

Trenco 818 Soundside Rd Edenton, NC 27932

Re: DLO230446

ANDREW DAVIS RESIDENCE

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Truss Builders, Inc..

Pages or sheets covered by this seal: I58463479 thru I58463482

My license renewal date for the state of North Carolina is December 31, 2023.

North Carolina COA: C-0844



May 19,2023

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job Truss Truss Type Qty Ply ANDREW DAVIS RESIDENCE 158463479 DLO230446 CT1 Roof Special 23 1 Job Reference (optional) 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri May 19 12:22:26 2023 Page 1

Truss Builders, Inc., Morrisville, NC - 27560.

9-2-8

9-2-8

-0₁10₆8

0-10-8

ID:URsBPXDbpmjfC_InBNvdl8yYRnx-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJČ?f 26-0-0 42-9-8 30-1-12 4-1-12 4-1-12 6-0-12 6-7-0 0-10-8

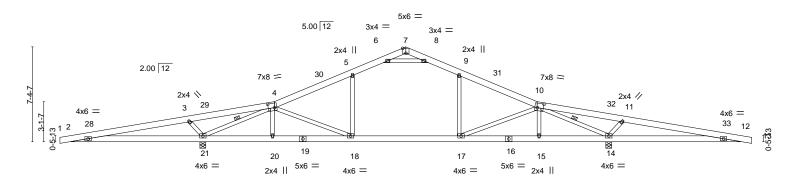
Structural wood sheathing directly applied or 1-4-12 oc purlins.

10-14, 4-21

Rigid ceiling directly applied or 4-6-7 oc bracing.

1 Row at midpt

Scale = 1:89.5



| <u> </u> | 10-2-12 | 15-9-8 | 21-10-4 | 30-1-12 | 36-2-8 | 41-9-4 | 52-0-0 |
|--|--|-------------------------------|--------------------|--------------------------------------|--------------------------------|---|--|
| Plate Offsets (X,Y) | 10-2-12 4:0-5-12,0-4-4], [7:0-3 | 5-6-12 -0,Edge], [10:0-5-1 | 6-0-12 2,0-4-4] | 8-3-8 | 6-0-12 | 5-6-12 | 10-2-12 |
| LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0 | SPACING Plate Grip Lumber D Rep Stres Code IBC | DOL 1.15 OL 1.15 | BC 0 | DEFL. 0.98 Vert(LL) 0.56 Vert(CT) MR | -0.25 18-20 > -0.35 18-20 > | defl L/d 999 240 999 180 n/a n/a | PLATES GRIP MT20 244/190 Weight: 320 lb FT = 6% |

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x6 SP No.2 **BOT CHORD** 2x6 SP No.2

WEBS 2x4 SP No.3 REACTIONS. (size) 14=0-5-8, 21=0-5-8

Max Horz 21=92(LC 16) Max Uplift 14=-285(LC 9), 21=-285(LC 8) Max Grav 14=2133(LC 2), 21=2133(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-584/2073, 3-4=-670/2531, 4-5=-1304/43, 5-6=-1116/73, 6-7=0/524, 7-8=0/524,

15-9-8

6-7-0

21-10-4

6-0-12

8-9=-1116/73, 9-10=-1304/42, 10-11=-671/2531, 11-12=-584/2073 **BOT CHORD** 2-21=-1985/601, 20-21=-286/987, 18-20=-287/985, 17-18=0/1123, 15-17=-287/985,

14-15=-286/987, 12-14=-1985/601

WEBS $9-17 = -47/254,\ 10-17 = -279/671,\ 10-14 = -3049/246,\ 11-14 = -780/202,\ 5-18 = -47/254,$

4-18=-279/671, 4-21=-3049/245, 3-21=-780/202, 6-8=-1665/37

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=15.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=285, 21=285.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



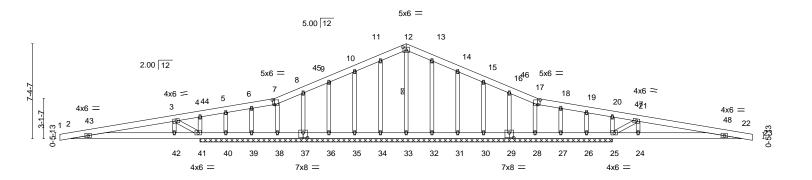
818 Soundside Road Edenton, NC 27932

Job Truss Truss Type Qty Ply ANDREW DAVIS RESIDENCE 158463480 DLO230446 **GABLE** 2 CT1GF Job Reference (optional) 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri May 19 12:22:30 2023 Page 1

Morrisville, NC - 27560. Truss Builders, Inc.,

> ID:URsBPXDbpmjfC_InBNvdl8yYRnx-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJČ?f 26-0-0 -0₁10₆8 0-10-8 10-2-8 10-2-8 15-9-8 15-9-8

> > Scale = 1:89.4



| | 1 10 | -U-U | | | | 32-0-0 | | | | |
|--|-------------------------------|--|--|---|---|--|-----------------------------|--------------------------|----------------------------------|-----------------------------|
| | 10 | -0-0 | | | | | | | | |
| Plate Offsets | (X,Y) [7:0-2 | -4,0-4-5], [12:0-3-0,0-2-1 | 2], [17:0-2-4,0- | -4-5], [29:0-4-0,0-4-8], [| 37:0-4-0,0-4-8] | | | | | |
| LOADING (p TCLL (roof) Snow (Pf) TCDL BCLL BCDL | 20.0 15.0 10.0 0.0 * | SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IBC2015/TF | 2-0-0 1.15 1.15 YES PI2014 | CSI. TC 0.86 BC 0.95 WB 0.40 Matrix-R | DEFL. Vert(LL) Vert(CT) Horz(CT) | in (loc) -0.04 22-23 -0.06 22-23 -0.06 25 | l/defl n/r n/r n/a | L/d 120 120 n/a | PLATES MT20 Weight: 345 lb | GRIP 244/190 FT = 6% |

BRACING-

TOP CHORD

BOT CHORD

WEBS

52-0-0

1 Row at midpt

Structural wood sheathing directly applied or 10-0-0 oc purlins.

12-33

Rigid ceiling directly applied or 2-2-0 oc bracing.

LUMBER-

TOP CHORD 2x6 SP No.1D *Except* 7-12,12-17: 2x6 SP No.2

BOT CHORD 2x6 SP No.2

WEBS 2x4 SP No.3 2x4 SP No.3 OTHERS

REACTIONS. All bearings 32-0-0.

(lb) -Max Horz 41=92(LC 12)

10-0-0

Max Uplift All uplift 100 lb or less at joint(s) 33, 34, 35, 36, 37, 32, 31, 30, 29 except 38=-471(LC 39), 39=-227(LC 8), 40=-1425(LC 43), 41=-729(LC 8), 28=-471(LC 42), 27=-228(LC 9), 26=-1425(LC 44), 25=-735(LC 9)

All reactions 250 lb or less at joint(s) 35, 36, 38, 31, 30, 28 except

33=1421(LC 2), 34=402(LC 3), 37=274(LC 43), 39=638(LC 39), 40=653(LC 8), 41=1995(LC 39), 32=402(LC 3), 29=274(LC 44), 27=638(LC 42), 26=659(LC 9),

25=1995(LC 42)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-548/1872, 3-4=-266/2014, 4-5=-226/1933, 5-6=-229/2008, 6-7=-199/1988,

7-8=-215/2116. 8-9=-175/2107. 9-10=-160/2110. 10-11=-142/2134. 11-12=-117/2044.

12-13=-117/2044, 13-14=-139/2134, 14-15=-156/2110, 15-16=-180/2107,

16-17=-210/2116, 17-18=-194/1988, 18-19=-225/2008, 19-20=-232/1933,

20-21=-262/2014. 21-22=-549/1872

BOT CHORD 2-42=-1792/557, 41-42=-1792/557, 40-41=-1958/266, 39-40=-1958/266, 38-39=-1958/266,

37-38=-1927/271, 36-37=-1927/271, 35-36=-1927/271, 34-35=-1927/271, 33-34=-1927/271, 32-33=-1927/271, 31-32=-1927/271, 30-31=-1927/271, 29-30=-1927/271, 28-29=-1927/271, 27-28=-1958/280, 26-27=-1958/280,

25-26=-1958/280, 24-25=-1792/557, 22-24=-1792/557

WEBS 12-33=-1381/89, 11-34=-354/39, 7-38=-109/457, 6-39=-393/163, 5-40=-303/688,

4-41=-1127/482, 3-42=-261/263, 13-32=-354/39, 17-28=-108/457, 18-27=-393/164

19-26=-306/688, 20-25=-1127/487, 21-24=-261/263, 3-41=-790/394, 21-25=-790/401

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=15.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

Continued on page 2





Design Valid to its 80 mly with win New Commercials. This design is based only upon parameters shown, and is for an individual orusining Component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932

| Job | Truss | Truss Type | Qty | Ply | ANDREW DAVIS RESIDENCE |
|------------|-------|------------|-----|-----|--------------------------|
| DI 0000440 | 07105 | OARLE | | l , | 158463480 |
| DLO230446 | CT1GE | GABLE | 2 | 1 | 1157 |
| | | | | | Job Reference (optional) |

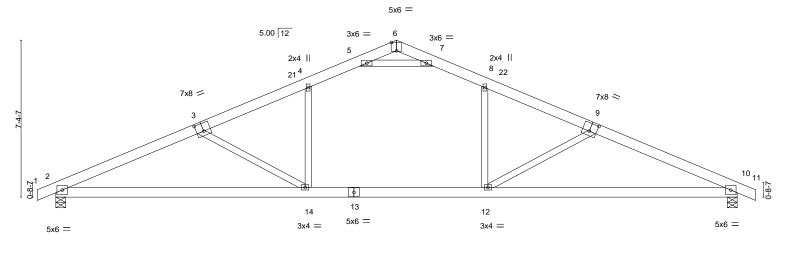
Morrisville, NC - 27560, Truss Builders, Inc.,

8.530 s Mar 9 2023 MiTek Industries, Inc. Fri May 19 12:22:30 2023 Page 2 ID:URsBPXDbpmjfC_InBNvdl8yYRnx-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.0 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 33, 34, 35, 36, 37, 32, 31, 30, 29 except (jt=lb) 38=471, 39=227, 40=1425, 41=729, 28=471, 27=228, 26=1425, 25=735.
- 12) Non Standard bearing condition. Review required.
- 13) This truss has large uplift reaction(s) from gravity load case(s). Proper connection is required to secure truss against upward movement at the bearings. Building designer must provide for uplift reactions indicated.

ANDREW DAVIS RESIDENCE Job Truss Truss Type Qty Ply 158463481 DLO230446 7 CT2 Common Job Reference (optional) 8.530 s Mar 9 2023 MiTek Industries, Inc. Fri May 19 12:22:31 2023 Page 1 Truss Builders, Inc., Morrisville, NC - 27560. ID:URsBPXDbpmjfC_lnBNvdl8yYRnx-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f -0-10-8 0-10-8 11-10-4 16-0-0 25-2-0 6-10-0 20-1-12 6-10-0 5-0-4 4-1-12 4-1-12 5-0-4 6-10-0 0-10-8

Scale = 1:54.1



| | 11-10-4 | 8- | -3-8 | 1 | 11- | 10-4 | |
|--|---|--|------|--|------------|------|-----------------------------------|
| Plate Offsets (X,Y) [3:0-4- | 0,0-4-8], [6:0-3-0,Edge], [9:0-4-0,0-4-8] | | | | | | |
| LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IBC2015/TPI2014 | CSI. TC 0.87 BC 0.63 WB 0.71 Matrix-MR | (/ | in (loc) l/defl -0.27 14-17 >999 -0.46 14-17 >840 0.06 10 n/a | 240 180 | _ | GRIP 244/190 FT = 6% |

TOP CHORD

BOT CHORD

20-1-12

LUMBER-BRACING-

TOP CHORD 2x6 SP No.2 2x6 SP No.2 **BOT CHORD WEBS** 2x4 SP No.3

> (size) 2=0-5-8, 10=0-5-8 Max Horz 2=92(LC 12)

Max Uplift 2=-50(LC 12), 10=-50(LC 13) Max Grav 2=1333(LC 2), 10=1333(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

11-10-4

TOP CHORD 2-3=-2513/106, 3-4=-2082/34, 4-5=-1759/81, 5-6=0/881, 6-7=0/882, 7-8=-1759/81,

8-9=-2082/34, 9-10=-2513/107

BOT CHORD 2-14=-120/2260, 12-14=0/1819, 10-12=-29/2260

WEBS 8-12=0/592, 9-12=-684/187, 4-14=0/592, 3-14=-684/186, 5-7=-2778/33

NOTES-

REACTIONS.

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=15.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.



32-0-0

Structural wood sheathing directly applied or 3-1-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932

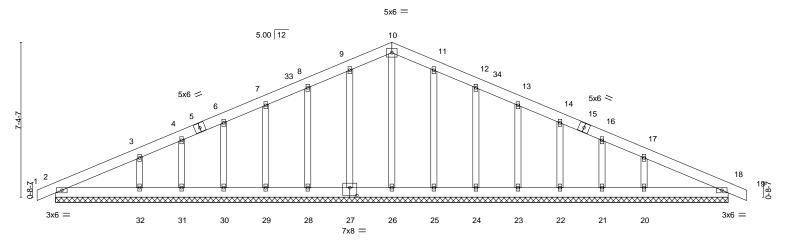
Job Truss Truss Type Qty Ply ANDREW DAVIS RESIDENCE 158463482 DLO230446 **GABLE** CT2GF 1 Job Reference (optional)

Truss Builders, Inc., Morrisville, NC - 27560.

-0-10-8 0-10-8 16-0-0 16-0-0

8.530 s Mar 9 2023 MiTek Industries, Inc. Fri May 19 12:22:33 2023 Page 1 ID:URsBPXDbpmjfC_InBNvdl8yYRnx-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 32-0-0 16-0-0 0-10-8

Scale = 1:54.8



32-0-0

| LOADING (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) //defl L/d MT20 244/190 | Plate Offsets (x, Y) [27:0-4-0,0-4-8] | | | | | | | | | | | | |
|--|---------------------------------------|-----------------------|---|---------------------|----------------|--------------|----------------------|------|----------|------------|------------|------|---------|
| BCDL 10.0 Code IBC2015/TPI2014 Matrix-R Weight: 234 lb FT = 6% | TCLL (roof) 2 Snow (Pf) 1 TCDL 1 BCLL | 15.0 10.0 0.0 * | Plate Grip DOL Lumber DOL Rep Stress Incr | 1.15 1.15 YES | TC BC WB | 0.05 0.10 | Vert(LL) Vert(CT) | 0.00 | 18 19 | n/r n/r | 120 120 | MT20 | 244/190 |

LUMBER-BRACING-

TOP CHORD 2x6 SP No.2 **BOT CHORD** 2x6 SP No.2 **OTHERS** 2x4 SP No.3

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 32-0-0. (lb) -Max Horz 2=92(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 18, 27, 28, 29, 30, 31, 32, 25, 24, 23, 22, 21, 20

Max Grav All reactions 250 lb or less at joint(s) 2, 18, 26, 27, 28, 29, 30, 31, 25, 24, 23, 22, 21 except

32=310(LC 30), 20=310(LC 31)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-10; Vult=115mph Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pf=15.0 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.0 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 18, 27, 28, 29, 30, 31, 32, 25, 24, 23, 22, 21, 20,





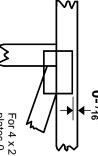
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



offsets are indicated. Center plate on joint unless x, y and fully embed teeth Apply plates to both sides of truss Dimensions are in ft-in-sixteenths



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

connector plates. required direction of slots in This symbol indicates the

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

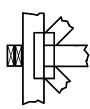
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



by text in the bracing section of the output. Use T or I bracing if indicated. ndicated by symbol shown and/or

BEARING



Min size shown is for crushing only number where bearings occur. reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

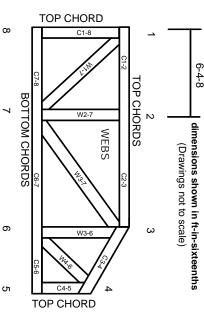
Industry Standards:

National Design Specification for Metal

DSB-89: ANSI/TPI1:

Building Component Safety Information. Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling Design Standard for Bracing. Plate Connected Wood Truss Construction.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

established by others. section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Ņ Truss bracing must be designed by an engineer. For bracing should be considered. may require bracing, or alternative Tor I wide truss spacing, individual lateral braces themselves
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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designer, erection supervisor, property owner and all other interested parties. Provide copies of this truss design to the building

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- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.

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- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication

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- 9 Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection. responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.