEIGHT
GREATER THAN 2".

NAIL SOLE
PLATE TO JOIST
PER TABLE
REQ. 2(1)

APPROVED BAND
OR RIM JOIST

APPROVED BAND
OR RIM JOIST
SECTION

 $\frac{\text{FRONT ELEVATION}}{\text{nch} = 25.4 \text{ mm}, 1 \text{ foot} = 305 \text{ mm}, 1 \text{ lb} = 4.45 \text{ N}.}$

OVER RAISED WOOD FLOOR - OVERLAP OPTION

OVER CONCRETE OR MASONRY BLOCK FOUNDATION

OVER RAISED WOOD FLOOR - FRAMING ANCHOR OPTION

FIGURE R602.10.1

METHOD PF—PORTAL FRAME CONSTRUCTION

TYPICAL PORTAL FRAME CONSTRUCTION

ANCHOR BOLTS PER SECTION R403.1.5

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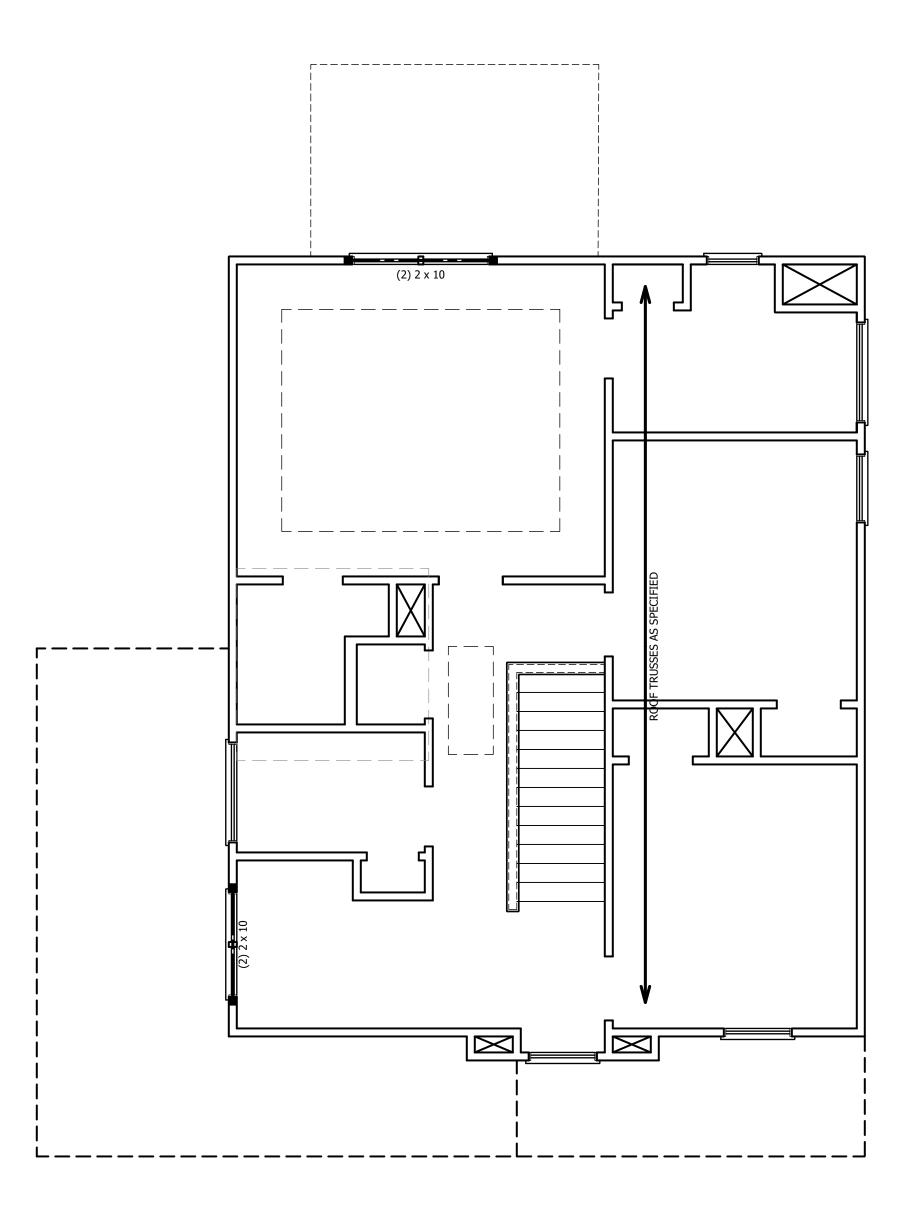


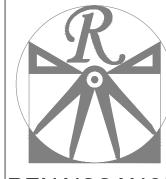
TABLE R602.7.5 MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (FEET)		SPACING (INCHES) E R602.3(5)
()	16	24
UP TO 3'	1	1
4'	2	1
8'	3	2
12'	5	3
16'	6	4

STRUCTURAL NOTES:

- ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET



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REVIEWED BY:

ATTIC FLOOR FRAMING PLAN

S-3

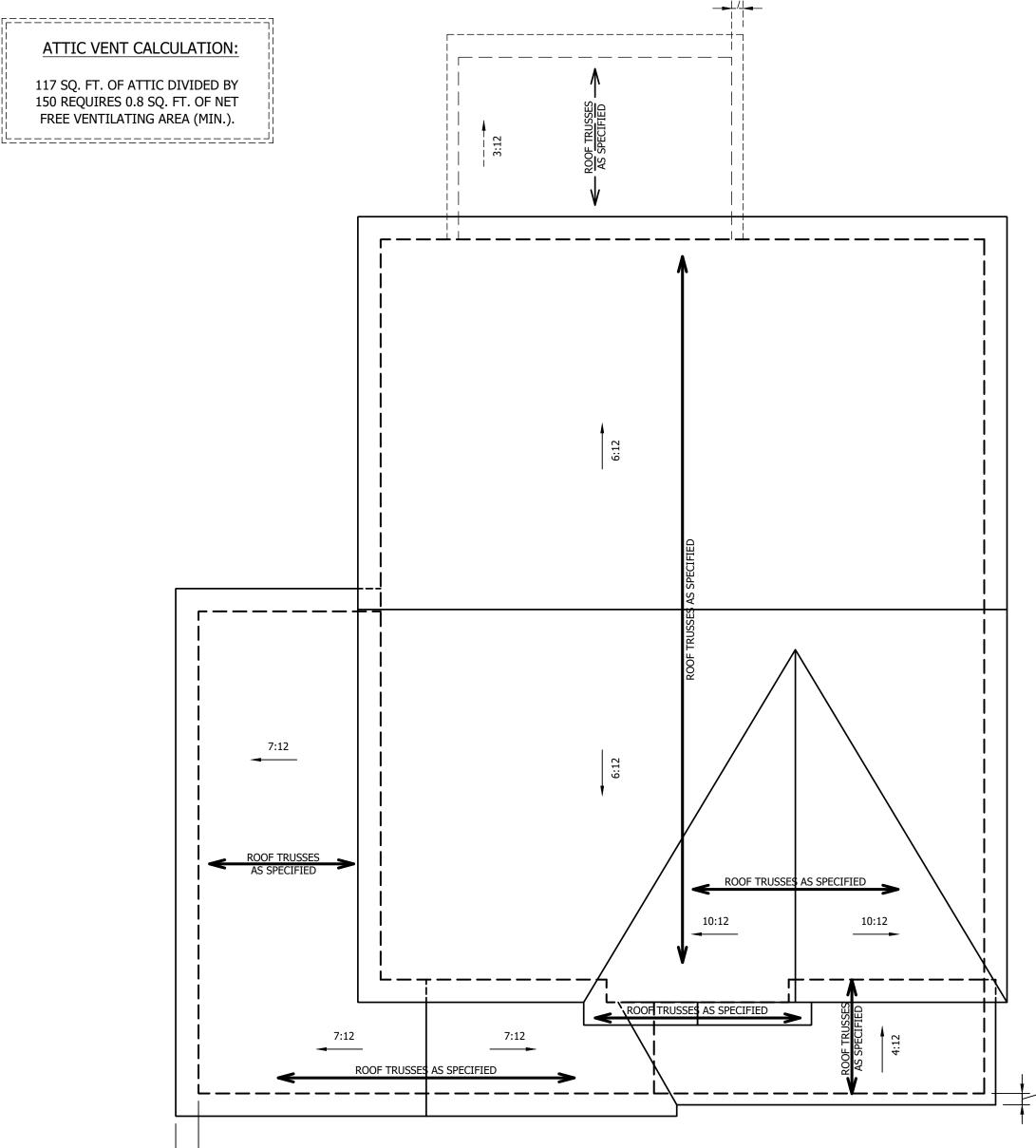
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ATTIC VENT CALCULATION:

117 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 0.8 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

12" O.H. (TYP.)

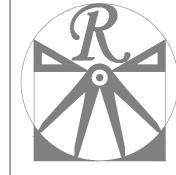


ATTIC VENT CALCULATION:

1302 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 8.7 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE #2 SPF (UNO).
- 2. HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
- 3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
- 4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
- 5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.



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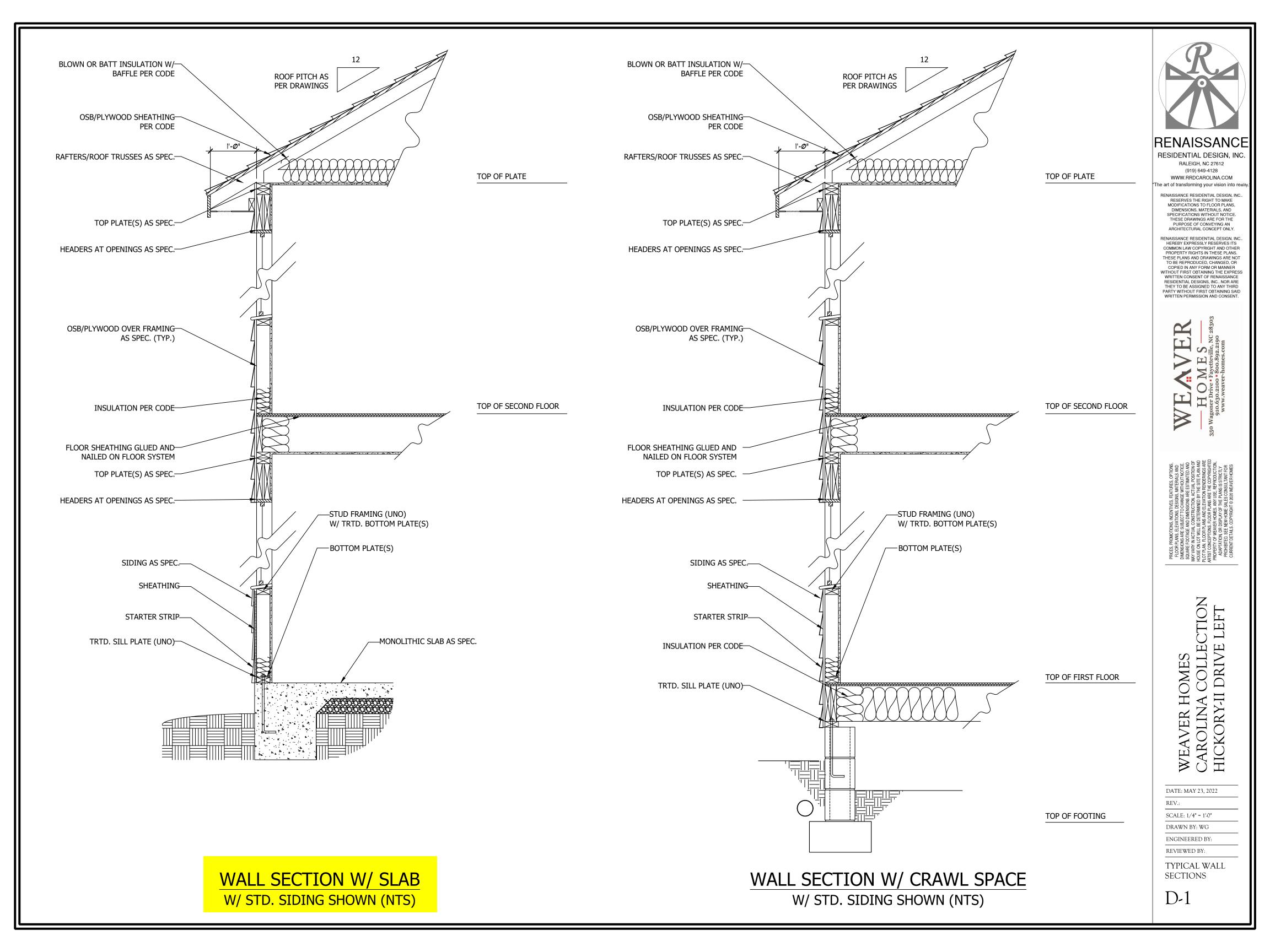
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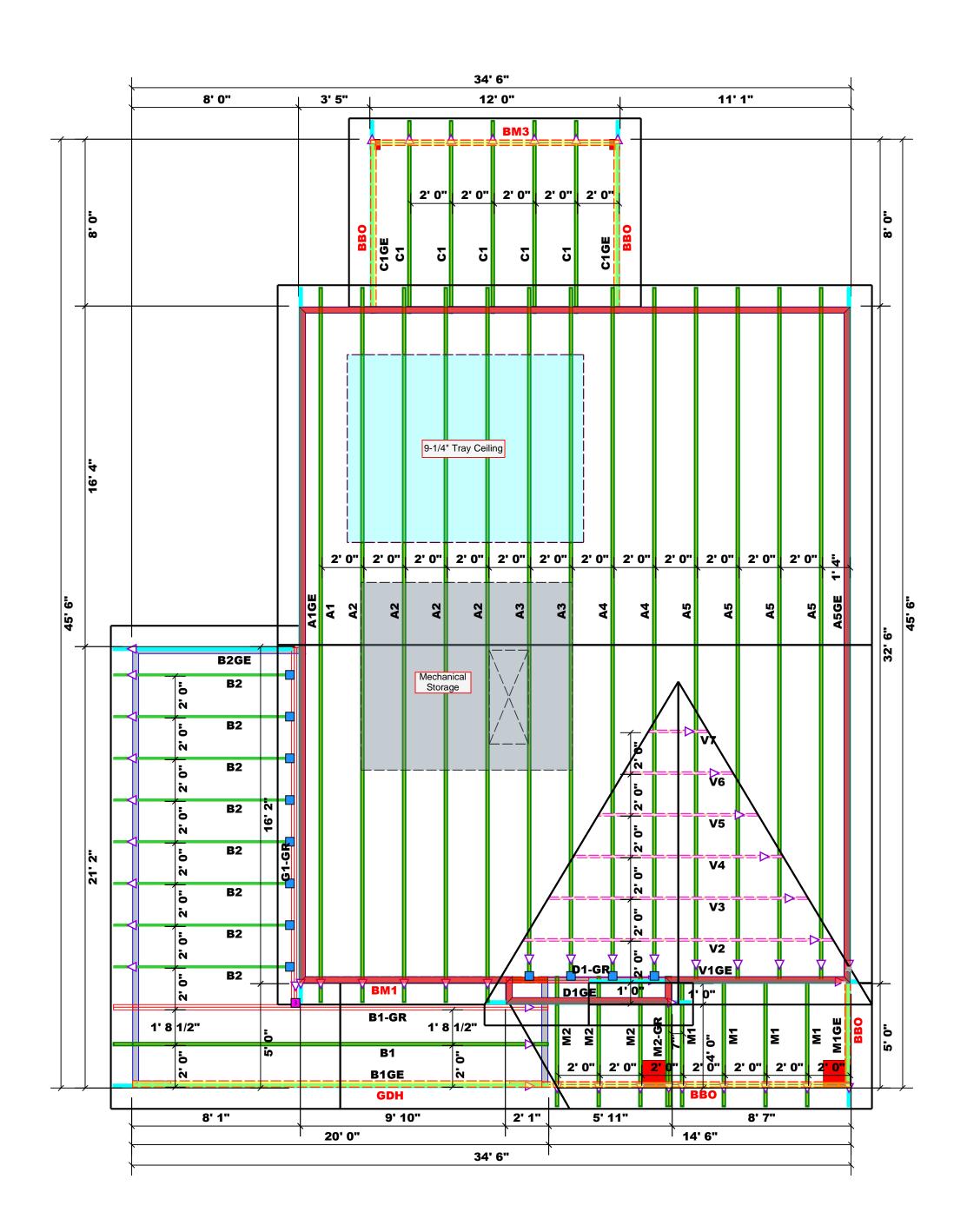
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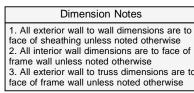
ROOF PLAN S-4

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED.

11x17 PRINTS ARE NOT TO SCALE







All Walls Shown Are Considered Load Bearing

Roof Area = 1692.08 sq.ft. Ridge Line = 52.07 ft. Hip Line = 0 ft. Horiz. OH = 115.69 ft. Raked OH = 175.3 ft. Decking = 58 sheets

Hatch Legend
Padded HVAC
2nd Floor Walls
Tray Ceiling
Drop Beam

	Conne	Nail Information				
Sym	Product	duct Manuf Qty Supported Member		Header	Truss	
	HUS26	USP	12	NA	16d/3-1/2"	16d/3-1/2"
3	THDH210-3	USP	1	Varies	16d/3-1/2"	16d/3-1/2"

Products							
PlotID	Length	Product	Plies	Net Qty	Fab Type		
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF		
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF		
BM3	12' 0"	2x10 SPF No.2	2	2	FF		
GDH	20' 0"	1-3/4"x 14" LVL Kerto-S	2	2	FF		

Truss Placement Plan
Scale: 1/4"=1'



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

dearing reactions less than or equal to 3000# are eemed to comply with the prescriptive Code equirements. The contractor shall refer to the ttached Tables (derived from the prescriptive Code equirements) to determine the minimum foundation ize and number of wood studs required to support eactions greater than 3000# but not greater than 5000#. A registered design professional shall be etained to design the support system for any eaction that exceeds those specified in the attached ables. A registered design professional shall be etained to design the support system for all eactions that exceed 15000#.

David Landry

David Landry

LOAD CHART FOR JACK STUDS

ı		(B	ASED O	N TABLE	S R502	5(1) & (1	2))		
ı	NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER								
	END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	8	END REACTION (UP TO)	4 4 CH	
ı	1700	1		2550	1		3400)	
ı	3400	2		5100	2		6800)	
	5100	3		7650	3		1020	0	
ı	6800	4		10200	4		1360	0	
ı	8500	5		12750	5		1700	0	
ı	10200	6		15300	6				
ı	11900	7							
ı	13600	8							
ı	15300	9							

CITY / CO.	CITY / CO. Sanford / Harnett
ADDRESS	Oleander Lane
WODEL	Roof
DATE REV.	01/18/23
DRAWN BY	DRAWN BY David Landry
SALES REP.	Lenny Norris

JOB NAME THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

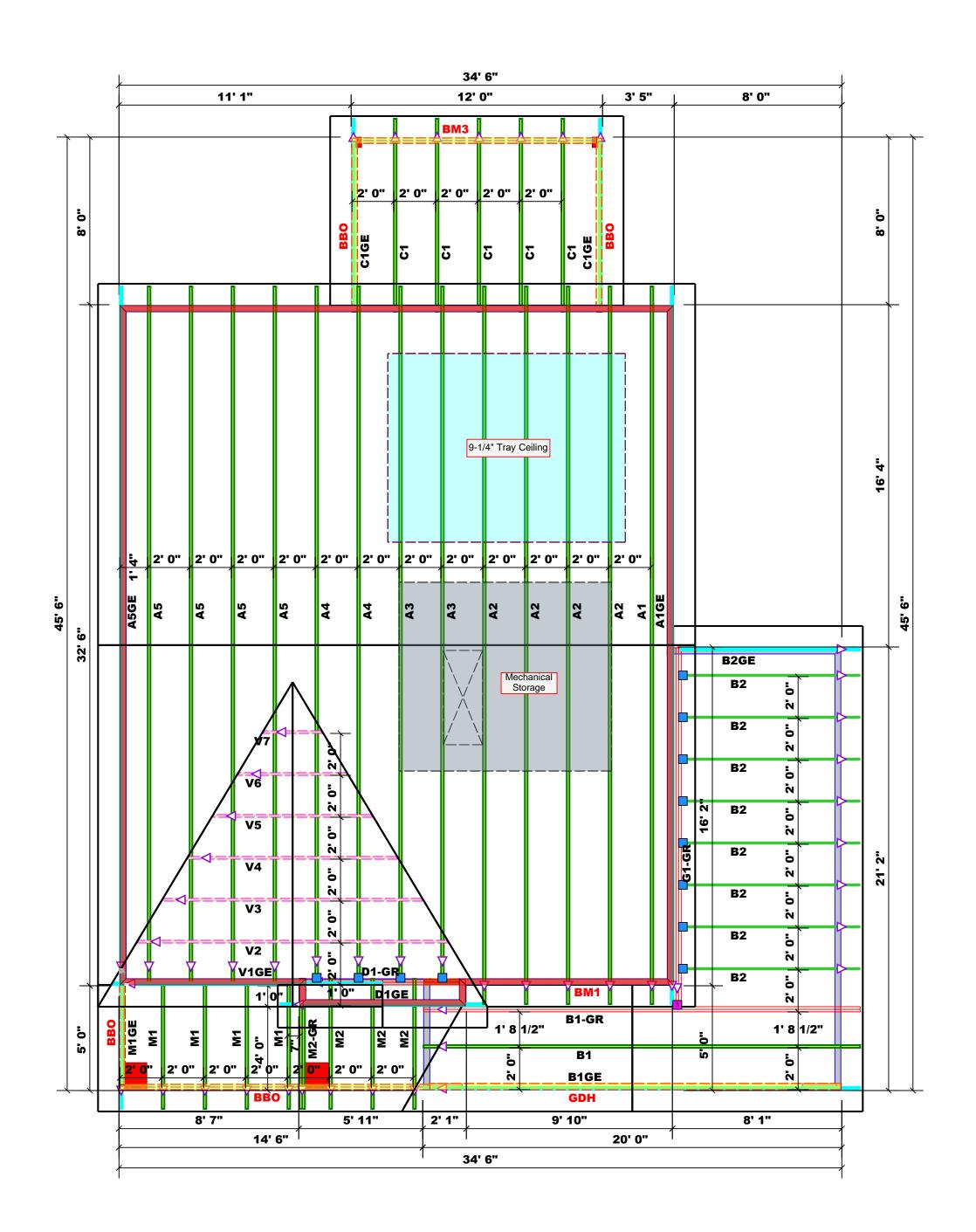
N/A

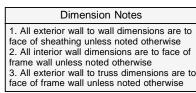
J0123-0221

Weaver Development

BUILDER

Lot 25 West Preser





All Walls Shown Are Considered Load Bearing

Roof Area = 1692.08 sq.ft. Ridge Line = 52.07 ft. Hip Line = 0 ft. Horiz. OH = 115.69 ft. Raked OH = 175.3 ft. Decking = 58 sheets

	_					
Hatch Legend						
	Padded HVAC					
	2nd Floor Walls					
	Tray Ceiling					
	Drop Beam					

	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	12	NA	16d/3-1/2"	16d/3-1/2"
3	THDH210-3	USP	1	Varies	16d/3-1/2"	16d/3-1/2"

		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM3	12' 0"	2x10 SPF No.2	2	2	FF
GDH	20' 0"	1-3/4"x 14" LVL Kerto-S	2	2	FF

Truss Placement Plan
Scale: 1/4"=1'



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

David Landry

David Landry

LOAD CHART FOR JACK STUDS
(BASED ON TABLES R502.5(1) & (b))

	(D	1000	IN INDEL	J KJUL	.J(1) U (1	-,,			
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER									
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER		END REACTION (UP TO)			
1700	1		2550	1		3400)		
3400	2		5100	2		6800)		
5100	3		7650	3		1020	0		
6800	4		10200	4		1360	0		
8500	5		12750	5		1700	0		
10200	6		15300	6					
11900	7								
13600	8								
15300	9								

CITY / CO.	CITY / CO. Sanford / Harnett
ADDRESS	Oleander Lane
WODEL	Roof
DATE REV.	01/18/23
DRAWN BY	DRAWN BY David Landry
SALES REP.	Lenny Norris

BUILDER Weaver Development

JOB NAME Lot 25 West Preserve

PLAN Hickory II "C" / 2GLF, CP

SEAL DATE N/A

QUOTE#

J0123-0221

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



RE: J0123-0221

Lot 25 West Preserve

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Weaver Development Project Name: J0123-0221 Lot/Block: 25 Model: Hickory II

Address: Oleander Lane Subdivision: West Preserve

City: Sanford State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4

Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 28 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	156188092	A1	1/18/2023	21	156188112	M2-GR	1/18/2023
2	156188093	A1GE	1/18/2023	22	I56188113	V1GE	1/18/2023
3	156188094	A2	1/18/2023	23	156188114	V2	1/18/2023
4	156188095	A3	1/18/2023	24	156188115	V3	1/18/2023
5	156188096	A4	1/18/2023	25	I56188116	V4	1/18/2023
6	156188097	A5	1/18/2023	26	I56188117	V5	1/18/2023
7	156188098	A5GE	1/18/2023	27	I56188118	V6	1/18/2023
8	156188099	B1	1/18/2023	28	I56188119	V7	1/18/2023
9	156188100	B1-GR	1/18/2023				
10	156188101	B1GE	1/18/2023				
11	156188102	B2	1/18/2023				
12	156188103	B2GE	1/18/2023				
13	156188104	C1	1/18/2023				
14	156188105	C1GF	1/18/2023				

1/18/2023

1/18/2023

1/18/2023

1/18/2023

1/18/2023

1/18/2023

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

D1-GR

D1GE

G1-GR

M1GE

M1

M2

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2023

North Carolina COA: C-0844

156188106

156188107

156188108

156188109

156188110

156188111

15

16

17

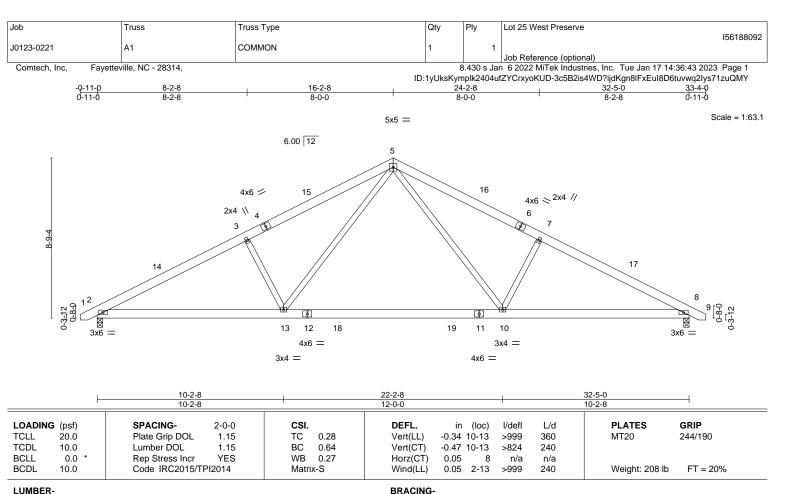
18

19

20

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

WEBS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-110(LC 10)

Max Uplift 2=-89(LC 12), 8=-89(LC 13) Max Grav 2=1337(LC 1), 8=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS 5-10=-147/921, 7-10=-454/288, 5-13=-147/921, 3-13=-454/288

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2 and 89 lb uplift at ioint 8.
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 4-11-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Lot 25 West Preserve 156188093 COMMON SUPPORTED GAB J0123-0221 A1GE Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:36:46 2023 Page 1

ID:1yUksKymplk2404ufZYCrxyoKUD-TBnKgkvyp8NHa53FSGlyZsWtAQHN5ILGkGAWkLzuQMV 16-2-8

L/d

120

120

n/a

(loc)

20

20

20

0.00

0.00

0.00

I/def

n/r

n/r

n/a

PLATES

Weight: 258 lb

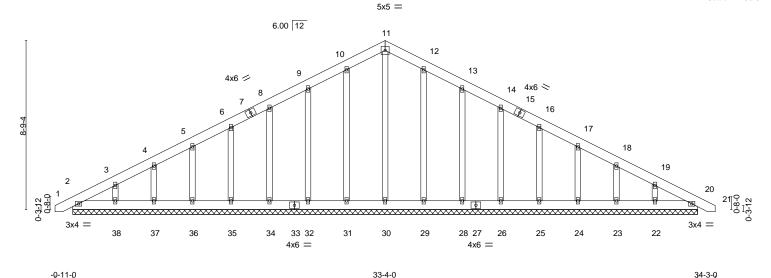
MT20

GRIP

244/190

FT = 20%

Scale = 1:59.8



DEFL.

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-LUMBER-

2-0-0

1.15

1.15

YES

16-2-8

TOP CHORD 2x6 SP No.1 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2x6 SP No.1 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 32-5-0.

2x4 SP No.2

-0-11-0 0-11-0

(lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22 Max Grav All reactions 250 lb or less at joint(s) 2, 30, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23,

CSI

TC

ВС

WB

Matrix-S

0.04

0.02

0.16

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 10-11=-114/284, 11-12=-114/284

LOADING (psf)

TCLL

TCDL

BCLL

BCDL

OTHERS

20.0

10.0

0.0

10.0

1) Unbalanced roof live loads have been considered for this design.

SPACING-

Plate Grip DOL

Rep Stress Incr

Code IRC2015/TPI2014

Lumber DOL

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOI = 1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22,
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Fayetteville, NC - 28314, Comtech, Inc.

6-0-0

ID:1yUksKymplk2404ufZYCrxyoKUD-PZu45PwDLld?pPCeZhKQeHc21EppZ2IZCafdpEzuQMT 18-11-8 23-11-8 29-10-8 32-2-0 33-1-0 2-3-8 0-11-0 3-0-0 5-0-0 5-11-0

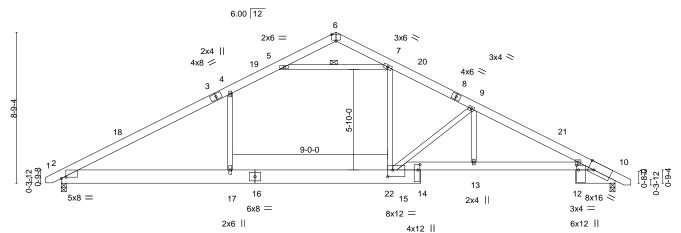
Scale = 1:66.9 4x6 =

Structural wood sheathing directly applied or 4-1-2 oc purlins.

5-7

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt



		9-11-8	18-1	1-8	20-10-8	23-11-8	29-10-8	32-2-0	
		9-11-8	9-0)-0	1-11-0	3-1-0	5-11-0	2-3-8	
Plate Offs	sets (X,Y)	[2:0-3-2,0-0-14], [6:0-3-0,Edge], [10:0	-4-0,Edge], [12:0-3-4,0-1-4	1], [14:0-3-12,0-1-8	3], [15:0-3	-8,0-4-12]			
LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc) I/de	fl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.72	Vert(LL)	-0.20	17 >99	9 360	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.66	Vert(CT)	-0.35	17 >99	9 240		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.71	Horz(CT)	0.09	10 n/	'a n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	0.15	2-17 >99	9 240	Weight: 245 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

2x6 SP No.1 TOP CHORD

BOT CHORD 2x10 SP No.1 *Except*

10-15: 2x6 SP 2400F 2.0E

WEBS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 10=0-3-8

Max Horz 2=-111(LC 10)

Max Uplift 2=-88(LC 12), 10=-90(LC 13) Max Grav 2=1386(LC 2), 10=1343(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-2191/400, 4-5=-1838/478, 7-9=-2220/513, 9-10=-2864/547 **BOT CHORD** 2-17=-187/1815, 15-17=-189/1827, 13-15=-368/2503, 10-13=-377/2503 **WEBS** 7-15=-110/948, 4-17=-17/415, 5-7=-1904/450, 9-15=-1085/235, 9-13=0/625

9-11-8

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.





Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188095 J0123-0221 **A3 ROOF SPECIAL** 2 Job Reference (optional)

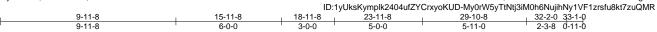
Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:36:50 2023 Page 1

Structural wood sheathing directly applied or 3-10-0 oc purlins.

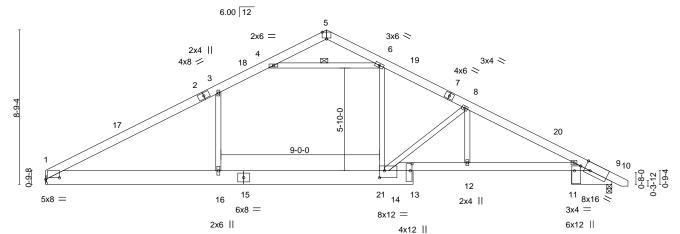
4-6

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt



Scale = 1:65.4 4x6 =



1	9-11-8	18-11-8	20-10-8 23-11-8	29-10-8 ₁ 32-2-0)
	9-11-8	9-0-0	1-11-0 3-1-0	5-11-0 2-3-8	
Plate Offsets (X,Y)	[1:0-9-6,0-1-2], [5:0-3-0,Edge], [9:0-4-0,E	Edge], [11:0-3-4,Edge], [13:0-4-12,	0-1-4], [14:0-3-8,0-4-12]		
LOADING (psf)	SPACING- 2-0-0	CSI. DE	FL. in (loc) I/defl	L/d PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.76 Ve	rt(LL) -0.21 16 >999	360 MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.66 Ve	rt(CT) -0.36 16 >999	240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.70 Ho	rz(CT) 0.09 9 n/a	n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S Wi	nd(LL) 0.16 1-16 >999	240 Weight: 24	43 lb FT = 20%
				[

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1

BOT CHORD 2x10 SP No.1 *Except*

9-14: 2x6 SP 2400F 2.0E

WEBS 2x4 SP No.2

REACTIONS. (size) 1=Mechanical, 9=0-3-8

Max Horz 1=-111(LC 8)

Max Uplift 1=-76(LC 12), 9=-90(LC 13) Max Grav 1=1345(LC 2), 9=1347(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-3=-2189/401, 3-4=-1853/491, 6-8=-2237/517, 8-9=-2874/549 **BOT CHORD** 1-16=-198/1827, 14-16=-200/1839, 12-14=-375/2511, 9-12=-384/2511 **WEBS** 6-14=-117/966, 3-16=-53/392, 4-6=-1931/474, 8-14=-1081/228, 8-12=0/620

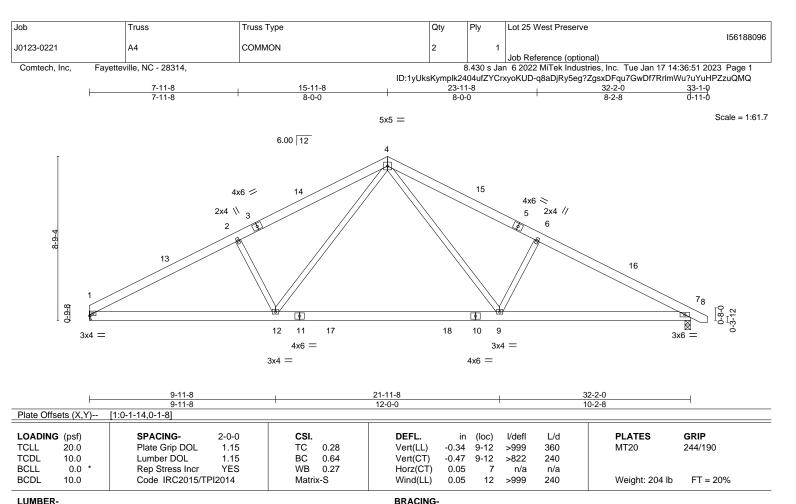
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



January 18,2023





TOP CHORD

BOT CHORD

LUMBER-

2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

REACTIONS.

(size) 1=Mechanical, 7=0-3-8

Max Horz 1=-111(LC 8)

Max Uplift 1=-76(LC 12), 7=-89(LC 13) Max Grav 1=1278(LC 1), 7=1331(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2276/496, 2-4=-2096/546, 4-6=-2113/532, 6-7=-2294/484

BOT CHORD 1-12=-319/1973, 9-12=-109/1291, 7-9=-324/1953

WFBS 4-9=-147/922, 6-9=-454/288, 4-12=-144/897, 2-12=-437/286

NOTES-

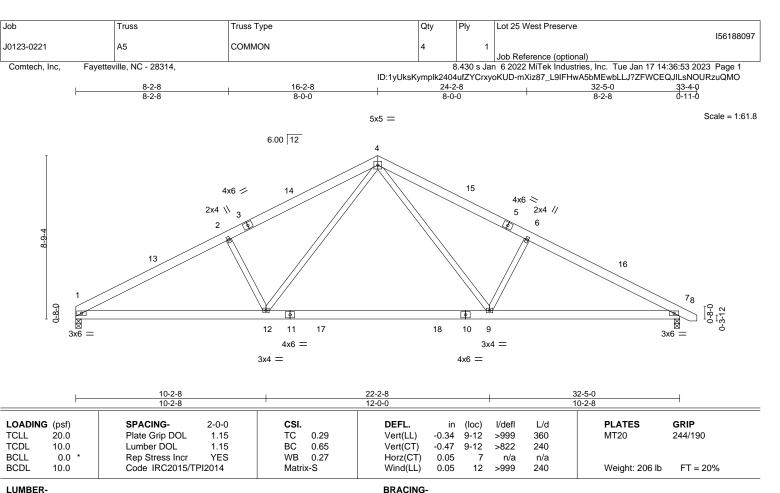
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-11-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

WEBS 2x4 SP No.2

REACTIONS. (size)

1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

BOT CHORD 1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS 4-9=-147/921, 6-9=-454/288, 4-12=-149/924, 2-12=-458/292

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-10-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Job Truss Truss Type Qty Lot 25 West Preserve 156188098 J0123-0221 A5GE COMMON SUPPORTED GAB Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:36:55 2023 Page 1

ID:1yUksKymplk2404ufZYCrxyoKUD-ivpkZo0chvV?9UF_Ufz3QmOPu2MUiMabp9sVYKzuQMM 16-2-8

L/d

120

120

n/a

(loc)

19

19

19

0.00

0.00

0.00

I/def

n/r

n/r

n/a

PLATES

Weight: 256 lb

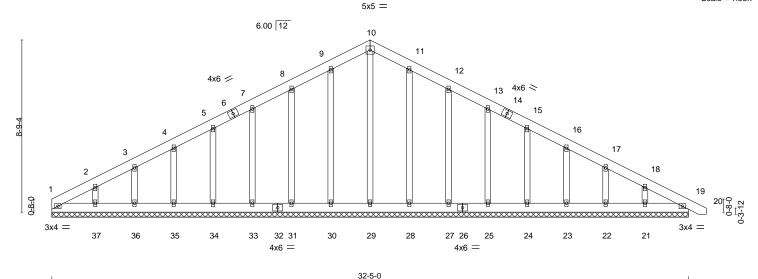
MT20

GRIP

244/190

FT = 20%

Scale = 1:58.7



DEFL.

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-LUMBER-

2-0-0

1.15

1.15

YES

16-2-8 16-2-8

TOP CHORD TOP CHORD 2x6 SP No.1 Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2x6 SP No.1 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. **OTHERS** 2x4 SP No.2

REACTIONS. All bearings 32-5-0. Max Horz 1=-175(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except

CSI.

TC

ВС

WB

Matrix-S

0.04

0.02

0.16

37=-101(LC 12)

SPACING-

Plate Grip DOL

Rep Stress Incr

Code IRC2015/TPI2014

Lumber DOL

All reactions 250 lb or less at joint(s) 1, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22, Max Grav

21, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 9-10=-114/284, 10-11=-114/284

NOTES-

LOADING (psf)

TCLL

TCDL

BCLL

BCDL

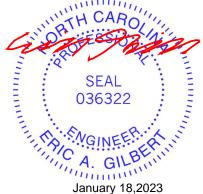
20.0

10.0

0.0

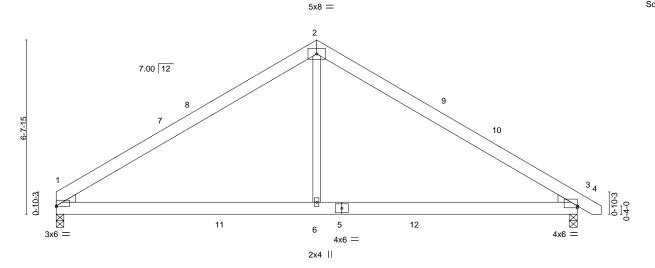
10.0

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.





Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188099 J0123-0221 B1 COMMON Job Reference (optional) Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:36:57 2023 Page 1 Comtech, Inc. ID:1yUksKymplk2404ufZYCrxyoKUD-flxU_U1sDWmjOnPNb4?XVBTdwsyNAGEuGTLcdDzuQMK 20-10-0 0-11-0 9-11-8 9-11-8 Scale = 1:44.0



9-11-8 Plate Offsets (X,Y)--[1:0-0-0,0-0-7], [3:0-0-0,0-0-15] LOADING (psf) SPACING-CSI. DEFL. in (loc) I/defI L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.52 Vert(LL) -0.06 3-6 >999 360 244/190 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.44 Vert(CT) -0.13 3-6 >999 240 BCLL 0.0 Rep Stress Incr YES WB 0.15 Horz(CT) 0.02 3 n/a n/a Code IRC2015/TPI2014 **BCDL** 10.0 Wind(LL) 0.04 3-6 >999 240 FT = 20%Matrix-S Weight: 114 lb

BRACING-

TOP CHORD

BOT CHORD

19-11-0

Structural wood sheathing directly applied or 5-11-11 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1 WEBS 2x4 SP No.2

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz 1=-149(LC 8)

Max Uplift 1=-43(LC 12), 3=-56(LC 13) Max Grav 1=900(LC 19), 3=951(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1193/217, 2-3=-1221/216

BOT CHORD 1-6=-30/956, 3-6=-30/956

WEBS 2-6=0/660

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 9-11-8, Exterior(2) 9-11-8 to 14-4-5, Interior(1) 14-4-5 to 20-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.





Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188100 J0123-0221 B1-GR COMMON GIRDER Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:00 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-3tddcW3kWR8IFF7xGDYE7p5Cq3?eNXeKyRZGEXzuQMH 20-10-0 0-11-0 14-11-8 19-11-0

5-0-0

5-0-0

Scale = 1:42.0 5x12 ||

19-11-0

Structural wood sheathing directly applied or 4-11-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

4-11-8

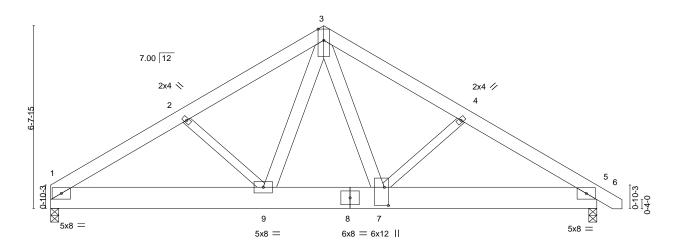


Plate Offsets (X,Y)--[7:0-8-0,0-1-12] LOADING (psf) SPACING-2-0-0 CSI DEFL. in (loc) I/def L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.28 Vert(LL) -0.07 >999 360 244/190 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.27 Vert(CT) -0.14 >999 240 **BCLL** 0.0 Rep Stress Incr NO WB 0.55 Horz(CT) 0.03 5 n/a n/a Code IRC2015/TPI2014 **BCDL** 10.0 Wind(LL) >999 240 Weight: 348 lb FT = 20%Matrix-S 0.05 7

BRACING-

TOP CHORD

BOT CHORD

12-2-0

LUMBER-

TOP CHORD 2x6 SP No.1 2x10 SP 2400F 2.0E **BOT CHORD** WEBS 2x6 SP No.1 *Except*

4-7,2-9: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 5=0-3-8

Max Horz 1=-148(LC 6)

Max Uplift 1=-423(LC 8), 5=-642(LC 9) Max Grav 1=4230(LC 2), 5=6213(LC 2)

4-11-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 1-2=-7512/795, 2-3=-7415/798, 3-4=-10654/1144, 4-5=-10705/1136 TOP CHORD

7-9-0

BOT CHORD 1-9=-679/6250, 7-9=-581/6245, 5-7=-887/8926

WEBS 3-7=-953/8878, 4-7=-281/486, 3-9=-44/404, 2-9=-284/309

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x10 - 4 rows staggered at 0-2-0 oc. Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=423, 5=642.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 9024 lb down and 972 lb up at 12-1-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 3-6=-60, 1-5=-20

Concentrated Loads (lb) Vert: 7=-7513(B)



January 18,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Lot 25 West Preserve 156188101 J0123-0221 B1GE COMMON SUPPORTED GAB Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:36:58 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-7UVsBq2U_qua0xzZ9oWm2O0wNGNDvki1V7499fzuQMJ 20-10-0 0-11-0 9-11-8 9-11-8 Scale = 1:42.1 5x5 = 6 7 7.00 12 9 3 10 0-10-3 3x4 =3x4 =22 21 20 19 18 17 16 15 14 13 4x6 = 19-11-0

19-11-0

DEFL.

BOT CHORD

BCLL 0.0 YES BCDL 10.0 Code IRC2015/TPI2014 Matrix-S

2-0-0

20.0 Plate Grip DOL TC Vert(LL) 0.00 1.15 0.03 10.0 Lumber DOL 1.15 ВС 0.02 Vert(CT) 0.00 Rep Stress Incr WB 0.07 Horz(CT) 0.00 **BRACING-**TOP CHORD

CSI.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

PLATES

Weight: 146 lb

MT20

GRIP

244/190

FT = 20%

L/d

120

120

n/a

(loc)

11

11

11

I/def

n/r

n/r

n/a

REACTIONS. All bearings 19-11-0.

2x6 SP No.1

2x6 SP No.1

2x4 SP No.2

Max Horz 1=-186(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 19, 20, 21, 16, 15, 14, 11 except 22=-116(LC 12),

13=-103(LC 13)

SPACING-

Max Grav All reactions 250 lb or less at joint(s) 1, 18, 19, 20, 21, 22, 16, 15, 14, 13, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

LOADING (psf)

TCLL

TCDL

LUMBER-

OTHERS

TOP CHORD

BOT CHORD

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 19, 20, 21, 16, 15, 14, 11 except (jt=lb) 22=116, 13=103.



January 18,2023



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Lot 25 West Preserve 156188102 J0123-0221 B2 JACK-CLOSED 8 Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:02 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-?GIN1C5?23O0VYHKOdbiCEBXeti6rZndQl2NIQzuQMF

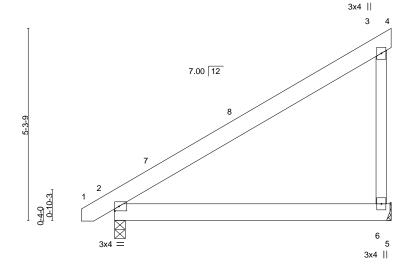
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

-0-11-0 0-11-0 7-7-8 7-7-8

Scale: 3/8"=1'



7-7-8

LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.33	Vert(LL)	-0.04	2-6	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.21	Vert(CT)	-0.07	2-6	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2	014	Matri	(-P	Wind(LL)	0.00	2	****	240	Weight: 48 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

2x4 SP No.2

6=Mechanical, 2=0-3-8 (size) Max Horz 2=158(LC 12) Max Uplift 6=-82(LC 12)

Max Grav 6=318(LC 19), 2=345(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-6=-288/220

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-0 to 3-7-13, Interior(1) 3-7-13 to 7-7-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6.





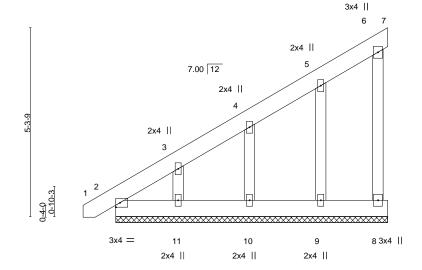
Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188103 J0123-0221 B2GE MONOPITCH SUPPORTED Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:03 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-TSIIFY6dpMWt6isWyL6xlSjn1H5Ta0ZmePowqszuQME

-0-11-0 0-11-0 7-7-8 7-7-8

Scale: 3/8"=1'



LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.03 BC 0.01	DEFL. i Vert(LL) -0.00 Vert(CT) -0.00	l/defl n/r n/r	L/d 120 120	PLATES GRIP MT20 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.03 Matrix-P	Horz(CT) -0.00	n/a	n/a	Weight: 57 lb FT = 20%

LUMBER-BRACING-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

2x4 SP No.2 WEBS **OTHERS** 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 7-7-8.

Max Horz 2=228(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 7, 8, 9, 10 except 11=-117(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 7, 8, 2, 9, 10, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

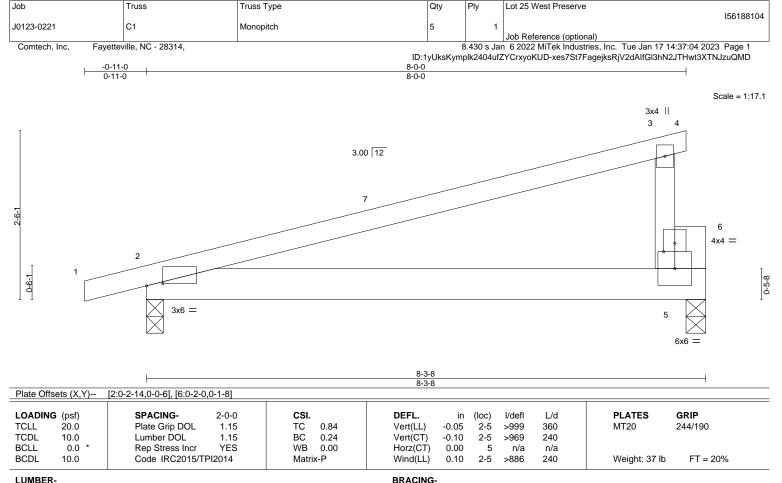
TOP CHORD 2-3=-269/227

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 8, 9, 10 except (jt=lb) 11=117.







TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1

WEBS 2x4 SP No.2 **OTHERS** 2x6 SP No.1

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8)

Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.



Structural wood sheathing directly applied or 5-3-4 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

b		Truss	Truss Type	Qty	Ply	Lot 25 West Preserve		150400405
123-0221		C1GE	GABLE	2	1			I56188105
125-0221		CIGL	GABLE	2	'	Job Reference (optional)		
Comtech, Inc,	Fayettev	rille, NC - 28314,			8.430 s Ja	an 6 2022 MiTek Industries, Inc	. Tue Jan 17 14:37:06 20:	23 Page 1
				ID:1yUksKymplk	2404ufZYC	rxyoKUD-u1_utZ8V6HuRzAb5c	dTfeN4LCIU2InMYCKN0at	RBzuQMB
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			2x4	2x4	H	2x4		
							5x10 M18AHS =	

BRACING-

TOP CHORD

BOT CHORD

Plate Offsets (X,	Plate Offsets (X,Y) [2:0-2-14,0-0-6], [8:Edge,0-2-0], [12:0-2-0,0-1-8]								
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP					
TCLL 20.0	Plate Grip DOL 1.15	TC 0.35	Vert(LL) 0.09 10-11 >999 240	MT20 244/190					
TCDL 10.0	Lumber DOL 1.15	BC 0.26	Vert(CT) -0.08 10-11 >999 240	M18AHS 186/179					
BCLL 0.0	* Rep Stress Incr YES	WB 0.01	Horz(CT) -0.00 8 n/a n/a						
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S		Weight: 41 lb FT = 20%					

LUMBER-

TOP CHORD 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1 **WEBS** 2x4 SP No.2

OTHERS 2x4 SP No.2 *Except* 8-12: 2x6 SP No.1

REACTIONS. (size) 2=0-3-0, 8=0-3-8

Max Horz 2=105(LC 8)

Max Uplift 2=-216(LC 8), 8=-188(LC 8) Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. BOT CHORD 2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216, 8=188.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188106 J0123-0221 D1-GR Common Girder Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:09 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-lcg0VbAOOCG0qdJglbDL?jzj1i0B_dVf1LFE2WzuQM8

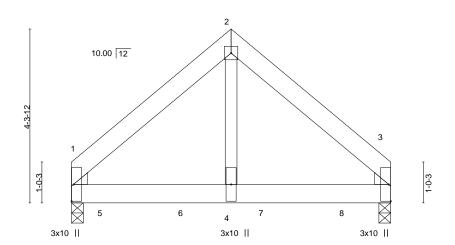
3-11-8 3-11-8

4x4 ||

Scale = 1:28.6

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



BRACING-

TOP CHORD

BOT CHORD

3-11-8 7-11-0 3-11-8

LOADING (psf) SPACING-2-0-0 CSI. DEFL. L/d (loc) I/defl 20.0 Plate Grip DOL -0.02 **TCLL** 1.15 TC 0.38 Vert(LL) 3-4 >999 360 TCDL 10.0 Lumber DOL 1.15 ВС 0.57 Vert(CT) -0.04 3-4 >999 240 **BCLL** 0.0 Rep Stress Incr NO WB 0.39 Horz(CT) 0.01 3 n/a n/a Code IRC2015/TPI2014 **BCDL** 10.0 Matrix-P Wind(LL) 0.01 3-4 >999 240

PLATES GRIP 244/190 MT20

Weight: 100 lb FT = 20%

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz 1=91(LC 24) Max Uplift 1=-191(LC 8), 3=-180(LC 9) Max Grav 1=2919(LC 1), 3=2779(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2418/177, 2-3=-2418/177 BOT CHORD 1-4=-100/1678, 3-4=-100/1678

WEBS 2-4=-154/3142

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=191, 3=180.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1261 lb down and 93 lb up at 0-9-12, 1258 lb down and 96 lb up at 2-9-12, and 1325 lb down and 96 lb up at 4-9-12, and 1325 lb down and 96 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 1-3=-20



January 18,2023

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Ply Lot 25 West Preserve I56188106 J0123-0221 D1-GR Common Girder

Comtech, Inc, Fayetteville, NC - 28314,

2 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:09 2023 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-lcg0VbAOOCG0qdJglbDL?jzj1i0B_dVf1LFE2WzuQM8

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)



818 Soundside Road Edenton, NC 27932

Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188107 J0123-0221 D1GE COMMON SUPPORTED GAB Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:08 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-qP6eIFAldv89DTkUkuh6SVRdulocFGmVohVhW4zuQM9 8-10-0 0-11-0 3-11-8 3-11-8 0-11-0 Scale = 1:28.6 4x4 =10.00 12 2x4 || 2x4 || 3 6 0-4-4 8 10 3x10 || 3x10 || 2x4 || 2x4 || 2x4 ||

8-10-0

7-11-0

DEFL.

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

9-9-0

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

PLATES

Weight: 60 lb

MT20

GRIP

244/190

FT = 20%

L/d

120

120

n/a

(loc)

6

6

6

0.00

0.00

0.00

I/def

n/r

n/r

n/a

10.0 LUMBER-

20.0

10.0

0.0

LOADING (psf)

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 **OTHERS** 2x4 SP No.2

WEDGE

Left: 2x4 SP No.2, Right: 2x4 SP No.2

REACTIONS. All bearings 7-11-0.

Max Horz 2=-118(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at ioint(s) 2. 6 except 10=-152(LC 12), 8=-148(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

-0-11-0

2-0-0

1.15

1.15

YES

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

SPACING-

Plate Grip DOL

Rep Stress Incr

Code IRC2015/TPI2014

Lumber DOL

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOI = 1.60

CSI.

TC

ВС

WB

Matrix-P

0.02

0.01

0.03

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb)
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



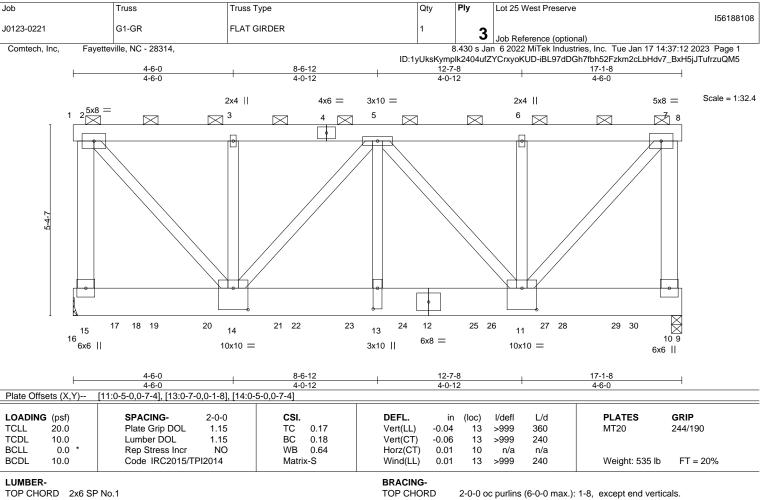


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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BOT CHORD

REACTIONS.

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x10 SP 2400F 2.0E 2x4 SP No.2 *Except* **WEBS** 2-15,7-10: 2x6 SP No.1

(size) 15=Mechanical, 10=0-3-8

Max Uplift 15=-952(LC 4), 10=-496(LC 5) Max Grav 15=9044(LC 2), 10=5622(LC 14)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-15=-4967/485, 2-3=-4297/388, 3-5=-4297/388, 5-6=-4012/329, 6-7=-4012/329, TOP CHORD

7-10=-4613/423

BOT CHORD 13-14=-445/5318 11-13=-445/5318

WEBS 2-14=-563/6307, 5-14=-1816/442, 7-11=-484/5953, 5-13=-233/2760, 5-11=-2053/179

1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x10 - 5 rows staggered at 0-4-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 15=952, 10=496,
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 5000 lb down and 537 lb up at 1-2-8, 276 lb down and 102 lb up at 1-9-12, 1043 lb down at 2-3-12, 276 lb down and 102 lb up at 3-9-12, 1043 lb down at 4-3-12 276 lb down and 102 lb up at 5-9-12, 1043 lb down at 6-3-12, 276 lb down and 102 lb up at 7-9-12, 1122 lb down at 8-3-12, 1122 lb down at 9-3-12, 276 lb down and 102 lb up at 9-9-12, 1122 lb down at 11-3-12, 276 lb down and 102 lb up at 11-9-12, 1122 lb down at 13-3-12, 276 lb down and 102 lb up at 13-9-12, and 1122 lb down at 15-3-12, and 276 lb down and 102 lb up at 15-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

JORTH

Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORF USF Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



January 18,2023

Edenton, NC 27932

Continued on page 2

Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188108 J0123-0221 FLAT GIRDER G1-GR

Fayetteville, NC - 28314, Comtech, Inc,

3 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:12 2023 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-iBL97dDGh7fbh52Fzkm2cLbHdv7_BxH5jJTufrzuQM5

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

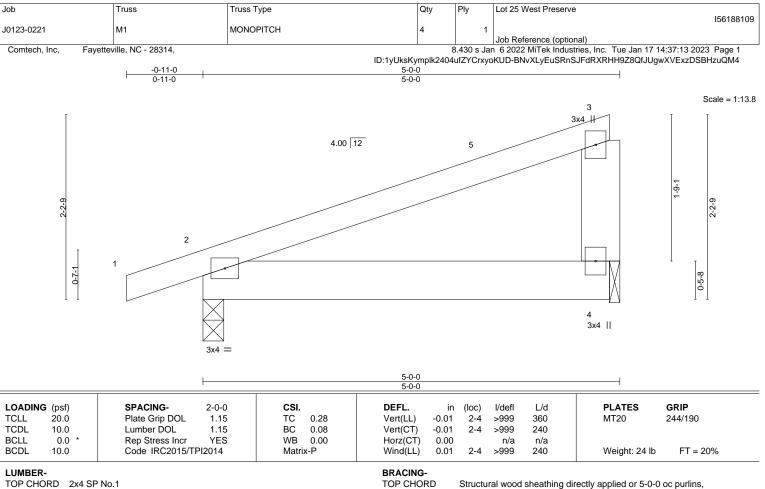
Uniform Loads (plf)

Vert: 1-2=-60, 2-7=-60, 7-8=-60, 9-16=-20

Concentrated Loads (lb)

Vert: 12=-276(F) 14=-255(B) 13=-291(B) 17=-5000(B) 18=-276(F) 19=-255(B) 20=-276(F) 21=-276(F) 22=-255(B) 23=-276(F) 24=-291(B) 25=-291(B) 26=-276(F) 21=-276(F) 21=-

27=-291(B) 28=-276(F) 29=-291(B) 30=-276(F)



TOP CHORD

BOT CHORD

2x4 SP No.1 2x6 SP No.1

BOT CHORD WEBS 2x6 SP No.1

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=63(LC 8)

Max Uplift 2=-102(LC 8), 4=-79(LC 8) Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psg bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.



Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



lob		Truss	Truss Type		Qty	Ply	Lot 25	West Preserve		I56188110
10123-0221		M1GE	GABLE		1	1	ı			100100110
								eference (optiona		
Comtech, Inc	c, Fayette	eville, NC - 28314,		ID.4.					es, Inc. Tue Jan 17 14	
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		0-11-0		5-0)-0					
									_	Scale = 1:13.8
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LOADING ((psf)	SPACING- 2-	0-0 CSI .	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
	20.0			.09 Vert(LL			>999	240	MT20	244/190
	10.0	Lumber DOL 1		.09 Vert(C)	r) -0.01	8	>999	240		
	0.0 *			.02 Horz(C	T) -0.00	6	n/a	n/a	W : L . 07 II	FT 000/
BCDL 1	10.0	Code IRC2015/TPI201	14 Matrix-S	'					Weight: 27 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1

BOT CHORD 2x6 SP No.1 **WEBS OTHERS** 2x4 SP No.2

REACTIONS.

(size) 2=0-3-0, 6=0-1-8 Max Horz 2=90(LC 8)

Max Uplift 2=-147(LC 8), 6=-115(LC 8) Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147, 6=115.



Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

January 18,2023



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chorembers only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Lot 25 West Preserve 156188111 J0123-0221 M2 Half Hip 3 Job Reference (optional) Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:16 2023 Page 1 Comtech, Inc. ID:1yUksKymplk2404ufZYCrxyoKUD-bybgz_GnlM90AiL0Car_nBmySWUy7uFhexR6oczuQM1 5-3-8 0-11-0 3-11-8 1-4-0 Scale = 1:12.9 3x4 || 3 4.00 12 9 0-7-1 10 8 4x6 = 4x4 || 3-11-8 3-11-8 LOADING (psf) SPACING-DEFL. L/d **PLATES** GRIP 2-0-0 CSI (loc) I/def 20.0 Plate Grip DOL 244/190 **TCLL** 1.15 TC 0.24 Vert(LL) -0.00 8 >999 360 MT20 TCDL 10.0 Lumber DOL 1.15 ВС 0.24 Vert(CT) -0.01 8 >999 240 **BCLL** 0.0 Rep Stress Incr NO WB 0.00 Horz(CT) -0.00 n/a n/a **BCDL** 10.0 Code IRC2015/TPI2014 Matrix-R Wind(LL) 0.02 8 >999 240 Weight: 28 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No 1 TOP CHORD Structural wood sheathing directly applied or 5-3-8 oc purlins, 2x6 SP No.1 **BOT CHORD** except end verticals, and 2-0-0 oc purlins: 3-8, 5-6. Except: **WEBS** 2x6 SP No.1 10-0-0 oc bracing: 3-5 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 7=0-3-8, 2=0-3-0 Max Horz 2=69(LC 12)

Max Uplift 7=-173(LC 8), 2=-138(LC 8) Max Grav 7=561(LC 19), 2=349(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-425/505, 5-8=-279/265, 5-6=-233/338, 6-7=-292/309 TOP CHORD

BOT CHORD 2-8=-546/359, 7-8=-338/233

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-7-4, Interior(1) 3-7-4 to 5-0-12 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=173, 2=138
- 7) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 5-9=-40, 6-9=-80, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-400

2) Dead + 0.75 Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-50, 3-4=-50, 5-9=-100, 6-9=-130, 2-7=-20



January 18,2023

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not

Design Valid to its 80 mly with win New Commercials. This design is based only upon parameters shown, and is for an individual orusining Component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



.loh Truss Truss Type Qty Lot 25 West Preserve 156188111 J0123-0221 M2 Half Hip 3 Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:16 2023 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-bybgz_GnlM90AiL0Car_nBmySWUy7uFhexR6oczuQM1

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 9=-350

3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-20, 3-4=-20, 5-6=-40, 2-7=-40

Concentrated Loads (lb)

Vert: 9=-300

4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=12, 2-8=52, 8-10=115, 7-10=52

Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55

Concentrated Loads (lb)

Vert: 9=548

5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=42, 2-8=52, 8-10=115, 7-10=52

Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55

Concentrated Loads (lb)

Vert: 9=566

6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9

Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51

Concentrated Loads (lb)

Vert: 9=-420

7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9

Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51

Concentrated Loads (lb)

Vert: 9=-420

8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-11, 2-8=10, 8-10=33, 7-10=10

Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7

Concentrated Loads (lb)

Vert: 9=154

9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12

Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27

Concentrated Loads (lb)

Vert: 9=43

10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2

Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34

Concentrated Loads (lb)

Vert: 9=-339

11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60

Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20

Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0

Concentrated Loads (lb)

Vert: 9=-234

12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12

Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39

Concentrated Loads (lb)

Vert: 9=43

13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12

Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27

Concentrated Loads (lb)

Vert: 9=43

14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12

Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb)

15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60

Continued on page 3





.loh Truss Truss Type Qty Lot 25 West Preserve 156188111 J0123-0221 M2 Half Hip 3 Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:16 2023 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-bybgz_GnlM90AiL0Car_nBmySWUy7uFhexR6oczuQM1

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27

Concentrated Loads (lb)

Vert: 9=43

16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12

Concentrated Loads (lb)

Vert: 9=-234

17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-21, 2-7=-20

Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0 Concentrated Loads (lb)

Vert: 9=-234

18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90

Uniform Loads (plf)

Vert: 1-3=-20, 3-4=-20, 5-6=-120, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-200

19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75 (0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-8=-3, 8-10=13, 7-10=-3

Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26

Concentrated Loads (lb)

Vert: 9=-454

20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-86, 6-9=-116, 2-7=-20

Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0

Concentrated Loads (lb)

Vert: 9=-375

21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-7=-20

Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9

Concentrated Loads (lb)

Vert: 9=-375

22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-86, 6-9=-116, 2-7=-20

Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0

Concentrated Loads (lb)

Vert: 9=-375

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 5-6=-40, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-400

24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-20, 3-4=-20, 5-9=-40, 6-9=-80, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-400

25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-50, 3-4=-50, 5-6=-100, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-350

26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-20, 3-4=-20, 5-9=-100, 6-9=-130, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-350



Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188112 HALF HIP J0123-0221 M2-GR Job Reference (optional) Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:18 2023 Page 1 Comtech, Inc. ID:1yUksKymplk2404ufZYCrxyoKUD-XLiQOgH1HzPkQ0VPK_tSscrKsKAxbok_5EwDsVzuQM? 0-11-0 3-11-8 1-4-0 Scale = 1:12.9 3x4 || 3 4.00 12 9 0-7-1 10 8 4x6 =3x4 || 3-11-8 3-11-8 LOADING (psf) SPACING-DEFL. **PLATES** GRIP 2-0-0 CSI (loc) I/def L/d 244/190 **TCLL** 20.0 Plate Grip DOL 1.15 TC 0.12 Vert(LL) -0.00 8 >999 360 MT20 TCDL 10.0 Lumber DOL 1.15 ВС 0.14 Vert(CT) -0.01 8 >999 240 **BCLL** 0.0 Rep Stress Incr NO WB 0.00 Horz(CT) -0.00 n/a n/a **BCDL** 10.0 Code IRC2015/TPI2014 Matrix-R Wind(LL) 0.01 8 >999 240 Weight: 55 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No 1 TOP CHORD Structural wood sheathing directly applied or 5-3-8 oc purlins, **BOT CHORD** 2x6 SP No.1 except end verticals, and 2-0-0 oc purlins: 3-8, 5-6. Except: 2x6 SP No.1 10-0-0 oc bracing: 3-5

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS

REACTIONS. (size) 7=0-3-8, 2=0-3-0

Max Horz 2=69(LC 12) Max Uplift 7=-24(LC 8), 2=-112(LC 8) Max Grav 7=710(LC 19), 2=375(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-484/446, 5-8=-334/210, 5-6=-280/291, 6-7=-390/210

BOT CHORD 2-8=-491/415, 7-8=-291/280

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-7-4, Interior(1) 3-7-4 to 5-0-12 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb) 2=112. 9) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified
- Building designer must review loads to verify that they are correct for the intended use of this truss.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 5-9=-160, 6-9=-200, 2-7=-20

ORTH

January 18,2023

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE
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.loh Truss Truss Type Qty Ply Lot 25 West Preserve 156188112 J0123-0221 M2-GR HALF HIP Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:18 2023 Page 2 ID:1yUksKymplk2404ufZYCrxyoKUD-XLiQOgH1HzPkQ0VPK_tSscrKsKAxbok_5EwDsVzuQM?

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 9=-400

2) Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-50, 3-4=-50, 5-9=-220, 6-9=-250, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-350

3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-20, 3-4=-20, 5-6=-160, 2-7=-40

Concentrated Loads (lb)

Vert: 9=-300

4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=-108, 2-8=52, 8-10=115, 7-10=52

Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55

Concentrated Loads (lb)

Vert: 9=548

5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=-78, 2-8=52, 8-10=115, 7-10=52

Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55

Concentrated Loads (lb)

Vert: 9=566

6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9

Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51

Concentrated Loads (lb)

Vert: 9=-420

7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9

Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51

Concentrated Loads (lb)

Vert: 9=-420

8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-131, 2-8=10, 8-10=33, 7-10=10

Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7

Concentrated Loads (lb)

Vert: 9=154

9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12

Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27

Concentrated Loads (lb)

Vert: 9=43

10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2

Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34

Concentrated Loads (lb)

Vert: 9=-339

11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20

Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0

Concentrated Loads (lb)

Vert: 9=-234

12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12

Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39

Concentrated Loads (lb)

Vert: 9=43

13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12

Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27

Concentrated Loads (lb)

Vert: 9=43

14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60

Continued on page 3





.loh Truss Truss Type Qty Ply Lot 25 West Preserve 156188112 J0123-0221 M2-GR HALF HIP Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:18 2023 Page 3 ID:1yUksKymplk2404ufZYCrxyoKUD-XLiQOgH1HzPkQ0VPK_tSscrKsKAxbok_5EwDsVzuQM?

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12

Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39

Concentrated Loads (lb)

Vert: 9=43

15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12

Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27

Concentrated Loads (lb)

Vert: 9=43

16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12

Concentrated Loads (lb)

Vert: 9=-234

17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20

Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0

Concentrated Loads (lb)

Vert: 9=-234

18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90

Uniform Loads (plf)

Vert: 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-200

19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3

Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26

Concentrated Loads (lb)

Vert: 9=-454

20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20

Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0

Concentrated Loads (lb)

Vert: 9=-375

21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20

Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9

Concentrated Loads (lb)

Vert: 9=-375

22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-206, 6-9=-236, 2-7=-20

Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0

Concentrated Loads (lb)

Vert: 9=-375

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 5-6=-160, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-400

24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-400

25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-50, 3-4=-50, 5-6=-220, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-350

26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20

Concentrated Loads (lb)

Vert: 9=-350







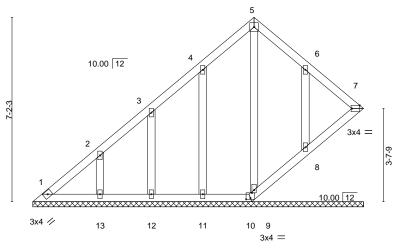
Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188113 J0123-0221 V1GE **ROOF SPECIAL STRUCTU** Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:20 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-TjqApMJHpafSfJfnRPvwx1wgG7u53h0GYYPJwNzuQLz

12-10-9 8-7-7 4-3-2

> Scale = 1:44.9 4x4 =



12-10-9 Plate Offsets (X V)-- [7:0-3-11 Edge] [9:0-1-6 0-1-0] [10:0-2-0 0-0-10]

Tiale Offsets (A	, 1) [7.	.0-3-11,Eugej, [3.0-1-0,0	J-1-0j, [10.0-2	0,0-0-10]									
LOADING (psf		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0		Plate Grip DOL	1.15	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
TCDL 10.0		Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999			
BCLL 0.0	*	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.00	7	n/a	n/a			
BCDL 10.0		Code IRC2015/TP	12014	Matri	x-S						Weight: 75 lb	FT = 20%	

BRACING-LUMBER-

TOP CHORD 2x4 SP No.1 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. **BOT CHORD** 2x4 SP No.1 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. **OTHERS** 2x4 SP No.2

REACTIONS. All bearings 12-10-9

Max Horz 1=231(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10 except 11=-112(LC 12), 12=-107(LC 12), 13=-133(LC 12),

8=-126(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-295/189

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.





Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188114 J0123-0221 V2 VALLEY Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:21 2023 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:1yUksKymplk2404ufZYCrxyoKUD-yvOZ0hKvZunJHTE_?7Q9UFTpoXCSo83QnC9tSqzuQLy 14-10-0 7-5-0 7-5-0 7-5-0

4x4 =

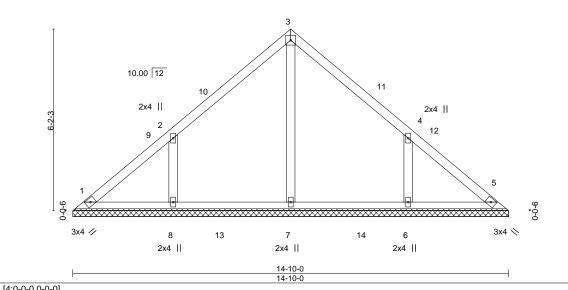


Plate Offsets (X,Y)	[4:0-0-0,0-0-0]			
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.14	DEFL. in (loc) I/defl L/d Vert(LL) n/a - n/a 999	PLATES GRIP MT20 244/190
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.15 WB 0.09 Matrix-S	Vert(CT) n/a - n/a 999 Horz(CT) 0.00 5 n/a n/a	Weight: 64 lb FT = 20%

LUMBER-**BRACING-**

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 **OTHERS** 2x4 SP No.2 TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. **BOT CHORD**

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 14-10-0.

(lb) -Max Horz 1=-140(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-135(LC 12), 6=-135(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=400(LC 19), 8=393(LC 19), 6=393(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-8=-338/247, 4-6=-338/247 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=135, 6=135



Scale = 1:39.2



Job Truss Truss Type Qty Lot 25 West Preserve 156188115 J0123-0221 V3 VALLEY Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:23 2023 Page 1

ID:1yUksKymplk2404ufZYCrxyoKUD-ulWJRNLA5V11WnOM7YTdZgY9RLuuG2_jFWe_XizuQLw 6-2-10 6-2-10

> Scale = 1:33.0 4x4 =

> > Structural wood sheathing directly applied or 6-0-0 oc purlins.

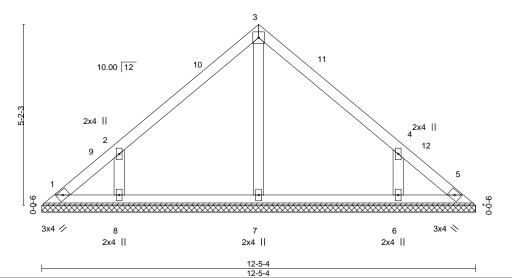


Plate Offsets (X,Y)--[4:0-0-0,0-0-0] SPACING-**PLATES** LOADING (psf) 2-0-0 CSI. DEFL. in (loc) I/defI L/d GRIP Plate Grip DOL 244/190 TCLL 20.0 1.15 TC 0.13 Vert(LL) 999 n/a n/a MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.09 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.06 Horz(CT) 0.00 5 n/a n/a Code IRC2015/TPI2014 FT = 20% **BCDL** 10.0 Weight: 52 lb Matrix-S

TOP CHORD

LUMBER-**BRACING-**

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 **OTHERS** 2x4 SP No.2

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-5-4. (lb) -Max Horz 1=-116(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-123(LC 12), 6=-123(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=326(LC 19), 6=326(LC 20)

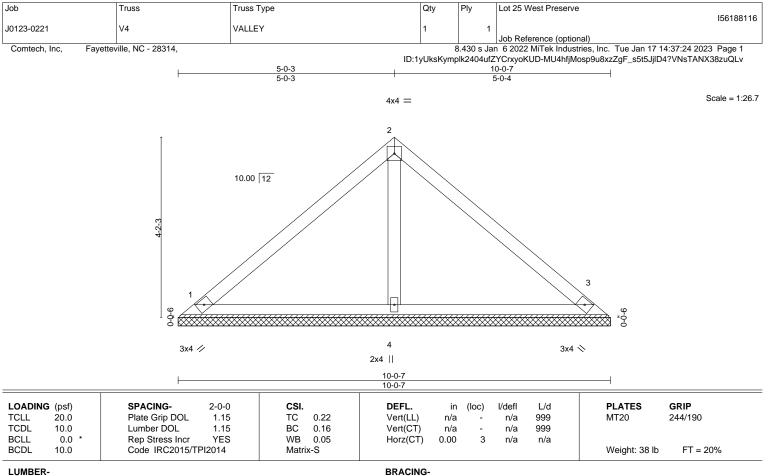
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-8=-312/241, 4-6=-312/241 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123,







TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.2

REACTIONS.

1=10-0-7, 3=10-0-7, 4=10-0-7 (size) Max Horz 1=-92(LC 8) Max Uplift 1=-22(LC 13), 3=-30(LC 13)

Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

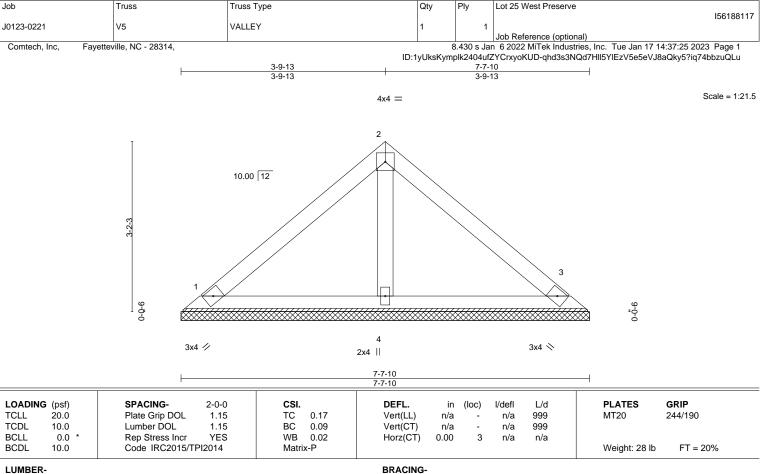
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.2

> 1=7-7-10, 3=7-7-10, 4=7-7-10 (size) Max Horz 1=68(LC 9)

Max Uplift 1=-24(LC 13), 3=-30(LC 13)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

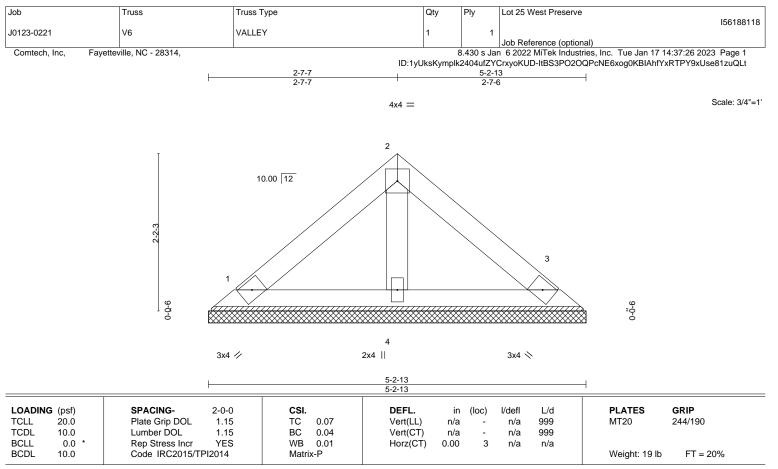


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chorembers only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rerection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 **OTHERS** 2x4 SP No.2

REACTIONS.

1=5-2-13, 3=5-2-13, 4=5-2-13 (size) Max Horz 1=-44(LC 8) Max Uplift 1=-15(LC 13), 3=-19(LC 13)

Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 5-2-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188119 J0123-0221 V7 VALLEY Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:37:28 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-EGJCU5Plw2gKcYGKv52oGjG2zMdyxJDSOoLlCwzuQLr 2-10-0 1-5-0 1-5-0 1-5-0 Scale = 1:8.7 3x4 =2 10.00 12 3 9-0-0 9-0-0 3x4 // 3x4 × 2-10-0 2-10-0 Plate Offsets (X,Y)--[2:0-2-0,Edge] SPACING-**PLATES** GRIP LOADING (psf) 2-0-0 CSI. DEFL. in (loc) I/defI L/d Plate Grip DOL TCLL 20.0 1.15 TC 0.01 Vert(LL) 999 MT20 244/190 n/a n/a TCDL 10.0 Lumber DOL 1.15 BC 0.03 Vert(CT) n/a n/a 999 **BCLL** 0.0 Rep Stress Incr YES WB 0.00 0.00 3 Horz(CT) n/a n/a Code IRC2015/TPI2014 FT = 20% **BCDL** 10.0 Matrix-P Weight: 8 lb

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

1=2-10-0, 3=2-10-0 (size)

Max Horz 1=-20(LC 8) Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 2-10-0 oc purlins.

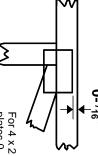
Rigid ceiling directly applied or 10-0-0 oc bracing.

Symbols

PLATE LOCATION AND ORIENTATION



offsets are indicated. Center plate on joint unless x, y and fully embed teeth Apply plates to both sides of truss Dimensions are in ft-in-sixteenths



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

connector plates. required direction of slots in This symbol indicates the

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

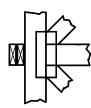
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



by text in the bracing section of the output. Use T or I bracing if indicated. ndicated by symbol shown and/or

BEARING



Min size shown is for crushing only number where bearings occur. reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

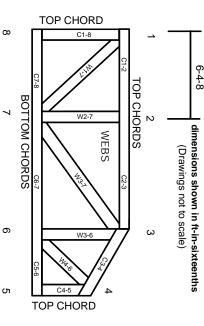
Industry Standards:

National Design Specification for Metal

DSB-89: ANSI/TPI1:

Building Component Safety Information. Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling Design Standard for Bracing. Plate Connected Wood Truss Construction.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

established by others. section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Ņ Truss bracing must be designed by an engineer. For bracing should be considered. may require bracing, or alternative Tor I wide truss spacing, individual lateral braces themselves
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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designer, erection supervisor, property owner and all other interested parties. Provide copies of this truss design to the building

4

- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.

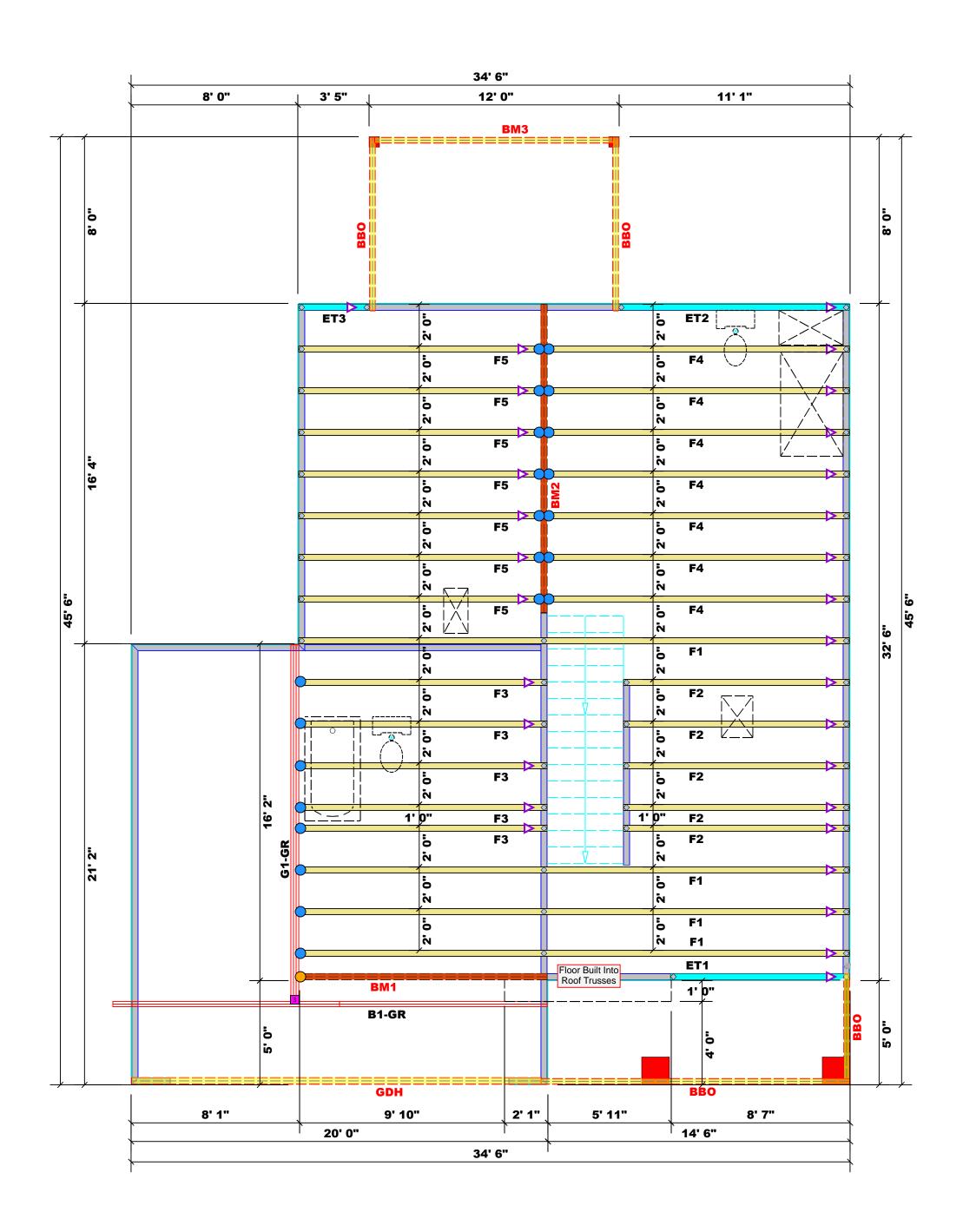
ტ. Ö

- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.

9

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- Camber is a non-structural consideration and is the camber for dead load deflection. responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.



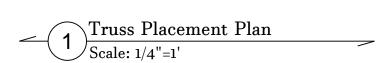
All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

1. Plumbing drop locations shown are NOT exact.
2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
3. Adjust spacing as needed not to exceed 24"oc.

	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS410	USP	22	NA	16d/3-1/2"	16d/3-1/2"
	THDH412	USP	1	NA	16d /3-1/2"	16d /3-1/2"

		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM3	12' 0"	2x10 SPF No.2	2	2	FF
GDH	20' 0"	1-3/4"x 14" LVL Kerto-S	2	2	FF





Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

ture David Landry

David Landry

COLD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

NOLLY

NO

ADDRESS Oleander Lane
MODEL Floor

DATE REV. 01/18/23

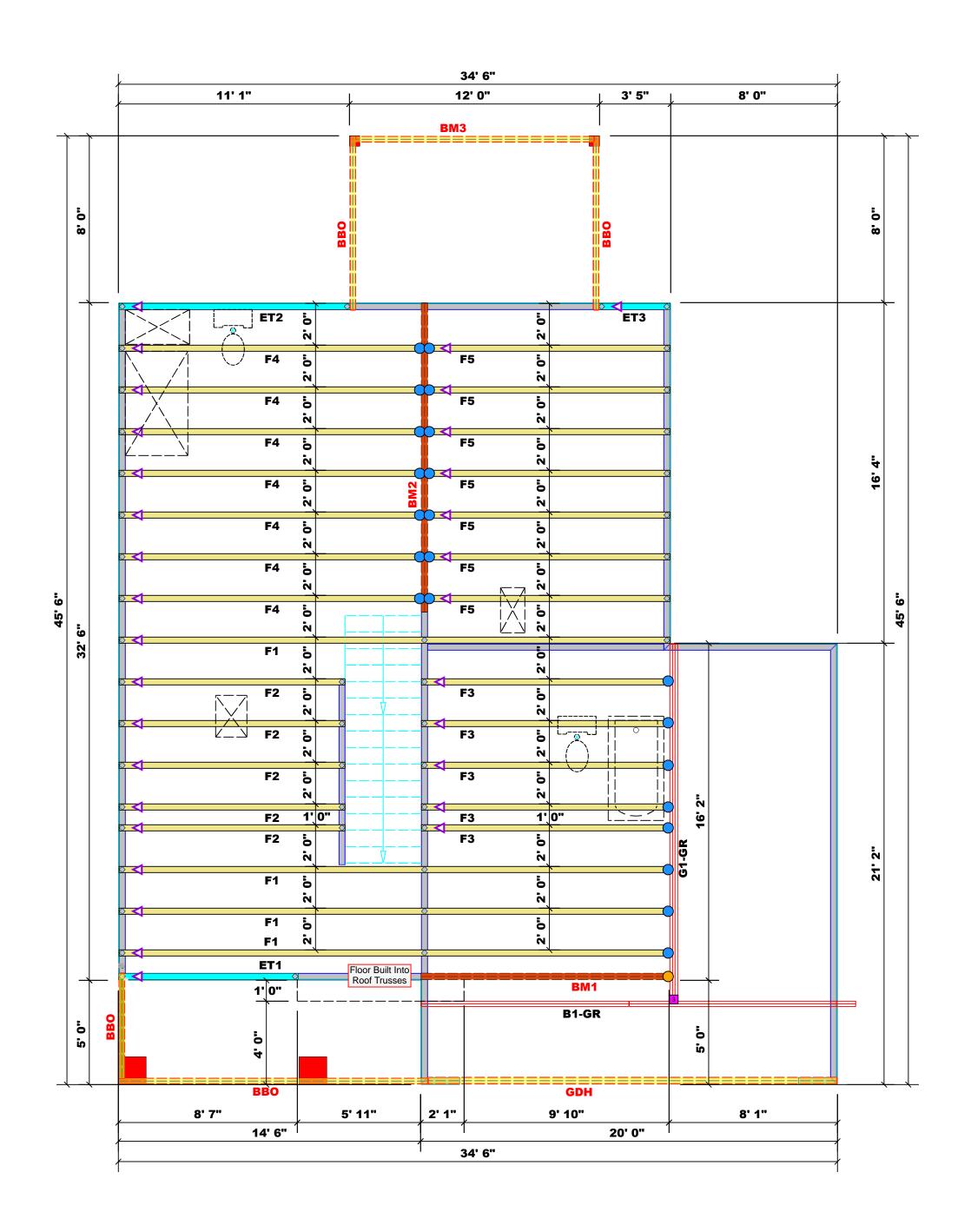
DRAWN BY David Landry

SALES REP. Lenny Norris

BUILDERWeaver DevelopmentJOB NAMELot 25 West PreservePLANHickory II "C" / 2GLF, CPSEAL DATEN/AQUOTE #J0123-0222

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of frame wall unless noted otherwise
3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes
 Plumbing drop locations shown are NOT exact. Contractor to verify ALL plumbing drop
locations prior to setting Floor Trusses. 3. Adjust spacing as needed not to exceed 24"oc.

	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS410	USP	22	NA	16d/3-1/2"	16d/3-1/2"
	THDH412	USP	1	NA	16d /3-1/2"	16d /3-1/2"

		Duadicata			
		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM3	12' 0"	2x10 SPF No.2	2	2	FF
GDH	20' 0"	1-3/4"x 14" LVL Kerto-S	2	2	FF

Truss Placement Plan
Scale: 1/4"=1'

ROOF & FLOOR TRUSSES & BEAMS

> Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are leemed to comply with the prescriptive Code equirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code equirements) to determine the minimum foundation size and number of wood studs required to support eactions greater than 3000# but not greater than 15000#. A registered design professional shall be etained to design the support system for any eaction that exceeds those specified in the attacher lables. A registered design professional shall be etained to design the support system for all eactions that exceed 15000#.

David Landry

David Landry

LOAD CHART FOR JACK STUDS
(BASED ON TABLES R502.5(1) & (b))

NUM	MBER C	STUDS R		A END OF	=
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR
1700	1	2550	1	3400	1
3400	2	5100	2	6800	3
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
0200	6	15300	6		
1900	7				
3600	8				
5300	9				

CITY / CO .	Sanford / Harnett
ADDRESS	Oleander Lane
MODEL	Floor
DATE REV.	01/18/23
DRAWN BY	David Landry
CALECDED	

BUILDER Weaver Development

JOB NAME Lot 25 West Preserve

Hickory II "C" / 2GLF

MOUTE # JO123-0222

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Client: Weaver Development

Project: Address:

Hickory II Oleander Lane

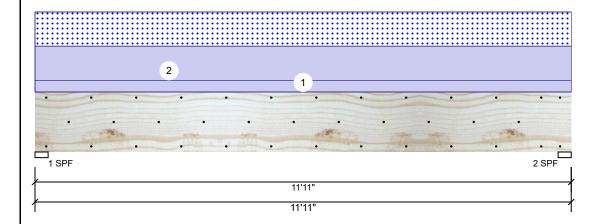
Sanford, NC 27546

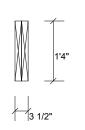
Date: 1/18/2023 Input by: David Landry

Job Name: Lot 25 West Preserve Project #: J0123-0222

Kerto-S LVL 2-Ply - PASSED 1.750" X 16.000" BM₁

Level: Level





Page 1 of 8

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temp <= 100°F Temperature:

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Not Checked Deck: Ceiling: Gypsum 1/2"

Reactions UNPATTERNED Ib (Uplift) Wind Brg Direction Live Dead Snow Const 0 2869 2079 Vertical 0 0 1 2 Vertical 0 2869 2079 0 0

Bearings

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+S 1 - SPF 3.500" Vert 2869 / 2079 4948 L 2 - SPF 3.500" Vert 95% 2869 / 2079 4948 L D+S

Analysis Results

. ,						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	13679 ft-lb	5'11 1/2"	39750 ft-lb	0.344 (34%)	D+S	L
Unbraced	13679 ft-lb	5'11 1/2"	13699 ft-lb	0.999 (100%)	D+S	L
Shear	3615 lb	1'7 1/2"	13739 lb	0.263 (26%)	D+S	L
LL Defl inch	0.069 (L/2000)	5'11 1/2"	0.287 (L/480)	0.240 (24%)	S	L
TL Defl inch	0.164 (L/840)	5'11 1/2"	0.383 (L/360)	0.428 (43%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 8'8 3/8" o.c.

7 Lateral slenderness ratio based on single ply width

, Eatorare	nonaonnoco rano bacca en	omigio pij maan									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall	
2	Uniform			Тор	349 PLF	0 PLF	349 PLF	0 PLF	0 PLF	A2	
	Self Weight				12 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. Lot 35 Briarwo Broadway, NO ay, NC USA 28314 910-864-TRUS







Client: Project:

Weaver Development Hickory II

Oleander Lane

Sanford, NC 27546

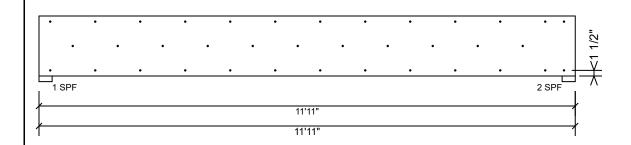
1/18/2023 Input by: David Landry

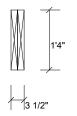
Job Name: Lot 25 West Preserve Project #: J0123-0222

1.750" X 16.000" **Kerto-S LVL** 2-Ply - PASSED BM₁

Address:

Level: Level





Page 2 of 8

Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

rasteri ali piles usirig 3 rows	101 100 box Halls (.120x3) at
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. Lot 35 Briarwood Bluff Broadway, NC USA 28314 910-864-TRUS





Client: Project: Address: Weaver Development Hickory II

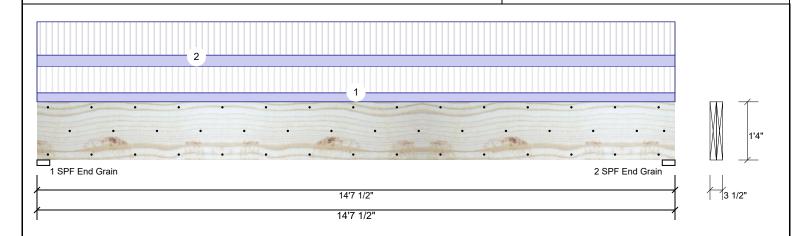
Oleander Lane Sanford, NC 27546 Date: 1/18/2023

Input by: David Landry Job Name: Lot 25 West Preserve

Project #: J0123-0222

2-Ply - PASSED **Kerto-S LVL** 1.750" X 16.000" BM₂

Level: Level



Member Information Type:

Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance:

Normal - II Temp <= 100°F Temperature:

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Not Checked Deck: Ceiling: Gypsum 1/2"

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	3868	1385	0	0	0
2	Vertical	3868	1385	0	0	0

Page 3 of 8

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	18077 ft-lb	7'3 3/4"	34565 ft-lb	0.523 (52%)	D+L	L
Unbraced	18077 ft-lb	7'3 3/4"	18085 ft-lb	1.000 (100%)	D+L	L
Shear	5044 lb	1'7 1/2"	11947 lb	0.422 (42%)	D+L	L
LL Defl inch	0.229 (L/743)	7'3 13/16"	0.355 (L/480)	0.646 (65%)	L	L
TL Defl inch	0.311 (L/547)	7'3 13/16"	0.473 (L/360)	0.658 (66%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top must be laterally braced at a maximum of 6'5 1/8" o.c.
- 6 Lateral slenderness ratio based on single ply width.

Bearings	Bearings													
Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.							
1 - SPF End Grain	3.500"	Vert	51%	1385 / 3868	5254	L	D+L							
2 - SPF End Grain	3.500"	Vert	51%	1385 / 3868	5254	L	D+L							

ID Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments Uniform 79 PLF 235 PLF 0 PLF 0 PLF 0 PLF F5 1 Near Face 98 PLF 294 PLF 0 PLF 0 PLF 2 Uniform Far Face 0 PLF Self Weight 12 PLF

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Damaged Beams must not be used

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. Lot 35 Briarwo Broadway, NO ay, NC USA 28314 910-864-TRUS







BM₂

Kerto-S LVL

Client: Project:

Address:

Hickory II

Weaver Development Oleander Lane

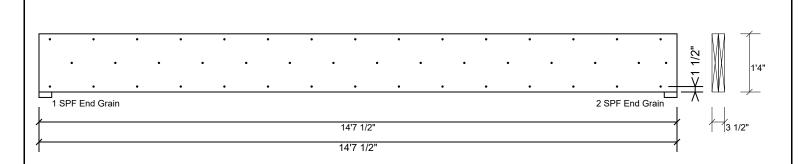
1/18/2023 Input by:

David Landry Job Name: Lot 25 West Preserve J0123-0222

Page 4 of 8

Sanford, NC 27546 Project #: 2-Ply - PASSED 1.750" X 16.000"

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c., Maximum end distance not to exceed 6".

		`	,
Capacity	79.8 %		
Load	196.0 PLF		
Yield Limit per Foot	245.6 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination	D+L		
Duration Factor	1.00		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Comtech, Inc. Lot 35 Briarwood Bluff Broadway, NC USA 28314 910-864-TRUS





Manufacturer Info





S-P-F #2

Client: Project: Address: Weaver Development Hickory II

Oleander Lane

velopment Date: Input by:

Sanford, NC 27546 Project #:

2.000" X 10.000" 2-Ply - PASSED

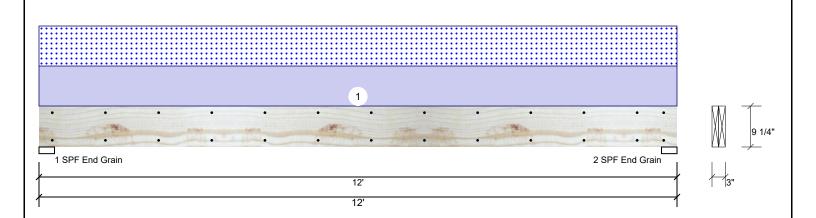
 Date:
 1/18/2023

 Input by:
 David Landry

 Job Name:
 Lot 25 West Preserve

 Project #:
 J0123-0222

Level: Level



Bearing Length

Dir.

Member Infor	mation			Rea	ctions U
Туре:	Girder	Application:	Floor	Brg	Direction
Plies:	2	Design Method:	ASD	1	Vertical
Moisture Condition	n: Dry	Building Code:	IBC/IRC 2015	2	Vertical
Deflection LL:	480	Load Sharing:	No		
Deflection TL:	360	Deck:	Not Checked		
Importance:	Normal - II	Ceiling:	Gypsum 1/2"		
Temperature:	Temp <= 100°F				
				Bea	rings

Rea	Reactions UNPATTERNED lb (Uplift)										
Brg	Direction	Live	Dead	Snow	Wind	Const					
1	Vertical	0	564	564	0	0					
2	Vertical	0	564	564	0	0					

Page 5 of 8

Analysis Results Analysis Actual Location Allowed Comb. Case Capacity Moment 3130 ft-lb 6' 3946 ft-lb 0.793 (79%) D+S L Unbraced 3130 ft-lb 6' 3131 ft-lb 1.000 L (100%)928 lb 10'11 1/4" 2872 lb 0.323 (32%) D+S Shear L LL Defl inch 0.135 (L/1022) 6' 0.289 (L/480) 0.470 (47%) S ı TL Defl inch 0.271 (L/511) 6' 0.385 (L/360) 0.704 (70%) D+S

1 - SPF 3.500" Vert 25% 564 / 564 1128 L D+S End Grain 2 - SPF 3.500" Vert 25% 564 / 564 1128 L D+S End Grain

Cap. React D/L lb

Total Ld. Case

Ld. Comb.

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 8'8 11/16" o.c.

7 Lateral slenderness ratio based on single ply width.

I	ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6 C	onst. 1.25	Comments
1	1	Uniform			Тор	94 PLF	0 PLF	94 PLF	0 PLF	0 PLF	C1

This design is valid until 11/3/2024

Manufacturer Info

Comtech, Inc.
Lot 35 Brianwood Bluff
Broadway, NC
USA
28314
910-864-TRUS



Client: Project: Address: Weaver Development Hickory II

Oleander Lane Sanford, NC 27546 Date: 1/18/2023 Input by: David Land

Input by: David Landry
Job Name: Lot 25 West Preserve

Page 6 of 8

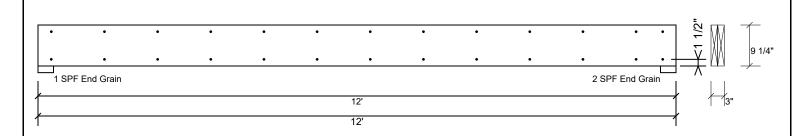
Project #: J0123-0222

BM3 S-P-F #2

2.000" X 10.000"

2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

, ,		`	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	157.4 PLF		
Yield Limit per Fastener	78.7 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Manufacturer Info

Comtech, Inc.
Lot 35 Briarwood Bluff
Broadway, NC
USA
28314
910-864-TRUS

This design is valid until 11/3/2024



GDH

Kerto-S LVL

Client: Weaver Development

Project: Address:

Hickory II Oleander Lane Sanford, NC 27546 Date: 1/18/2023 Input by:

David Landry Job Name: Lot 25 West Preserve

J0123-0222

Page 7 of 8

Wind

0

0

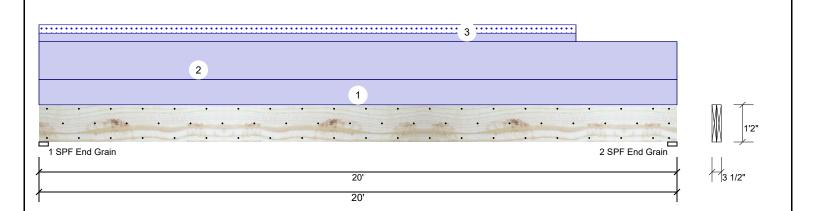
Const

0

0

Project #: 2-Ply - PASSED 1.750" X 14.000"

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Application: Live Type: Floor Brg Direction Plies: 2 Design Method: ASD 0 Vertical 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 Vertical 0 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal - II Ceiling: Gypsum 1/2" Temp <= 100°F Temperature:

Bearings	5						
Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	19%	1804 / 196	2000	L	D+S
2 - SPF End	3.500"	Vert	18%	1750 / 141	1891	L	D+S

Dead

1804

1750

Snow

196

141

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8592 ft-lb	9'11 11/16"	24299 ft-lb	0.354 (35%)	D	Uniform
Unbraced	9503 ft-lb	9'11 1/2"	9509 ft-lb	0.999 (100%)	D+S	L
Shear	1553 lb	1'5 1/2"	9408 lb	0.165 (17%)	D	Uniform
LL Defl inch	0.041 (L/5726)	9'11 1/16"	0.489 (L/480)	0.084 (8%)	S	L
TL Defl inch	0.430 (L/546)	9'11 7/8"	0.651 (L/360)	0.660 (66%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 11'9 1/16" o.c.

7 Lateral slenderness ratio based on single bly width

/ Lateral S	ienuenness raub baseu	on single ply widin.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall	
2	Uniform			Тор	90 PLF	0 PLF	0 PLF	0 PLF	0 PLF	B1GE	
3	Tie-In	0-0-0 to 16-10-0	1-0-0	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	Roof Load	
	Self Weight				11 PLF						

Grain

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Comtech, Inc. Lot 35 Briarwo Broadway, NC ay, NC USA 28314 910-864-TRUS



Manufacturer Info





GDH

Kerto-S LVL

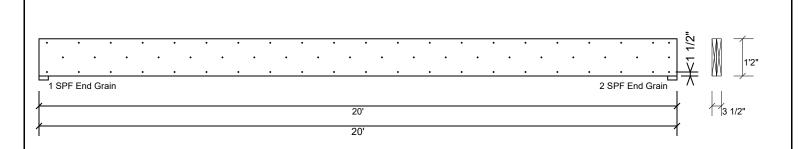
Client: Weaver Development

Project: Hickory II Address:

1/18/2023 Input by: David Landry Job Name: Lot 25 West Preserve Oleander Lane J0123-0222

Sanford, NC 27546 Project #: 2-Ply - PASSED 1.750" X 14.000"

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c., Maximum end distance not to exceed 6".

1 3		`	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	245.6 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. Lot 35 Briarwood Bluff Broadway, NC USA 28314 910-864-TRUS



Page 8 of 8





RE: J0123-0222

Lot 25 West Preserve

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Weaver Development Project Name: J0123-0222 Lot/Block: 25 Model: Hickory II

Address: Oleander Lane Subdivision: West Preserve

City: Sanford State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4

Wind Code: N/A Wind Speed: N/A mph Roof Load: N/A psf Floor Load: 55.0 psf

This package includes 8 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	I56188153	ET1	1/18/2023
2	I56188154	ET2	1/18/2023
3	I56188155	ET3	1/18/2023
4	I56188156	F1	1/18/2023
5	I56188157	F2	1/18/2023
6	I56188158	F3	1/18/2023
7	I56188159	F4	1/18/2023
8	156188160	F5	1/18/2023

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2023

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



January 18, 2023

Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188153 J0123-0222 ET1 **GABLE** Job Reference (optional)

Comtech, Inc,

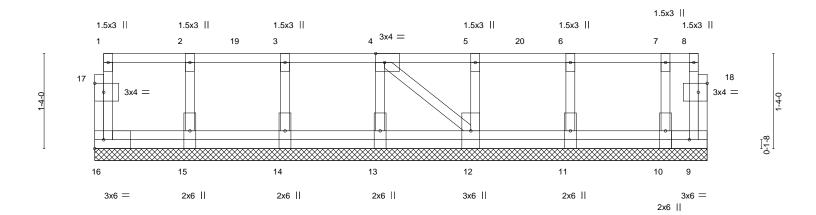
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Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:45:53 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-yPK6UmW4?Clgi9NO?fcDEDgKFwzamjanWsAzQ8zuQDy

0_1_8

Scale: 3/4"=1'



	1-4-0	2-8-0	4-0-0	5-4-0	1	6-8-0	1	8-0-0	3-7-0 ₁
	1-4-0	1-4-0	1-4-0	1-4-0		1-4-0	ı	1-4-0)-7-0
Plate Offsets (X	(,Y) [4:0-1-8,Edge], [17:0-1-8,0-1-8], [18:0-1-	8,0-1-8]						
LOADING (psf	SPACING	3- 2-0-0	CSI.	DEFL.	in ((loc) I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip	DOL 1.00	TC 0.10	Vert(LL)	n/a	. n/a	999	MT20	244/190
TCDL 10.0	Lumber D	OL 1.00	BC 0.00	Vert(CT)	n/a	- n/a	999		
BCLL 0.0	Rep Stres	ss Incr YES	WB 0.04	Horz(CT)	0.00	9 n/a	n/a		
BCDL 5.0	Code IRC	C2015/TPI2014	Matrix-P	, ,				Weight: 54 lb	FT = 20%F, 11%E
									<u> </u>

LUMBER-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) **WEBS**

2x4 SP No.3(flat) **OTHERS** 2x4 SP No.3(flat) BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 8-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Plates checked for a plus or minus 1 degree rotation about its center.
- 2) Gable requires continuous bottom chord bearing.
- 3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 4) Gable studs spaced at 1-4-0 oc.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 9-16=-10. 1-8=-100

Concentrated Loads (lb) Vert: 4=-71 7=-77 19=-71 20=-71



January 18,2023

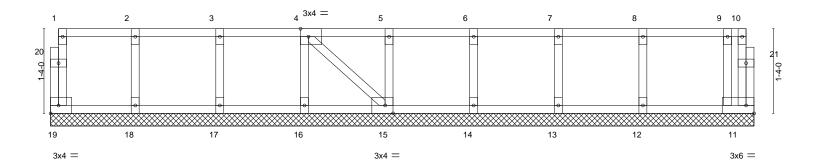
Job	Truss	Truss Type	Qty	Ply	Lot 25 West Preserve
J0123-0222	ET2	GABLE	1	1	I56188154
30123-0222	E12	GABLE	'	'	Job Reference (optional)

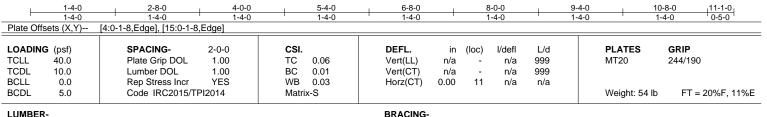
Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:45:54 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-QcuUh6XilWQXJJyaYM7SmQDWZKJgVAzwlWwWzbzuQDx

0₁1₃8

0₁1₃8 Scale = 1:18.2





2x4 SP No.1(flat) TOP CHORD BOT CHORD 2x4 SP No.1(flat) **WEBS** 2x4 SP No.3(flat) **OTHERS** 2x4 SP No.3(flat) TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 11-1-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 19, 11, 18, 17, 16, 15, 14, 13, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



January 18,2023



Job Truss Truss Type Qty Lot 25 West Preserve 156188155 J0123-0222 ET3 **GABLE** Job Reference (optional)
8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:45:55 2023 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:1yUksKymplk2404ufZYCrxyoKUD-uoStuSYKWpYOxTXm64fhJelhVkf?Ecl4_Af4V1zuQDw Q-1-8 4 1.5x3 || 1 3x4 || 2 1.5x3 || 3 Scale = 1:9.4 9 3x4 =8 7 6 5 3x4 || 3x4 = 1.5x3 || 3x4 = 1-0-0 1-0-0 1-4-0 Plate Offsets (X,Y)--[1:Edge,0-1-8], [3:0-1-8,Edge], [7:0-1-8,Edge], [8:Edge,0-1-8], [9:0-1-8,0-1-8] LOADING (psf) SPACING-L/d **PLATES** GRIP 2-0-0 CSI. in (loc) I/defI

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

n/a

n/a

0.00

n/a

n/a

n/a

except end verticals.

5

999

999

n/a

LUMBER-

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat)

Code IRC2015/TPI2014

Plate Grip DOL

Rep Stress Incr

Lumber DOL

BOT CHORD **WEBS** 2x4 SP No.3(flat) **OTHERS** 2x4 SP No.3(flat)

40.0

10.0

0.0

5.0

REACTIONS. All bearings 3-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Plates checked for a plus or minus 1 degree rotation about its center.
- 2) Gable requires continuous bottom chord bearing.
- 3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

1.00

1.00

YES

- 4) Gable studs spaced at 1-4-0 oc.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

TC

BC

WB

Matrix-P

0.05

0.01

0.03

6) CAUTION, Do not erect truss backwards.



244/190

FT = 20%F, 11%E

MT20

Structural wood sheathing directly applied or 3-5-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 22 lb



Job	Truss	Truss Type	Qty	Ply	Lot 25 West Preserve
10400 0000		_			I56188156
J0123-0222	F1	Floor	4	1	
					Job Reference (optional)

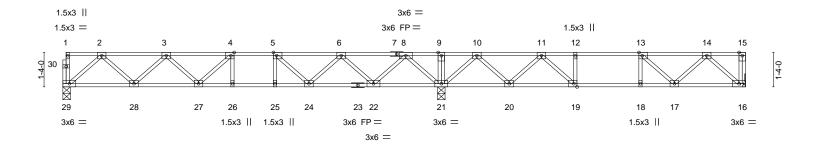
Comtech, Inc, Fayetteville, NC - 28314,

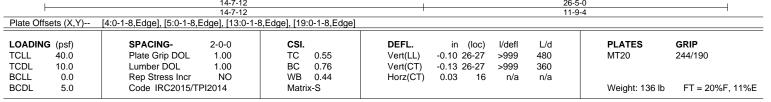
8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:45:56 2023 Page 1



2-4-12

0-1-8 Scale = 1:44.6





BOT CHORD

LUMBER-**BRACING-**

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat) TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing.

(size) 29=0-3-8, 16=Mechanical, 21=0-3-8

Max Grav 29=727(LC 10), 16=1063(LC 4), 21=1671(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 15-16=-537/0, 2-3=-1257/0, 3-4=-1883/0, 4-5=-2002/0, 5-6=-1658/0, 6-8=-756/225,

8-9=0/1401, 9-10=0/1401, 10-11=-468/367, 11-12=-1158/0, 12-13=-1158/0,

13-14=-884/0

BOT CHORD 28-29=0/771, 27-28=0/1717, 26-27=0/2002, 25-26=0/2002, 24-25=0/2002,

22-24=-34/1340, 21-22=-444/147, 20-21=-637/19, 19-20=-175/900, 18-19=0/1158,

17-18=0/1158, 16-17=0/586

2-29=-1023/0, 2-28=0/677, 3-28=-640/0, 8-21=-1289/0, 14-16=-780/0, 14-17=0/415, **WEBS**

13-17=-372/90, 10-21=-1064/0, 8-22=0/923, 6-22=-882/0, 6-24=0/527, 5-24=-633/0,

10-20=0/687, 11-20=-690/0, 11-19=0/582, 12-19=-278/0

NOTES-

REACTIONS.

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 16-29=-10, 1-15=-100 Concentrated Loads (lb)

Vert: 15=-500



January 18,2023



Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188157 Floor J0123-0222 F2 5 Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:45:57 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-rBZdJ8Za2Ro5Amh9EUh9O3ry7XELiTuNRU8AZvzuQDu

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

0-1-8 1-3-0

2-5-8

0₇1₇8 Scale = 1:18.3

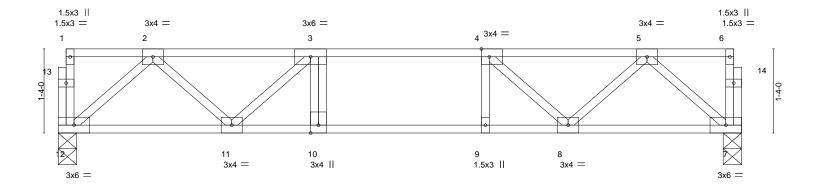


Plate Offsets (X,Y)--[4:0-1-8,Edge] SPACING-DEFL. **PLATES** GRIP LOADING (psf) 2-0-0 CSI. in (loc) I/defI L/d Plate Grip DOL **TCLL** 40.0 1.00 TC 0.36 Vert(LL) -0.07 10 >999 480 MT20 244/190 TCDL 10.0 Lumber DOL 1.00 BC 0.46 Vert(CT) -0.09 10 >999 360 **BCLL** 0.0 Rep Stress Incr YES WB 0.21 Horz(CT) 0.01 n/a n/a **BCDL** Code IRC2015/TPI2014 FT = 20%F, 11%E 5.0 Weight: 57 lb Matrix-S

BRACING-

TOP CHORD

BOT CHORD

10-10-0

LUMBER-

TOP CHORD 2x4 SP No.1(flat) **BOT CHORD** 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 12=0-3-8, 7=0-3-8

Max Grav 12=576(LC 1), 7=576(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-923/0, 3-4=-1242/0, 4-5=-925/0

BOT CHORD 11-12=0/606, 10-11=0/1242, 9-10=0/1242, 8-9=0/1242, 7-8=0/603 2-12=-805/0, 2-11=0/441, 3-11=-469/0, 5-7=-800/0, 5-8=0/449, 4-8=-475/0 WEBS

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



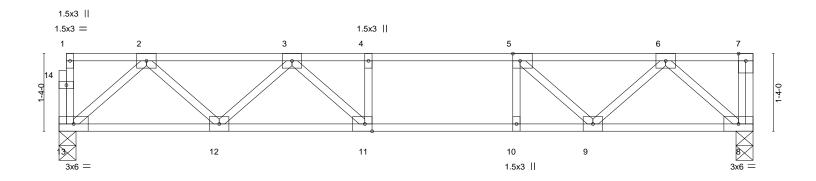


Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188158 J0123-0222 F3 Floor 5 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:45:58 2023 Page 1

Comtech, Inc, Fayetteville, NC - 28314,

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11-11-0 Plate Offsets (X,Y)--[5:0-1-8,Edge], [11:0-1-8,Edge] SPACING-DEFL. **PLATES** GRIP LOADING (psf) CSI. in (loc) I/defl L/d TCLL 40.0 Plate Grip DOL 1.00 TC 0.60 Vert(LL) -0.13 11-12 >999 480 MT20 244/190 TCDL 10.0 Lumber DOL 1.00 BC 0.70 Vert(CT) -0.16 11-12 >894 360 **BCLL** 0.0 Rep Stress Incr NO WB 0.27 Horz(CT) 0.02 8 n/a n/a **BCDL** Code IRC2015/TPI2014 FT = 20%F, 11%E 5.0 Weight: 62 lb Matrix-S

11-11-0

BRACING-LUMBER-

TOP CHORD 2x4 SP No.1(flat) TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, **BOT CHORD** 2x4 SP No.1(flat) except end verticals.

WEBS 2x4 SP No.3(flat) **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 13=0-3-8, 8=0-3-8

Max Grav 13=635(LC 1), 8=1142(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 7-8=-548/0, 2-3=-1060/0, 3-4=-1495/0, 4-5=-1495/0, 5-6=-1059/0 **BOT CHORD** 12-13=0/676, 11-12=0/1395, 10-11=0/1495, 9-10=0/1495, 8-9=0/659 2-13=-898/0, 2-12=0/534, 3-12=-466/0, 3-11=-19/356, 6-8=-877/0, 6-9=0/557, WEBS

5-9=-616/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 8-13=-10, 1-7=-100 Concentrated Loads (lb) Vert: 7=-500





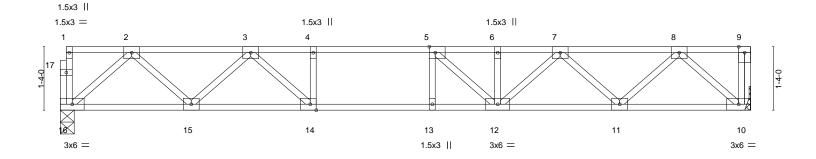
Job	Truss	Truss Type	Qty	Ply	Lot 25 West Preserve	
					I56188159	۱ ا
J0123-0222	F4	Floor	7	1		
					Job Reference (optional)	

Comtech, Inc, Fayetteville, NC - 28314,

0-1-8

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:46:00 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-FmFmy9cTLMAg1EPkvdEs0hTPGIA1voYp7SNrAEzuQDr





			14-6-0	
Plate Offsets (X,Y)	[5:0-1-8,Edge], [14:0-1-8,Edge]			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.62 BC 0.78 WB 0.35	DEFL. in (loc) l/defl L/d Vert(LL) -0.17 12-13 >999 480 Vert(CT) -0.22 12-13 >790 360 Horz(CT) 0.03 10 n/a n/a	PLATES GRIP MT20 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S		Weight: 76 lb FT = 20%F, 11%E

14-6-0

LUMBER-**BRACING-**

2x4 SP No.1(flat) TOP CHORD TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x4 SP No.1(flat) except end verticals.

WEBS 2x4 SP No.3(flat) **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 16=0-3-8, 10=Mechanical Max Grav 16=778(LC 1), 10=784(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1348/0, 3-4=-2244/0, 4-5=-2244/0, 5-6=-2152/0, 6-7=-2152/0, 7-8=-1359/0 **BOT CHORD**

15-16=0/834, 14-15=0/1856, 13-14=0/2244, 12-13=0/2244, 11-12=0/1857, 10-11=0/835 WEBS 2-16=-1109/0, 2-15=0/714, 3-15=-707/0, 3-14=0/697, 4-14=-339/0, 8-10=-1111/0,

8-11=0/729, 7-11=-693/0, 7-12=0/401, 5-12=-438/123

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.





Job Truss Truss Type Qty Ply Lot 25 West Preserve 156188160 Floor J0123-0222 F5 Job Reference (optional) Fayetteville, NC - 28314, Comtech, Inc.

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Jan 17 14:46:01 2023 Page 1 ID:1yUksKymplk2404ufZYCrxyoKUD-jyp89Vc56flXfO_wTKl5Zv?do9ageHJyM66OihzuQDq

Structural wood sheathing directly applied or 6-0-0 oc purlins,

2-1-8 0₁1₈

Scale = 1:19.4

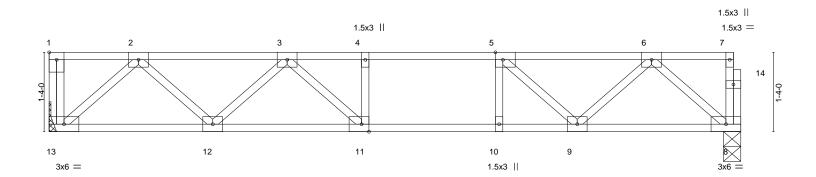


Plate Offsets (X,Y)--[1:Edge,0-1-8], [5:0-1-8,Edge], [11:0-1-8,Edge] SPACING-**PLATES** GRIP LOADING (psf) CSI. DEFL. (loc) I/defl L/d -0.10 11-12 **TCLL** 40.0 Plate Grip DOL 1.00 TC 0.44 Vert(LL) >999 480 244/190 MT20 TCDL 10.0 Lumber DOL 1.00 ВС 0.56 Vert(CT) -0.13 11-12 >999 360 **BCLL** 0.0 Rep Stress Incr YES WB 0.25 Horz(CT) 0.02 8 n/a n/a BCDL Code IRC2015/TPI2014 Weight: 61 lb FT = 20%F, 11%E 5.0 Matrix-S

LUMBER-**BRACING-**

TOP CHORD 2x4 SP No.1(flat) TOP CHORD

BOT CHORD 2x4 SP No.1(flat) except end verticals. WEBS 2x4 SP No.3(flat) **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 13=Mechanical, 8=0-3-8 Max Grav 13=626(LC 1), 8=619(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1025/0, 3-4=-1427/0, 4-5=-1427/0, 5-6=-1022/0

BOT CHORD 12-13=0/658, 11-12=0/1345, 10-11=0/1427, 9-10=0/1427, 8-9=0/644

2-13=-876/0, 2-12=0/510, 3-12=-446/0, 3-11=-41/323, 6-8=-854/0, 6-9=0/526, WEBS

5-9=-570/0

1-3-0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.



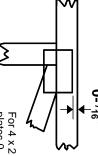


Symbols

PLATE LOCATION AND ORIENTATION



offsets are indicated. Center plate on joint unless x, y and fully embed teeth Apply plates to both sides of truss Dimensions are in ft-in-sixteenths



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

connector plates. required direction of slots in This symbol indicates the

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

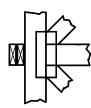
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



by text in the bracing section of the output. Use T or I bracing if indicated. ndicated by symbol shown and/or

BEARING



Min size shown is for crushing only number where bearings occur. reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

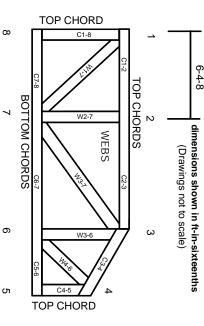
Industry Standards:

National Design Specification for Metal

DSB-89: ANSI/TPI1:

Building Component Safety Information. Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling Design Standard for Bracing. Plate Connected Wood Truss Construction.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

established by others. section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Ņ Truss bracing must be designed by an engineer. For bracing should be considered. may require bracing, or alternative Tor I wide truss spacing, individual lateral braces themselves
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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designer, erection supervisor, property owner and all other interested parties. Provide copies of this truss design to the building

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- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.

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- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.

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- Camber is a non-structural consideration and is the camber for dead load deflection. responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.