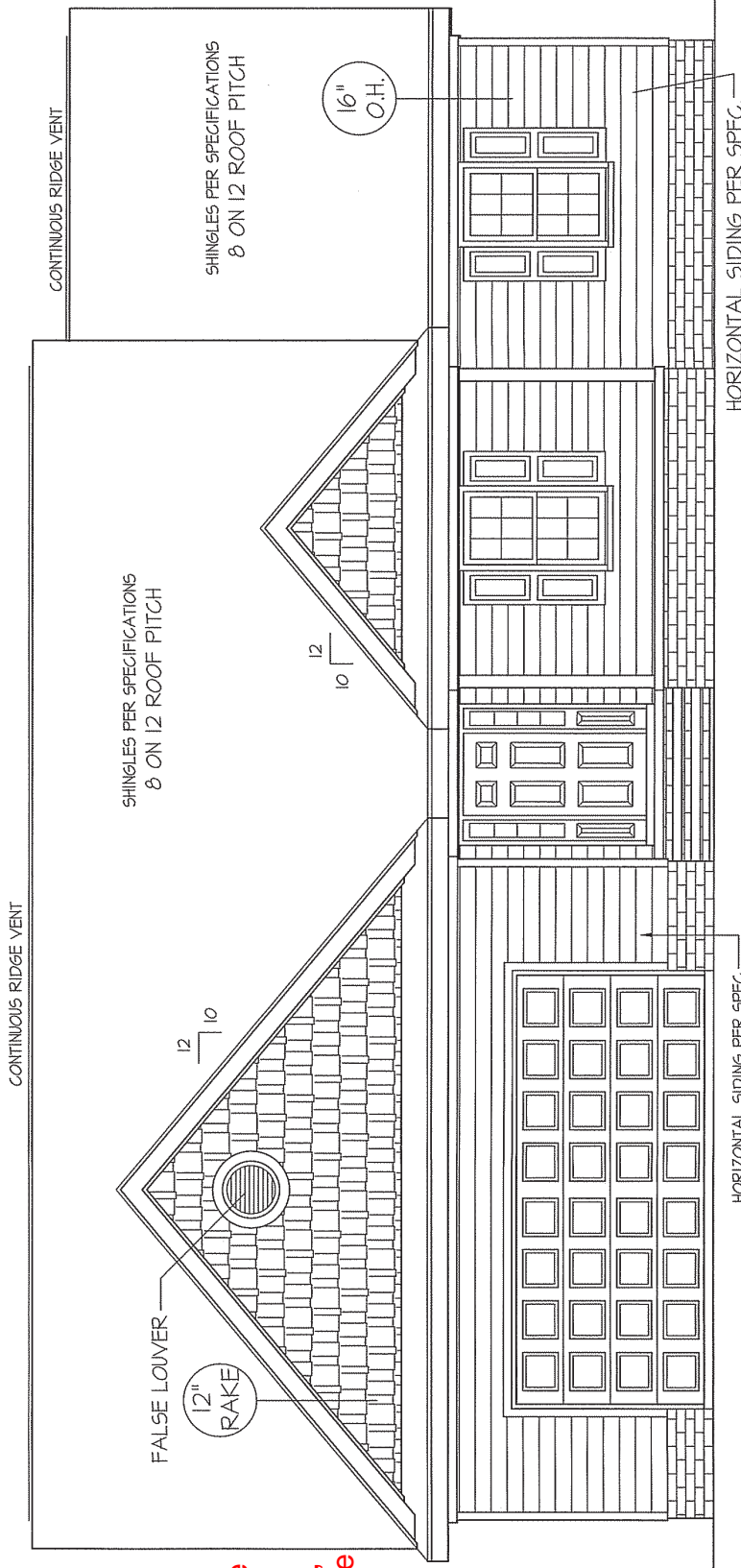


THIS PLAN IS DESIGNED TO MEET THE REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL CODE 2018 EDITION



While this is in a flood plane, the elevation shown on the site plan is at the edge of the flood plane. If flood vents/crawlspace were used, the water would not reach the vents.



FRONT ELEVATION

SCALE: 1/4" = 1'-0"

ROOF VENTILATION REQ. MTS.
 2510 ATTIC SQ. FT. / 300 = 8.37
 PROVIDED ON PLAN
 97 L.F. RIDGE VENT = 18.19
 124 L.F. SOFFIT VENT = 7.75
 TOTAL = 25.94 S.F. FREE NET AREA

WIND ZONES (PER TABLE R301.2(4))	
COUNTY	MPH
HARNETT	120
JOHNSTON	120
SAMPSON	130
WAKE	115

INSULATION and FENESTRATION REQUIREMENTS		
CLIMATE ZONE	ZONE-3	ZONE-4
FENESTRATION U-FACTOR	0.35	0.35
GLAZED FENESTRATION SHGC	0.30	0.30
MINIMUM CEILING R-VALUE	R-30	R-30
MINIMUM FLOOR R-VALUE	R-15, 19, 25	R-15, 19, 25
MIN. CRAWL SPACE WALL R-VALUE	R-19	R-14
MIN. SLAB R-VALUE	5/8	10/15
	0	R-10

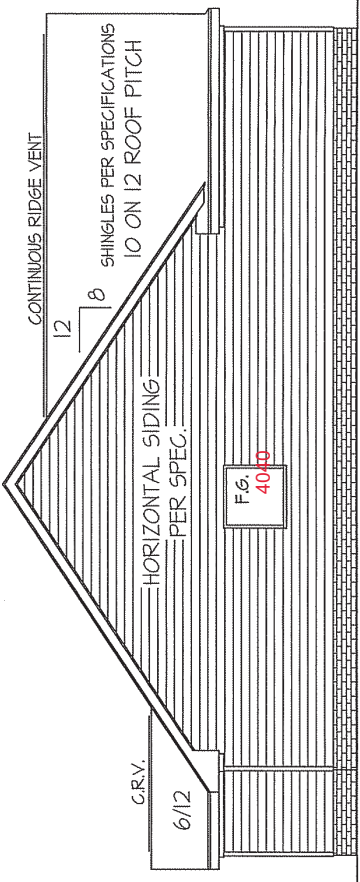
PROVIDE STEPS AS REQUIRED
 GRADE MAY VARY - BUILDER TO VERIFY

ALL EXTERIOR WALLS TO BE SHEATHED WITH CS-MSP (7/16" OSB) IN ACCORDANCE WITH SECTION R602.10.3 UNLESS OTHERWISE NOTED.



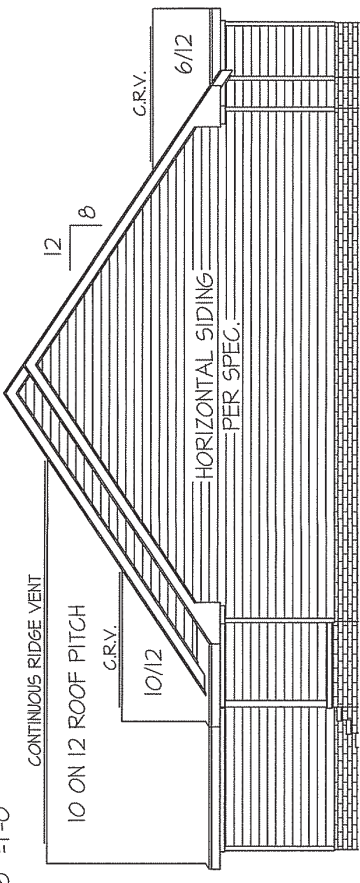
REAR ELEVATION

SCALE: 1/8" = 1'-0"



LEFT ELEVATION

SCALE: 1/8" = 1'-0"

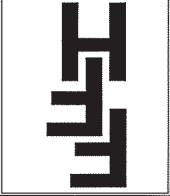


RIGHT ELEVATION

SCALE: 1/8" = 1'-0"

DATE:
 OCT. 18, 2019

FREEDOM FAMILY HOMES
 P.O. BOX 608
 DUNN, N.C. - 28335
 O: (910) 892-1231 FAX: (910) 892-5680
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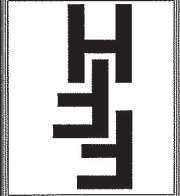


EXCLUSIVE PLAN FOR
 FREEDOM FAMILY HOMES
 WILSON ~ LEFT

SHEET NO.
1

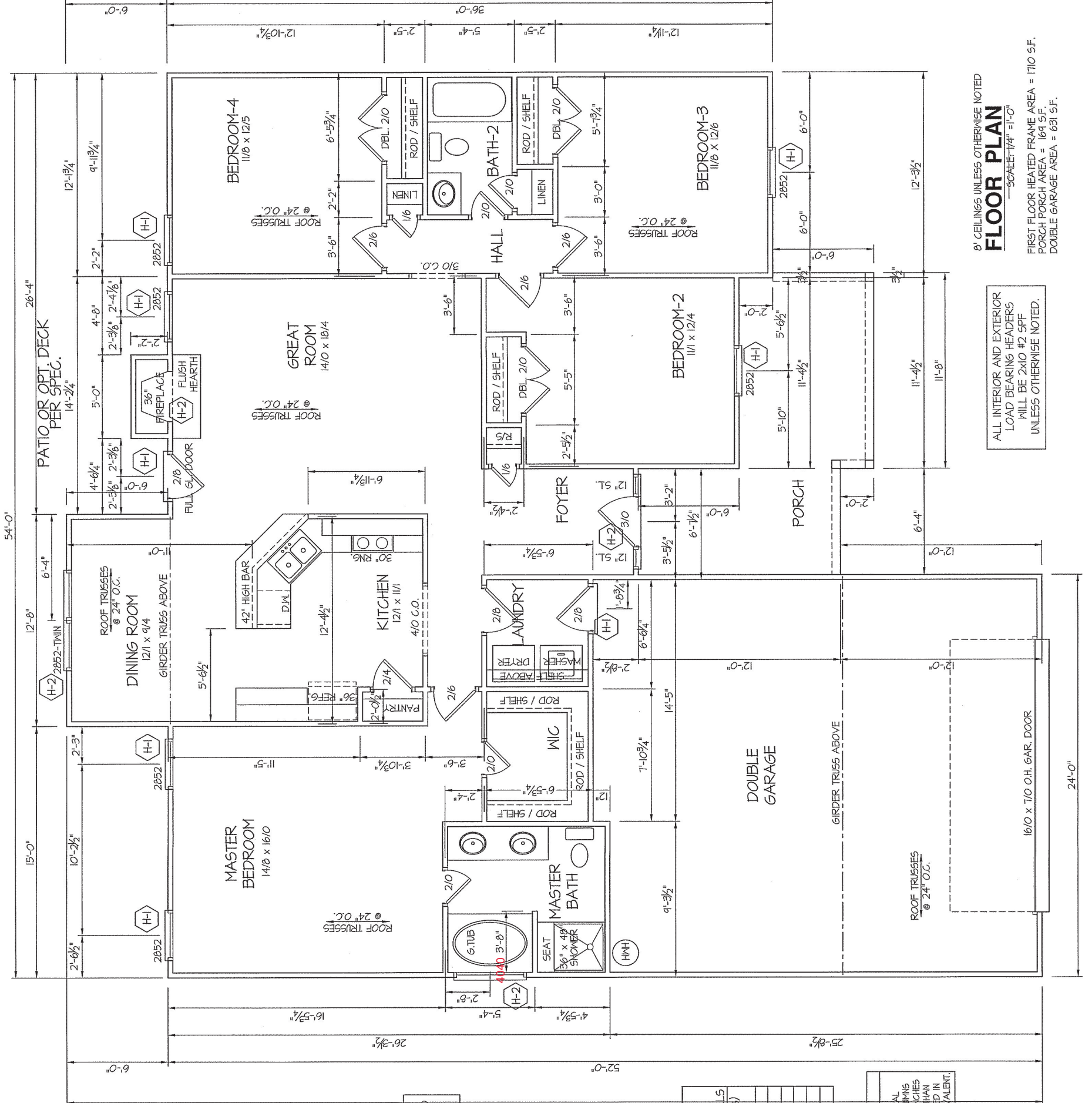
DATE:
OCT. 18, 2019

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PLAN:
**EXCLUSIVE PLAN FOR
FREEDOM FAMILY HOMES
WILSON ~ LEFT**

SHEET NO.
2



WIND ZONES (PER TABLE R301.2(4))

COUNTY	MPH
HARNETT	120
JOHNSTON	120
SAMPSON	130
WAKE	115

HEADER SCHEDULE

SYMBOL #	SIZE	JACKS
H-1	(2) 2x10	1
H-2	(2) 2x10	2
H-3	(2) 2x8	2
H-4	(2) 2x12	2
H-5	(2) 1.15 x 9.25 LVL	3

ALL EXTERIOR WALLS TO BE SHEATHED WITH CS-NSP (7/16" OSB) IN ACCORDANCE WITH SECTION R602.10.3 UNLESS OTHERWISE NOTED.

GIRDER AND HEADER SIZES AND JACK STUD REQUIREMENTS ON EXTERIOR AND INTERIOR LOAD BEARING WALLS ARE TO COINCIDE WITH TABLE EXT.-R602.1 (1) AND INT.-R602.1 (2).

TABLE R602.1.5
MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS
MAX. STUD SPACING (inches)
[per Table R602.3(5)]

HEADER SPAN (feet)	MINIMUM NUMBER OF STUDS
< 3'	24
4'	2
8'	3
12'	5
16'	6

SECTION R407-COLUMNS
R407.3 STRUCTURAL REQUIREMENTS
THE COLUMNS SHALL BE RESTRAINED TO PREVENT LATERAL DISPLACEMENT AT THE TOP AND BOTTOM END. WOOD COLUMNS SHALL BE NOT LESS IN NOMINAL SIZE THAN 4 INCHES BY 4 INCHES (102 mm BY 102 mm). STEEL COLUMNS SHALL BE NOT LESS THAN 3-INCH DIAMETER (76 mm) SCHEDULE 40 PIPE MANUFACTURED IN ACCORDANCE WITH ASTM A53 GRADE B OR APPROVED EQUIVALENT.

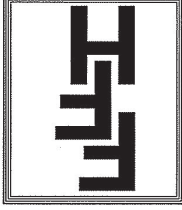
GARAGE DOOR WALL TO BE PORTAL FRAME CONSTRUCTION, COMPLYING SECTION R602.10.1 AND FIGURE-R602.10.1 METHOD PFG

8' CEILINGS UNLESS OTHERWISE NOTED
FLOOR PLAN
SCALE: 1/4" = 1'-0"
FIRST FLOOR HEATED FRAME AREA = 1710 SF.
PORCH PORCH AREA = 169 SF.
DOUBLE GARAGE AREA = 631 SF.

ALL INTERIOR AND EXTERIOR LOAD BEARING HEADERS WILL BE 2x10 #2 SFF UNLESS OTHERWISE NOTED.

DATE:
OCT. 18, 2019

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EXCLUSIVE PLAN FOR
FREEDOM FAMILY HOMES
WILSON ~ LEFT

PLAN:

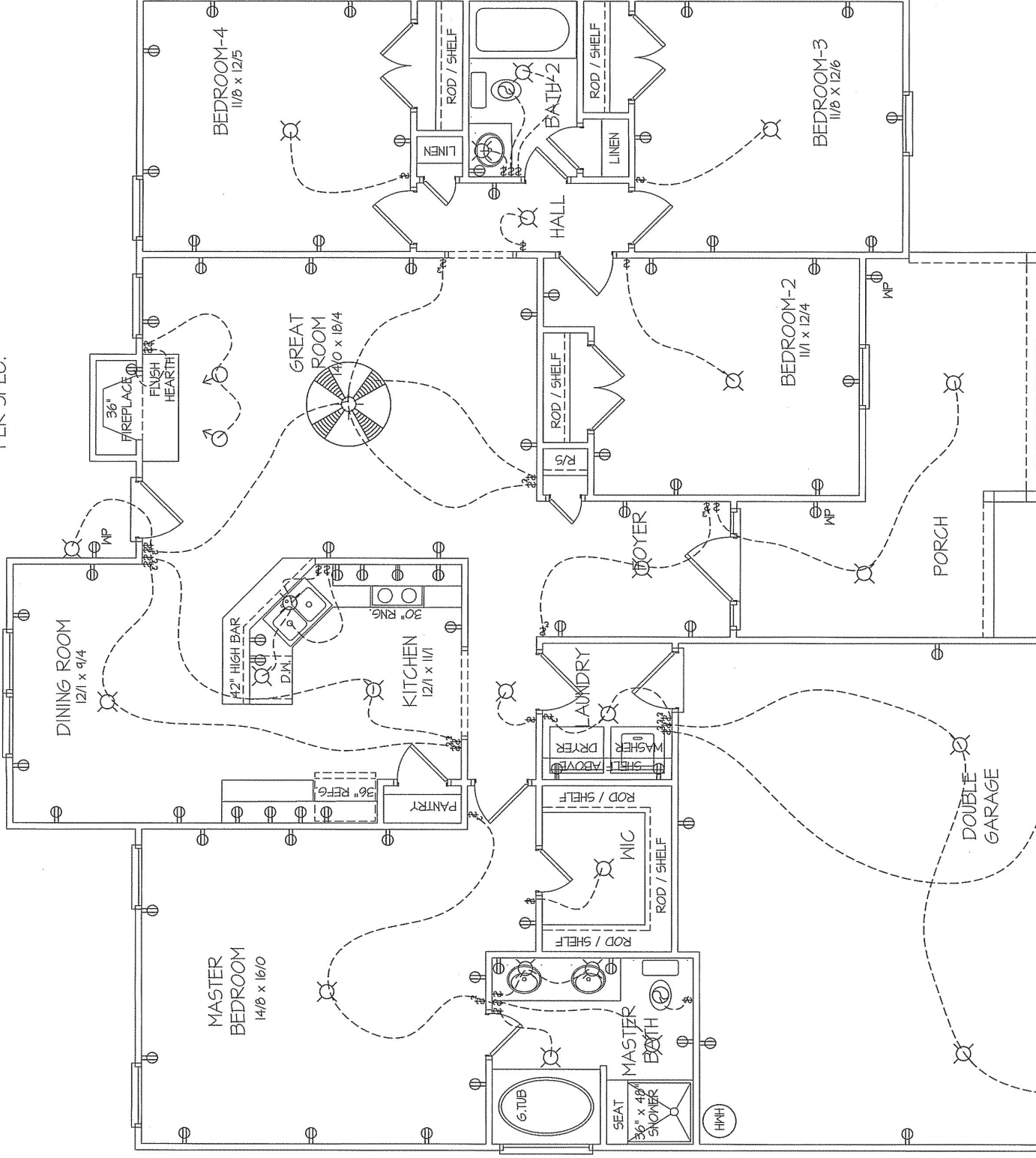
SHEET NO.

4

8' C.L.G. UNLESS OTHERWISE NOTE
**ELECTRICAL
FLOOR PLAN**
SCALE: 1/4" = 1'-0"

FIRST FLOOR HEATED FRAME AREA = 1110 S.F.
PORCH PORCH AREA = 169 S.F.
DOUBLE GARAGE AREA = 631 S.F.

PATIO OR OPT. DECK
PER SPEC.



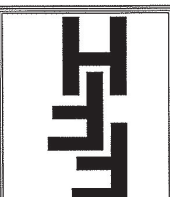
ELECTRICAL LEGEND

PROVIDE BURGLAR ALARMS AND FIRE DETECTORS AS SHOWN. PROVIDE CENTRAL VACUUM SYSTEMS AS PER MANUFACTURER'S INSTRUCTIONS. ALL PANELS TO BE CONTROLLED BY 240V SPLIT AND DIRECTIONAL SWITCHES.

⊕ SURF. MOUNTED LIGHT	⊕ TYPICAL WALL RECEPT.	⊕ TYPICAL SWITCH
○ RECESSED LIGHT	⊕ TOP 1/2 HOT W/SWITCH	⊕ 3-WAY SWITCH
⊕ EYEBALL LIGHT	⊕ CEILING RECEPTACLE	⊕ 4-WAY SWITCH
⊕ FAN/LIGHT COMB.	⊕ FLOOR RECEPTACLE	⊕ DIMMER SWITCH
⊕ FLUORESCENT TUBE	⊕ WATERPROOF RECEPT.	⊕ ELEC. PANEL BOX
⊕ FLUOR. LIGHT FIXTURE	⊕ GROUND FAULT	⊕ T.V. CABLE RECEPT.
⊕ EXHAUST FAN	⊕ DISPOSAL UNIT	⊕ TELEPHONE JACK
⊕ C.L.G. FAN	⊕ 220 VOLT RECEPTACLE	⊕ COMPUTER JACK
⊕ FLOOD LIGHT		

DATE:
OCT. 18, 2019

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0: (910) 892-1231 FAX: (910) 892-5680

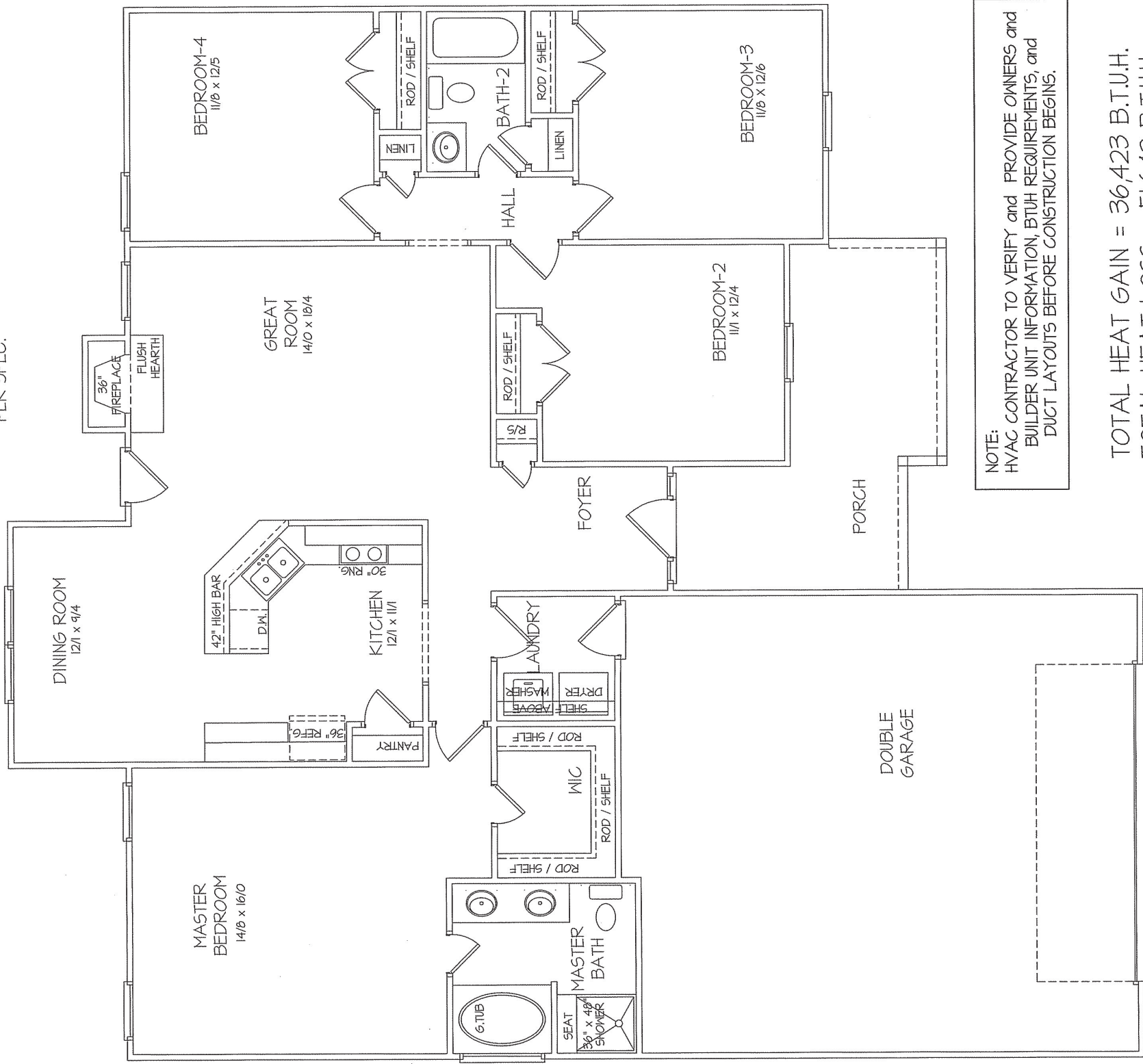


EXCLUSIVE PLAN FOR
FREEDOM FAMILY HOMES
WILSON~LEFT

SHEET NO.
5

8' CEILINGS UNLESS OTHERWISE NOTED
HVAC FLOOR PLAN
SCALE: 1/4" = 1'-0"
FIRST FLOOR HEATED FRAME AREA = 1710 S.F.
PORCH PORCH AREA = 169 S.F.
DOUBLE GARAGE AREA = 631 S.F.

PATIO OR OPT. DECK
PER SPEC.

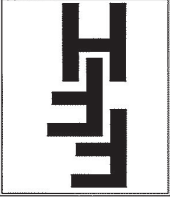


NOTE:
HVAC CONTRACTOR TO VERIFY and PROVIDE OWNERS and
BUILDER UNIT INFORMATION, BTUH REQUIREMENTS, and
DUCT LAYOUTS BEFORE CONSTRUCTION BEGINS.

TOTAL HEAT GAIN = 36,423 B.T.U.H.
TOTAL HEAT LOSS = 51,642 B.T.U.H.

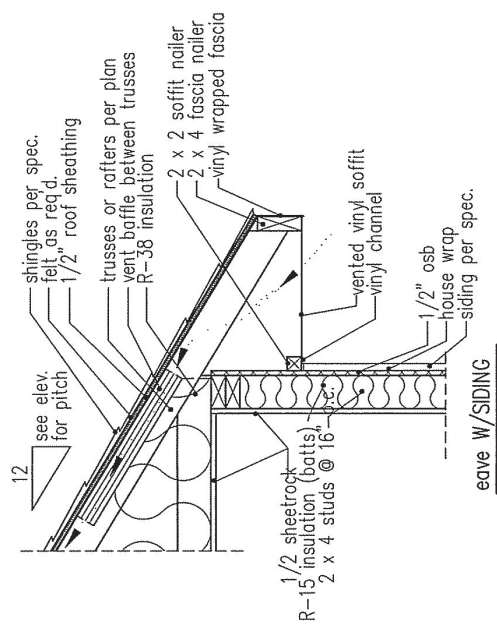
DATE:
OCT. 18, 2019

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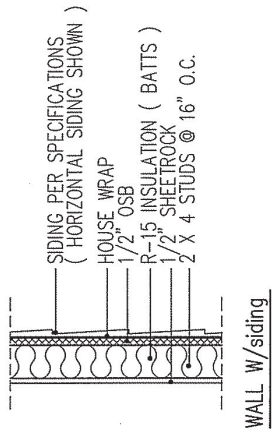
EXCLUSIVE PLAN FOR
FREEDOM FAMILY HOMES
PLAN:

SHEET NO.
6

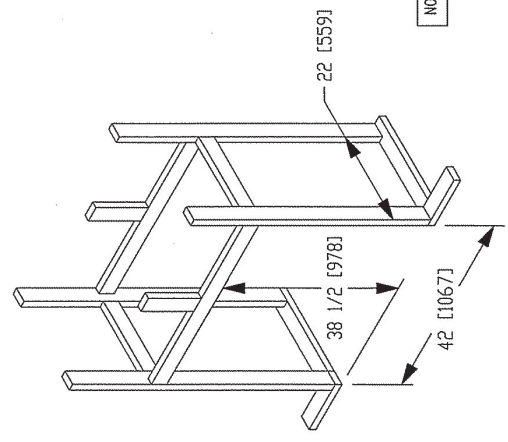


NOTE: OVERHANG DISTANCE NOTED ON ELEVATION SHEET IS ALWAYS MEASURED FROM FRAME LINE

standard eave details



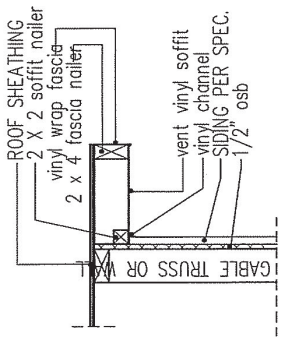
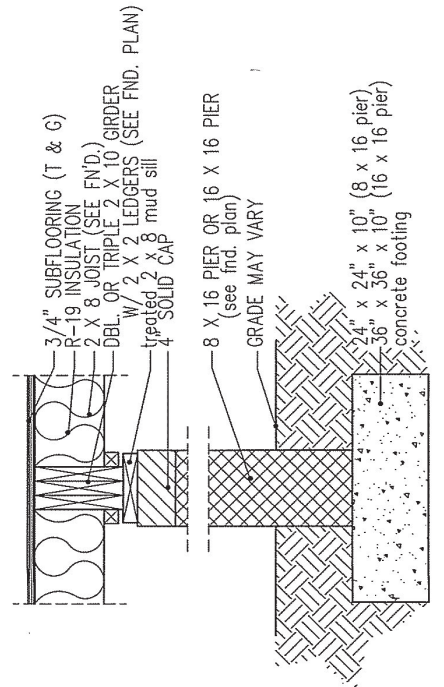
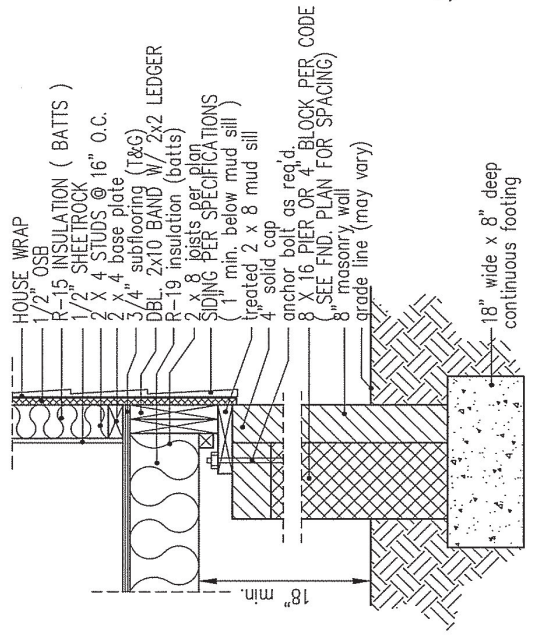
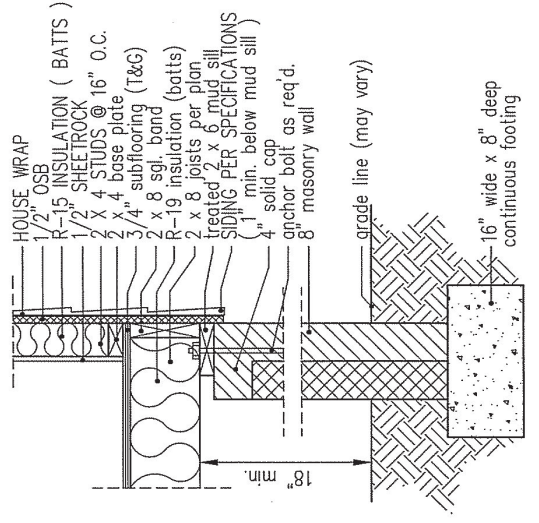
intermediate wall details



DIMENSIONS IN () ARE MM

Model 600-TR	Height		Front Width		Back Width		Depth		Glass Size	JBU Input
	Actual	Framing	Actual	Framing	Actual	Framing	Actual	Framing		
Inches	38	38-1/2	41	42	28-1/2	42	21-1/2	22		

Reference dimensions only. We recommend measuring individual units at installation.



NOTE: OVERHANG DISTANCE NOTED ON ELEVATION SHEET IS ALWAYS MEASURED FROM FRAME LINE

TABLE R602.15
MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (feet)	MAX. STUD SPACING (inches) [per Table R602.3(5)]	24
< 3'	1	1
4'	2	2
8'	3	3
12'	5	5
16'	6	6

GIRDER AND HEADER SIZES AND JACK STUD REQUIREMENTS ON EXTERIOR AND INTERIOR LOAD BEARING WALLS ARE TO COINCIDE WITH TABLE EXT.-R602.1 (1) AND INT.-R602.1 (2).



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

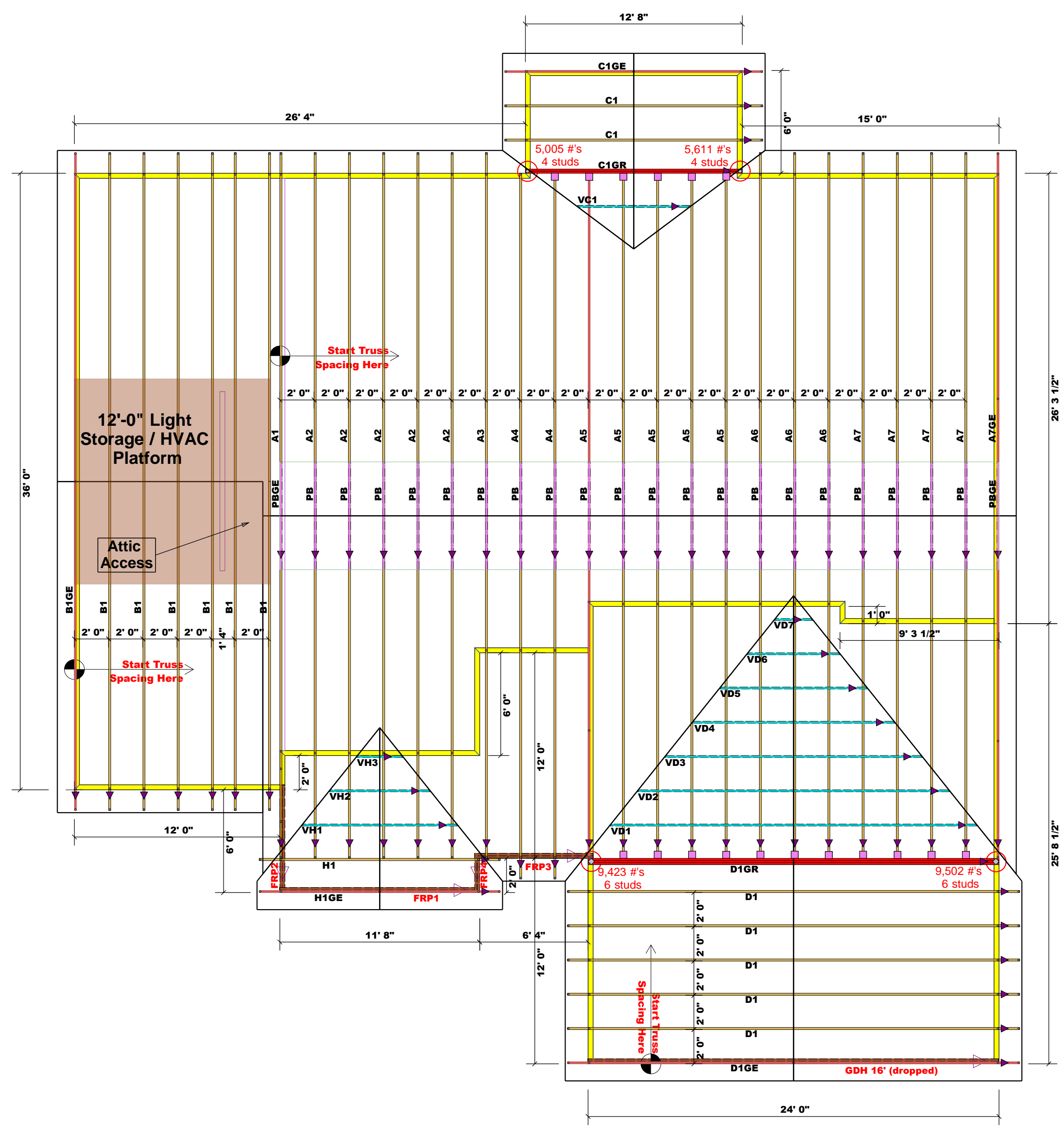
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature Lenny Norris
Lenny Norris

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION (UP TO)	REQ. D. STUDS FOR (1) PLY HEADER	END REACTION (UP TO)	REQ. D. STUDS FOR (1) PLY HEADER	END REACTION (UP TO)	REQ. D. STUDS FOR (1) PLY HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				



Estimation			
Name	Selection	Formula	Calculation
Roof Area	1st Floor	Roof Area	3372.22
Roof Decking	1st Floor	Roof Decking	116

BEAM LEGEND					
PlotID	Length	Product	Plies	Net Qty	Fab Type
GDH 16' (dropped)	24' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
FRP1	12' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
FRP2	7' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
FRP3	7' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
FRP4	3' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF

Truss Placement Plan

SCALE: 3/16" = 1'-0"

■ HUS28 USP 17 16d/3-1/2 16d/3-1/2"

▲ = Denotes Left End of Truss
(Reference Engineered Truss Drawing)

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

○ -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

BUILDER	CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
Freedom Constructors, Inc.	Harnett Co.	Lot 17 The Cape	Wilson	Roof	Lenny Norris	Lenny Norris
JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #		
				J0523-2061		

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
 Fayetteville, N.C. 28309
 Phone: (910) 864-8787
 Fax: (910) 864-4444

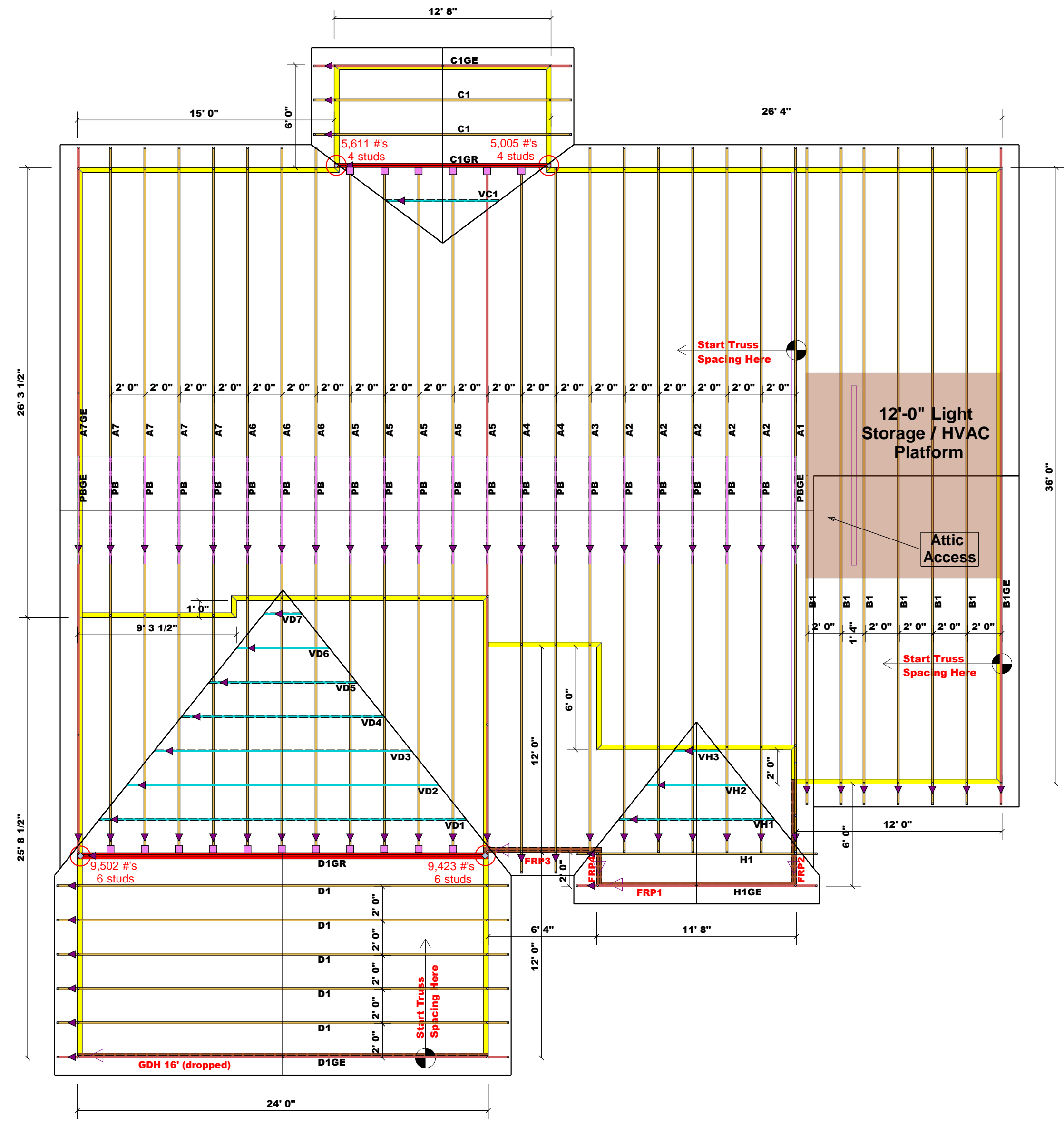
Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature Lenny Norris
Lenny Norris

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))
 NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION (UP TO)	REQ. D. STUDS FOR (1) PLY HEADER	END REACTION (UP TO)	REQ. D. STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ. D. STUDS FOR (3) PLY HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				



Estimation			
Name	Selection	Formula	Calculation
Roof Area	1st Floor	Roof Area	3372.22
Roof Decking	1st Floor	Roof Decking	116

BEAM LEGEND					
PlotID	Length	Product	Plies	Net Qty	Fab Type
GDH 16' (dropped)	24' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
FRP1	12' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
FRP2	7' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
FRP3	7' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
FRP4	3' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF

Truss Placement Plan
SCALE: 3/16" = 1'-0"

■ HUS28 ■ USP ■ 17 ■ 16d/3-1/2 ■ 16d/3-1/2"

▲ = Denotes Left End of Truss
 (Reference Engineered Truss Drawing)

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

○ -- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

BUILDER	Freedom Constructors, Inc.	CITY / CO.	Harnett Co.
JOB NAME	Lot 17 The Cape	ADDRESS	
PLAN	Wilson	MODEL	Roof
SEAL DATE	Seal Date	DATE REV.	/ /
QUOTE #	Quote #	DRAWN BY	Lenny Norris
JOB #	J0523-2061	SALES REP.	Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

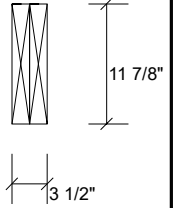
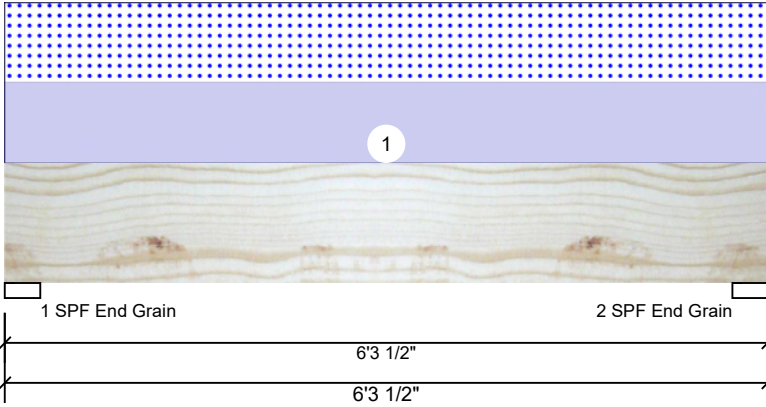


Client: FREEDOM
 Project:
 Address:

Date: 5/1/2023
 Input by: Lenny Norris
 Job Name: WILSON
 Project #:

BP3 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	526	497	0	0
2	Vertical	0	526	497	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	10%	526 / 497	1023	L	D+S
2 - SPF End Grain	3.500"	Vert	10%	526 / 497	1023	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1383 ft-lb	3'1 3/4"	22897 ft-lb	0.060 (6%)	D+S	L
Unbraced	1383 ft-lb	3'1 3/4"	14335 ft-lb	0.097 (10%)	D+S	L
Shear	616 lb	5' 1/8"	10197 lb	0.060 (6%)	D+S	L
LL Defl inch (L/11519)	0.006	3'1 3/4"	0.146 (L/480)	0.042 (4%)	S	L
TL Defl inch (L/5596)	0.013	3'1 3/4"	0.292 (L/240)	0.043 (4%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	158 PLF	0 PLF	158 PLF	0 PLF	0 PLF	G1 TRUSS
	Self Weight				9 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

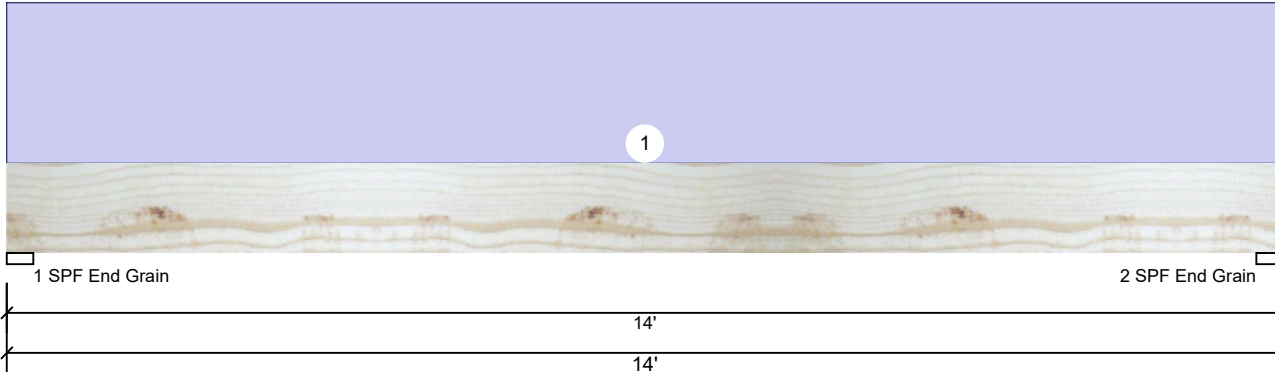
Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



BP2 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	765	0	0	0
2	Vertical	0	765	0	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	7%	765 / 0	765	Uniform	D
2 - SPF End Grain	3.500"	Vert	7%	765 / 0	765	Uniform	D

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2504 ft-lb	7'	17919 ft-lb	0.140 (14%)	D	Uniform
Unbraced	2504 ft-lb	7'	7212 ft-lb	0.347 (35%)	D	Uniform
Shear	634 lb	1'3 3/8"	7980 lb	0.079 (8%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.092 (L/1775)	7' 1/16"	0.677 (L/240)	0.135 (14%)	D	Uniform

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	100 PLF	0 PLF	0 PLF	0 PLF	0 PLF	GABLE END
	Self Weight				9 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

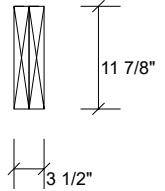
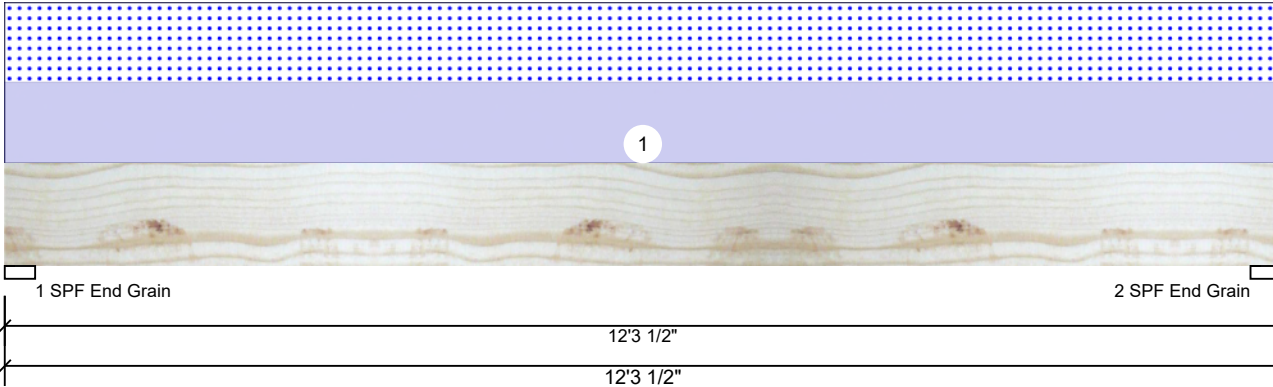
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 910-864-TRUS



BP1 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	813	756	0	0
2	Vertical	0	813	756	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	15%	813 / 756	1569	L	D+S
2 - SPF End Grain	3.500"	Vert	15%	813 / 756	1569	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4468 ft-lb	6'1 3/4"	22897 ft-lb	0.195 (20%)	D+S	L
Unbraced	4468 ft-lb	6'1 3/4"	8162 ft-lb	0.547 (55%)	D+S	L
Shear	1251 lb	11' 1/8"	10197 lb	0.123 (12%)	D+S	L
LL Defl inch	0.062 (L/2308)	6'1 3/4"	0.296 (L/480)	0.208 (21%)	S	L
TL Defl inch	0.128 (L/1112)	6'1 3/4"	0.592 (L/240)	0.216 (22%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	123 PLF	0 PLF	123 PLF	0 PLF	0 PLF	G1,C1,C2 TRUSSES
	Self Weight				9 PLF					

Notes

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Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

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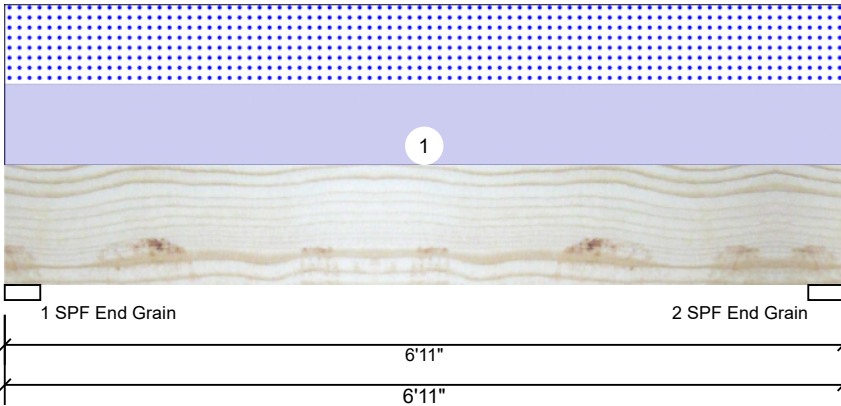


Client: FREEDOM
 Project:
 Address:

Date: 5/1/2023
 Input by: Lenny Norris
 Job Name: WILSON
 Project #:

FRP3 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1484	1453	0	0
2	Vertical	0	1484	1453	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	29%	1484 / 1453	2937	L	D+S
2 - SPF End Grain	3.500"	Vert	29%	1484 / 1453	2937	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4428 ft-lb	3'5 1/2"	22897 ft-lb	0.193 (19%)	D+S	L
Unbraced	4428 ft-lb	3'5 1/2"	13146 ft-lb	0.337 (34%)	D+S	L
Shear	1858 lb	1'3 3/8"	10197 lb	0.182 (18%)	D+S	L
LL Defl inch	0.023 (L/3384)	3'5 1/2"	0.161 (L/480)	0.142 (14%)	S	L
TL Defl inch	0.046 (L/1674)	3'5 1/2"	0.323 (L/240)	0.143 (14%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	420 PLF	0 PLF	420 PLF	0 PLF	0 PLF	A4,A3 TRUSS
	Self Weight				9 PLF					

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

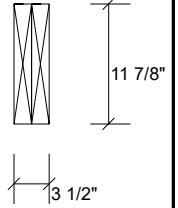
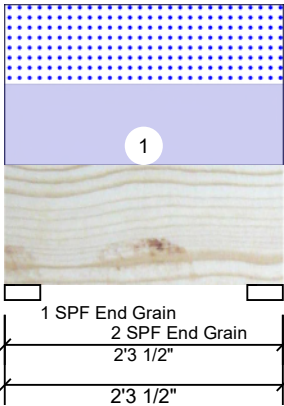
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FRP4 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	162	151	0	0
2	Vertical	0	162	151	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	3%	162 / 151	313	L	D+S
2 - SPF End Grain	3.500"	Vert	3%	162 / 151	313	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	115 ft-lb	1'1 3/4"	22897 ft-lb	0.005 (1%)	D+S	L
Unbraced	115 ft-lb	1'1 3/4"	22013 ft-lb	0.005 (1%)	D+S	L
Shear	28 lb	1'3 3/8"	10197 lb	0.003 (0%)	D+S	L
LL Defl inch	0.000 (L/116983)	1'1 13/16"	0.046 (L/480)	0.004 (0%)	S	L
TL Defl inch	0.000 (L/56514)	1'1 13/16"	0.092 (L/240)	0.004 (0%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	132 PLF	0 PLF	132 PLF	0 PLF	0 PLF	H1,H1GE TRUSS
	Self Weight				9 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

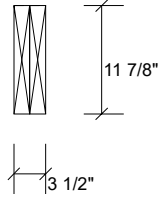
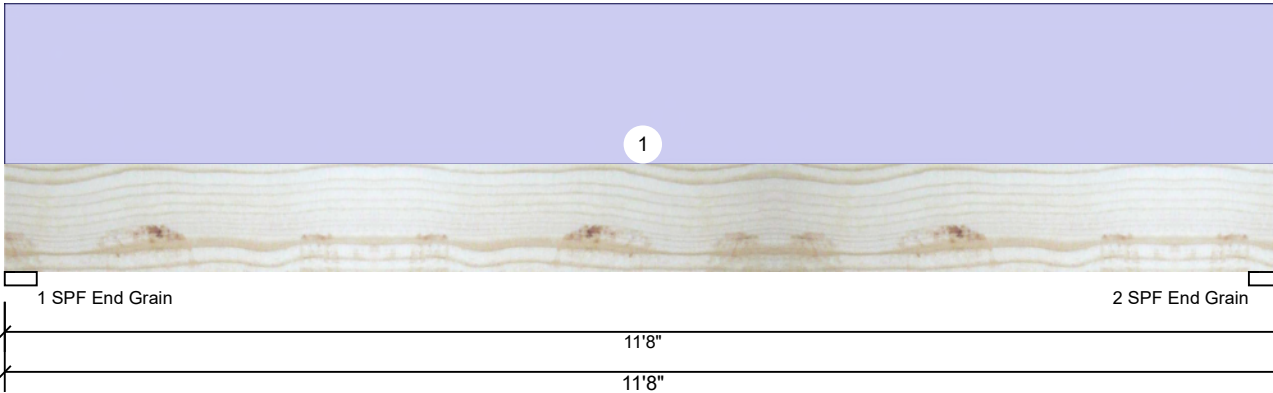
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FRP1 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	637	0	0	0
2	Vertical	0	637	0	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	6%	637 / 0	637	Uniform	D
2 - SPF End Grain	3.500"	Vert	6%	637 / 0	637	Uniform	D

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1715 ft-lb	5'10"	17919 ft-lb	0.096 (10%)	D	Uniform
Unbraced	1715 ft-lb	5'10"	8417 ft-lb	0.204 (20%)	D	Uniform
Shear	506 lb	10'4 5/8"	7980 lb	0.063 (6%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.044 (L/3025)	5'10"	0.560 (L/240)	0.079 (8%)	D	Uniform

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	100 PLF	0 PLF	0 PLF	0 PLF	0 PLF	GABLE END
	Self Weight				9 PLF					

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

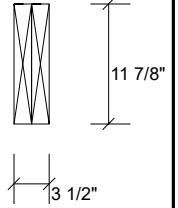
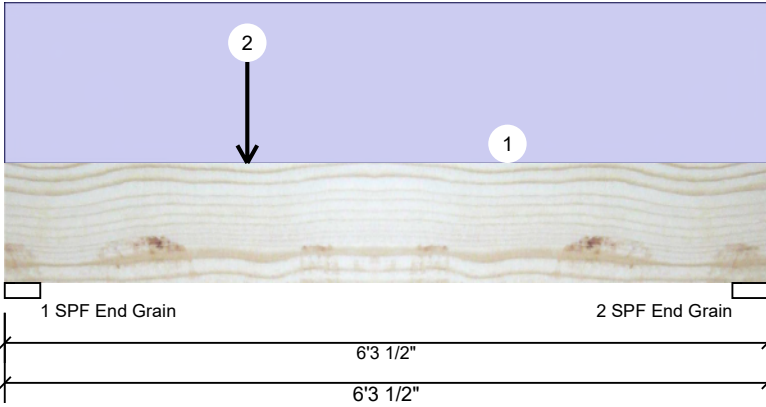
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FRP2 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal - II		
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	527	184	0	0
2	Vertical	0	424	80	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	7%	527 / 184	711	L	D+S
2 - SPF End Grain	3.500"	Vert	5%	424 / 80	504	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1044 ft-lb	2'	22897 ft-lb	0.046 (5%)	D+S	L
Unbraced	1044 ft-lb	2'	14335 ft-lb	0.073 (7%)	D+S	L
Shear	581 lb	1'3 3/8"	10197 lb	0.057 (6%)	D+S	L
LL Defl inch (L/30093)	0.002	2'4 13/16"	0.146 (L/480)	0.016 (2%)	S	L
TL Defl inch (L/8041)	0.009	2'9 9/16"	0.292 (L/240)	0.030 (3%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	100 PLF	0 PLF	0 PLF	0 PLF	0 PLF	GABLE END
2	Point	2-0-0		Top	264 lb	0 lb	264 lb	0 lb	0 lb	H1 TRUSS
	Bearing Length	0-3-8								
	Self Weight				9 PLF					

Notes

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Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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