

RE: J0424-2449 Lot 16 Duncans Creek Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J0424-2449 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 20 individual, dated Truss Design Drawings and 0 Additional Drawings.

No. 1 2 3 4 5 6 7 8 9 10 11 2 3 14 15 16 17 18	Seal# I65230014 I65230015 I65230016 I65230017 I65230019 I65230020 I65230021 I65230022 I65230023 I65230024 I65230025 I65230026 I65230027 I65230028 I65230029 I65230030 I65230031	Truss Name A01GE A02 A03 A03A A03B A04 A05GE B01GE B02-GR C01GE C02 M01GE M02 M03 M04GE VB1 VB2 VB3	Date 4/30/2024 4/30/2024 4/30/2024 4/30/2024 4/30/2024 4/30/2024 4/30/2024 4/30/2024 4/30/2024 4/30/2024 4/30/2024 4/30/2024 4/30/2024 4/30/2024 4/30/2024
18	165230031	VB3	4/30/2024
19	165230032	VB4	4/30/2024
20	165230033	VB5	4/30/2024

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

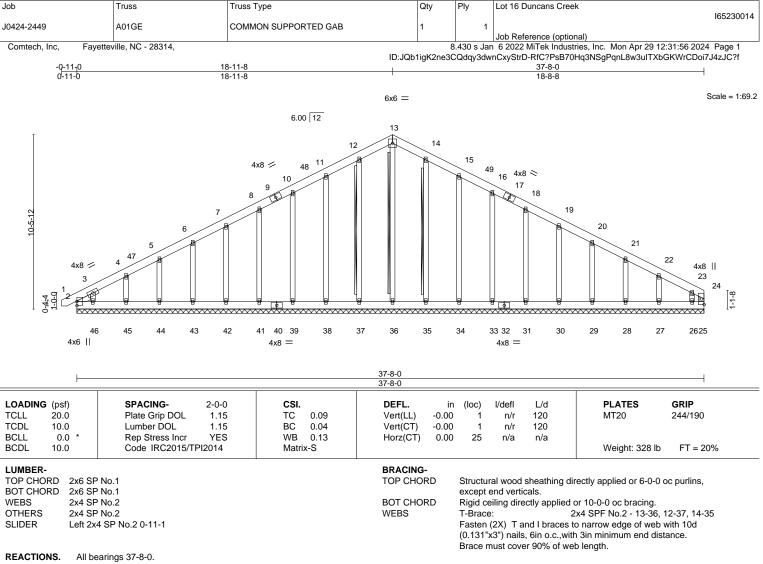
My license renewal date for the state of North Carolina is December 31, 2024

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



(lb) - Max Horz 2=216(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 25, 2, 37, 38, 39, 41, 42, 43, 44, 45, 35, 34, 33, 31, 30, 29, 28, 27 except 46=-175(LC 12), 26=-234(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 36, 37, 38, 39, 41, 42, 43, 44, 45, 46, 35, 34, 33, 31, 30, 29, 28, 27, 26 except 25=255(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-340/113, 10-11=-101/306, 11-12=-124/370, 12-13=-137/406, 13-14=-137/408, 14-15=-124/372, 15-16=-101/309, 16-18=-81/250

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-9-2 to 3-7-11, Exterior(2) 3-7-11 to 18-11-8, Corner(3) 18-11-8 to 23-4-5, Exterior(2) 23-4-5 to 37-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

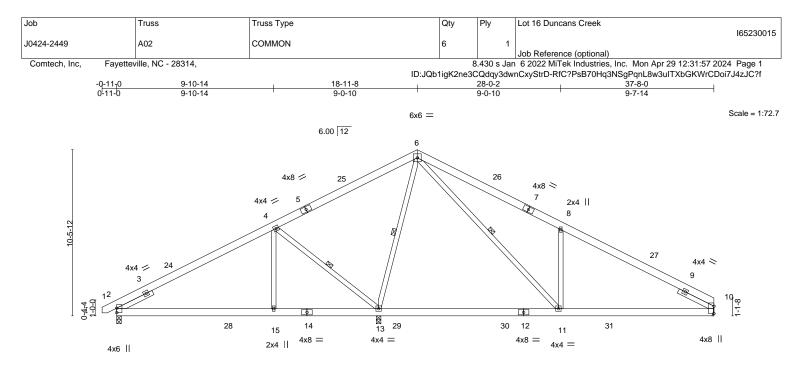
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 2, 37, 38, 39, 41, 42, 43, 44, 45, 35, 34, 33, 31, 30, 29, 28, 27 except (jt=lb) 46=175, 26=234.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

A MiTek A 818 Soundside Road Edenton, NC 27932



	<u>9-10-14</u> 9-10-14	16-6-4 6-7-6	28-0-2 11-5-14		37-8-0 9-7-14	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.27 BC 0.49 WB 0.45 Matrix-AS	DEFL. in (loc) Vert(LL) -0.24 11-13 Vert(CT) -0.33 11-13 Horz(CT) 0.02 2 Wind(LL) 0.03 11-22	l/defl L/d >999 360 >773 240 n/a n/a >999 240	PLATES MT20 Weight: 258 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD	2x6 SP No.1 *Except*
	1-5: 2x6 SP 2400F 2.0E
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2
SLIDER	Left 2x4 SP No.2 2-6-0, Right 2x4 SP No.2 2-6-0

BRACING-TOP CHORD BOT CHORD

WEBS

Structural wood sheathing directly applied. Rigid ceiling directly applied. 1 Row at midpt 4-13, 6-13, 6-11

REACTIONS. (size) 2=0-3-8, 13=0-3-8, 10=Mechanical Max Horz 2=131(LC 9) Max Uplift 2=-90(LC 12), 13=-20(LC 12), 10=-106(LC 13) Max Grav 2=692(LC 23), 13=1826(LC 2), 10=836(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-670/296, 4-6=-108/289, 6-8=-1202/546, 8-10=-1101/342

BOT CHORD 2-15=-148/579, 13-15=-148/579, 10-11=-169/963

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WEBS
        4-15=0/308, 4-13=-793/281, 6-13=-955/161, 6-11=-307/1303, 8-11=-586/356
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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-2 to 3-7-11, Interior(1) 3-7-11 to 18-11-8, Exterior(2) 18-11-8 to 23-4-5, Interior(1) 23-4-5 to 37-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

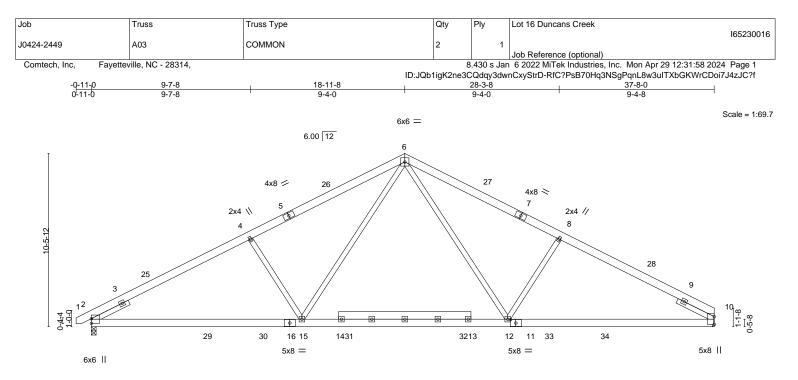
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 13 except (it=lb) 10=106

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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	12-8-13	25-2	2-3 37-8-0)
	12-8-13	12-5	5-5 12-5-1	3
Plate Offsets (X,	Y) [11:0-3-14,0-2-8]			

LOADIN	G (psf)	SPACING- 2	2-0-0	CSI.		DEFL.	in (lo	oc) I	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.31	Vert(LL)	-0.18 12-	-15 >	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.61	Vert(CT)	-0.28 12-	-15 >	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.33	Horz(CT)	0.07	10	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI20	014	Matri	x-AS	Wind(LL)	0.06 12-	-15 >	>999	240	Weight: 267 lb	FT = 20%

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2
SLIDER	Left 2x4 SP No.2 2-6-0, Right 2x4 SP No.2 2-6-0

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied. Rigid ceiling directly applied.

REACTIONS. (size) 2=0-3-8, 10=Mechanical Max Horz 2=131(LC 9) Max Uplift 2=-101(LC 12), 10=-89(LC 13) Max Grav 2=1662(LC 2), 10=1627(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-2675/551, 4-6=-2472/577, 6-8=-2434/580, 8-10=-2629/555

BOT CHORD 2-15=-367/2364, 12-15=-143/1612, 10-12=-357/2268

WEBS 6-12=-119/955, 8-12=-482/296, 6-15=-126/1015, 4-15=-510/302

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-2 to 3-7-11, Interior(1) 3-7-11 to 18-11-8, Exterior(2) 18-11-8 to 23-4-5, Interior(1) 23-4-5 to 37-8-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 4x4 MT20 unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

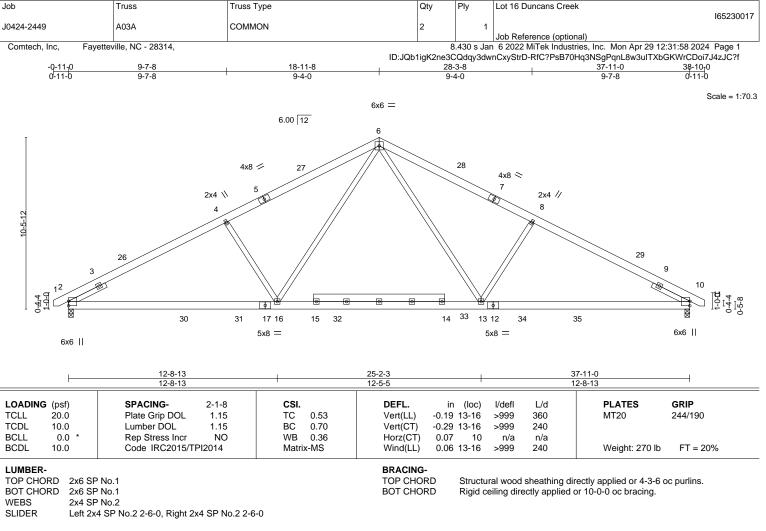
6) Refer to girder(s) for truss to truss connections.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 except (jt=lb) 2=101.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=-136(LC 10) Max Uplift 2=-107(LC 12), 10=-107(LC 13) Max Grav 2=1777(LC 2), 10=1777(LC 2)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-4=-2870/592, 4-6=-2653/619, 6-8=-2653/619, 8-10=-2870/592

BOT CHORD 2-16=-365/2544, 13-16=-129/1731, 10-13=-376/2490

WEBS 6-13=-141/1087, 8-13=-553/330, 6-16=-141/1087, 4-16=-553/330

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) -0-9-2 to 3-7-11, Interior(1) 3-7-11 to 18-11-8, Exterior(2) 18-11-8 to 23-4-5, Interior(1) 23-4-5 to 38-8-2 zone; C-C

for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 4x4 MT20 unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

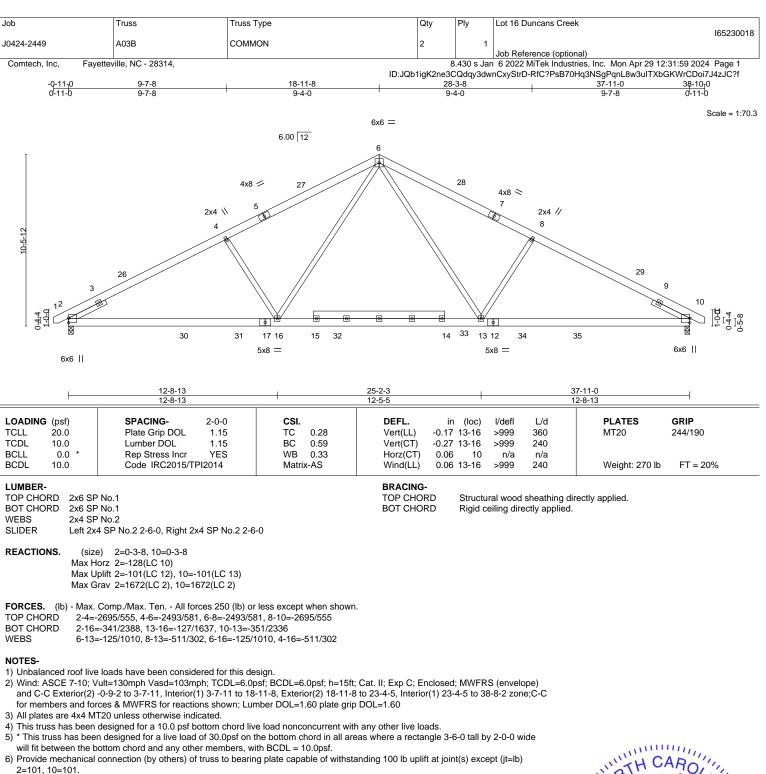
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=107, 10=107.



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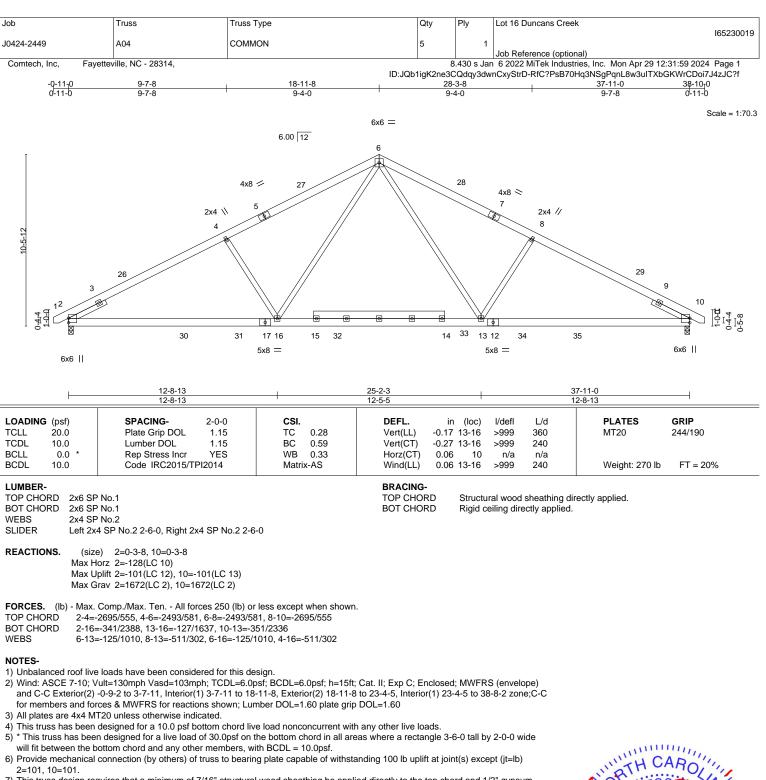




7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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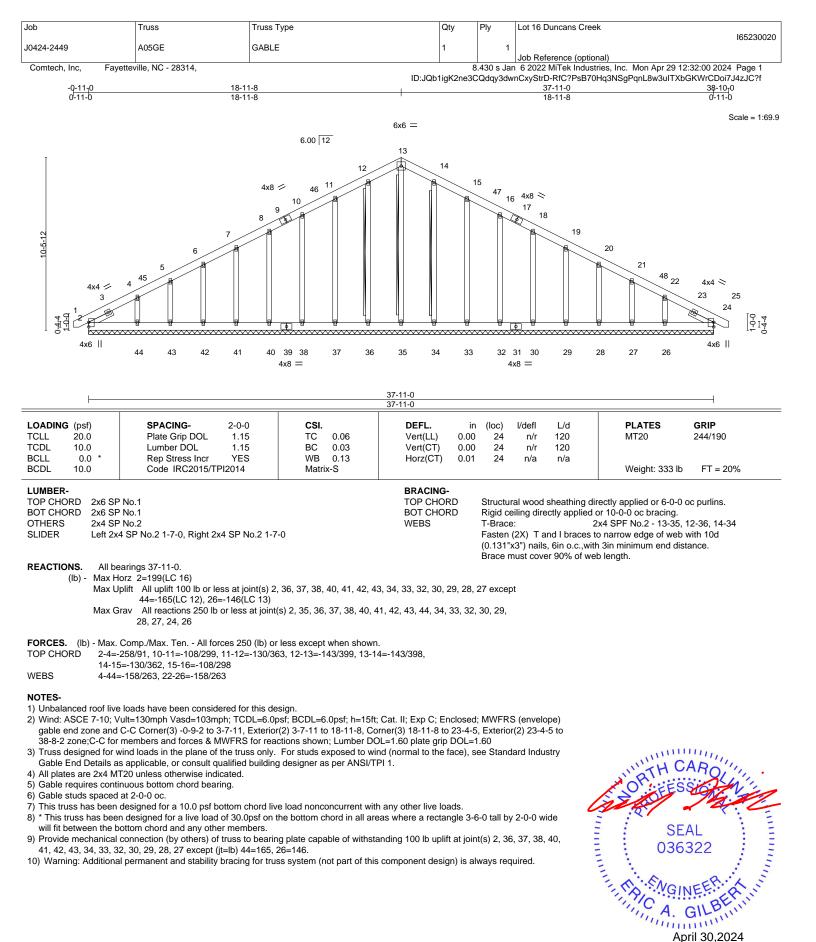


2=101, 10=101. 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum

sheetrock be applied directly to the bottom chord.



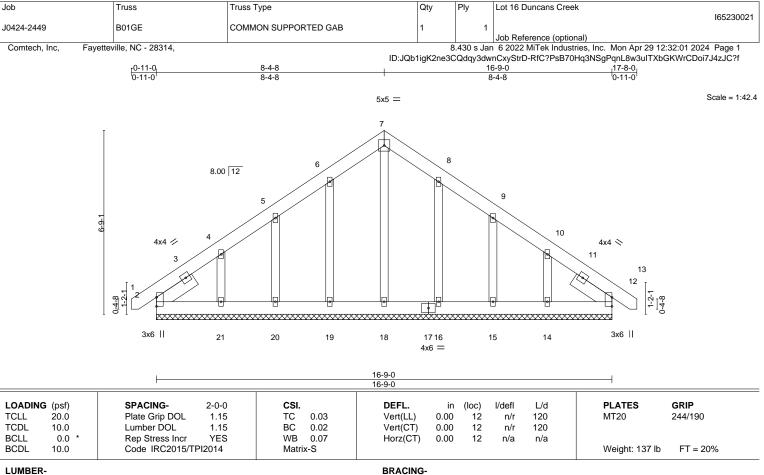
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818 Soundside Road



TOP CHORD

BOT CHORD

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

2x4 SP No.2 OTHERS

SLIDER Left 2x6 SP No.1 1-7-11, Right 2x6 SP No.1 1-7-11

REACTIONS. All bearings 16-9-0.

(lb) -Max Horz 2=-186(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 20, 16, 15, 12 except 21=-159(LC 12), 14=-147(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 18, 19, 20, 21, 16, 15, 14, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

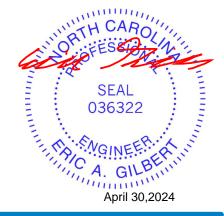
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-9-7 to 3-7-6, Exterior(2) 3-7-6 to 8-4-8, Corner(3) 8-4-8 to 12-9-5, Exterior(2) 12-9-5 to 17-6-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 8) will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 20, 16, 15, 12 except (jt=lb) 21=159, 14=147.



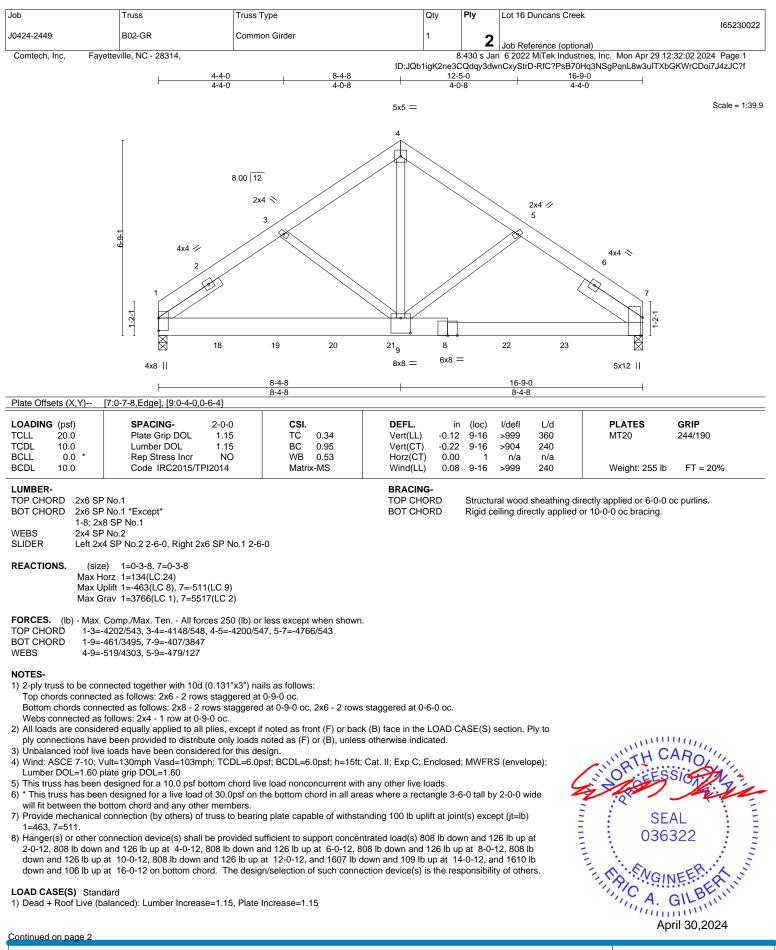
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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818 Soundside Road



TRENCO AMITEK Affiliate

818 Soundside Road Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	Lot 16 Duncans Creek
					165230022
J0424-2449	B02-GR	Common Girder	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fayettev	rille, NC - 28314,		8	.430 s Jan	6 2022 MiTek Industries, Inc. Mon Apr 29 12:32:02 2024 Page 2
		ID:JQb	1igK2ne3C	Qdqy3dw	nCxyStrD-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

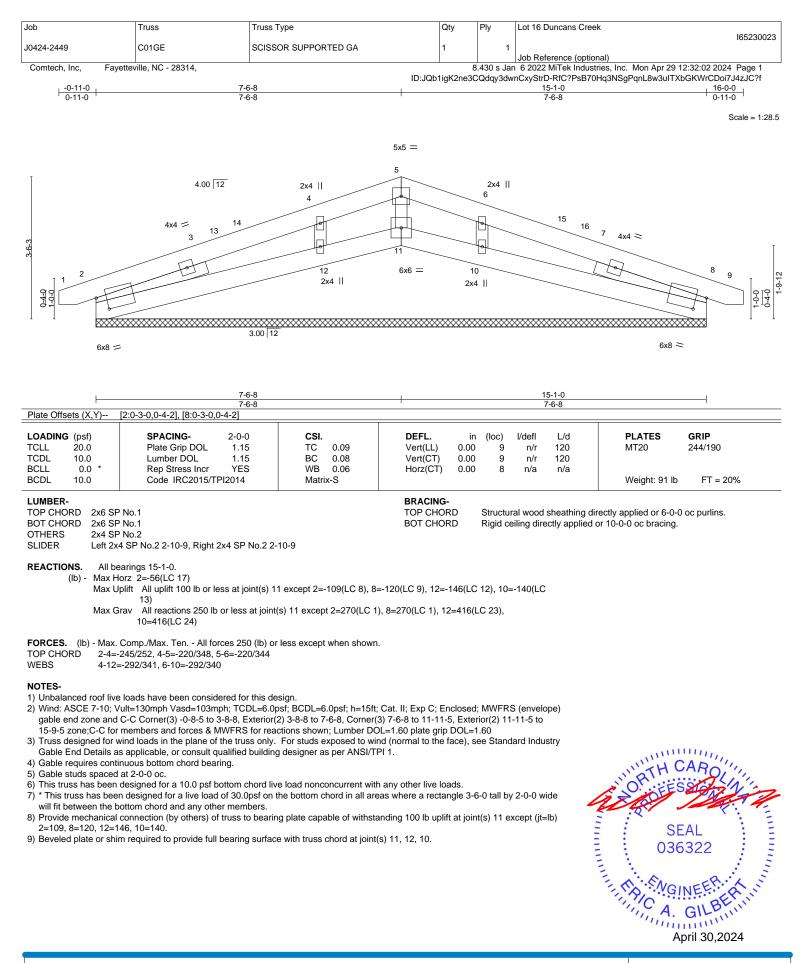
LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 10-14=-20

Concentrated Loads (lb) Vert: 8=-808(B) 16=-1489(B) 18=-808(B) 19=-808(B) 20=-808(B) 21=-808(B) 22=-808(B) 23=-1486(B)

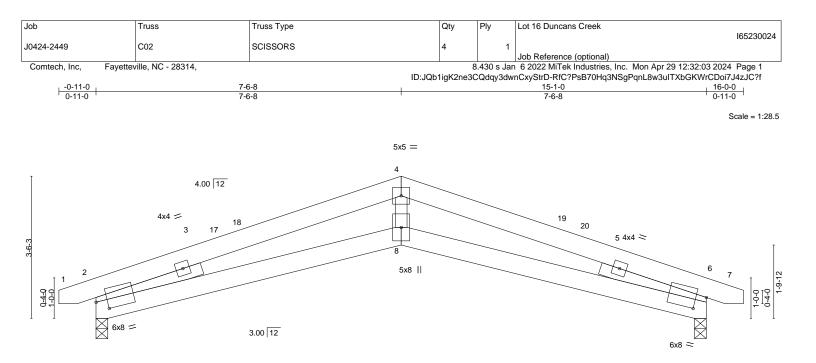
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





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⊢	<u>7-6-8</u> 7-6-8			<u>15-1-0</u> 7-6-8		
Plate Offsets (X,Y)	[2:0-3-6,0-2-12], [6:0-3-0,0-4-2]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.17 BC 0.29 WB 0.21 Matrix-AS	Vert(LL) -0.0 Vert(CT) -0.1 Horz(CT) 0.1	>999 240 n/a n/a	MT20	GRIP 244/190 FT = 20%
BOT CHORD 2x6 S WEBS 2x4 S	P No.1 P No.1 P No.2 k4 SP No.2 2-9-0, Right 2x4 SP No.2 2-9	-0	BRACING- TOP CHORD BOT CHORD	ural wood sheath ceiling directly ap	ning directly applied. oplied.	
Max I Max I	ze) 2=0-3-8, 6=0-3-8 Horz 2=-33(LC 13) Jplift 2=-72(LC 8), 6=-72(LC 9) Grav 2=645(LC 1), 6=645(LC 1)					
()	. Comp./Max. Ten All forces 250 (lb) or	less except when shown.				

TOP CHORD 2-4=-1829/501, 4-6=-1829/492

BOT CHORD 2-8=-395/1733, 6-8=-395/1733

WEBS 4-8=-122/844

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-5 to 3-8-8, Interior(1) 3-8-8 to 7-6-8, Exterior(2) 7-6-8 to 11-11-5, Interior(1) 11-11-5 to 15-9-5 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

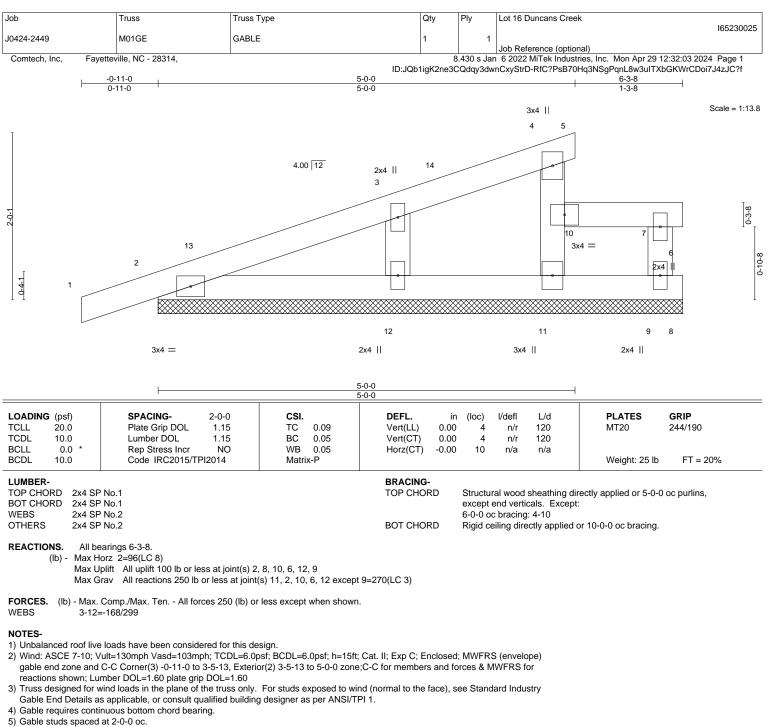
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 10, 6, 12, 9.

10) Magnitude of user added load(s) on this truss have been applied uniformly across all gravity load cases with no adjustments.

LOAD CASE(S) Standard

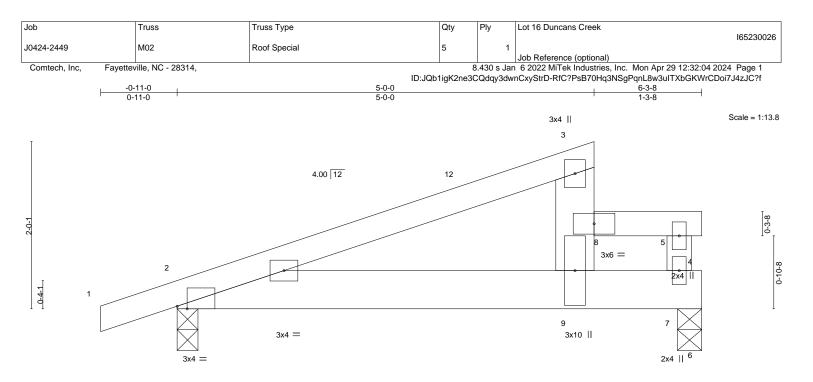
 Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-4=-60, 4-5=-20, 2-8=-20, 7-10=-170, 6-7=-20





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5-0-0
5-0-0

Plate Offsets (X,Y)	[2:0-1-7,Edge]						
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) l/c	lefl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.28	Vert(LL) -0.05	9-11 >9	99 360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.69	Vert(CT) -0.11	9-11 >6	50 240		
CLL 0.0 *	Rep Stress Incr NO	WB 0.03	Horz(CT) -0.00	7	n/a n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-MP	Wind(LL) 0.18	9-11 >4	02 240	Weight: 30 lb	FT = 20%
UMBER-	P No.1	·	BRACING- TOP CHORD	Structural v	vood sheathing dir	ectly applied or 5-0-0	oc purlins,

TOP CHORD	2x4 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 5-0-0 oc purlins,
BOT CHORD	2x4 SP No.1 *Except*		except end verticals. Except:
	2-6: 2x6 SP No.1		6-0-0 oc bracing: 3-8
WEBS	2x6 SP No.1 *Except*	BOT CHORD	Rigid ceiling directly applied or 7-9-5 oc bracing.
	5-7: 2x4 SP No.2		

REACTIONS. (size) 2=0-3-0, 7=0-3-8 Max Horz 2=67(LC 8)

Max Uplift 2=-174(LC 8), 7=-313(LC 8) Max Grav 2=410(LC 1), 7=705(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 8-9=-665/957

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=174, 7=313.
- 6) Magnitude of user added load(s) on this truss have been applied uniformly across all gravity load cases with no adjustments.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-60, 2-6=-20, 5-8=-130, 4-5=-20 Concentrated Loads (lb)

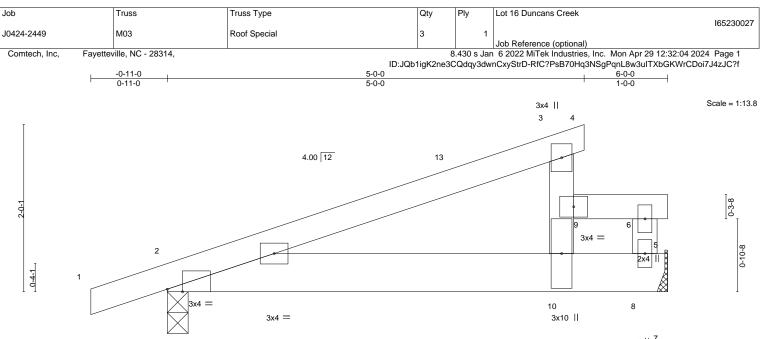
Vert: 8=-460



6-3-8

1-3-8

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2x4 || ⁷

	L				5-0-0					6-0-0	
					5-0-0					1-0-0	1
Plate Offsets (X,Y)	[2:0-2-3,Edge]										
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.24	Vert(LL)	-0.04	10-12	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.53	Vert(CT)	-0.08	10-12	>864	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB	0.01	Horz(CT)	-0.00	8	n/a	n/a		
BCDL 10.0	Code IRC2015/T	PI2014	Matrix	x-MP	Wind(LL)	0.12	10-12	>548	240	Weight: 28 lb	FT = 20%
LUMBER-		L.			BRACING-						
TOP CHORD 2x4 S	P No.1				TOP CHOR	D	Structu	ral wood	sheathing di	rectly applied or 5-0-0	oc purlins,
BOT CHORD 2x4 S	P No.1 *Except*						except	end verti	cals. Except	:	
2-7: 2	x6 SP No.1						6-0-0 o	c bracino	: 3-9		
WEBS 2x4 S	P No.2				BOT CHOR	п	Rigid c	eiling dire	, actly applied	or 10-0-0 oc bracing.	

REACTIONS. (size) 2=0-3-0, 8=Mechanical Max Horz 2=69(LC 8)

Max Uplift 2=-156(LC 8), 8=-258(LC 8) Max Grav 2=372(LC 1), 8=603(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 9-10=-597/850

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 5-0-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=156, 8=258.
- 7) Magnitude of user added load(s) on this truss have been applied uniformly across all gravity load cases with no adjustments.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

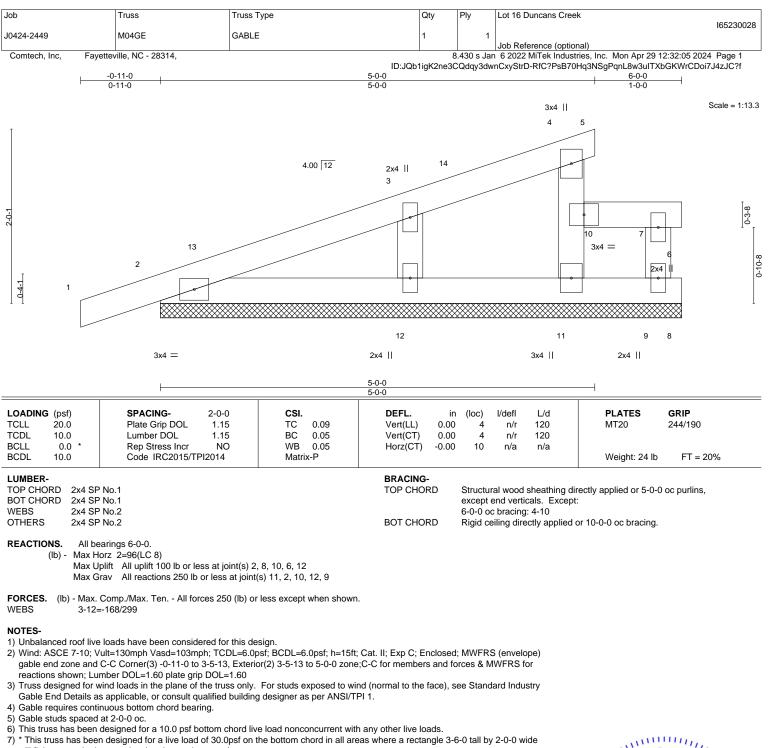
- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-3=-60, 3-4=-20, 2-7=-20, 6-9=-30(F=-10), 5-6=-20 Concentrated Loads (lb)
 - Vert: 9=-460



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will fit between the bottom chord and any other members.

8) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 10, 6, 12.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

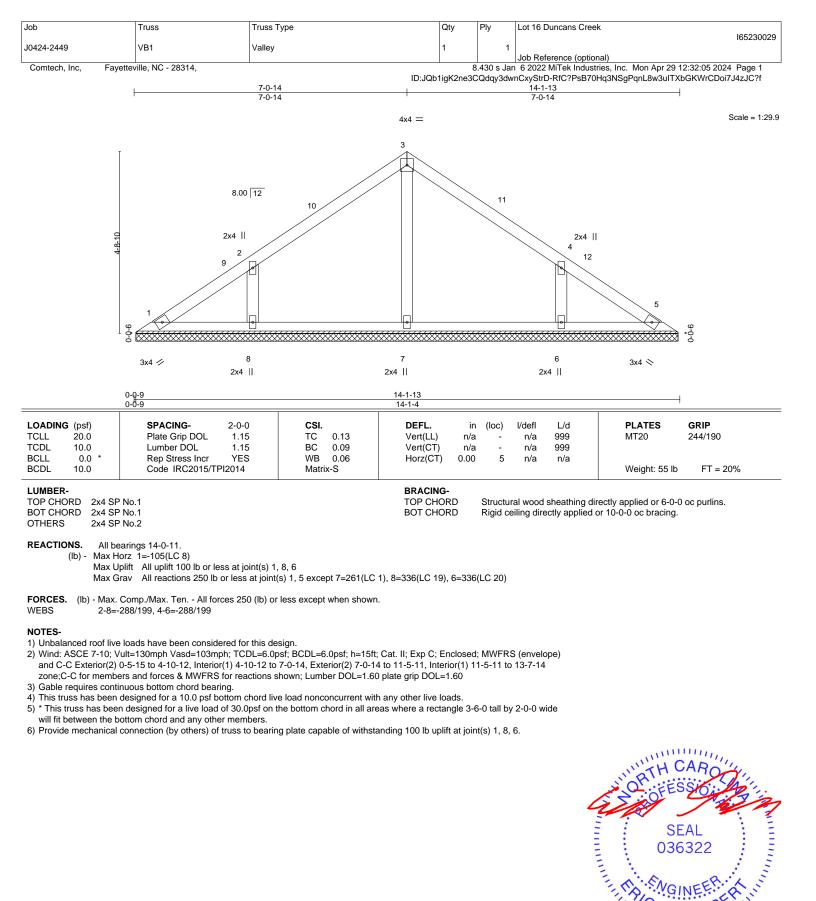
Vert: 1-4=-60, 4-5=-20, 2-8=-20, 7-10=-30, 6-7=-20



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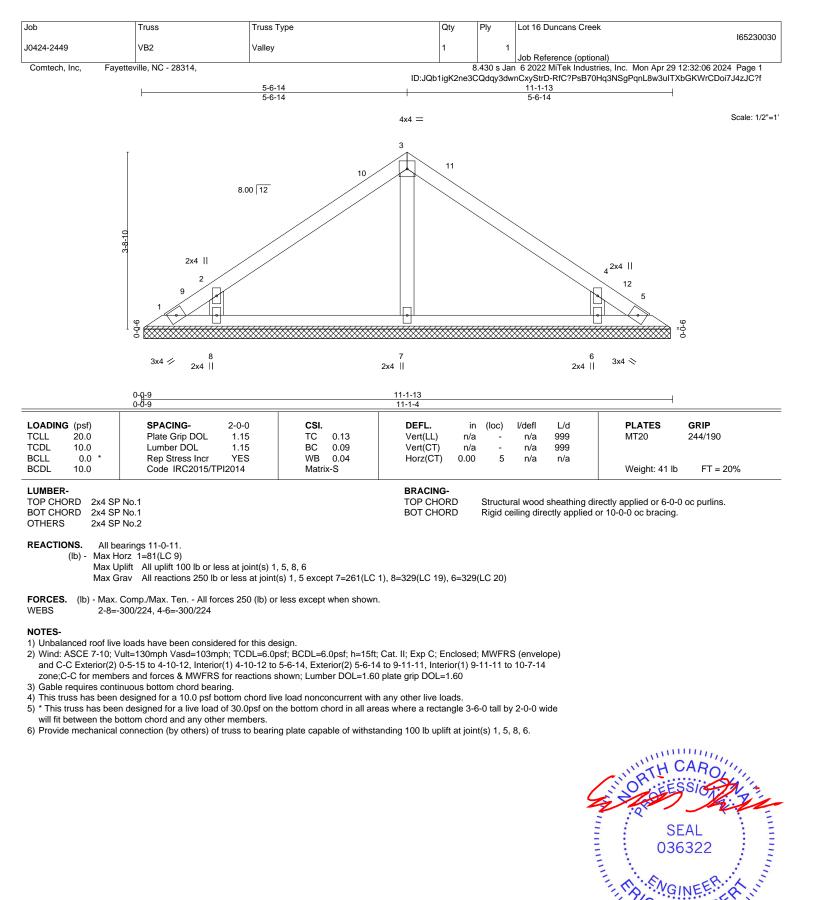
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April 30,2024

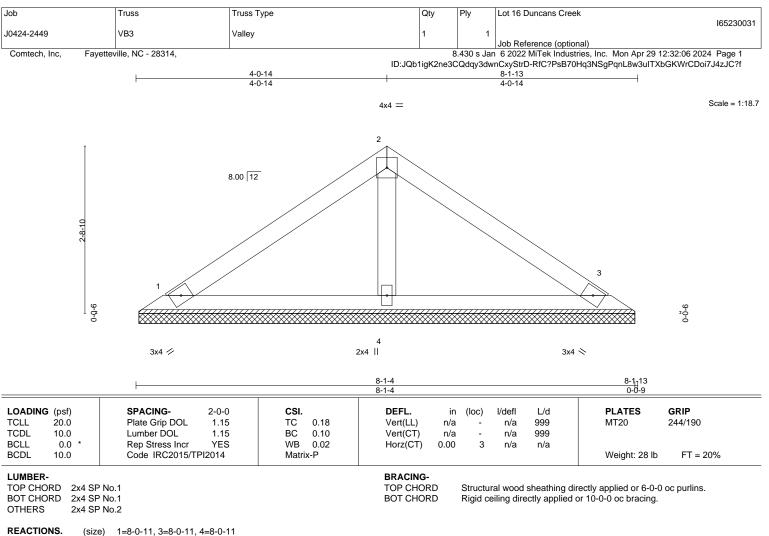


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Edenton, NC 27932

G mm April 30,2024



Max Hord 1=-57(LC 12), 3=-30(LC 13) Max Grav 1=156(LC 1), 3=156(LC 1), 4=261(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

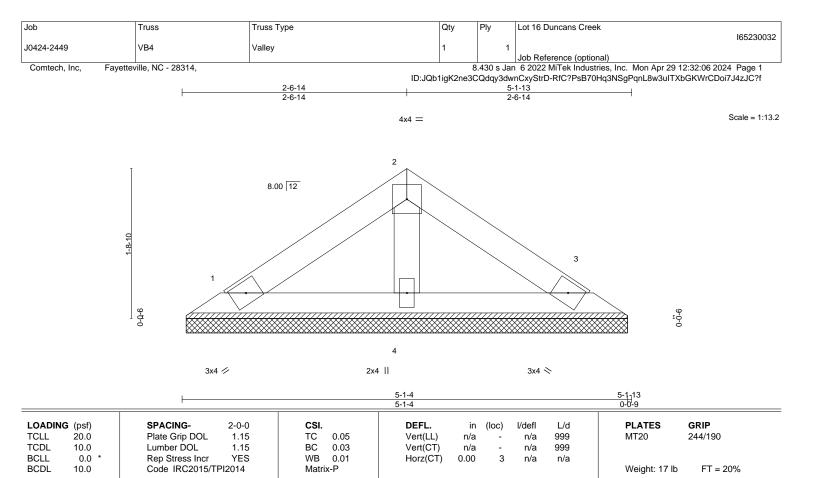
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

OTHERS 2x4 SP No.2

REACTIONS. (size) 1=5-0-11, 3=5-0-11, 4=5-0-11 Max Horz 1=-33(LC 8) Max Uplift 1=-14(LC 12), 3=-18(LC 13) Max Grav 1=91(LC 1), 3=91(LC 1), 4=152(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

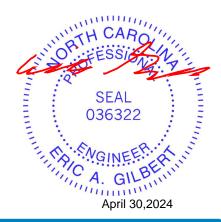
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

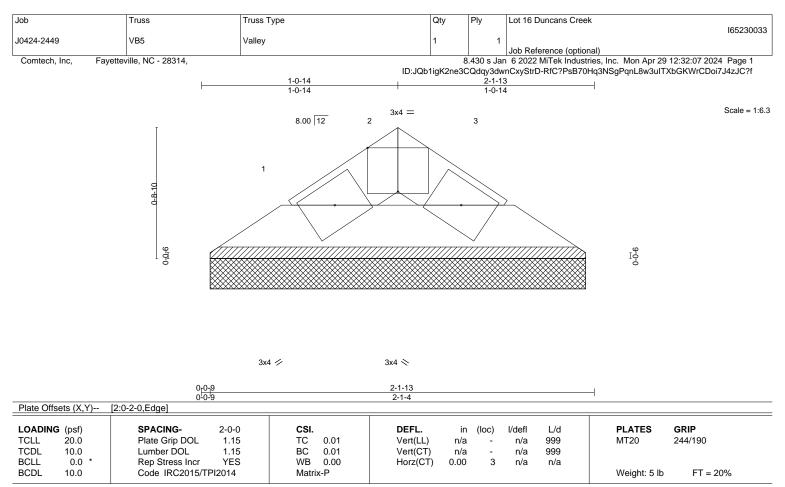


Structural wood sheathing directly applied or 5-1-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1

REACTIONS. (size) 1=2-0-11, 3=2-0-11 Max Horz 1=9(LC 9) Max Uplift 1=-2(LC 12), 3=-2(LC 13) Max Grav 1=46(LC 1), 3=46(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 2-1-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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