NOTE: MONO SLAB - STONE TO RUN TO THE BOTTOM OF WINDOW STEM WALL - STONE TO FOUNDATION HEIGHT ONLY



BOARD & BATTEN

FRONT FACING ONLY

PLANS DESIGNED TO THE **2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE**

MEAN ROOF HEIGHT: 25'-6	HEIGHT TO RIDGE: 29'-9'					
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A			
FENESTRATION U-FACTOR	0.35	0.35	0.35			
SKYLIGHT U-FACTOR	0.55	0.55	0.55			
GLAZED FENESTRATION SHGC	0.30	0.30	0.30			
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci			
WALL R-VALUE	15	15	19			
FLOOR R-VALUE	19	19	30			
* BASEMENT WALL R-VALUE	5/13	10/15	10/15			
** SLAB R-VALUE	0	10	10			
* CRAWL SPACE WALL R-VALUE	5/13	10/15	10/19			
	ULATION OF 5 42 O					

* "10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION

** INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF FOOTING; INSULATION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL DESTONED FOR WIND SPEED OF 120 MPH 3 SECOND CLIST (03 EASTEST MILE) EXPOSURE "R"

DESIGNED FOR WIN	D SPEED	UF IZU MF	H, J SEUC	JND GUST	(93 FAST	EST MILE)	EXPUSUR	E B.
COMPONENT	** *= *	DDING			=		WING L	OADS
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'				TO 45'
ZONE 1	14.2	-15.0	14.9	-15.8		-16.4		
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6		
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4
DESIGNED FOR WIN	d speed	OF 130 MF	PH, 3 SECO	OND GUST	(101 FAS	TEST MILE) EXPOSU	RE "B"
COMPONENT	& CLA	DDING	DESIG	NED FC	R THE	FOLLO	WING L	OADS
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'
ZONE 1	16.7	-18.0	17.5	-18.9	18.2	-19.6	18.7	-20.2
ZONE 2	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 3	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 4	18.2	-19.0	19.1	-20.0	19.8	-20.7	20.4	-21.3
ZONE 5	18.2	-24.0	19.1	-25.2	19.8	-26.2	20.4	-26.9

ROOF VENTILATION

SECTION R806

SQUARE FOOTAGE OF ROOF TO BE VENTED = 1,344 SQ.FT. NET FREE CROSS VENTILATION NEEDED: WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 8.96 SQ.FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 4.48 SQ.FT.

AIR LEAKAGE

Section N1102.4

N1102.4.1 Building thermal envelope. The building thermal envelope shall be durably sealed with an air barrier system to limit infiltration. The sealing methods between dissimilar materials shall allow for differential expansion and contraction. For all homes, where present, the following shall be caulked, gasketed, weather stripped or otherwise sealed with an air barrier material or solid material consistent with Appendix E-2.4 of this code:

- 1. Blocking and sealing floor/ceiling systems and under knee walls open to unconditioned or exterior space.
- 2. Capping and sealing shafts or chases, including flue shafts.
- 3. Capping and sealing soffit or dropped ceiling areas.

GUARD RAIL NOTES

SECTION R312

R312.1 Where required. Guards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or grade below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a guard.

R312.2 Height. Required guards at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads. Exceptions:

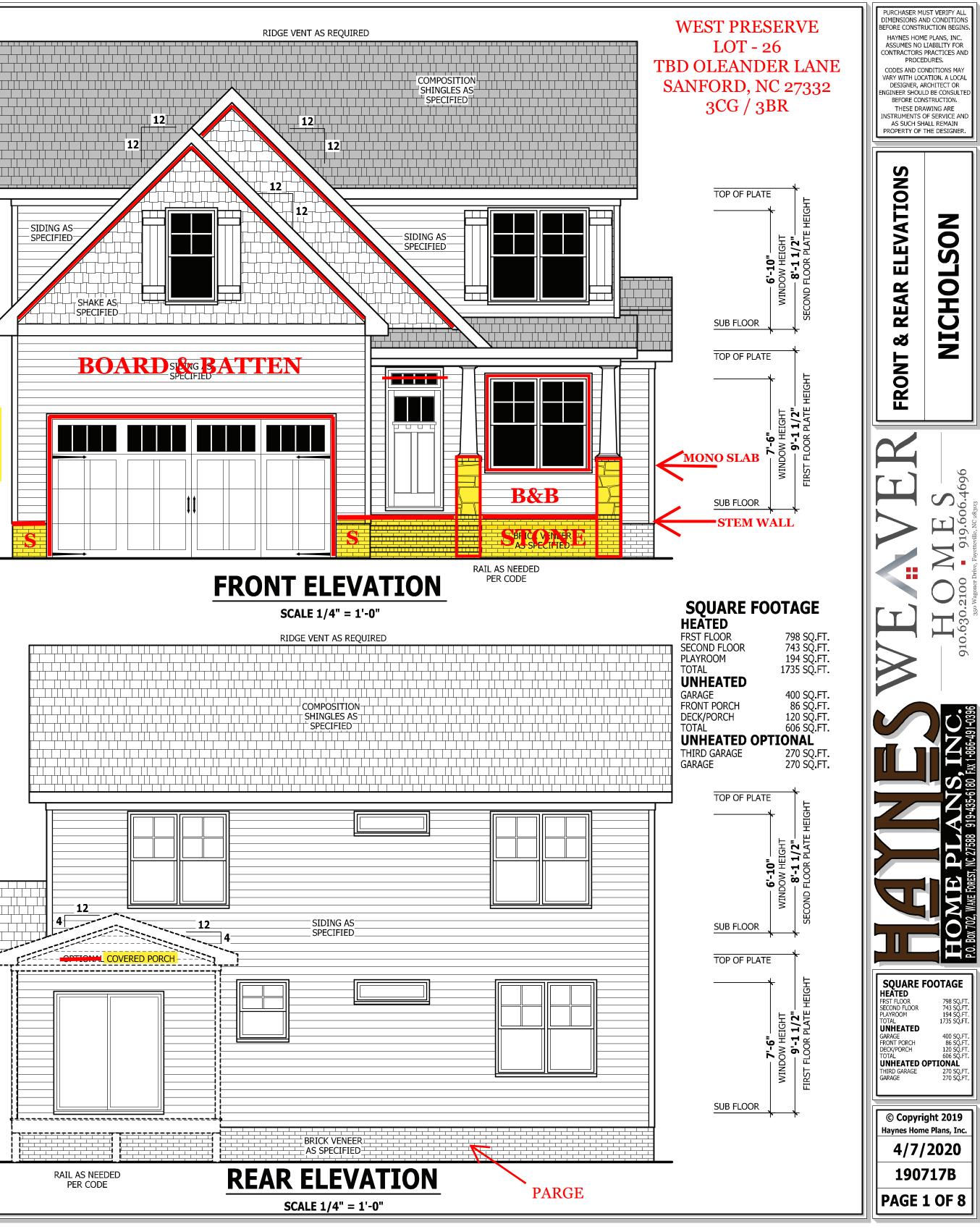
1. *Guards* on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the treads.

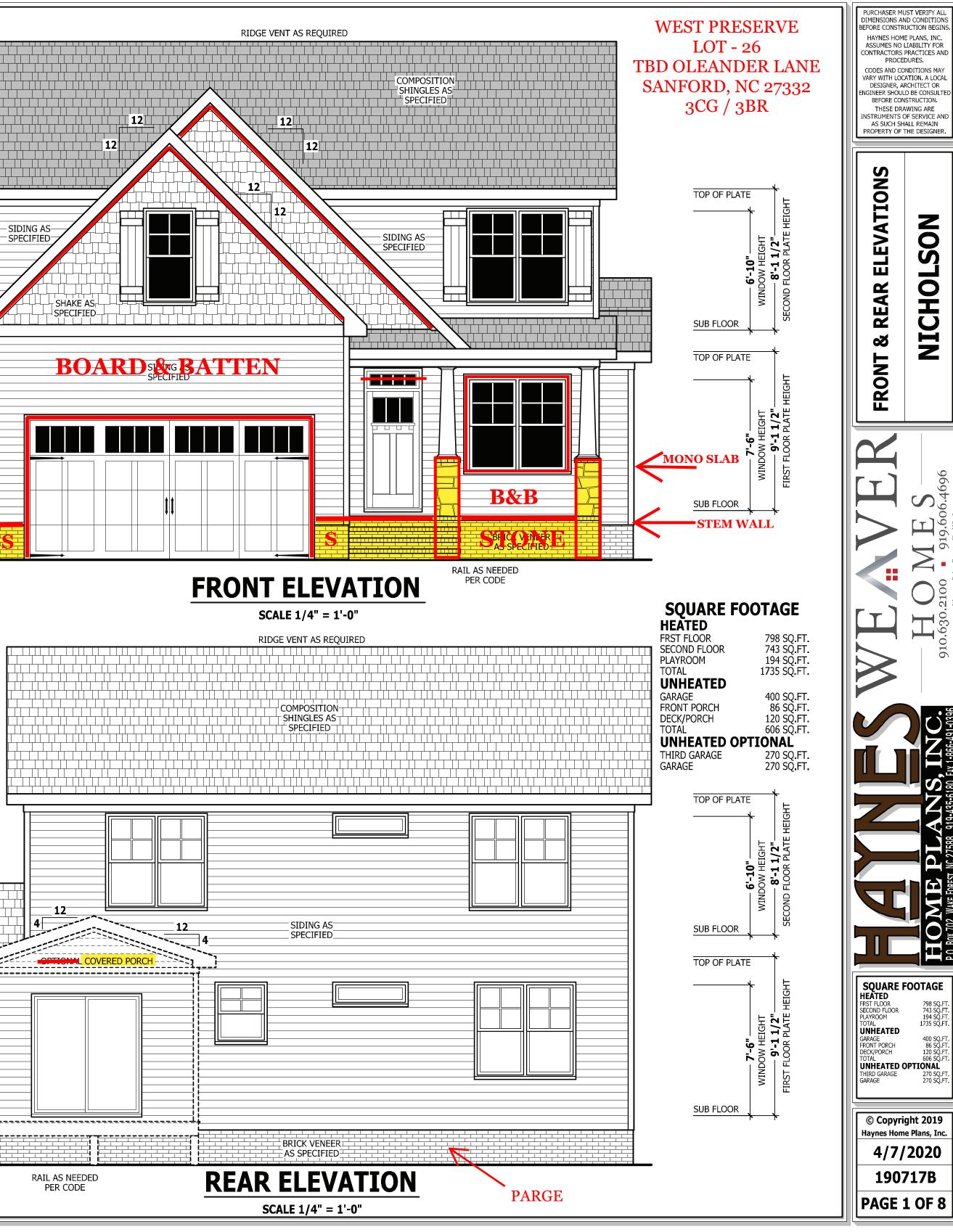
2. Where the top of the *guard* also serves as a handrail on the open sides of stairs, the top of the *guard* shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

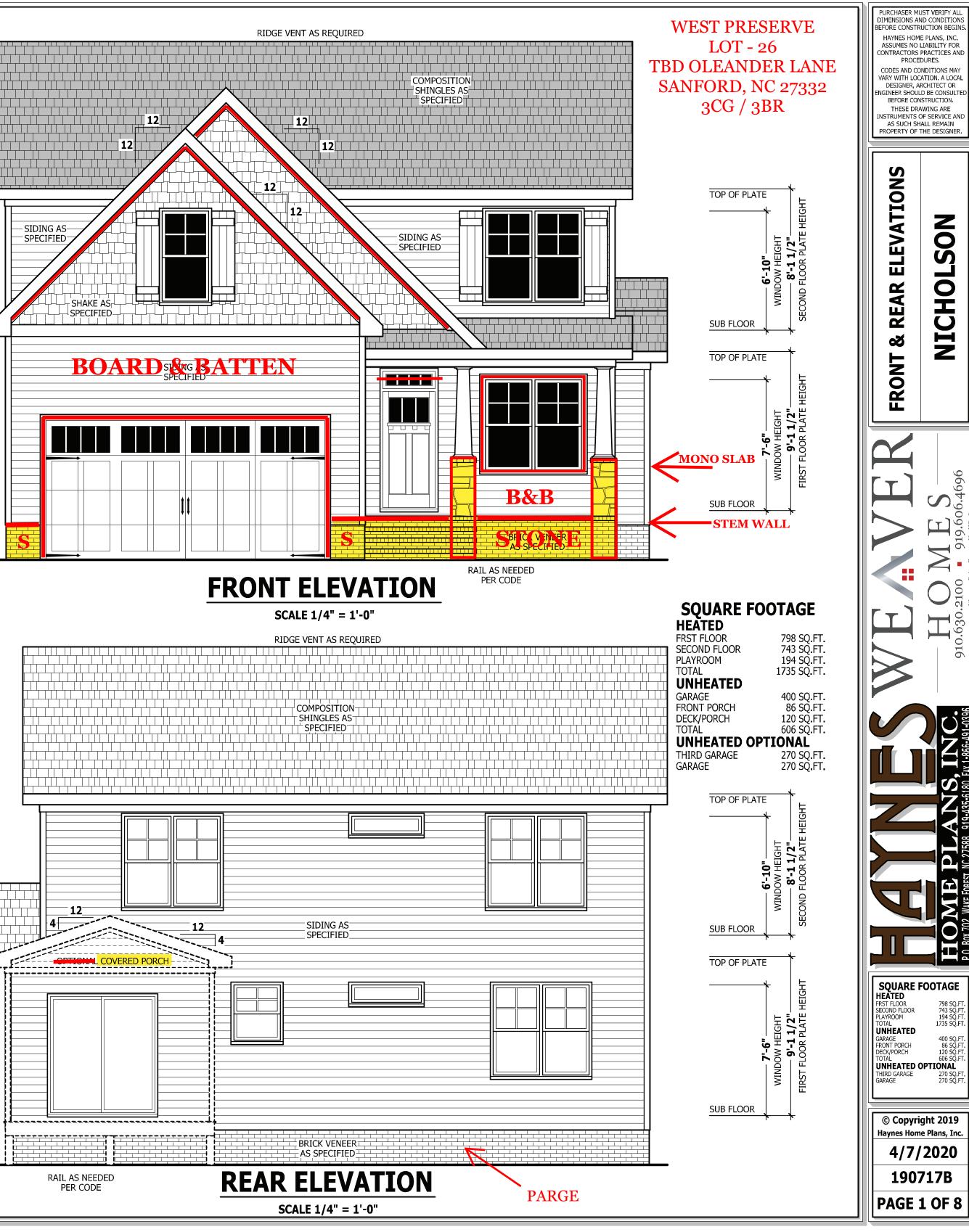
R312.3 Opening limitations. Required guards shall not have openings from the walking surface to the required *guard* height which allow passage of a sphere 4 inches (102 mm)in diameter. Exceptions:

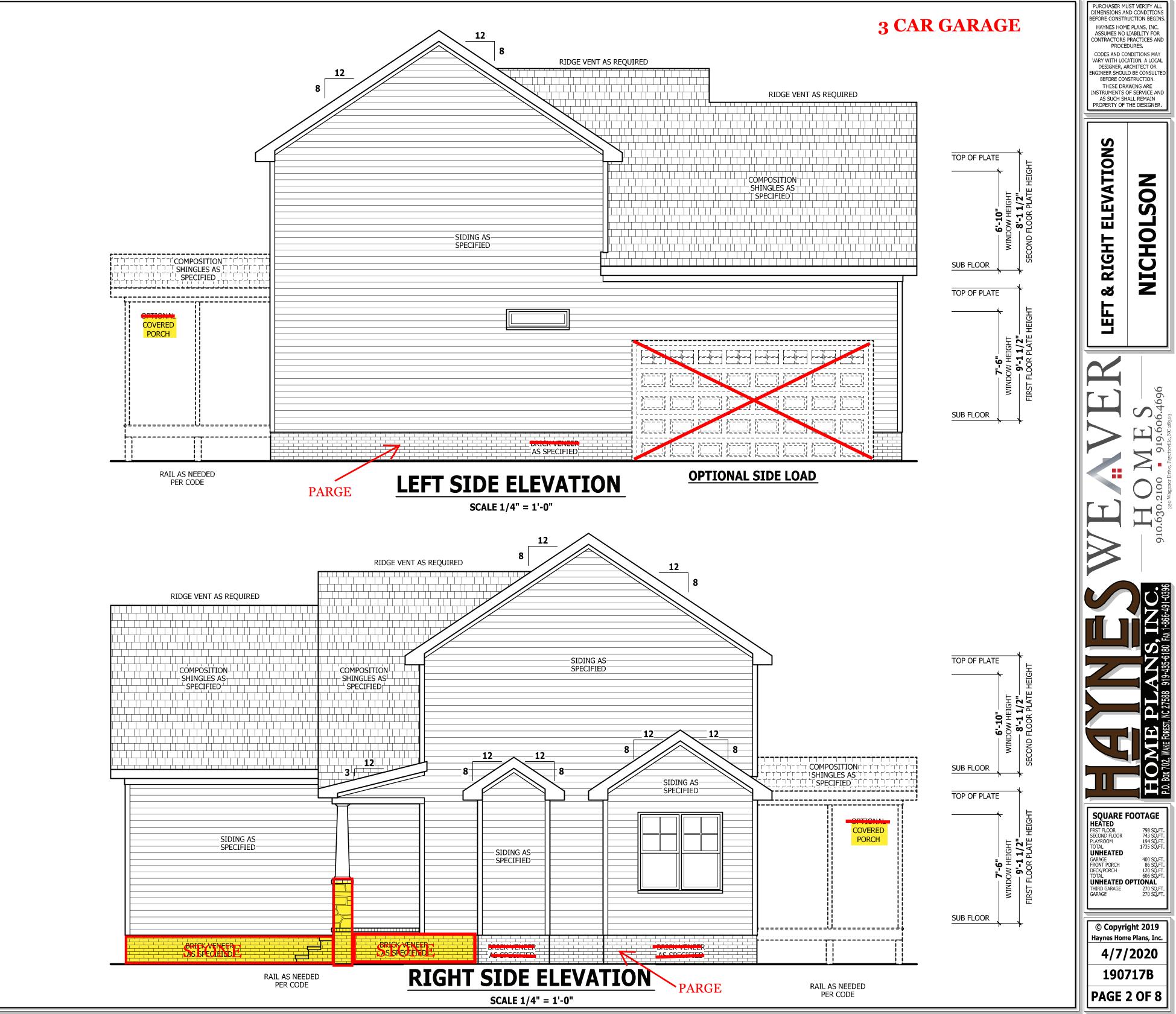
1. The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a *guard*, shall not allow passage of a sphere 6 inches (153 mm) in diameter.

2. *Guards* on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter.









WALL THICKNESSES

Exterior walls and walls adjacent to a garage area are drawn as 4" or as noted 2 X 6 are drawn as 6" to include 1/2" sheathing or gypsum. Subtract 1/2" for stud face. Interior walls are drawn as 3 1/2" or as noted 2 X 6

are drawn as 5 1/2", and do not include gypsum.

DWELLING / GARAGE SEPARATION

REFER TO SECTIONS R302.5, R302.6, AND R302.7

WALLS. A minimum 1/2" gypsum board must be installed on all walls supporting floor/ceiling assemblies used for separation required by this section. **STAIRS.** A minimum of 1/2" gypsum board must be installed on the underside and exposed sides of all stairways.

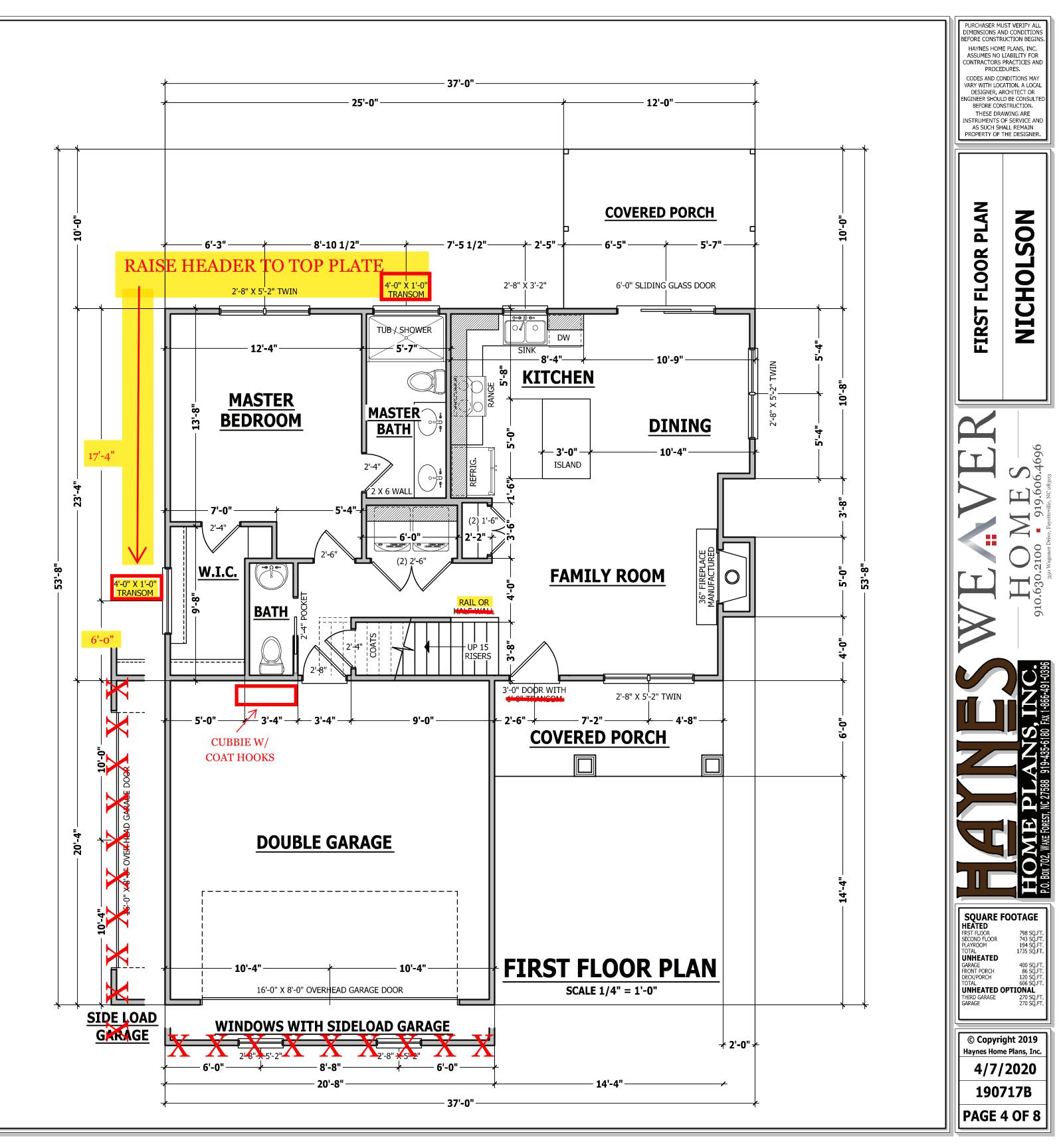
CEILINGS. A minimum of 1/2" gypsum must be installed on the garage ceiling if there are no habitable room above the garage. If there are habitable room above the garage a minimum of 5/8" type X gypsum board must be installed on the garage ceiling. **OPENING PENETRATIONS.** Openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 1 3/8 inches (35 mm) thick, or 20-minute fire-rated doors.

DUCT PENETRATIONS. Ducts in the garage and ducts penetrating the walls or ceilings separating the *dwelling* from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other *approved* material and shall have no openings into the garage.

OTHER PENETRATIONS. Penetrations through the separation required in Section R302.6 shall be protected as required by Section R302.11, Item 4.

SQUARE FOOTAGE

ΠΕΑΙΕν	
FRST FLOOR	798 SQ.FT.
SECOND FLOOR	743 SQ.FT.
PLAYROOM	194 SQ.FT.
TOTAL	1735 SQ.FT.
UNHEATED	-
GARAGE	400 SQ.FT.
FRONT PORCH	86 SQ.FT.
DECK/PORCH	120 SQ.FT.
TOTAL	606 SQ.FT.
UNHEATED (OPTIONAL
THIRD GARAGE	270 SQ.FT.
GARAGE	270 SQ.FT.
	•



STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code. JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no

liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

•	U		
DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x10⁶ PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. **LINTELS:** Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing. ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters.

CONCRETE AND SOILS: See foundation notes.

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

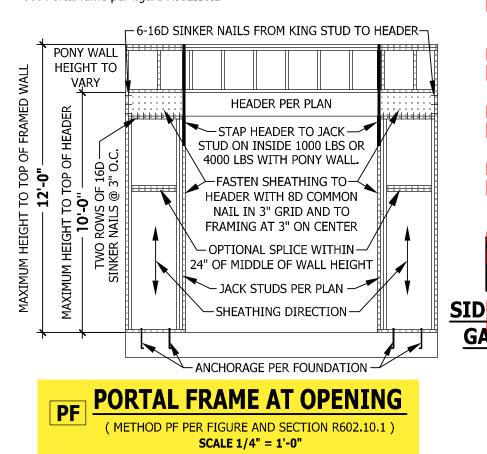
GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

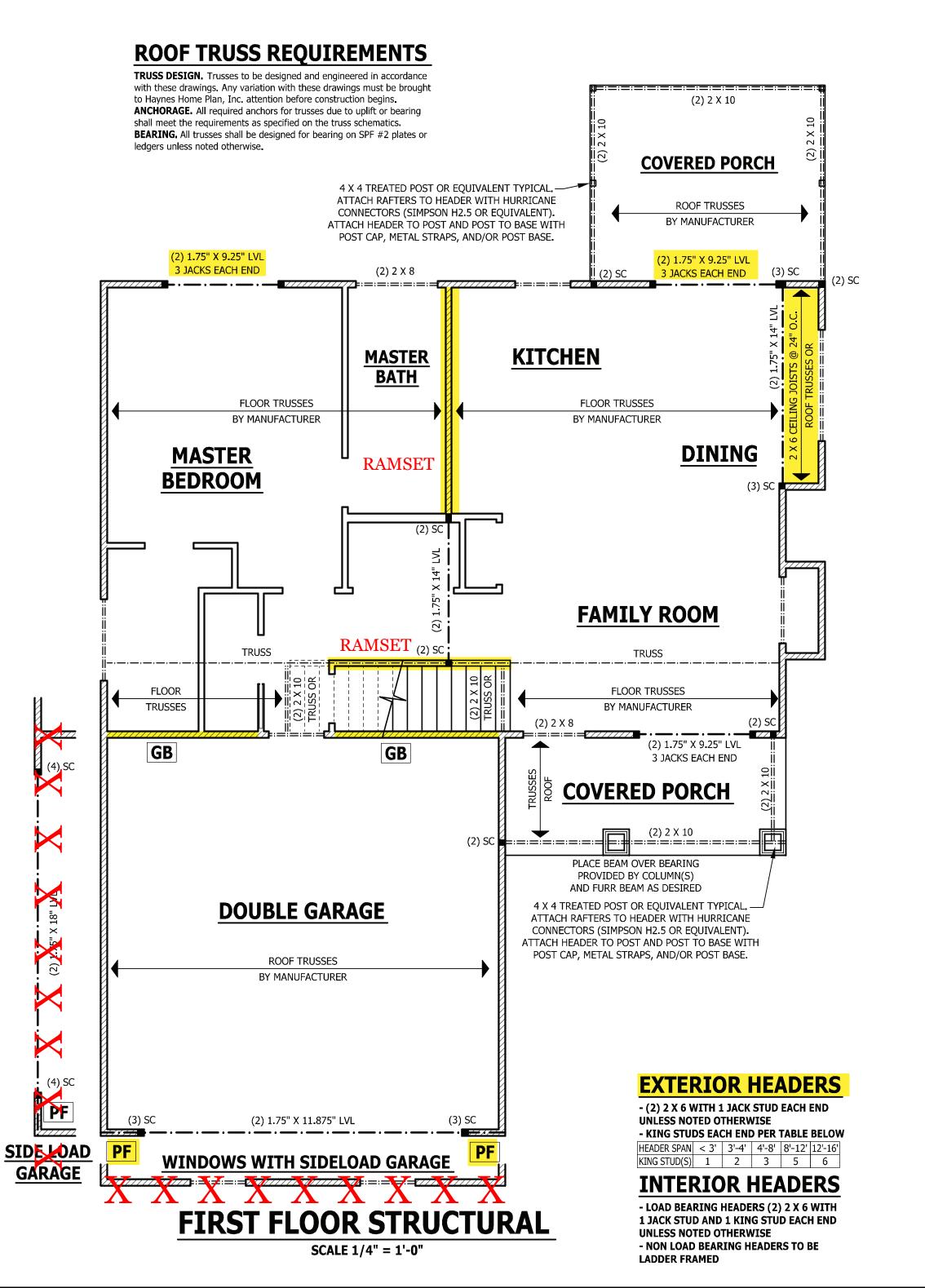
REQUIRED LENGTH OF BRACING: Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length. HD: 800 lbs hold down hold down device fastened to the edge of the brace wall panel closets to the corner.

Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or $8d(2 1/2" \log x 0.113" diameter)$. CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with $1 \frac{1}{2}$ long x 0.12" diameter galvanized roofing nails

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. **PF**: Portal fame per figure R602.10.1







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DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions. TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. **LINTELS:** Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing. **ROOF SHEATHING:** OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters. **CONCRETE AND SOILS:** See foundation notes.

ATTIC ACCESS

SECTION R807

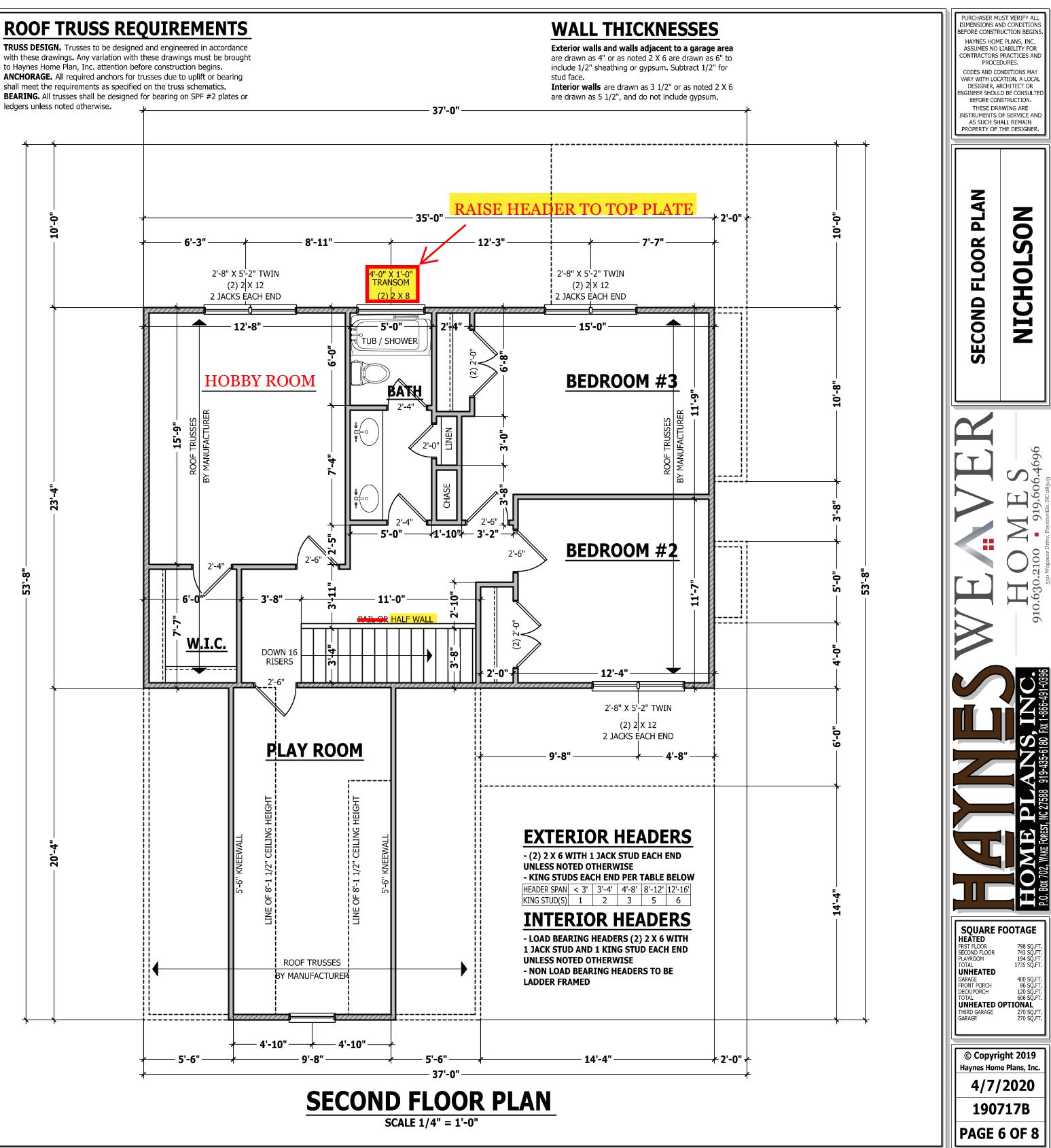
R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net clear opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

Exceptions:

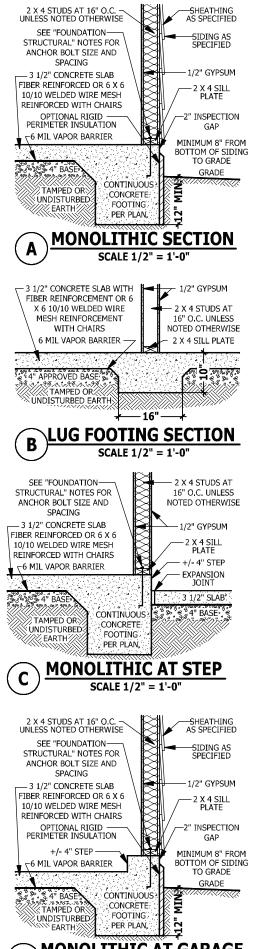
1. Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.

2. Pull down stair treads, stringers, handrails, and hardware may protrude into the net clear opening.

with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **ANCHORAGE.** All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or



:\Builder\Weaver Development Company, Inc\200129B Nicholson\200129B Nicholson - Left.aec



D MONOLITHIC AT GARAGE SCALE 1/2" = 1'-0"

FOUNDATION STRUCTURAL

115 to 130 mph wind zone (1 1/2 to 2 1/2 story)

CONTINUOUS FOOTING: 16" wide and 8" thick minimum. 20" wide minimum at brick veneer. Must extended 2" to either side of supported wall. **GIRDERS:** (3) 2 X 10 girder unless noted otherwise.

PIERS: 16" X 16" piers with 8" solid masonry cap on 30" X 30" X 10" concrete footing with maximum pier height of 64" with hollow masonry and 160" with solid masonry.

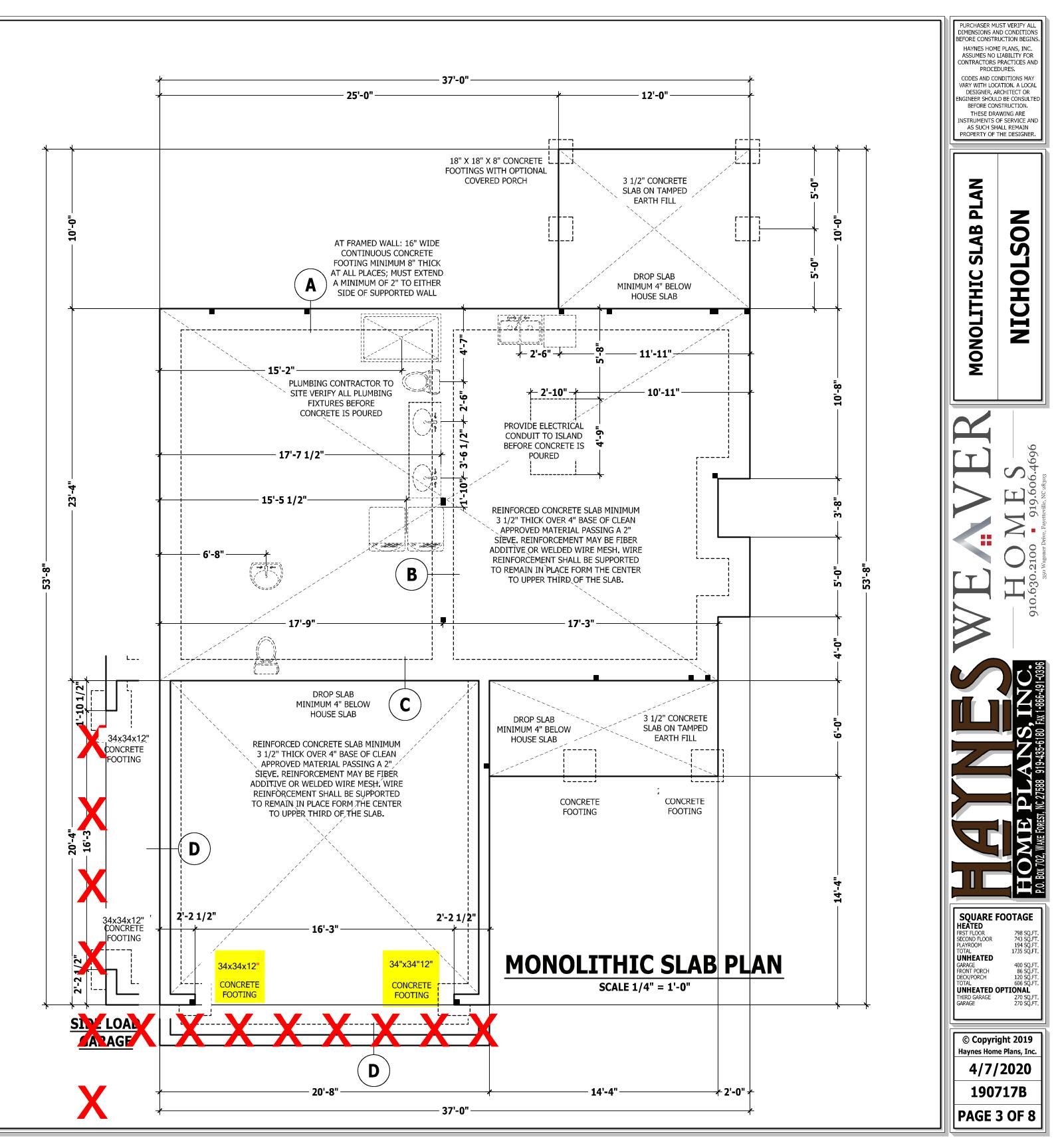
POINT LOADS: designates significant point load and should have solid blocking to pier, girder or foundation wall.

115 and 120 MPH ANCHORS BOLTS: 1/2" diameter anchor bolts embedded minimum 7", maximum 6'-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate.

130 MPH ANCHORS BOLTS: 1/2" diameter anchor bolts embedded minimum 15", maximum 4'-0" on center, within 12" of plate ends, and minimum two anchor bolts per plate.

CONCRETE: Concrete shall have a minimum 28 day strength of 3000 psi and a maximum 5" slump. Air entrained per table 402.2. All concrete shall be in accordance with ACI standards. All samples for pumping shall be taken from the exit end of the pump.

SOILS: Allowable soil bearing pressure assumed to be 2000 PSF. The contractor must contact a geotechnical engineer and a structural engineer if unsatisfactory subsurface conditions are encountered. The surface area adjacent to the foundation wall shall be provided with adequate drainage, and shall be graded so as to drain surface water away from foundation walls.

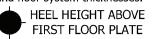


ROOF TRUSS REQUIREMENTS

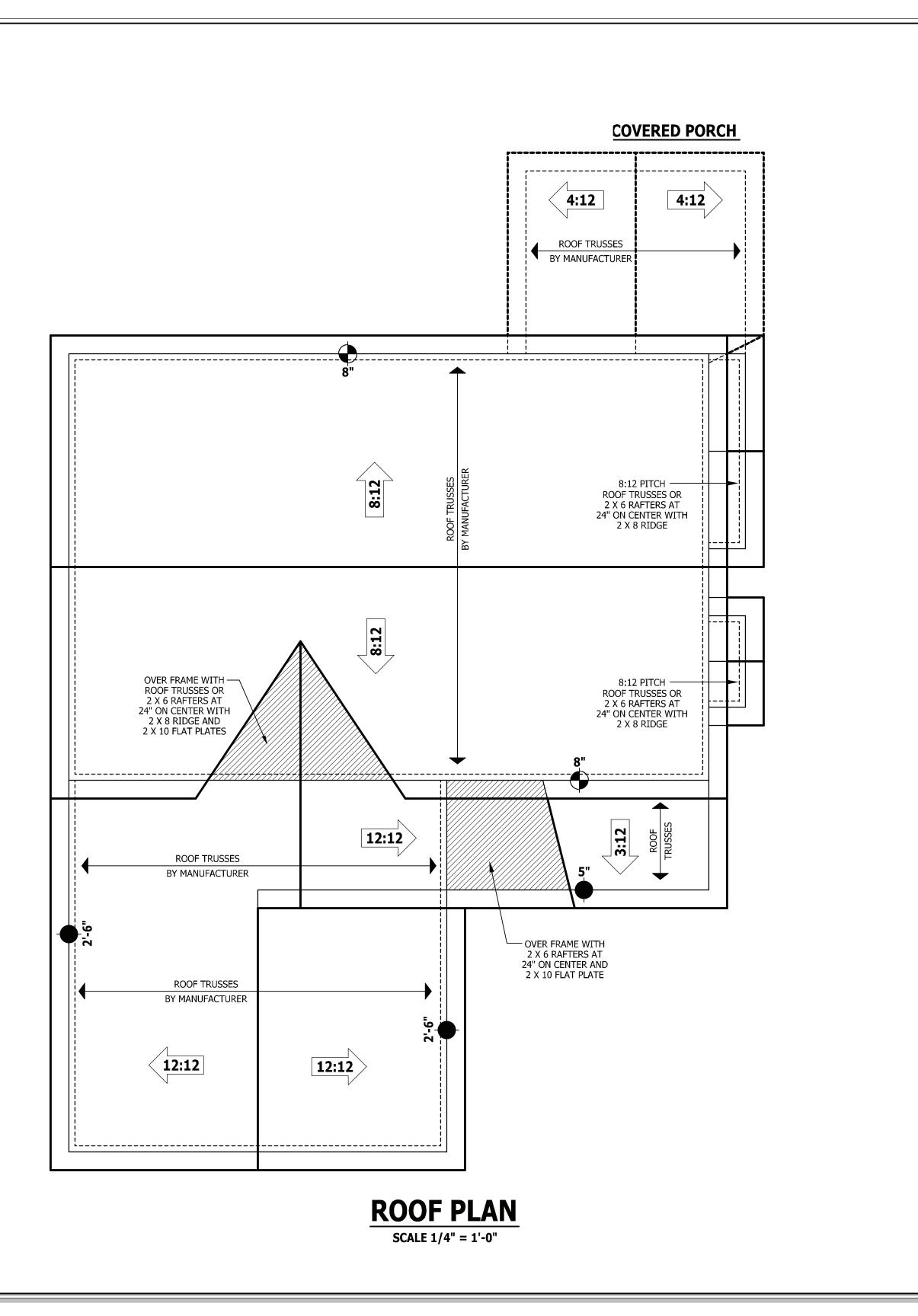
TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **KNEE WALL AND CEILING HEIGHTS.** All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

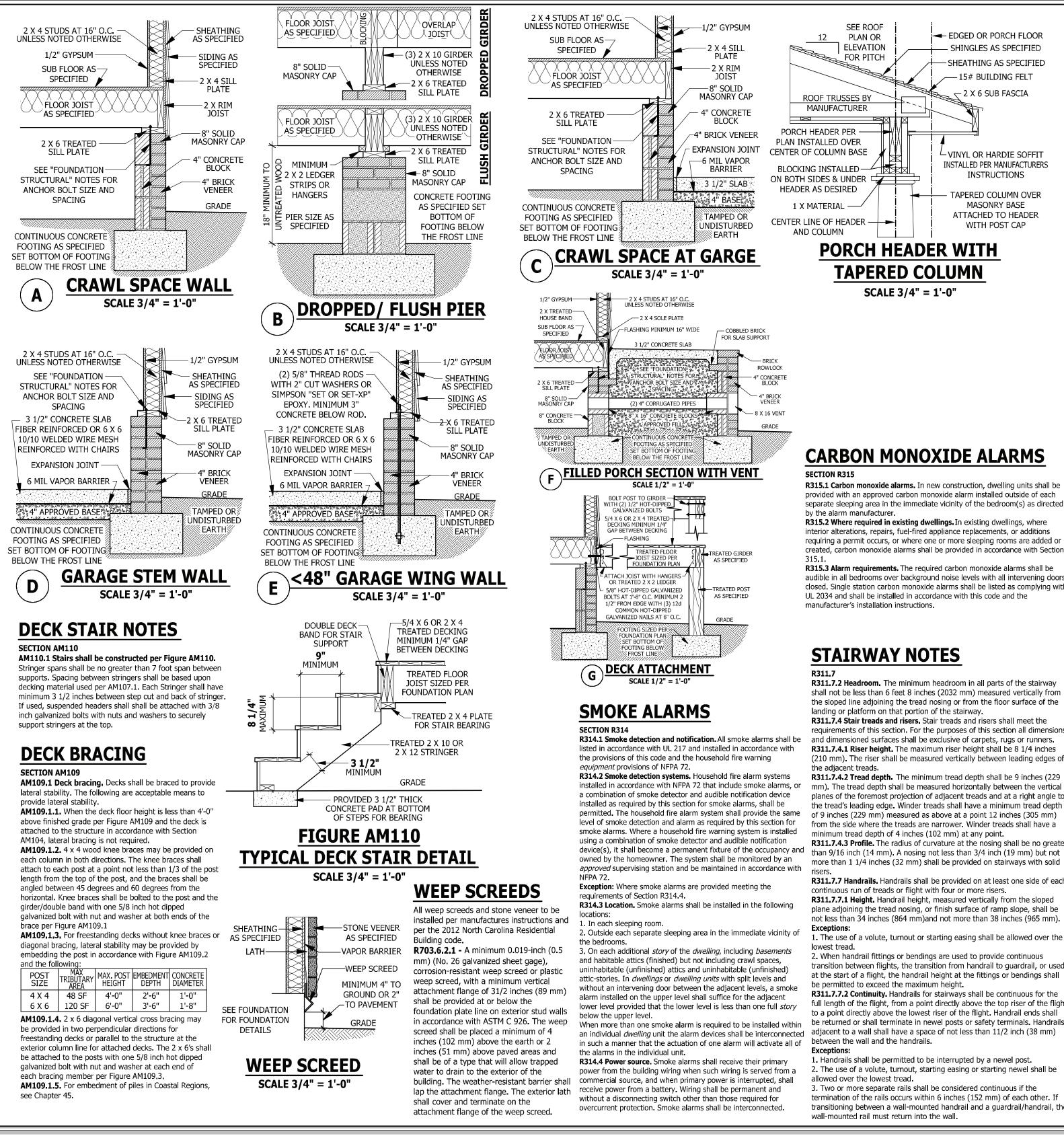
Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.



HEEL HEIGHT ABOVE SECOND FLOOR PLATE







R315.1 Carbon monoxide alarms. In new construction, dwelling units shall be provided with an approved carbon monoxide alarm installed outside of each separate sleeping area in the immediate vicinity of the bedroom(s) as directed

interior alterations, repairs, fuel-fired appliance replacements, or additions requiring a permit occurs, or where one or more sleeping rooms are added or created, carbon monoxide alarms shall be provided in accordance with Section

audible in all bedrooms over background noise levels with all intervening doors closed. Single station carbon monoxide alarms shall be listed as complying with

shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the

requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners. R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches

R311.7.4.2 Tread depth. The minimum tread depth shall be 9 inches (229

mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stairways with solid

R311.7.7 Handrails. Handrails shall be provided on at least one side of each

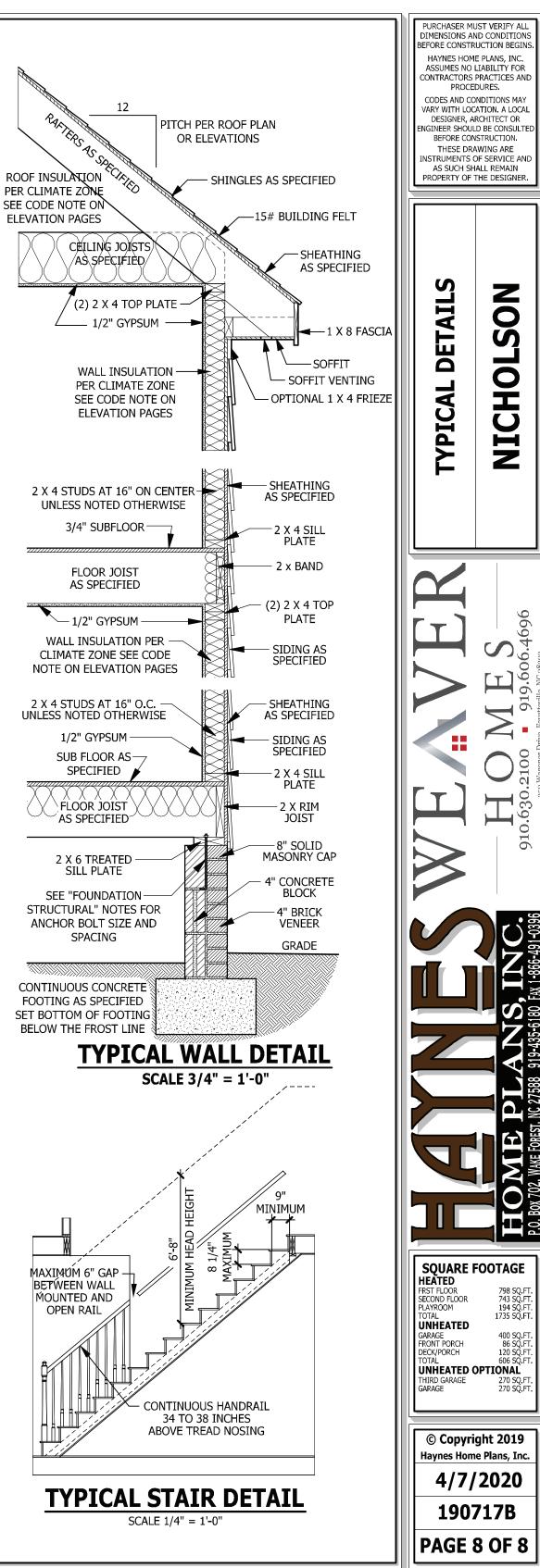
plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm).

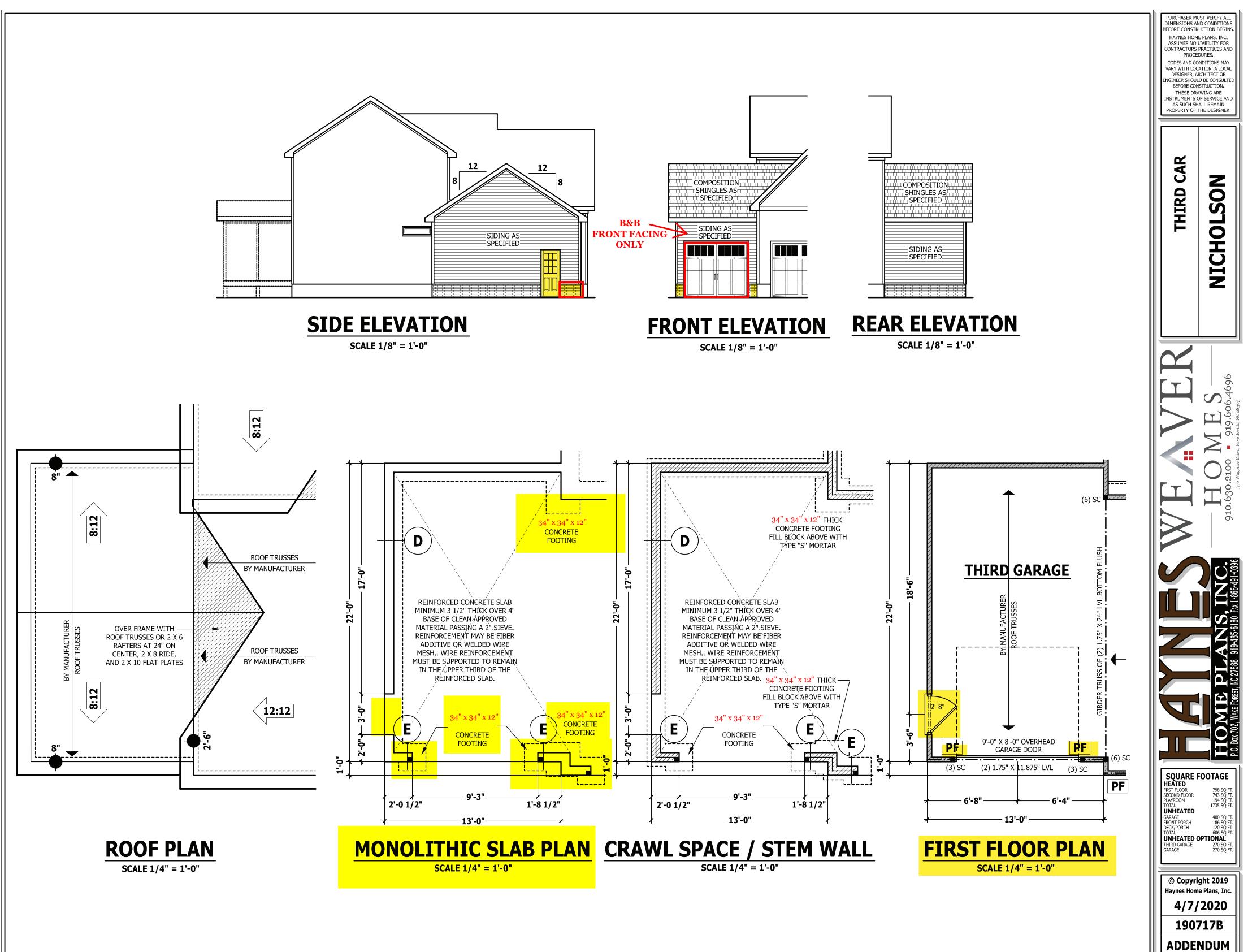
1. The use of a volute, turnout or starting easing shall be allowed over the

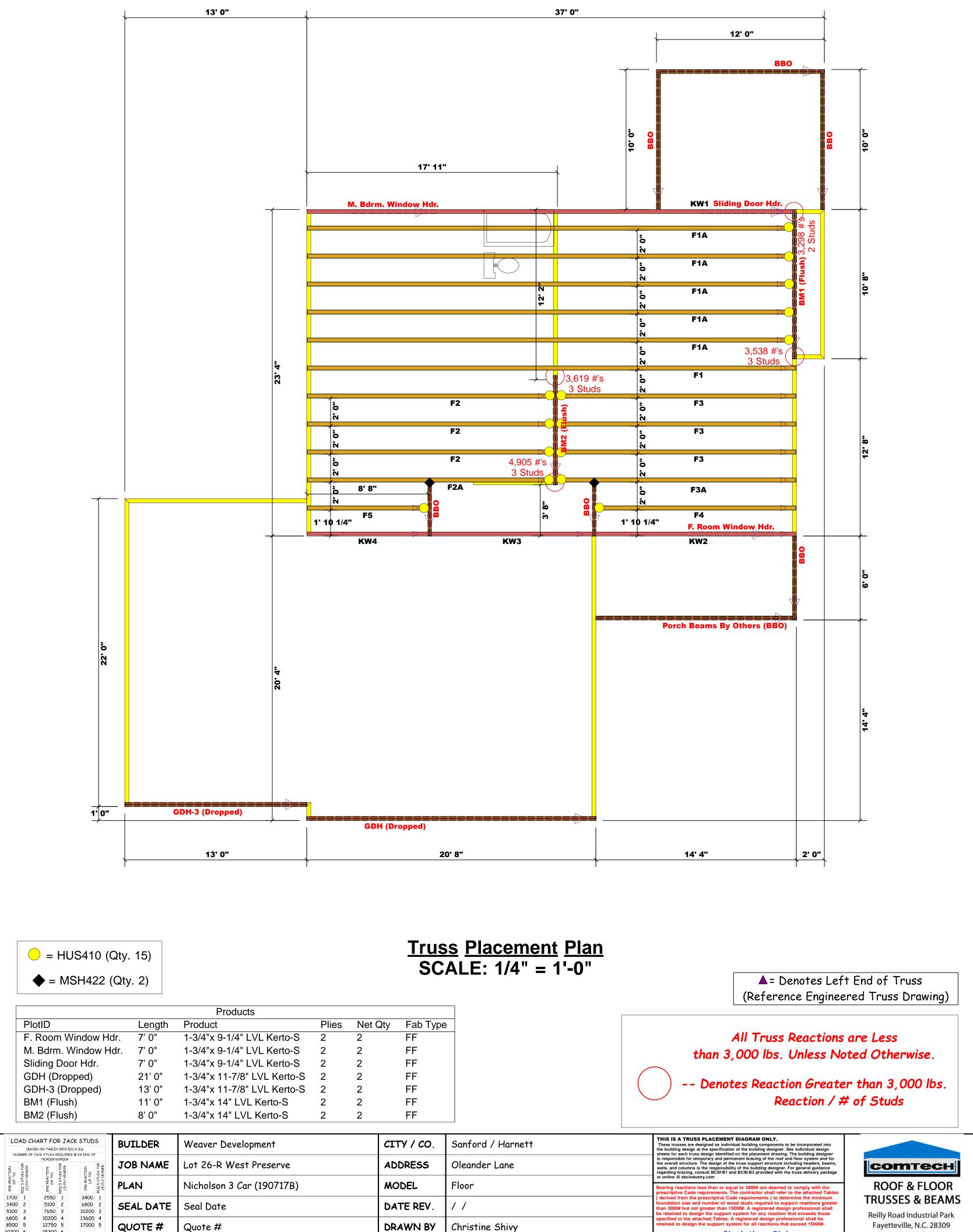
transition between flights, the transition from handrail to guardrail, or used at the start of a flight, the handrail height at the fittings or bendings shall

full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails

termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the







10200 6

11900 7

13600 8

15300 9

15300 6

JOB #

J0123-0305

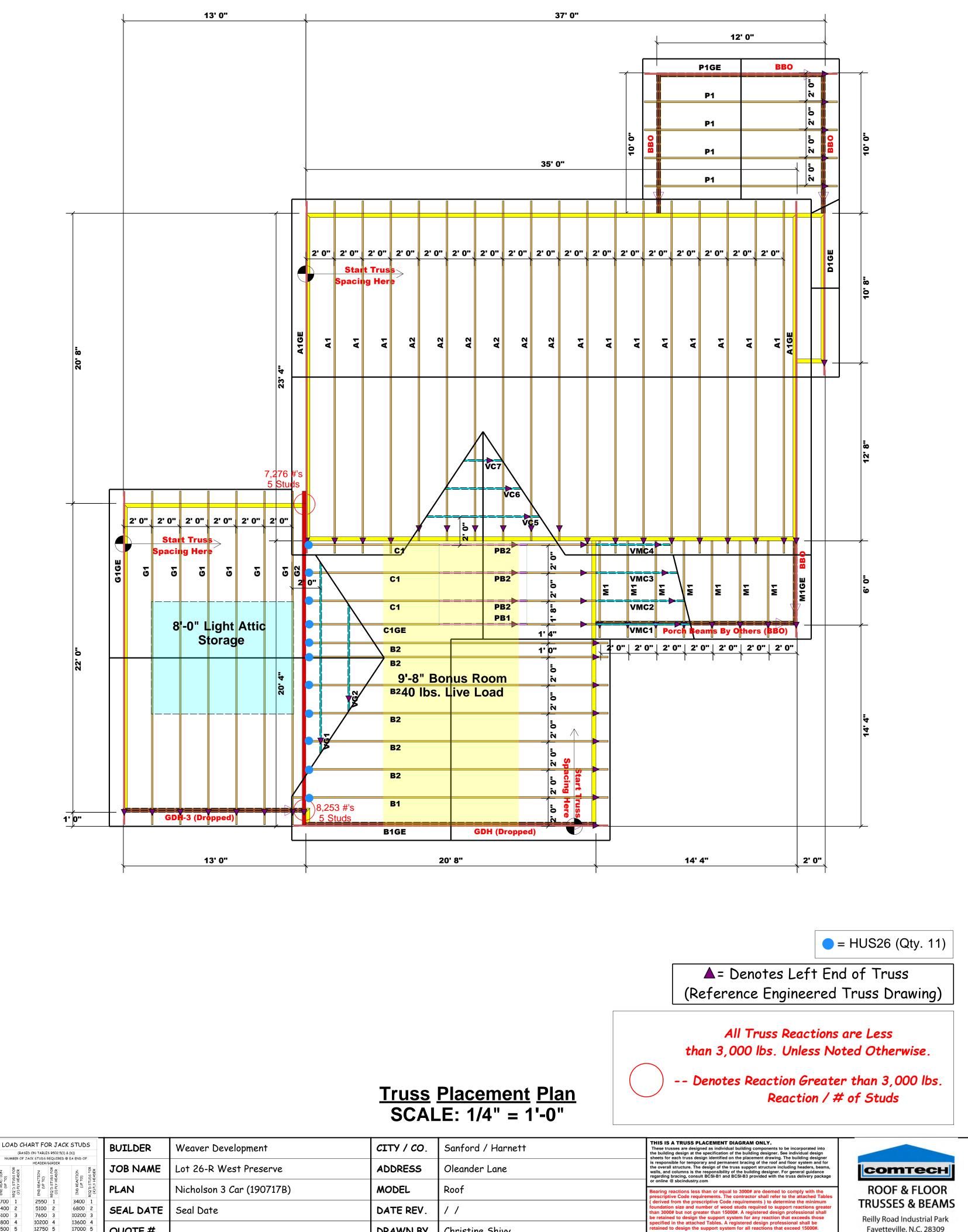
SALES REP.

Lenny Norris

Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Christine Shivy

Christine Shivy



DRAWN BY

SALES REP.

Christine Shivy

Lenny Norris

END REACTION (UP TO) REQ'D STUDS FC (2) PLY HEADEI

1700 1

3400 2

5100 3

6800 4

8500 5

10200 6

11900 7

13600 8

15300 9

10200 4

12750 5

15300 6

13600 4

17000 5

QUOTE #

J0123-0304

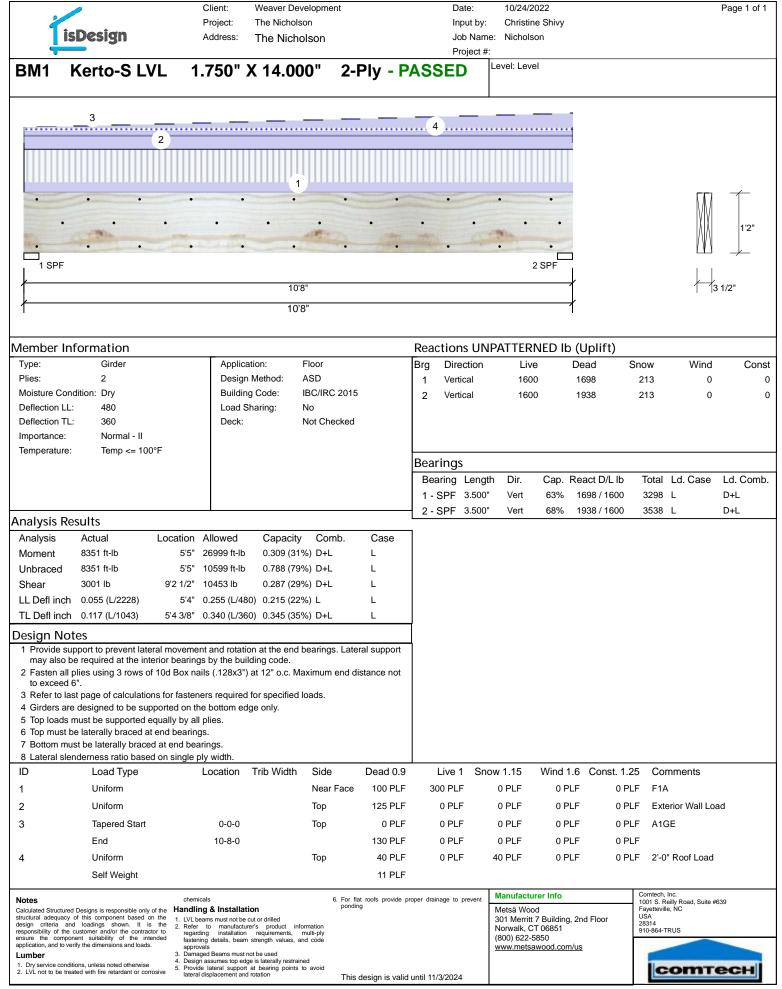
JOB #

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Christine Shivy

Christine Shivy

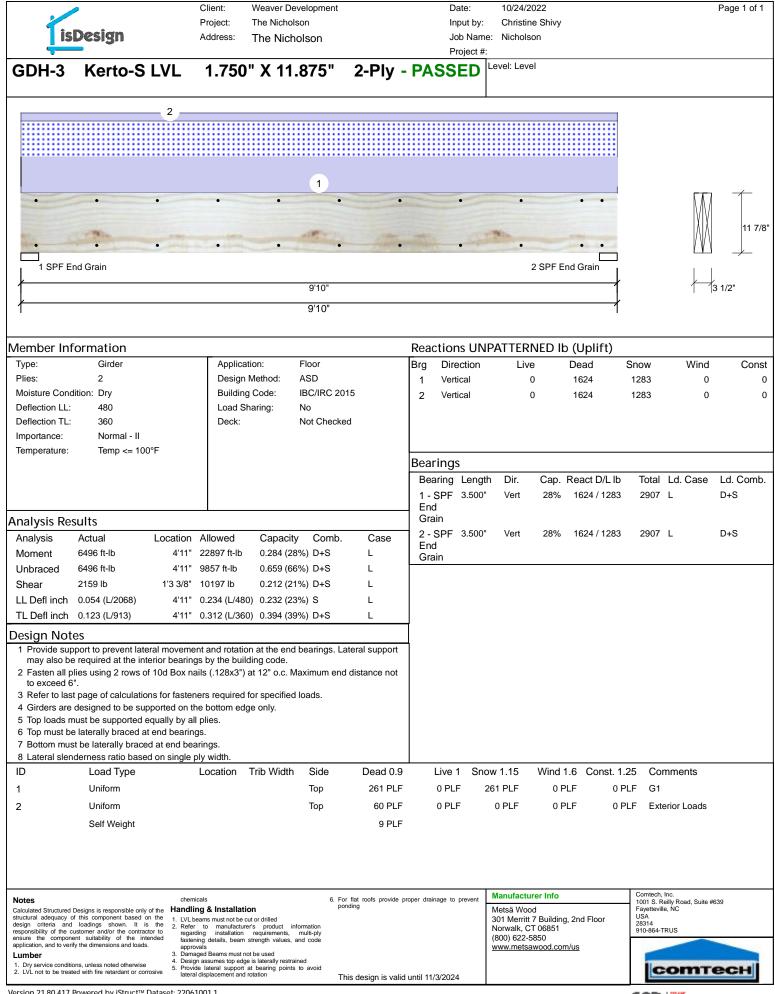
Signatur



	/	Client:	Weaver Devel	opment		Da	ite:	10/24/202	22				Page 1 of 1
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lombor I	nformation					Reaction	e LIND			lift)			
Type:	Girder	App	ication: Fl	oor		Brg Dire		Live	Dea Dea		Snow	Wind	Const
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	ondition: Dry		0	C/IRC 2015		2 Vertio	cal	2679	93	9	0	0	C
Deflection L		Load	I Sharing: No	o ot Checked									
Deflection T Importance		Dec		ot Checked									
Temperatur		-											
						Bearings							
						Bearing	-		Cap. Read			Ld. Case	Ld. Comb.
						1 - SPF		Vert		1/3644	4905		D+L
Analysis F	Results	•				2 - SPF	3.500"	Vert	70% 93	9 / 2679	3619	L	D+L
Analysis	Actual L	ocation Allowed	Capacity	Comb.	Case]							
Moment		3'11 1/2" 26999 ft-	. ,		L								
Unbraced		3'11 1/2" 13870 ft-	. ,		L								
Shear	3460 lb h 0.038 (L/2320)	1'5 1/2" 10453 lb 3'11 1/8" 0.183 (L/	0.331 (33%) 480) 0.207 (21%)		L								
		3'11 1/8" 0.244 (L/			L								
Design N	· /	0111/0 0.211(E	000) 0.210 (2170)	DIE	-	1							
<u> </u>	support to prevent lateral	I movement and rot	ation at the end be	arings. Later	al support	1							
may also	be required at the interi	or bearings by the t	uilding code.										
2 Fasten a to excee	II plies using 3 rows of 1 d 6".	Ud Box nalls (.128x	3") at 12" o.c. Max	mum ena al	stance not								
	last page of calculations				oria								
present.	rated load fastener speci		Ū	ers il a riarig	eris								
	are designed to be suppo t be laterally braced at e		edge only.										
	nust be laterally braced a	•											
	lenderness ratio based o												
ID	Load Type	Location		Side	Dead 0.9	Live 1			Wind 1.6			mments	
1	Point	0-3-8		Near Face	306 lb	917 lb		0 lb	0 lb		lb F3A		
2	Point	0-3-8		Far Face	264 lb	790 lb		0 lb	0 lb		Ib F2A	A Contraction of the second se	
3	Part. Uniform	1-2-4 to 7-9-8		Near Face	115 PLF	344 PLF		0 PLF	0 PLF	0 PL			
4	Part. Uniform Self Weight	1-2-4 to 7-9-8		Far Face	119 PLF 11 PLF	355 PLF		0 PLF	0 PLF	UPL	_F F2		
	Sen weight												
Notes		chemicals		6. For fla	t roofs provide p	roper drainage to p	Jieveni	lanufacture	r Info		Comtech, 1001 S. R	eilly Road, Suite #	639
structural adequa	red Designs is responsible only of the component based on the transformation of the component based on the component based	he 1. LVL beams must not	be cut or drilled	pondin	A			Netsä Wood 801 Merritt 7	Building, 2nd I	loor	Fayettevill USA	e, NC	
responsibility of the ensure the com	and loadings shown. It is the ne customer and/or the contractor ponent suitability of the intender	he 2. Refer to manufa to regarding installa	cturer's product inform ion requirements, mul am strength values, and	ti-ply			N	lorwalk, CT (800) 622-585	06851		28314 910-864-1	RUS	
application, and to	verify the dimensions and loads.	approvals 3. Damaged Beams m	ist not be used					www.metsaw					
1. Dry service con	nditions, unless noted otherwise treated with fire retardant or corrosiv	 Design assumes top Provide lateral curp 	edge is laterally restrained ort at bearing points to a								le	omt	есн
-		taset: 22061001.1		This	aesign is valid	until 11/3/2024							

isD	esign	Client: Project: Address:	Weaver Deve The Nicholso The Nichol	'n		Date Input Job I Proje	t by: Name:	10/24/20 Christine Nicholso	Shivy				Page 1 of
F. Room W	indow Hdr.	Kerto-S LVL	1.750	" X 9.250'	' 2-P	ly - PASS	ED ^{Le}	vel: Level					
4	2		1		(3)								
• • • •	• • •		•	· .	2	SPF End Grain	•						9
ł			6'1"									<u>}</u>	3 1/2"
ſ			6'1"				1						
lember Infor	mation					Reactions			IFD Ib	(Unlift)			
Type: Plies: Moisture Conditic Deflection LL: Deflection TL:	Girder 2 Dry 480 360	Applica Design Building Load SI Deck:	Method: A Code: II naring: N	Floor NSD BC/IRC 2015 Io Iot Checked		Brg Directi 1 Vertica 2 Vertica	ion I	Live 122 122	[Dead 1375 1375	Snow 928 928	Wind 0 0	Cor
Importance: Temperature:	Normal - II Temp <= 100°F					Bearings Bearing Lo 1 - SPF 3. End	-	Dir. Vert	Cap. R 22%	eact D/L lb 1375 / 928		al Ld. Case 3 L	Ld. Com D+S
Moment 29 Unbraced 29	ctual Lo 995 ft-lb 995 ft-lb 504 lb 019 (L/3521)	cation Allowed 3' 1/2" 14423 ft-lb 3' 1/2" 10944 ft-lb 1' 3/4" 7943 lb 3' 1/2" 0.141 (L/480 3' 1/2" 0.188 (L/360		5) D+S 5) D+S 5) D+S 5) S	Case L L L L	Grain 2 - SPF 3. End Grain	500"	Vert	22%	1375 / 928	230	3 L	D+S
 may also be re 2 Fasten all pliet to exceed 6". 3 Refer to last paid 4 Girders are de 5 Top loads musion 6 Top must be lation 7 Bottom must be 	rt to prevent lateral n equired at the interior s using 2 rows of 10c age of calculations fo	l bearings. end bearings.	ding code. at 12" o.c. Ma for specified lo	ximum end dista									
ID 1	Load Type Uniform	Location	Trib Width	Side [Top	Dead 0.9 125 PLF	Live 1 0 PLF	Snow	1.15) PLF	Wind 1.6 0 PLI	6 Const. 1 = 0		omments /all Load	
2	Uniform			Тор	249 PLF	0 PLF		9 PLF	0 PLI		PLF A		
3 4	Uniform Uniform Self Weight			Тор Тор	15 PLF 56 PLF 7 PLF	40 PLF 0 PLF) PLF 3 PLF	0 PLI 0 PLI		PLF 1 PLF N	'-0" Floor Load I1	1
structural adequacy of th design criteria and lo esponsibility of the custo ansure the component application, and to verify th Lumber 1. Dry service conditions,	s component based on the adings shown. It is the mer and/or the contractor to suitability of the intended e dimensions and loads.	chemicals Handling & Installati 1. LVL beams must not be c 2. Refer to manufactur regarding installation fastening details, beam approvals 3. Damaged Beams must n 4. Design assumes top edg 5. Provide lateral support lateral displacement and	ut or drilled tr's product infor requirements, m strength values, and to be used a is laterally restrained at bearing points to	ponding mation ulti-ply d code d avoid		oper drainage to pret until 11/3/2024	M 30 N (8	lanufacture letsä Wood 01 Merritt 7 orwalk, CT 300) 622-58 ww.metsaw	Building, 2 06851 50		1001 S Fayette USA 28314 910-86	ch, Inc. J. Reilly Road, Suite sville, NC 4-TRUS	

is	Design		The Nicholson The Nicholson			Christine Shivy e: Nicholson			
GDH	Kerto-S LVL	1.750" >	(11.875" 2	-Ply - P	Project #:	Level: Level			
1 SPF En	d Grain	2	1	-			2 SPF End Gr		11 7/8"
ł			16'10)"					
					<u> </u>		41		
Aember Ini Type: Plies: Moisture Cond Deflection LL: Deflection TL: Importance:	Girder 2 dition: Dry 480	Applicati Design I Building Load Sh Deck:	Nethod: ASD Code: IBC/IRC 201		Brg Direction 1 Vertical 2 Vertical	Live 0	Dead Sr 2098	now Wind 337 0 337 0	Con
Temperature:	Temp <= 100°F				Bearings Bearing Lengtl 1 - SPF 3.500" End Grain	n Dir. Cap. Vert 24%	React D/L lb 2098 / 337	Total Ld. Case 2434 L	Ld. Com D+S
nalysis Re Analysis Moment Unbraced Shear LL Defl inch	Actual Loc 8354 ft-lb 9694 ft-lb 1788 lb 1 0.070 (L/2809) 8'5	ation Allowed 8'5" 17919 ft-lb 8'5" 9704 ft-lb 3 3/8" 7980 lb 1/16" 0.409 (L/480 1/16" 0.546 (L/360		Case Uniform L Uniform L	Grain 2 - SPF 3.500" End Grain	Vert 24%	2098 / 337	2434 L	D+S
Design Not 1 Provide suy may also b 2 Fasten all g to exceed 6 3 Refer to las 4 Girders are 5 Top loads r 6 Top must b 7 Bottom must	CES oport to prevent lateral m e required at the interior blies using 2 rows of 10d	ovement and rotation bearings by the build Box nails (.128x3") a r fasteners required f ed on the bottom edg y by all plies. aximum of 9'6 3/4" o. end bearings.	a at the end bearings. Lat ing code. It 12" o.c. Maximum end or specified loads. e only.	teral support					
ID 1	Load Type Uniform	Location	Trib Width Side Top	Dead 0.9 200 PLF	Live 1 Sno 0 PLF		1.6 Const. 1.25 PLF 0 PLF		Siding/
2	Uniform Self Weight		Тор	40 PLF 9 PLF				Plywood, etc.)	-
structural adequacy of design criteria and esponsibility of the of ansure the comport application, and to ver Lumber 1. Dry service conditi	Designs is responsible only of the of this component based on the I loadings shown. It is the customer and/or the contractor to ent suitability of the intended ify the dimensions and loads. ions, unless noted otherwise ted with fire retardant or corrosive	chemicals Handling & Installatio 1. LVL beams must not be cu 2. Refer to manufacturer regarding installation fastering details, beam s 3. Damaged Beams must not 4. Design assumes top edge 5. Provide lateral support a lateral displacement and ro	n pond tor drilled 's product information requirements, multi-ply trength values, and code be used is laterally restrained bearing points to avoid	flat roofs provide pr ding s design is valid	roper drainage to prevent	Manufacturer Info Metsä Wood 301 Merritt 7 Buildin Norwalk, CT 06851 (800) 622-5850 www.metsawood.com	g, 2nd Floor	Comtech, Inc. 1001 S. Reilly Road, Suite # Fayetteville, NC USA 28314 910-864-TRUS	639 ECH



Version 21.80.417 Powered by iStruct™ Dataset: 22061001.1

	•	Client:	Weaver Dev	relopment			Date:	10/24/	2022				F	Page 1 of
	Destan	Project:	The Nichols				Input by	·	ne Shivy					
	Design	Address:	The Nicho	olson			Job Na Project	me: Nichol	son					
M. Bdrm. V	Window Hdr.	Kerto-S LVL	. 1.750"	X 9.250"	2-Ply	- PAS		Level: Lev	/el					
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	End Grain				2	2 SPF E	ind Grain							
1			6'1"										3	3 1/2"
ł			6'1"					\neg						
lember Inf		— • ··	ation			_		NPATTER		<u> </u>			<i>l</i>	
Type: Plies:	Girder 2			Floor ASD		Brg 1	Direction Vertical		/e 22	Dead 1205	Snov 75		/ind 0	Con
Moisture Cond		°		IBC/IRC 2015		2	Vertical		22	1205	75		0	
Deflection LL:	480		•	No										
Deflection TL: Importance:	360 Normal - II	Deck:		Not Checked										
Temperature:	Temp <= 100°F													
						Bear								
							ring Len	•	•	React D/L		otal Ld. C		Ld. Com
						1 - 8 End	SPF 3.50 I	0" Vert	19%	1205 / 7	57 1	962 L		D+S
nalysis Res	sults					Gra		0" \/	400/	1005 / 7				
Analysis		ocation Allowed	Capacity	Comb.	Case	End	SPF 3.50 I	0" Vert	19%	1205 / 7	5/ I	962 L	1	D+S
Moment Unbraced	2552 ft-lb 2552 ft-lb	3' 1/2" 14423 ft-lb 3' 1/2" 10944 ft-lb	·		L	Gra	in							
Shear	1282 lb	1' 3/4" 7943 lb	0.161 (16)		L									
LL Defl inch	0.016 (L/4312)	3' 1/2" 0.141 (L/4			L									
TL Defl inch	0.041 (L/1664)	3' 1/2" 0.188 (L/3	60) 0.216 (229	%) D+S	L									
Design Note]								
	port to prevent lateral required at the interio			pearings. Latera	al support									
	lies using 2 rows of 10		-	aximum end dis	stance not									
	t page of calculations f	for fasteners require	d for specified	oads.										
	designed to be suppor nust be supported equa		dge only.											
	e laterally braced at en													
	t be laterally braced at derness ratio based or	-												
ID	Load Type	Location	Trib Width	Side	Dead 0.9		Live 1 S	now 1.15	Wind	1.6 Cons	t. 1.25	Comment	s	
1	Uniform			Тор	125 PLF		0 PLF	0 PLF	0 F	۲LF	0 PLF	Exterior Wa	all Load	1
2	Uniform			Тор	249 PLF		0 PLF	249 PLF	0 F	۲LF	0 PLF	A1		
3	Uniform			Тор	15 PLF	4	0 PLF	0 PLF	0 F	۲LF	0 PLF	1'0" Floor L	.oad	
	Self Weight				7 PLF									
											0	tooh lee		
Notes Calculated Structured [Designs is responsible only of the	chemicals e Handling & Installa	ation	For flat ponding	roofs provide p	roper drain	age to prevent	Manufact Metsä Wo			100	ntech, Inc. 1 S. Reilly Road, etteville, NC	Suite #639	9
structural adequacy of design criteria and	f this component based on the loadings shown. It is the	 1. LVL beams must not b 2. Refer to manufact 	e cut or drilled urer's product info	rmation					t 7 Building	g, 2nd Floor	US/ 283	1		
ensure the compone	ustomer and/or the contractor to ent suitability of the intended fy the dimensions and loads.	regarding installation	n requirements,	multi-ply				(800) 622		n/us	910	-004-1KUS		
		 Damaged Beams mus 												
Lumber	ons, unless noted otherwise	 Design assumes top e Provide lateral support 										con	100 Shorts (pd)	

2	•	Client Proje		evelopment olson			Date: Input by:	10/24/20 Christine				Page 1 of
is 🛛	Design	Addre		holson				: Nicholso	•			
							Project #:					
Sliding I	Door Kerto-	-S LVL	1.750" X	9.250"	2-Ply -	PAS	SSED	Level: Level				
						3						
	2		1									
•	•	•	•		•	•		•			N.M.	1 1
				-100				-				9
	•	•	-	ALC: NO	•			•			/ V	
1 SPF	End Grain						2 SPF End	Grain				
			6'7"								1	3 1/2"
1			6'7"					1				
lember Inf	formation					Read	ctions UNI	PATTERN	IED Ib (Uplift)			
Туре:	Girder		pplication:	Floor		Brg	Direction	Live	Dead	Snow	Wind	Cor
Plies:	2 dition: Dru		esign Method: suilding Code:	ASD	5	1	Vertical	132		820	0	
Moisture Cond Deflection LL:	-		oad Sharing:	IBC/IRC 201 No	5	2	Vertical	132	1386	820	0	
Deflection TL:			eck:	Not Checked	b							
mportance:	Normal - II											
Temperature:	Temp <= 100°F											
						-	rings					
							aring Length		Cap. React D/L			Ld. Corr
						1-3 Enc	SPF 3.500"	Vert	21% 1386 / 82	20 2206	• L	D+S
nalysis Re	sults					Gra						
Analysis		ocation Allow	-	-	Case	2 - 3 Enc	SPF 3.500"	Vert	21% 1386 / 82	20 2206	i L	D+S
Moment		3'3 1/2" 14423		22%) D+S	L	Gra						
Unbraced		3'3 1/2" 1045'		30%) D+S	L							
Shear	1500 lb	1' 3/4" 7943	`	19%) D+S	L							
			(L/480) 0.139 ((L/360) 0.280 (2		L							
	. ,	331/2 0.204	(L/300) 0.200 (A	20 %) D+3	L	┥						
esign Not	es oport to prevent lateral r	movement and	rotation at the er	d bearings La	teral support	4						
may also b	e required at the interior plies using 2 rows of 10	r bearings by th	ne building code.	Ũ								
to exceed 6		or footonoro ro	wired for aposific	dloodo								
	t page of calculations for designed to be suppor			u iuaus.								
	nust be supported equa											
·	e laterally braced at end st be laterally braced at	•										
	iderness ratio based or	-	th.									
ID	Load Type	Locat		h Side	Dead 0.9		Live 1 Sno	w 1.15	Wind 1.6 Const	1.25 Co	omments	
1	Uniform			Тор	150 PLF		0 PLF	0 PLF	0 PLF	0 PLF Ex	terior Wall Lo	ad
2	Uniform			Тор	249 PLF		0 PLF 2	49 PLF	0 PLF	0 PLF A1		
3	Uniform			Тор	15 PLF	4	40 PLF	0 PLF	0 PLF	0 PLF 1'-	0" Floor Load	
	Self Weight				7 PLF							
							Г	Manufacture	ar Info	Comtect	n. Inc.	
Notes Calculated Structured	Designs is responsible only of the	chemicals Handling & In	stallation	6. For pon	flat roofs provide p ding	proper drain	nage to prevent	Metsä Wood		1001 S. Fayettev	Reilly Road. Suite #	639
structural adequacy of design criteria and	of this component based on the loadings shown. It is the	 LVL beams mus Refer to ma 	t not be cut or drilled anufacturer's product	information					Building, 2nd Floor	USA 28314		
responsibility of the o ensure the compon	customer and/or the contractor to ent suitability of the intended	regarding ins fastening detail	tallation requirements, s, beam strength values	multi-ply				(800) 622-58	50	910-864	TRUS	
Lumber	ify the dimensions and loads.	approvals 3. Damaged Bean 4. Design assume	is must not be used	rained				www.metsav	vood.com/us			
	ons, unless noted otherwise ted with fire retardant or corrosive	Provide lateral	s top edge is laterally rest support at bearing poir nent and rotation	ts to avoid	ia dooi <i>ne in th</i>	، م م الدور ا	2/2024				Omt	есн
		set: 22061001.1		í hi	is design is valid	i unui 11/	J/2024			CSD		