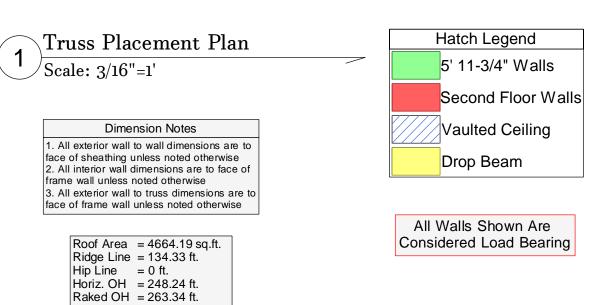


		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	6
BM2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
BM3	20' 0"	1-3/4"x 23-7/8" LVL Kerto-S	3	3
GDH	24' 0"	1-3/4"x 16" LVL Kerto-S	2	2
GDH2	13' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2



Decking = 160 sheets

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)

ROOF & FLOOR TRUSSES & BEAMS

> Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are leemed to comply with the prescriptive Code equirements. The contractor shall refer to the ttached Tables (derived from the prescriptive Coc equirements) to determine the minimum foundatic are and number of wood studs required to suppor eactions greater than 3000# but not greater than 5000#. A registered design professional shall be etained to design the support system for any eaction that exceeds those specified in the attache Tables. A registered design professional shall be etained to design the support system for all eactions that exceed 15000#.

Jonathan Landry

Jonathan Landry

LOAD CHART FOR JACK STUDS (BASED ON TABLES R502.5(1) & (b))

NUA	ABER C	STUDS R		A END OF	:
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

BUILDER	Glover Design Build	CITY / CO.	CITY / CO. Fuquay Varina / Harnett	
JOB NAME	JOB NAME Lot 19 Purfoy Place	ADDRESS	438 Lambert Lane	,
PLAN	Paxton / 3GRF, CP	MODEL	Roof	
SEAL DATE N/A	N/A	DATE REV.	03/27/23	
флоте #		DRAWN BY	DRAWN BY Jonathan Landry	
JOB #	J0922-4569	SALES REP.	SALES REP. Lenny Norris	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0922-4569 Lot 19 Purfoy Place

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I57394434 thru I57394461

My license renewal date for the state of North Carolina is December 31, 2023.

North Carolina COA: C-0844



March 27,2023

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157394434 J0922-4569 Α1 **ROOF SPECIAL** 3 | Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:45:57 2023 Page 1 Comtech, Inc, Fayetteville, NC - 28314,

ID:X5az_D23vLwLuiTNLuG6bHyGfxb-ptt1xlnzXl6eVhhv?BsJzrrvbXefDJ8UTai5hXzWnde

43-11-0 44-10-0 0-11-0 -0-11-0 0-11-0 6-1-12 4-9-12 4-0-0 10-0-0 10-0-0 8-11-8

> Scale: 1/8"=1' 5x8 =

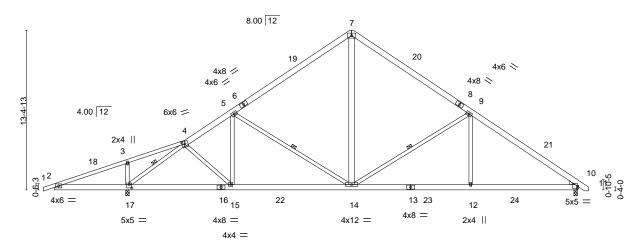
> > Structural wood sheathing directly applied or 4-8-8 oc purlins.

4-17, 5-14, 9-14

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 2-17.

1 Row at midpt



	0-1-12	14-11-0	1	24-11-0	34-11-0	1	43-11-0	
	6-1-12	8-9-12	ı	10-0-0	10-0-0		8-11-8	
Plate Offsets (X,Y)	[4:0-2-12,0-2-8], [10:0-0-0	,0-2-2], [17:0-2-8,0-3-8]						
LOADING (psf)	SPACING-	2-0-0 CS	il.	DEFL.	in (loc) I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15 TC	0.46	Vert(LL)	-0.11 14-15 >999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15 BC	0.43	Vert(CT)	-0.20 14-15 >999	240		
BCLL 0.0 *	Rep Stress Incr	YES WE	3 0.66	Horz(CT)	0.06 10 n/a	n/a		
BCDL 10.0	Code IRC2015/TP	I2014 Ma	trix-S	Wind(LL)	0.06 14-15 >999	240	Weight: 317 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x6 SP No.1 *Except*

1-4: 2x4 SP No.1 2x6 SP No.1

BOT CHORD WEBS 2x4 SP No.2 *Except*

7-14: 2x6 SP No.1

WEDGE Right: 2x4 SP No.3

REACTIONS. (size) 17=0-3-8, 10=0-3-8

Max Horz 17=428(LC 11)

Max Uplift 17=-407(LC 12), 10=-272(LC 13) Max Grav 17=2101(LC 1), 10=1741(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1030/770, 3-4=-898/721, 4-5=-2076/500, 5-7=-1695/602, 7-9=-1704/634,

9-10=-2482/689

BOT CHORD 2-17=-663/1045, 15-17=-282/1769, 14-15=-288/1972, 12-14=-366/1898, 10-12=-366/1898

WEBS 3-17=-403/327, 4-17=-2428/1189, 4-15=-285/334, 5-15=-63/345, 5-14=-812/325,

7-14=-277/1180, 9-14=-1074/470, 9-12=0/587

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 24-11-8, Exterior(2) 24-11-8 to 29-4-5, Interior(1) 29-4-5 to 44-8-1 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 407 lb uplift at joint 17 and 272 lb uplift at joint 10.



March 27,2023



Job Truss Truss Type Qty Lot 19 Purfoy Place 157394435 J0922-4569 A1SG **GABLE** | Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:00 2023 Page 1

Comtech, Inc, Fayetteville, NC - 28314,

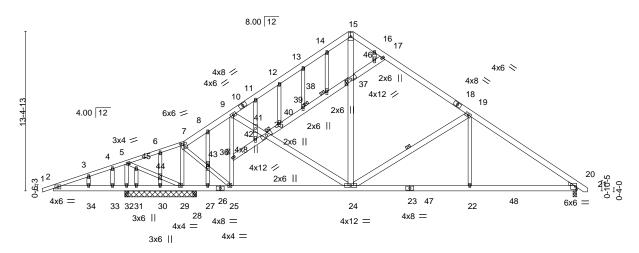
ID:X5az_D23vLwLuiTNLuG6bHyGfxb-ESYAZmprqgUDM9QUgJP0aUTS8lfhQkCw9YwmlszWndb

24-11-8 10-0-0 10-0-0 4-0-0

> Scale: 1/8"=1' 5x8 ||

> > Structural wood sheathing directly applied or 5-0-15 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing, Except:



		0-0-0	0-1-12 4-9-12	0-9-0 3-3-0		10-0-0	10	-0-0		0-11-0	
Plate Offse	ets (X,Y)	[7:0-3-0,0-2-0]									
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.37	Vert(LL)	-0.06 22-24	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	-0.12 22-24	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.38	Horz(CT)	0.03 20	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matrix	c-S	Wind(LL)	0.07 20-22	>999	240	Weight: 412 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-**BRACING-**

2x6 SP No.1 *Except* TOP CHORD 1-7: 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1

10-0-0 oc bracing: 24-25,22-24,20-22. WEBS 2x6 SP No.1 *Except* **WEBS** 1 Row at midpt 5-32,5-29,7-29,7-25,9-25,19-22: 2x4 SP No.2 **JOINTS** 1 Brace at Jt(s): 35, 36, 37, 38, 39, 43

11-8-8 14-11-8

2x4 SP No.2 **OTHERS**

WEDGE

Right: 2x4 SP No.3

REACTIONS. All bearings 6-0-0 except (jt=length) 20=0-3-8, 28=0-3-8.

(lb) -Max Horz 32=553(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) except 32=-1265(LC 8), 29=-345(LC 12), 20=-514(LC 13), 30=-176(LC 8), 31=-544(LC 23), 28=-185(LC 12)

All reactions 250 lb or less at joint(s) except 32=1193(LC 23)

32=1137(LC 1), 29=1059(LC 19), 20=1516(LC 20), 30=313(LC 1), 31=571(LC 8),

28=464(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-962/724, 3-4=-902/722, 4-5=-861/715, 5-6=-386/467, 6-7=-328/445, 7-8=-784/348,

8-9=-883/352, 9-11=-1162/483, 11-12=-1143/486, 12-13=-1218/580, 13-14=-1265/646,

14-15=-1197/626, 15-16=-1159/597, 16-17=-1241/635, 17-19=-1323/633,

19-20=-2071/749

2-34=-660/960, 33-34=-660/960, 32-33=-660/960, 31-32=-780/932, 30-31=-780/932, **BOT CHORD**

29-30=-780/932, 28-29=-571/492, 27-28=-571/492, 25-27=-571/492, 24-25=-253/853,

22-24=-404/1545, 20-22=-404/1545

WEBS 5-32=-390/502, 5-45=-489/331, 44-45=-488/332, 29-44=-501/340, 7-29=-1456/714,

7-43=-603/1314, 25-43=-577/1263, 25-36=-704/516, 9-36=-574/418, 9-41=-5/345,

35-41=-63/423, 24-35=-149/387, 24-37=-240/736, 15-37=-266/760, 19-24=-1050/585,

19-22=0/594, 8-43=-262/69

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2



March 27,2023



Job	Truss	Truss Type	Qty	Ply	Lot 19 Purfoy Place
10000 4500		OARLE			I57394435
J0922-4569	A1SG	GABLE	1	1	
					Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:00 2023 Page 2 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-ESYAZmprqgUDM9QUgJP0aUTS8lfhQkCw9YwmlszWndb

NOTES-

- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1265 lb uplift at joint 32, 345 lb uplift at joint 29, 514 lb uplift at joint 20, 176 lb uplift at joint 30, 544 lb uplift at joint 31 and 185 lb uplift at joint 28.



818 Soundside Road Edenton, NC 27932

Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157394436 J0922-4569 A2 **ROOF SPECIAL** 5 Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:01 2023 Page 1

 $ID: X5 az_D23 vLwLuiTNLuG6 bHyGfxb-ie6 Ym6 qTb_c4_J_gE1 wF7 h? cJ90S99W3OBgJqJzWnda$ 24-11-8 39-11-0 14-11-8 34-11-8 6-1-12 4-9-12 4-0-0 10-0-0 10-0-0 4-11-8

5x8 ||

Structural wood sheathing directly applied or 5-4-1 oc purlins,

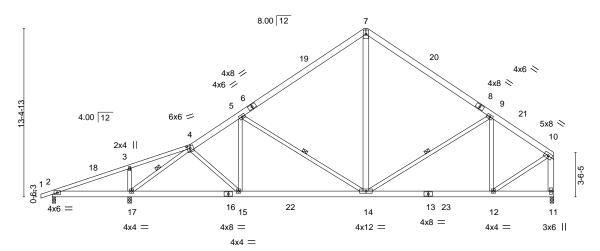
4-17, 9-14, 5-14

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

1 Row at midpt

Scale = 1:91.7



14-11-8 34-11-8 39-11-0 8-9-12 4-11-8

BRACING-

TOP CHORD

BOT CHORD

WEBS

Plate Offset	ts (X,Y)	[4:0-2-12,0-2-8]									
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.41	Vert(LL)	-0.08 14-15	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.37	Vert(CT)	-0.15 14-15	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.51	Horz(CT)	0.03 11	n/a	n/a		
BCDL	10.0	Code IRC2015/TP	I2014	Matri	x-S	Wind(LL)	0.05 14-15	>999	240	Weight: 308 lb	FT = 20%

LUMBER-

WEBS

REACTIONS.

2x6 SP No.1 *Except* TOP CHORD

1-4: 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1 2x4 SP No.2 *Except*

10-11,7-14: 2x6 SP No.1

(size) 2=0-3-0, 17=0-3-8, 11=0-3-8

Max Horz 2=421(LC 9)

Max Uplift 2=-138(LC 8), 17=-377(LC 12), 11=-206(LC 13) Max Grav 2=182(LC 23), 17=1739(LC 1), 11=1410(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-203/368, 3-4=-116/369, 4-5=-1783/601, 5-7=-1381/579, 7-9=-1381/588,

9-10=-1251/444, 10-11=-1423/487

BOT CHORD 2-17=-368/118, 15-17=-437/1454, 14-15=-448/1651, 12-14=-307/1013

WEBS 3-17=-384/287, 4-17=-2012/635, 4-15=-16/270, 10-12=-366/1223, 7-14=-219/812,

9-14=-282/259, 9-12=-525/312, 5-14=-825/401, 5-15=0/275

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 24-11-8, Exterior(2) 24-11-8 to 29-4-5, Interior(1) 29-4-5 to 39-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 2, 377 lb uplift at joint 17 and 206 lb uplift at joint 11.





 Job
 Truss
 Truss Type
 Qty
 Ply
 Lot 19 Purfoy Place

 J0922-4569
 A3
 ROOF TRUSS
 3
 1

 Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

Job Reference (optional)
8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:03 2023 Page 1

Structural wood sheathing directly applied or 1-11-14 oc purlins,

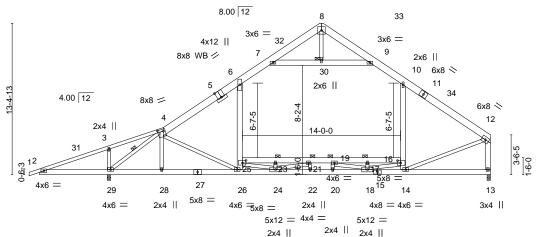
Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

except end verticals.

6-0-0 oc bracing: 2-29

ID:X5az_D23vLwLuiTNLuG6bHyGfxb-e1ElBork7bsnDc83MSzjC65pTybZdysMrV9QuBzWndY
0₇11₇0 6-1-12 10-11-8 17-11-8 24-11-8 31-11-8 39-11-0
0₇11-0 6-1-12 4-9-12 7-0-0 7-0-0 7-0-0 7-11-8

8x8 = Scale = 1:102.0



	6-1-12	10-11-8	17-11-8	20-10-10	23-11-8	25-11-8	29-0-6	31-11-8	39-11-0	1
	6-1-12	4-9-12	7-0-0	2-11-2	3-0-14	2-0-0	3-0-14	2-11-2	7-11-8	7
= .	0.40 [-1] [40.0	0 4 0 0 01 1	05-0 0 40 0 0 401							

Plate Off	sets (X,Y)	[4:0-6-0,0-2-4], [5:0-4-0,Edge], [16:0-	3-4,0-2-8], [25:0-3-12,0-2-1	2]	
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.94	Vert(LL) -0.35 26 >999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.78	Vert(CT) -0.76 26 >531 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.90	Horz(CT) 0.06 13 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.30 26-28 >999 240	Weight: 381 lb FT = 20%

LUMBER- BRACING-

TOP CHORD 2x8 SP No.1 *Except* TOP CHORD

6-26,10-14,12-13,7-9: 2x6 SP No.1 9-6-15 oc bracing: 13-14.

OTHERS 2x4 SP No.2 WEBS 1 Row at midst 4.

OTHERS 2x4 SP No.2 WEBS 1 Row at midpt 4-29

JOINTS 1 Brace at Jt(s): 30, 21, 19, 23, 17

REACTIONS. (size) 29=0-3-8, 13=0-3-8 Max Horz 29=419(LC 9)

Max Horz 29=419(LC 9) Max Uplift 29=-38(LC 12)

Max Grav 29=2604(LC 2), 13=2254(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-583/770, 3-4=-463/732, 4-6=-2702/36, 6-7=-1948/233, 9-10=-2046/258,

10-12=-2429/55, 12-13=-2257/115

BOT CHORD 2-29=-664/628, 28-29=0/2506, 26-28=0/2498, 24-26=0/2949, 22-24=0/4355,

20-22=0/4355, 18-20=0/4355, 14-18=-37/1376, 23-25=-2487/0, 21-23=-2487/0,

19-21=-2628/0, 17-19=-1186/180, 16-17=-1186/180

WEBS 3-29=-393/300, 4-29=-3419/259, 4-26=-397/107, 6-25=0/1020, 10-16=0/714, 12-14=0/2069, 7-30=-2178/188, 9-30=-2178/188, 19-20=-14/263, 23-24=-399/0,

17-18=-429/0, 24-25=0/2032, 21-24=-424/513, 18-19=-1738/0, 16-18=0/2050

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 24-11-8, Exterior(2) 24-11-8 to 29-4-5, Interior(1) 29-4-5 to 39-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 4-6, 6-7, 9-10, 7-30, 9-30; Wall dead load (5.0psf) on member(s).6-25, 10-16

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 23-25, 21-23, 19-21, 17-19, 16-17

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 29.

8) Attic room checked for L/360 deflection.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not

Design valid for use only with Mir leave connectors. Inits design is based only upon parameters snown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



818 Soundside Road Edenton, NC 27932 Job Truss Truss Type Qty Lot 19 Purfoy Place 157394438 **ROOF TRUSS** J0922-4569 A4 3 Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:05 2023 Page 1 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-aPM3cUt_fC6VTwlSTt?BHXA9JmG25uEfJpeXz4zWndW

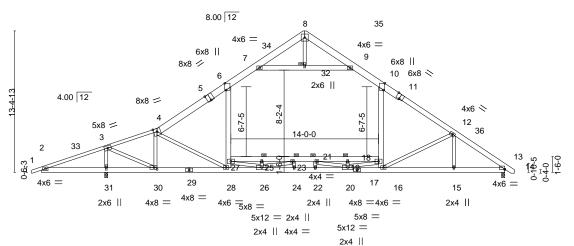
38-11-8

Structural wood sheathing directly applied or 2-1-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

31-11-8 7-0-0 7-0-0 7-0-0

> Scale = 1:109.2 8x8 =



		' 6-1-12	2 4-9-1	2 ' 7	-0-0	2-11-2 ' 3-0-14 '2-0-0	3-0-14 2-11-2	2 ' 7	-0-0	' 4-11-8 '	
Plate Off	sets (X,Y)	[5:0-4-0,Edge], [6:0-7-11	,Edge], [10:0-7	7-11,Edge], [1	1:0-4-0,Edg	ge], [13:0-0-0,0-0-1	0], [30:0-3-8,0-2	2-0]			
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	I/defI	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.92	Vert(LL)	-0.29 16-20	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.78	Vert(CT)	-0.57 22-24	>795	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.78	Horz(CT)	0.09 13	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matrix	k-S	Wind(LL)	0.29 15-16	>999	240	Weight: 393 lb	FT = 20%

TOP CHORD

20-10-10 23-11-8 25-11-8 29-0-6 31-11-8

LUMBER-BRACING-

2x8 SP No.1 *Except* TOP CHORD

1-4: 2x4 SP No.1, 11-14: 2x6 SP No.1 **BOT CHORD BOT CHORD** 2x6 SP No.1

6-0-0 oc bracing: 2-31,30-31. WEBS 2x4 SP No.2 *Except* **JOINTS** 1 Brace at Jt(s): 32, 21, 23, 25, 19

17-11-8

10-16,6-28,7-9: 2x6 SP No.1

10-11-8

Max Grav 31=2854(LC 2), 13=2387(LC 21)

(size) 31=0-3-8, 13=0-3-8 Max Horz 31=425(LC 11) Max Uplift 31=-20(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-581/774, 3-4=-2620/0, 4-6=-3232/64, 6-7=-2475/258, 7-8=-63/326, 8-9=-57/321,

9-10=-2453/276, 10-12=-3251/128, 12-13=-3697/209

BOT CHORD 2-31=-668/626, 30-31=-733/618, 28-30=0/2782, 26-28=0/2761, 24-26=0/4799, 22-24=0/4799, 20-22=0/4799, 16-20=0/2562, 15-16=-52/2930, 13-15=-52/2930,

25-27=-1722/0, 23-25=-1722/0, 21-23=-2543/0, 19-21=-1827/68, 18-19=-1827/68 3-31=-2614/470, 12-16=-670/340, 16-18=0/495, 10-18=0/1260, 27-28=-59/395,

6-27=0/1159, 7-32=-2956/217, 9-32=-2956/217, 4-30=-1384/215, 25-26=-421/0, 19-20=-420/0, 26-27=0/2034, 23-26=-1090/291, 20-21=-979/160, 18-20=0/2044,

3-30=-243/3176

NOTES-

WEBS

REACTIONS.

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 24-11-8, Exterior(2) 24-11-8 to 29-4-5, Interior(1) 29-4-5 to 44-8-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 4-6, 6-7, 9-10, 7-32, 9-32; Wall dead load (5.0psf) on member(s).10-18, 6-27

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 25-27, 23-25, 21-23, 19-21,

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 31.

8) Attic room checked for L/360 deflection.





Job Truss Truss Type Qty Lot 19 Purfoy Place Ply 157394439 **ROOF TRUSS** J0922-4569 A4SG Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:07 2023 Page 1

ID:X5az_D23vLwLuiTNLuG6bHyGfxb-XoTp19uEBqMDiESqbl1fMyFXfZz0Zowym77d1yzWndU

31-11-8 7-0-0 43-11-0 44-10-0 4-11-8 0-11-0 7-0-0 7-0-0 7-0-0

> Scale = 1:111.5 8x8 =

> > Structural wood sheathing directly applied or 3-1-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

8-42

1 Brace at Jt(s): 45, 46, 47, 48, 54, 30, 32, 34, 28

6-0-0 oc bracing: 2-44,43-44,42-43.

1 Row at midpt

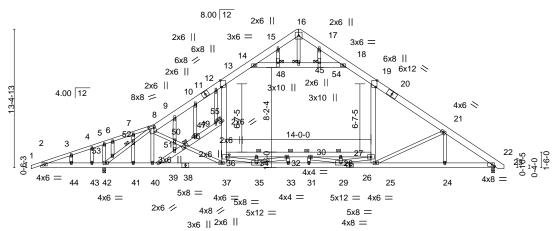


Plate Offsets (X,Y)	[8:0-6-0,0-2-4], [13:0-7-11,Edge], [19:0		[-8]	3-0-14 2-11-2	7-0-0	4-11-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.80 BC 0.74 WB 0.83 Matrix-S	Vert(CT) Horz(CT)	in (loc) l/defl -0.24 25-29 >999 -0.49 30 >915 0.10 22 n/a 0.30 24-25 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 450 lb	GRIP 244/190 FT = 20%

WEBS

JOINTS

20-10-10 23-11-8 25-11-8 29-0-6 31-11-8

LUMBER-**BRACING-**

2x8 SP No.1 *Except* TOP CHORD TOP CHORD 1-8: 2x4 SP No.1 **BOT CHORD**

BOT CHORD 2x6 SP No.1 2x4 SP No.2 *Except* **WEBS**

19-25,13-37,14-18,46-47,39-46: 2x6 SP No.1

2x4 SP No.2 **OTHERS**

REACTIONS. 42=0-3-8, 22=0-3-8 (size) Max Horz 42=549(LC 11)

Max Uplift 42=-509(LC 12), 22=-234(LC 13)

Max Grav 42=2751(LC 2), 22=2356(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1013/729, 3-4=-950/727, 4-5=-928/719, 5-6=-852/727, 6-7=-842/706, 7-8=-784/695, 8-9=-3196/165, 9-10=-3350/228, 10-12=-3467/267, 12-13=-3557/245,

13-14=-2441/421, 15-16=0/472, 16-17=0/464, 18-19=-2429/407, 19-21=-3226/265,

21-22=-3722/410

BOT CHORD 2-44=-665/1009, 43-44=-665/1009, 42-43=-665/1009, 41-42=-170/2818, 40-41=-170/2818,

39-40=-158/2803, 37-39=-522/3299, 35-37=-511/2598, 33-35=0/4653, 31-33=0/4653, 29-31=0/4653, 25-29=-6/2471, 24-25=-227/2978, 22-24=-227/2977, 34-36=-1517/331,

32-34=-1517/331, 30-32=-2301/0, 28-30=-1650/15, 27-28=-1650/15

5-42=-24/256, 42-53=-3703/774, 52-53=-3559/760, 8-52=-3456/735, 8-40=-326/476,

8-50=-494/551, 46-50=-461/468, 37-46=-827/524, 16-45=-422/0, 21-25=-765/537, 25-27=-63/525, 19-27=0/1248, 36-37=-68/589, 36-47=0/1314, 13-47=0/1875,

14-48=-2890/366, 45-48=-2883/367, 45-54=-2883/367, 18-54=-2888/366, 46-49=-367/454,

49-55=-285/628, 47-55=-169/770, 39-51=-661/462, 46-51=-542/420, 15-48=-11/447,

10-49=-447/133, 9-50=-399/164, 50-51=-321/129, 17-54=0/464, 12-55=-429/35,

34-35=-425/0, 28-29=-426/0, 27-29=0/1944, 29-30=-924/326, 32-35=-1341/366,

35-36=0/2097

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Ceiling dead load (10.0 psf) on member(s). 13-14, 18-19, 14-48, 45-48, 45-54, 18-54; Wall dead load (5.0psf) on member(s).19-27,

Conใimded bin plage 2



SEAL 036322

Job	Truss	Truss Type	Qty	Ply	Lot 19 Purfoy Place
					157394439
J0922-4569	A4SG	ROOF TRUSS	1	1	
					LJob Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:07 2023 Page 2 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-XoTp19uEBqMDiESqbl1fMyFXfZz0Zowym77d1yzWndU

NOTES-

- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 34-36, 32-34, 30-32, 28-30, 27-28
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 509 lb uplift at joint 42 and 234 lb uplift at joint 22.
- 9) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job Truss Truss Type Qty Lot 19 Purfoy Place Ply 157394440 J0922-4569 B1 COMMON Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:09 2023 Page 1 Fayetteville, NC - 28314, Comtech, Inc.

8-0-0

15-11-8

8-0-0

ID:X5az_D23vLwLuiTNLuG6bHyGfxb-TBbaSrwVjRdxxXcDii37SNL_DNmN1jMFERck6rzWndS 31-11-0 32-10₋0 0-11-0 23-11-8 8-0-0 7-11-8

Scale = 1:72.8 5x5 =

Structural wood sheathing directly applied or 5-3-8 oc purlins.

Fasten (2X) T and I braces to narrow edge of web with 10d

(0.131"x3") nails, 6in o.c., with 3in minimum end distance.

2x4 SPF No.2 - 6-12, 4-12

Rigid ceiling directly applied or 10-0-0 oc bracing.

Brace must cover 90% of web length.

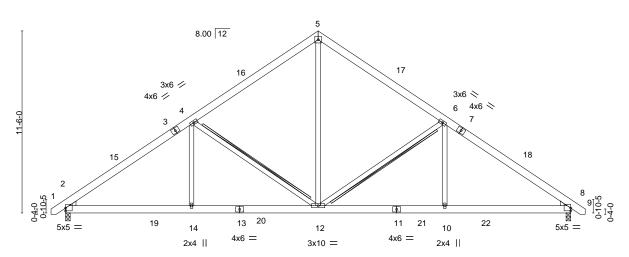


Plate Offsets (X,Y)--[2:0-0-0,0-2-2], [8:0-0-0,0-2-2] LOADING (psf) SPACING-2-0-0 CSI DEFL. in (loc) I/defl L/d **PLATES** GRIP -0.05 12-14 TCLL 20.0 Plate Grip DOL 1.15 TC 0.35 Vert(LL) >999 360 MT20 244/190 -0.10 12-14 TCDL 10.0 Lumber DOL 1.15 ВС 0.30 Vert(CT) >999 240 **BCLL** 0.0 Rep Stress Incr YES WB 0.71 Horz(CT) 0.04 8 n/a n/a Code IRC2015/TPI2014 **BCDL** 10.0 Wind(LL) 0.04 2-14 >999 240 FT = 20%Matrix-S Weight: 230 lb

BRACING-

WEBS

TOP CHORD

BOT CHORD

23-11-8

8-0-0

LUMBER-

2x6 SP No.1 TOP CHORD **BOT CHORD** 2x6 SP No.1 WEBS 2x4 SP No.2

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=356(LC 11)

Max Uplift 2=-233(LC 12), 8=-233(LC 13) Max Grav 2=1472(LC 19), 8=1472(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-2046/647, 4-5=-1483/609, 5-6=-1483/609, 6-8=-2047/647 **BOT CHORD** 2-14=-345/1807, 12-14=-345/1807, 10-12=-348/1560, 8-10=-348/1560 5-12=-342/1092, 6-12=-832/389, 6-10=0/444, 4-12=-831/389, 4-14=0/444 **WEBS**

7-11-8

7-11-8

7-11-8 7-11-8

-0-11₋0

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-8-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 2 and 233 lb uplift at joint 8.
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer
- 7) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



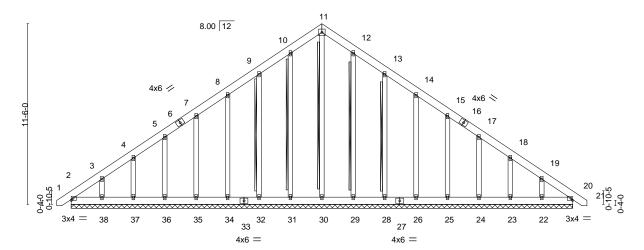
Job Truss Truss Type Qty Lot 19 Purfoy Place 157394441 J0922-4569 B1GE **GABLE** Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:11 2023 Page 1 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-PZjKtXylE2tfBrlbq76bXoQOABW1VmDYhl5rBjzWndQ

32-10-0 -0-11-0 0-11-0 15-11-8 15-11-8

> Scale = 1:73.3 5x5 =



LOADING (psf) SPACING-DEFL. **PLATES GRIP** 2-0-0 CSI (loc) I/def L/d Plate Grip DOL 244/190 **TCLL** 20.0 1.15 TC 0.06 Vert(LL) 0.00 20 n/r 120 MT20 TCDL 10.0 Lumber DOL 1.15 ВС 0.04 Vert(CT) 0.00 20 n/r 120 **BCLL** 0.0 Rep Stress Incr YES WB 0.17 Horz(CT) 0.01 20 n/a n/a **BCDL** 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 295 lb FT = 20%

32-10-0

LUMBER-

TOP CHORD 2x6 SP No 1 2x6 SP No.1 **BOT CHORD OTHERS** 2x4 SP No.2 BRACING-

TOP CHORD **BOT CHORD WEBS**

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

2x4 SPF No.2 - 11-30, 10-31, 9-32, 12-29, 13-28

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 31-11-0.

Max Horz 2=445(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 20, 30, 31, 29 except 2=-150(LC 8), 32=-148(LC 12), 34=-133(LC 12), 35=-131(LC 12), 36=-132(LC 12), 37=-132(LC 12), 38=-212(LC 12), 28=-153(LC 13), 26=-133(LC 13), 25=-131(LC 13),

24=-132(LC 13), 23=-131(LC 13), 22=-198(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 20, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22 except 2=257(LC 9), 30=285(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-489/359, 3-4=-339/289, 4-5=-278/252, 8-9=-200/281, 9-10=-288/346, 10-11=-334/381, 11-12=-334/381, 12-13=-288/321, 19-20=-383/255

BOT CHORD 2-38=-224/347, 37-38=-224/347, 36-37=-224/347, 35-36=-224/347, 34-35=-224/347,

32-34=-224/347, 31-32=-224/347, 30-31=-224/347, 29-30=-224/347, 28-29=-224/347,

26-28=-224/347, 25-26=-224/347, 24-25=-224/347, 23-24=-224/347, 22-23=-224/347,

20-22=-224/347

WEBS 11-30=-261/174

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 30, 31, 29 except (jt=lb) 2=150, 32=148, 34=133, 35=131, 36=132, 37=132, 38=212, 28=153, 26=133, 25=131, 24=132, 23=131, 22=198.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building

Contidesigner page 2



March 27,2023

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 19 Purfoy Place
	5.405	0.5.5	l.	_	l57394441
J0922-4569	B1GE	GABLE	1	1	
					Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:11 2023 Page 2 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-PZjKtXylE2tfBrlbq76bXoQOABW1VmDYhl5rBjzWndQ

NOTES-

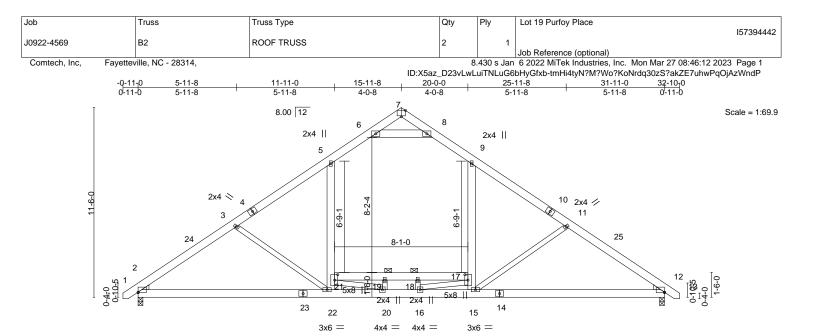
11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





		11-11-0 14-11-8	16-11-8 20-0-0	25-11-8	31-11-0	
	5-11-8	5-11-8 3-0-8	2-0-0 3-0-8	5-11-8	5-11-8	
Plate Offsets (X,Y)	[2:0-0-0,0-0-6], [7:0-3-0,Edge], [12:0-0	-0,0-0-6], [17:0-4-0,0-2-4]	, [21:0-4-0,0-2-4]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) I/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.57	Vert(LL) -0.16 1:	2-15 >999 360) MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.46	Vert(CT) -0.34 1:	2-15 >999 240)	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.53	Horz(CT) 0.06	12 n/a n/a	a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.17	2-22 >999 240	Weight: 266 I	b FT = 20%

BRACING-

JOINTS

TOP CHORD

BOT CHORD

LUMBER-

2x6 SP 2400F 2.0E *Except* TOP CHORD

1-4,10-13: 2x6 SP No.1

BOT CHORD 2x6 SP No.1

2x4 SP No.2 *Except* **WEBS**

9-15,5-22,6-8: 2x6 SP No.1

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. 2=0-3-8, 12=0-3-8 (size)

Max Horz 2=-356(LC 10)

Max Uplift 2=-14(LC 12), 12=-14(LC 13) Max Grav 2=1810(LC 20), 12=1810(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2606/327, 3-5=-2315/243, 5-6=-1746/318, 6-7=-96/1026, 7-8=-96/1025,

8-9=-1746/318, 9-11=-2314/243, 11-12=-2606/327

BOT CHORD 2-22=-125/2314, 20-22=0/2028, 16-20=0/2465, 15-16=0/1768, 12-15=-123/2048, 19-21=-713/0, 18-19=-713/0, 17-18=-713/0

WEBS 11-15=-619/356, 3-22=-619/356, 15-17=-60/585, 9-17=0/907, 21-22=-60/585,

5-21=0/907, 6-8=-3181/491, 16-18=-330/43, 19-20=-330/43, 20-21=-155/898,

16-17=-151/895

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 15-11-8, Exterior(2) 15-11-8 to 20-2-12, Interior(1) 20-2-12 to 32-8-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 4x6 MT20 unless otherwise indicated.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Ceiling dead load (10.0 psf) on member(s). 5-6, 8-9, 6-8; Wall dead load (5.0psf) on member(s).9-17, 5-21
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 19-21, 18-19, 17-18
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building
- 10) Attic room checked for L/360 deflection.



Structural wood sheathing directly applied or 4-9-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 18, 19

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Lot 19 Purfoy Place 157394443 J0922-4569 C₁ **ROOF SPECIAL** 21 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:13 2023 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:X5az_D23vLwLuiTNLuG6bHyGfxb-Lyr4HDz?mg7NQ9v_xY83cDVfr_7qzckq93ayFczWndO 17-9-0 0-11-0 11-11-8 5-11-8 6-0-0 5-9-8 Scale = 1:36.4 8x8 = 1 3.77 12 3x4 = 10 11 3 12.00 | 12 12 5x8 8 3x6 2x4 || 3x4 =

Plate Offsets (X,Y)--[2:0-1-15,0-1-8], [4:0-5-8,0-4-0], [5:0-6-4,0-0-0]

LOADING	(psf)	SPACING-	2-0-0	CSI.	
TCLL	20.0	Plate Grip DOL	1.15	TC 0.44	
TCDL	10.0	Lumber DOL	1.15	BC 0.27	
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.43	
BCDL	10.0	Code IRC2015/Ti	PI2014	Matrix-S	

5-11-8

Wind(LL) BRACING-

11-11-8

6-0-0

DEFL.

Vert(LL)

Vert(CT)

Horz(CT)

in (loc)

5-7

5-7

5-7

6

-0.05

-0.10

0.09

0.05

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

17-9-0

GRIP

244/190

FT = 20%

PLATES

Weight: 118 lb

MT20

Rigid ceiling directly applied or 10-0-0 oc bracing.

3-4-4

L/d

360

240

n/a

240

I/def

>999

>999

>999

n/a

LUMBER-

WEBS

2x6 SP No.1 *Except* TOP CHORD 4-6: 2x10 SP No.1

BOT CHORD 2x6 SP No.1 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 6=0-3-8

Max Horz 2=166(LC 11)

Max Uplift 2=-189(LC 8), 6=-108(LC 8) Max Grav 2=746(LC 1), 6=711(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1503/605, 3-4=-857/407, 4-5=-926/397, 5-6=-451/249 TOP CHORD

BOT CHORD 2-8=-396/1365, 7-8=-396/1365, 5-7=-104/761

WEBS 3-7=-650/307, 4-7=-85/424

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-6 to 3-9-6, Interior(1) 3-9-6 to 11-11-8, Exterior(2) 11-11-8 to 16-4-5, Interior(1) 16-4-5 to 17-7-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=189, 6=108.





Job Truss Truss Type Qty Lot 19 Purfoy Place 157394444 J0922-4569 C1A **ROOF SPECIAL** 2 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:14 2023 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:X5az_D23vLwLuiTNLuG6bHyGfxb-q8OSVZ_dXzFE2IUAVGfl9R2ncOS?i1K_NjJVn2zWndN 11-11-8 0-11-0

6-0-0

5-9-8

Structural wood sheathing directly applied or 5-5-10 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Scale = 1:36.4 8x8 = 1 3.77 12 3x4 = 10 11 3 12.00 | 12 12 5x8 8 3x6 2x4 || 3x4 =11-11-8 17-9-0 5-11-8 Plate Offsets (X,Y)--[2:0-1-15,0-1-8], [4:0-5-8,0-4-0], [5:0-6-4,0-0-0]

BRACING-

TOP CHORD

BOT CHORD

5-11-8

LOADING (p	osf)	SPACING-	2-6-0	CSI.		DEFL.	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL 20	0.0	Plate Grip DOL	1.15	TC	0.63	Vert(LL)	-0.06	5-7	>999	360	MT20	244/190
TCDL 10	0.0	Lumber DOL	1.15	BC	0.34	Vert(CT)	-0.12	5-7	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.53	Horz(CT)	0.12	6	n/a	n/a		
BCDL 10	0.0	Code IRC2015/TF	PI2014	Matrix	x-S	Wind(LL)	0.06	5-7	>999	240	Weight: 118 lb	FT = 20%

LUMBER-

WEBS

2x6 SP No.1 *Except* TOP CHORD

4-6: 2x10 SP No.1 **BOT CHORD** 2x6 SP No.1 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 6=0-3-8

Max Horz 2=207(LC 11)

Max Uplift 2=-236(LC 8), 6=-134(LC 8) Max Grav 2=933(LC 1), 6=889(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1879/756, 3-4=-1072/508, 4-5=-1157/496, 5-6=-564/311

BOT CHORD 2-8=-495/1707. 7-8=-495/1707. 5-7=-130/951 **WEBS** 3-8=0/295, 3-7=-813/383, 4-7=-106/530

NOTES-

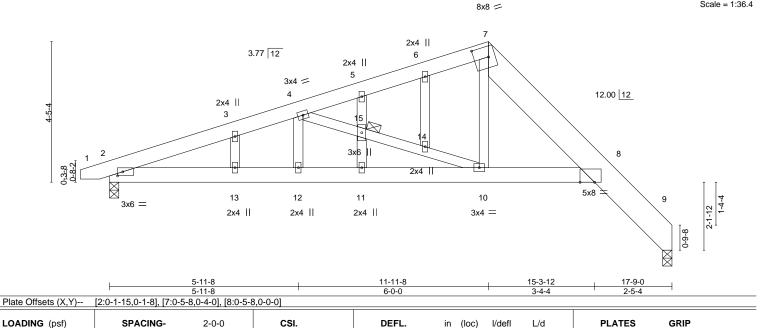
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-6 to 3-9-6, Interior(1) 3-9-6 to 11-11-8, Exterior(2) 11-11-8 to 16-4-5, Interior(1) 16-4-5 to 17-7-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=236, 6=134.





Job Truss Truss Type Qty Lot 19 Purfoy Place 157394445 J0922-4569 C1GE **GABLE** 2 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:15 2023 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:X5az_D23vLwLuiTNLuG6bHyGfxb-ILyriv?FIHN4fS3N3zAXheb?LopGRbH7cN32KVzWndM 0-11-0 11-11-8 5-11-8 6-0-0 5-9-8



Vert(LL)

Vert(CT)

Horz(CT)

Wind(LL)

BRACING-

JOINTS

TOP CHORD

BOT CHORD

-0.05

-0.10

0.09

0.06

>999

>999

>999

1 Brace at Jt(s): 15

n/a

11

9

11

8-10

360

240

n/a

240

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TCLL

TCDL

BCLL

BCDL

2x6 SP No.1 *Except* TOP CHORD

7-9: 2x10 SP No.1 2x6 SP No.1

BOT CHORD WEBS 2x4 SP No.2 **OTHERS** 2x4 SP No.2

20.0

10.0

10.0

0.0

REACTIONS. (size) 2=0-3-8, 9=0-3-8

Max Horz 2=-227(LC 13)

Max Uplift 2=-350(LC 8), 9=-214(LC 13) Max Grav 2=746(LC 1), 9=711(LC 1)

Plate Grip DOL

Rep Stress Incr

Code IRC2015/TPI2014

Lumber DOL

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1469/782, 3-4=-1392/828, 4-5=-855/460, 5-6=-820/493, 6-7=-797/512,

7-8=-931/488, 8-9=-451/278

BOT CHORD 2-13=-593/1328, 12-13=-593/1328, 11-12=-593/1328, 10-11=-593/1328, 8-10=-172/765

1.15

1.15

YES

TC

ВС

WB

Matrix-S

0.44

0.27

0.10

WEBS 4-15=-609/440, 14-15=-604/434, 10-14=-617/445, 7-10=-190/418

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=350. 9=214.



244/190

FT = 20%

MT20

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Weight: 126 lb

Scale = 1:36.4

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Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157394446 J0922-4569 C2 **ROOF SPECIAL**

Comtech, Inc, Fayetteville, NC - 28314, Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:16 2023 Page 1

ID:X5az_D23vLwLuiTNLuG6bHyGfxb-mXWDwE?u3bVxHceZcqhmEs7CpCBKA3NHr1ocsxzWndL 9-1-12 3-4-4 3-4-4 2-5-4

> Scale = 1:37.8 8x8 =

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

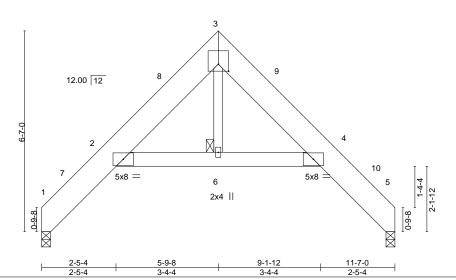


Plate Off	fsets (X,Y)	[3:0-4-0,0-2-12]										
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.26	Vert(LL)	-0.03	6	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.16	Vert(CT)	-0.06	2-6	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.09	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	k-S	Wind(LL)	0.03	2-6	>999	240	Weight: 89 lb	FT = 20%

BRACING-

JOINTS

TOP CHORD

BOT CHORD

1 Brace at Jt(s): 6

LUMBER-

TOP CHORD 2x10 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 5=0-3-8

Max Horz 1=-180(LC 8)

Max Uplift 1=-56(LC 12), 5=-56(LC 13) Max Grav 1=466(LC 1), 5=466(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-368/196, 2-3=-514/219, 3-4=-539/220, 4-5=-346/194

BOT CHORD 2-6=-36/499, 4-6=-36/499

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 5-9-8, Exterior(2) 5-9-8 to 10-2-5, Interior(1) 10-2-5 to 11-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.





Job Truss Truss Type Qty Lot 19 Purfoy Place 157394447 J0922-4569 C2GE **GABLE** Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:17 2023 Page 1

ID:X5az_D23vLwLuiTNLuG6bHyGfxb-Ej4b7a0WqudovmDIAOD?m3gNZbXXvVLQ3hY9ONzWndK 9-1-12 3-4-4 3-4-4 2-5-4

> Scale = 1:37.8 8x8 =

> > Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 9

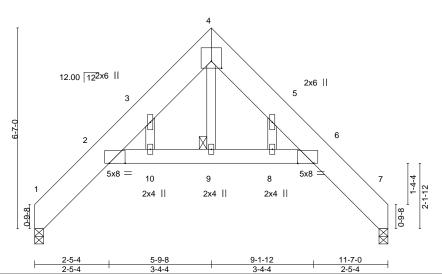


Plate Offsets (X,Y)-- [2:0-6-4,Edge], [4:0-4-0,0-2-12], [6:0-6-4,Edge]

LOADING	(psf)	SPACING- 2-0	0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.	.15	TC	0.26	Vert(LL)	-0.03	8	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1.	.15	BC	0.16	Vert(CT)	-0.06	8	>999	240		
BCLL	0.0 *	Rep Stress Incr Y	ΈS	WB	0.07	Horz(CT)	0.09	7	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI201	14	Matrix	x-S	Wind(LL)	0.04	10	>999	240	Weight: 92 lb	FT = 20%

BRACING-

JOINTS

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

2x10 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 **OTHERS** 2x4 SP No.2

(size) 1=0-3-8, 7=0-3-8

Max Horz 1=-225(LC 8)

Max Uplift 1=-132(LC 12), 7=-132(LC 13) Max Grav 1=466(LC 1), 7=466(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-391/243, 2-3=-483/175, 3-4=-565/274, 4-5=-565/274, 5-6=-539/190, 6-7=-346/195

BOT CHORD 2-10=-84/548, 9-10=-78/539, 8-9=-78/539, 6-8=-78/544

WEBS 4-9=-109/318

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=132, 7=132,





Job Truss Truss Type Qty Lot 19 Purfoy Place 157394448 J0922-4569 D1 COMMON 3 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:18 2023 Page 1 Fayetteville, NC - 28314, Comtech, Inc. ID:X5az_D23vLwLuiTNLuG6bHyGfxb-iwezLw18bClfWwoyk5kEJHDYX?oUeuRalLHjwpzWndJ 0-11-0 16-6-0

7-9-8

Scale = 1:38.1 5x5 =

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 9-7-8 oc bracing.

7-9-8

15-7-0

0-11-0

3 8.00 12 9 10 6 6x6 6x6 2x4

7-9-8 Plate Offsets (X,Y)--[2:0-0-0,0-2-6], [4:Edge,0-2-6]

LOADING (psf) SPACING-CSI. in (loc) I/defI L/d **PLATES** GRIP 240 TCLL 20.0 Plate Grip DOL 1.15 TC 0.31 Vert(LL) 0.07 4-6 >999 244/190 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.43 Vert(CT) -0.05 2-6 >999 240 BCLL 0.0 Rep Stress Incr YES WB 0.33 Horz(CT) 0.01 n/a n/a BCDL Code IRC2015/TPI2014 Weight: 95 lb FT = 20% 10.0 Matrix-S

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1 WEBS 2x4 SP No.2

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (size) 2=0-3-0, 4=0-3-0

Max Horz 2=182(LC 11)

Max Uplift 2=-192(LC 9), 4=-192(LC 8) Max Grav 2=685(LC 2), 4=685(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-807/920, 3-4=-807/917

BOT CHORD 2-6=-564/567, 4-6=-564/567

3-6=-674/457 **WEBS**

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 7-9-8, Exterior(2) 7-9-8 to 12-2-5, Interior(1) 12-2-5 to 16-4-1 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property danage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



157394449 J0922-4569 D1GE **GABLE** Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:19 2023 Page 1 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-A6CLYG2mMWtW84M8lpFTsUlmgPE9NP?jX?1GTGzWndl |-0-11-0 | 0-11-0 16-6-0 7-9-8 7-9-8 0-11-0 Scale = 1:36.0 5x5 =6 7 8.00 12 8 10 3x4 = 3x4 = 18 17 16 15 14 13 12 |-0-11-0 | 0-11-0 17-5-0 0-11-0 16-6-0 LOADING (psf) SPACING-2-0-0 CSI. DEFL. L/d **PLATES GRIP** (loc) I/defl 20.0 Plate Grip DOL -0.00 120 244/190 **TCLL** 1.15 TC 0.03 Vert(LL) 10 n/r MT20 TCDL 10.0 Lumber DOL 1.15 ВС 0.02 Vert(CT) 0.00 10 n/r 120 **BCLL** 0.0 Rep Stress Incr YES WB 0.06 Horz(CT) 0.00 10 n/a n/a BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Weight: 116 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Qty

Lot 19 Purfoy Place

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

Job

Truss

Truss Type

TOP CHORD 2x6 SP No.1 2x6 SP No.1

BOT CHORD **OTHERS** 2x4 SP No.2

REACTIONS. All bearings 15-7-0.

Max Horz 2=227(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 10 except 16=-121(LC 12), 17=-139(LC 12), 18=-168(LC 12),

14=-116(LC 13), 13=-141(LC 13), 12=-160(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 10, 15, 16, 17, 18, 14, 13, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10 except (jt=lb) 16=121, 17=139, 18=168, 14=116, 13=141, 12=160.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Lot 19 Purfoy Place 157394450 J0922-4569 D2 COMMON 3 Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:20 2023 Page 1 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-eImklc3O7p?NmDxKrWmiOilt5pWe6n0smfmp?izWndH 15-7-0 7-9-8 7-9-8 Scale = 1:37.3 5x8 = 2 8.00 12 6 - 0 - 53 0 - 10 - 0 \bigotimes 9 10 4 5x5 =5x5 = 2x4 || Plate Offsets (X,Y)--[1:0-0-0,0-1-15], [3:0-0-0,0-1-15] SPACING-GRIP LOADING (psf) CSI. DEFL. in (loc) I/defI L/d **PLATES** 240 TCLL 20.0 Plate Grip DOL 1.15 TC 0.31 Vert(LL) 0.07 1-4 >999 244/190 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.26 Vert(CT) -0.05 1-4 >999 240

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

0.01

3

n/a

n/a

Rigid ceiling directly applied or 9-6-4 oc bracing.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

LUMBER-

BCLL

BCDL

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

0.0

10.0

REACTIONS. (size) 1=0-3-8, 3=0-3-8 Max Horz 1=-176(LC 8)

Max Uplift 1=-183(LC 9), 3=-183(LC 8) Max Grav 1=638(LC 2), 3=638(LC 2)

Rep Stress Incr

Code IRC2015/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-804/923, 2-3=-804/923 TOP CHORD **BOT CHORD** 1-4=-580/569, 3-4=-580/569

WFBS 2-4=-669/452

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 7-9-8, Exterior(2) 7-9-8 to 12-2-5, Interior(1) 12-2-5 to 15-5-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB

Matrix-S

0.33

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=183, 3=183.



FT = 20%

Weight: 89 lb



Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157394451 J0922-4569 G1 COMMON 6 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:21 2023 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:X5az_D23vLwLuiTNLuG6bHyGfxb-6UK6zy30u77EONWWPEHxxvq3JDqdrHI0_JWNX8zWndG 21-11-0

7-0-0

7-0-0

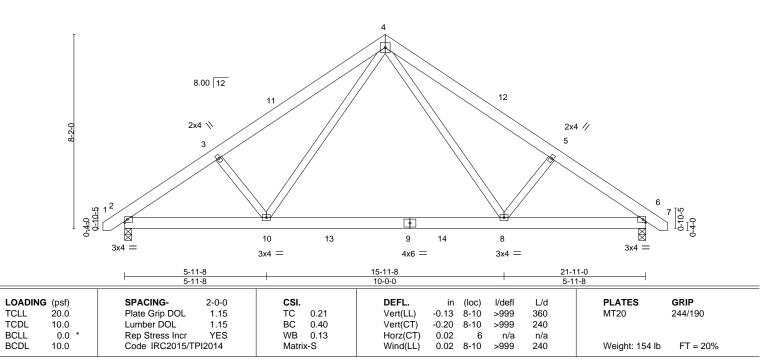
Scale: 1/4"=1' 5x5 =

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

3-11-8

22-10-0 0-11-0



BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

REACTIONS.

(size) 2=0-3-8, 6=0-3-8 Max Horz 2=187(LC 11)

Max Uplift 2=-57(LC 12), 6=-57(LC 13) Max Grav 2=960(LC 19), 6=960(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1386/278, 3-4=-1267/323, 4-5=-1268/323, 5-6=-1386/278

3-11-8

BOT CHORD 2-10=-158/1188. 8-10=0/718. 6-8=-163/1048

WEBS 4-8=-83/607, 5-8=-299/225, 4-10=-83/607, 3-10=-299/225

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-9-12, Interior(1) 3-9-12 to 10-11-8, Exterior(2) 10-11-8 to 15-4-5, Interior(1) 15-4-5 to 22-8-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.





818 Soundside Road Edenton, NC 27932

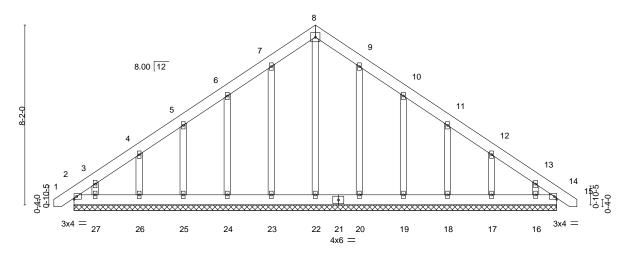
Job Truss Truss Type Qty Lot 19 Purfoy Place 157394452 J0922-4569 G1GE **GABLE** Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:23 2023 Page 1

Comtech, Inc, Fayetteville, NC - 28314, $ID: X5az_D23vLwLuiTNLuG6bHyGfxb-3tRsOe5HQkNydhgvXfKP0KwSe0c7JCtJSd?Uc1zWndEurustander (Control of the Control of the Control$

22-10-0 23-9-0 -0-11-0 0-11-0 10-11-8 10-11-8

5x5 =

Scale = 1:52.3



	-0-11-0 0-11-0	22-10-0 21-11-0		23-9-0 0-11-0
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.03 Vert(BC 0.02 Vert(WB 0.13 Horze Matrix-S	LL) -0.00 14 n/r 120 CT) -0.00 14 n/r 120	PLATES GRIP MT20 244/190 Weight: 178 lb FT = 20%

LUMBER-BRACING-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP No.1 **OTHERS**

2x4 SP No.2

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

All bearings 21-11-0.

REACTIONS. Max Horz 2=234(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 23, 24, 25, 26, 20, 19, 18, 17 except 27=-134(LC 12),

16=-120(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 14, 22, 23, 24, 25, 26, 27, 20, 19, 18, 17, 16

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-275/196

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 23, 24, 25, 26, 20, 19, 18, 17 except (jt=lb) 27=134, 16=120.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

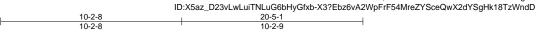
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157394453 J0922-4569 V1GE VALLEY

Comtech, Inc, Fayetteville, NC - 28314, Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:24 2023 Page 1



Scale = 1:65.0 3x4 =

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

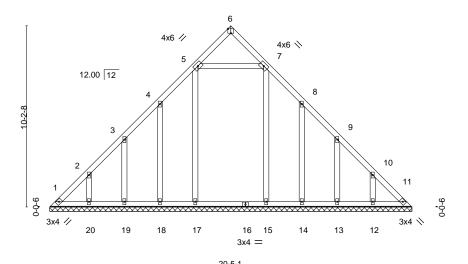


Plate Off	rsets (X,Y)	[6:0-2-0,Edge], [7:0-0-0,0	-0-0], [8:0-0-0,	0-0-0], [9:0-	0-0,0-0-0], [1	0:0-0-0,0-0-0]						
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.13	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.23	Horz(CT)	0.01	11	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 133 lb	FT = 20%

LUMBER-BRACING-

2x4 SP No.1 TOP CHORD BOT CHORD 2x4 SP No.1 **WEBS** 2x4 SP No.2 **OTHERS** 2x4 SP No.2

TOP CHORD **BOT CHORD**

REACTIONS. All bearings 20-5-1. Max Horz 1=-393(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 11, 17 except 1=-174(LC 10), 18=-200(LC 12), 19=-200(LC 12),

20=-217(LC 12), 14=-197(LC 13), 13=-201(LC 13), 12=-217(LC 13)

All reactions 250 lb or less at joint(s) 11, 18, 19, 20, 14, 13, 12 except 1=265(LC 9), 17=404(LC 19), Max Grav

15=340(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-399/324, 2-3=-293/256, 3-4=-245/253, 4-5=-282/345, 7-8=-282/296,

BOT CHORD 1-20=-146/267, 19-20=-146/267, 18-19=-146/267, 17-18=-146/267, 15-17=-146/267,

14-15=-146/267, 13-14=-146/267, 12-13=-146/267, 11-12=-146/267

WEBS 5-7=-236/292

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 17 except (jt=lb) 1=174, 18=200, 19=200, 20=217, 14=197, 13=201, 12=217.



March 27,2023



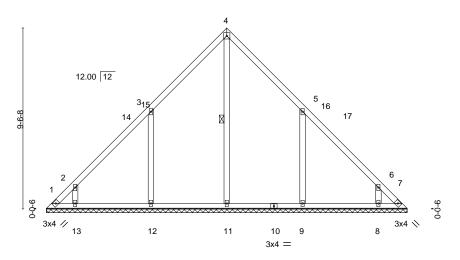
Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157394454 J0922-4569 V2 VALLEY

Fayetteville, NC - 28314, Comtech, Inc.

Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:26 2023 Page 1

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> Scale = 1:60.8 4x4 =



19-1-1

_Plate Off	sets (X,Y)	[5:0-0-0,0-0-0], [6:0-0-0,0	-0-0]									
LOADIN	\(\(\)	SPACING-	2-0-0	CSI.	0.24	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15 YES	BC WB	0.19	Vert(CT)	n/a	-	n/a	999		
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code IRC2015/TP		Matri	0.23	Horz(CT)	0.01	,	n/a	n/a	Waight: 00 lb	FT = 20%
DODL	10.0	Code IRC2015/1F	12014	Iviatri	x-3						Weight: 98 lb	F1 = 20%

BRACING-LUMBER-

TOP CHORD 2x4 SP No.1 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2x4 SP No.1 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. **OTHERS** 2x4 SP No.2 **WEBS** 1 Row at midpt 4-11

REACTIONS. All bearings 19-0-5.

(lb) -Max Horz 1=-293(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) except 1=-189(LC 10), 7=-140(LC 11), 12=-280(LC 12),

13=-203(LC 12), 9=-279(LC 13), 8=-204(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 7 except 1=255(LC 12), 11=447(LC 22), 12=523(LC 19),

13=304(LC 19), 9=523(LC 20), 8=304(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-357/311, 3-4=-273/261, 4-5=-273/261, 6-7=-355/311 **WEBS** 3-12=-511/429, 2-13=-395/368, 5-9=-511/429, 6-8=-395/368

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 9-6-8, Exterior(2) 9-6-8 to 13-11-5, Interior(1) 13-11-5 to 18-8-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 189 lb uplift at joint 1, 140 lb uplift at joint 7, 280 lb uplift at joint 12, 203 lb uplift at joint 13, 279 lb uplift at joint 9 and 204 lb uplift at joint 8.
- 7) N/A





Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157394455 J0922-4569 V3 VALLEY Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:27 2023 Page 1

Comtech, Inc, Fayetteville, NC - 28314, $ID: X5az_D23vLwLuiTNLuG6bHyGfxb-xehNE?8nTzuN6l_gmUOLAA44ndxWF_juNEzhlozWndA$

8-10-8 8-10-9

> Scale = 1:59.6 4x4 =

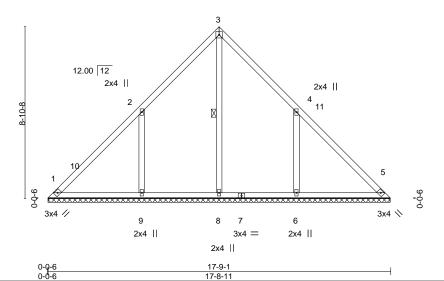


Plate Of	fsets (X,Y)	[4:0-0-0,0-0-0]										
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.28	Vert(LL)	n/a		n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.18	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.20	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2015/Ti	PI2014	Matri	x-S						Weight: 87 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 **OTHERS** 2x4 SP No.2 BRACING-

TOP CHORD **BOT CHORD WEBS**

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 3-8

REACTIONS. All bearings 17-8-5.

(lb) -Max Horz 1=-272(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=-328(LC 12), 6=-327(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=427(LC 22), 9=594(LC 19), 6=594(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-255/243, 3-4=-255/244 WEBS 2-9=-583/475, 4-6=-583/475

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-10-8, Interior(1) 4-10-8 to 8-10-8, Exterior(2) 8-10-8 to 13-3-5, Interior(1) 13-3-5 to 17-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 9=328, 6=327.
- 6) N/A





Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157394456 J0922-4569 V4 VALLEY

Comtech, Inc, Fayetteville, NC - 28314,

Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:28 2023 Page 1 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-PrFIRL9PEG0EjSZtJCvajOdGd1Hh_Sv2buiEHEzWnd9

15-8-9 7-10-5 7-10-4

> Scale = 1:50.6 4x4 =

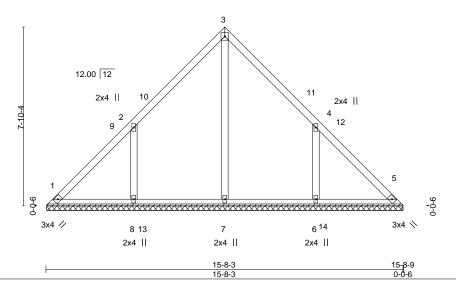


Plate Off	sets (X,Y)	[4:0-0-0,0-0-0]										
LOADIN	G (psf)	SPACING- 2-	-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1	.15	TC	0.21	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL 1	.15	BC	0.18	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr Y	′ES	WB	0.14	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI201	14	Matri	x-S						Weight: 75 lb	FT = 20%

LUMBER-

2x4 SP No.1 TOP CHORD BOT CHORD 2x4 SP No.1 **OTHERS** 2x4 SP No.2 BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 15-7-13.

Max Horz 1=240(LC 9) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-284(LC 12), 6=-284(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=425(LC 22), 8=512(LC 19), 6=512(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-8=-511/431, 4-6=-511/431 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 7-10-5, Exterior(2) 7-10-5 to 12-3-1, Interior(1) 12-3-1 to 15-4-5 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=284, 6=284,





Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157394457 J0922-4569 V5 VALLEY

Comtech, Inc, Fayetteville, NC - 28314, Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:29 2023 Page 1

ID:X5az_D23vLwLuiTNLuG6bHyGfxb-t1o7ehA1?a85Lc73tvQpGbASvRdTjwxBqYSophzWnd8 6-6-5 6-6-5 13-0-9 6-6-4

> Scale = 1:42.4 4x4 =

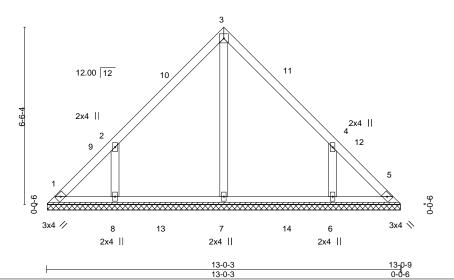


Plate Off	sets (X,Y)	[4:0-0-0,0-0-0]										
LOADIN	G (psf)	SPACING- 2	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.17	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.15	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.09	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI20	014	Matri	x-S						Weight: 60 lb	FT = 20%

LUMBER-

2x4 SP No.1 TOP CHORD BOT CHORD 2x4 SP No.1 **OTHERS** 2x4 SP No.2 BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-11-13.

Max Horz 1=197(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-245(LC 12), 6=-245(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=390(LC 19), 8=404(LC 19), 6=404(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-8=-451/401, 4-6=-451/402 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-6-5, Exterior(2) 6-6-5 to 10-11-1, Interior(1) 10-11-1 to 12-8-5 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=245, 6=245,





Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157394458 J0922-4569 V₆ VALLEY

Fayetteville, NC - 28314, Comtech, Inc.

Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:30 2023 Page 1

ID:X5az_D23vLwLuiTNLuG6bHyGfxb-MDMWs1AgmuGyzmiFRdy2opicDr_gSNEL3CBLL7zWnd7 5-2-5 5-2-5 5-2-4

> Scale = 1:34.2 4x4 =

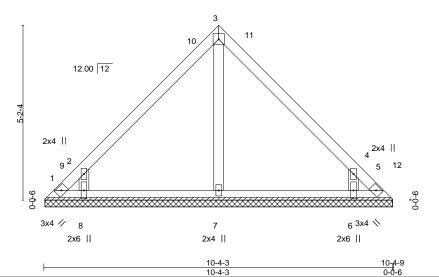


Plate Offsets (X,Y)--[4:0-0-0,0-0-0] SPACING-L/d **PLATES** LOADING (psf) 2-0-0 CSI. DEFL. in (loc) I/defI GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.20 Vert(LL) 999 244/190 n/a n/a MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.09 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.09 0.00 Horz(CT) n/a n/a Code IRC2015/TPI2014 FT = 20% **BCDL** 10.0 Matrix-S Weight: 44 lb

LUMBER-**BRACING-**

2x4 SP No.1 TOP CHORD TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2x4 SP No.1 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. **OTHERS** 2x4 SP No.2

REACTIONS. All bearings 10-3-13.

(lb) -Max Horz 1=-154(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) except 1=-178(LC 10), 5=-153(LC 11), 8=-276(LC 12), 6=-276(LC

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=416(LC 19), 6=416(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

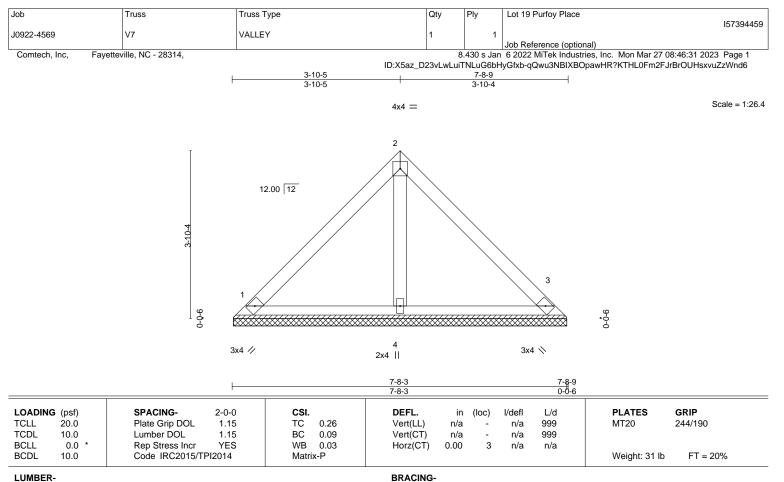
TOP CHORD 1-2=-257/213, 4-5=-258/213 WEBS 2-8=-529/498, 4-6=-530/498

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 5-2-5, Exterior(2) 5-2-5 to 9-7-1, Interior(1) 9-7-1 to 10-0-5 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 1, 153 lb uplift at joint 5, 276 lb uplift at joint 8 and 276 lb uplift at joint 6.







TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.2

REACTIONS.

1=7-7-13, 3=7-7-13, 4=7-7-13 (size) Max Horz 1=-112(LC 10) Max Uplift 1=-55(LC 13), 3=-55(LC 13)

Max Grav 1=171(LC 1), 3=171(LC 1), 4=219(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

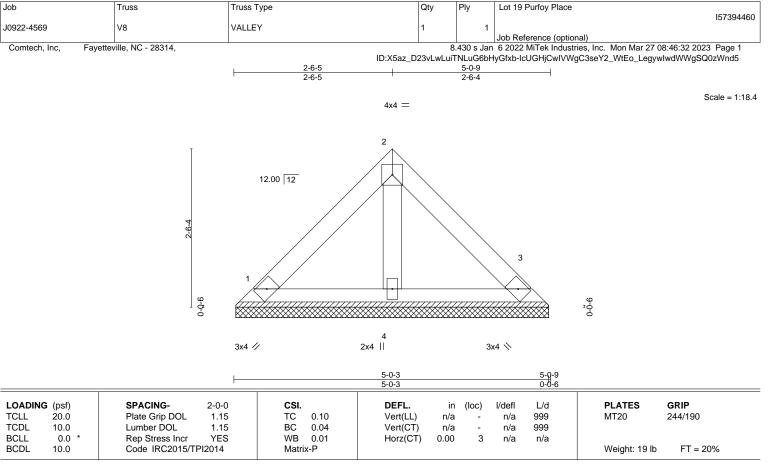
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 55 lb uplift at joint 1 and 55 lb uplift at joint 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 **OTHERS** 2x4 SP No.2

REACTIONS.

1=4-11-13, 3=4-11-13, 4=4-11-13 (size) Max Horz 1=-69(LC 8) Max Uplift 1=-34(LC 13), 3=-34(LC 13)

Max Grav 1=106(LC 1), 3=106(LC 1), 4=136(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 1 and 34 lb uplift at joint 3.



Structural wood sheathing directly applied or 5-0-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157394461 J0922-4569 V9 VALLEY Job Reference (optional) Comtech, Inc, Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Mar 27 08:46:33 2023 Page 1 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-mo2eU2DY3peXqDRq6IVIQRKAO20OflKnlAQ?ySzWnd4 1-2-5 1-2-5 2-4-9 1-2-4 Scale = 1:8.7 3x4 =2 12.00 12 3 9-0-0 9-0-0 3x4 // 3x4 N Plate Offsets (X,Y)--[2:0-2-0,Edge] SPACING-L/d **PLATES** GRIP LOADING (psf) 2-0-0 CSI. DEFL. in (loc) I/defI Plate Grip DOL TCLL 20.0 1.15 TC 0.02 Vert(LL) 999 MT20 244/190 n/a n/a TCDL 10.0 Lumber DOL 1.15 BC 0.02 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a Code IRC2015/TPI2014 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

BCDL

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

10.0

1=2-3-13, 3=2-3-13 (size)

Max Horz 1=-27(LC 8) Max Uplift 1=-10(LC 12), 3=-10(LC 12) Max Grav 1=67(LC 1), 3=67(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-P

- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 1 and 10 lb uplift at joint 3.



Weight: 7 lb

Structural wood sheathing directly applied or 2-4-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

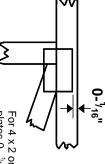


Symbols

PLATE LOCATION AND ORIENTATION



offsets are indicated. Center plate on joint unless x, y and fully embed teeth Apply plates to both sides of truss Dimensions are in ft-in-sixteenths



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

connector plates. required direction of slots in This symbol indicates the

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

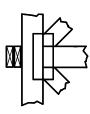
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



by text in the bracing section of the output. Use T or I bracing if indicated. ndicated by symbol shown and/or

BEARING



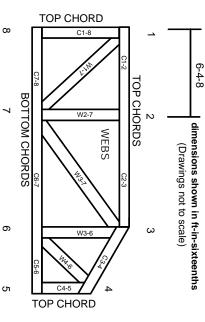
Min size shown is for crushing only number where bearings occur. reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

Industry Standards:

National Design Specification for Metal Building Component Safety Information. Installing & Bracing of Metal Plate Connected Wood Trusses. Guide to Good Practice for Handling Design Standard for Bracing. Plate Connected Wood Truss Construction.

DSB-89: ANSI/TPI1:

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

established by others. section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Ņ Truss bracing must be designed by an engineer. For bracing should be considered. may require bracing, or alternative Tor I wide truss spacing, individual lateral braces themselves
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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designer, erection supervisor, property owner and all other interested parties. Provide copies of this truss design to the building

4

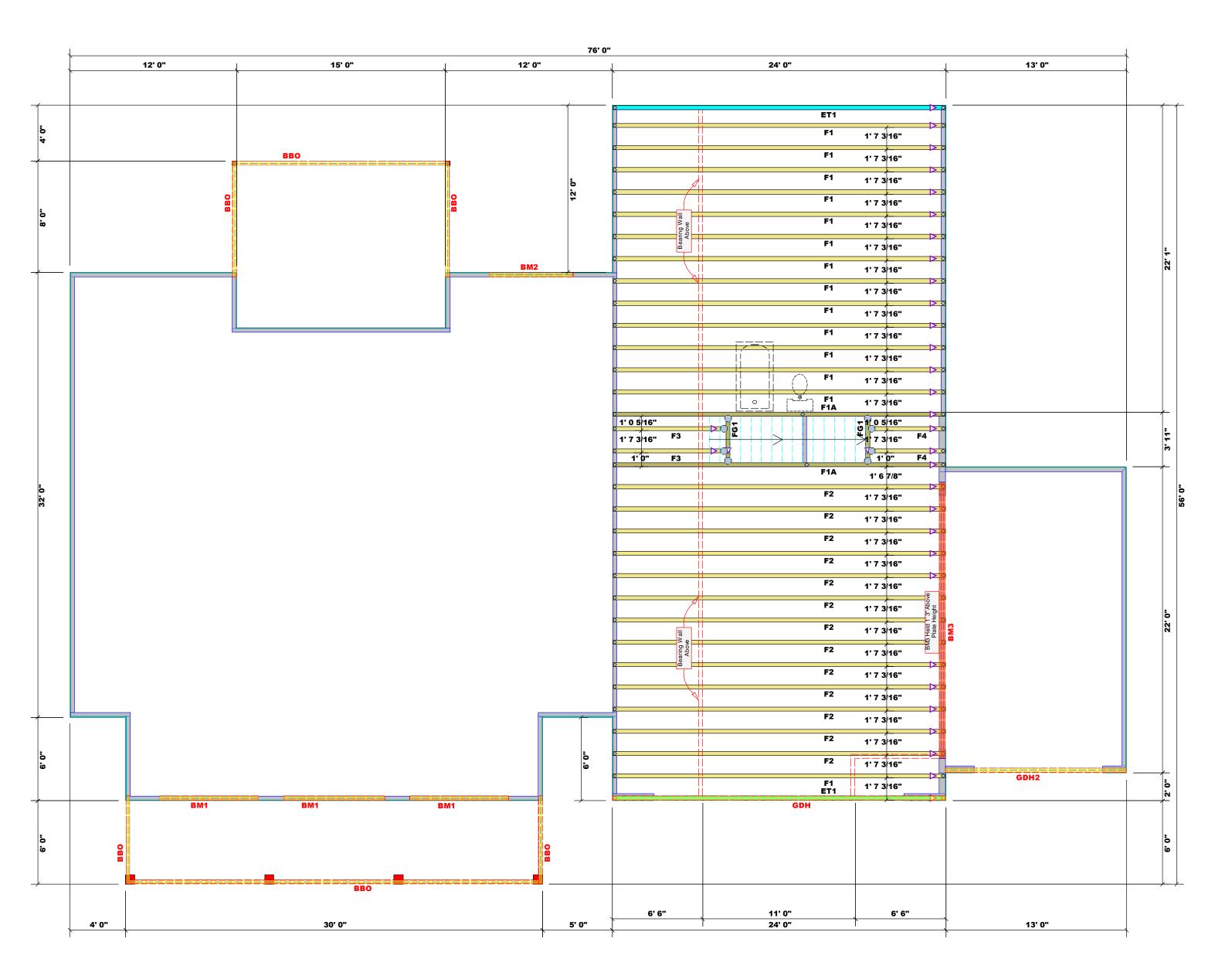
- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.

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- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication

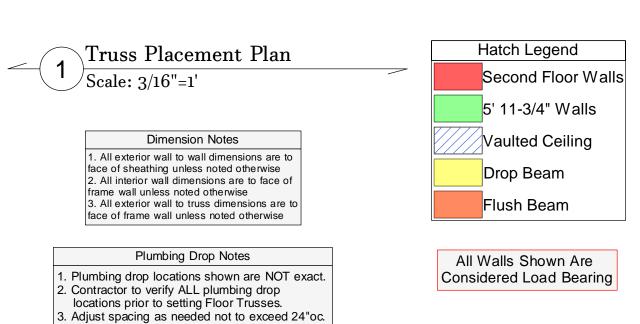
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- 9 Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection. responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.



		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	6
BM2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
BM3	20' 0"	1-3/4"x 18" LVL Kerto-S	3	3
GDH	24' 0"	1-3/4"x 16" LVL Kerto-S	2	2
GDH2	13' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2

	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
0	MSH422	USP	8	Varies	10d/3"	10d/3"



▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)

ROOF & FLOOR TRUSSES & BEAMS

> Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are leemed to comply with the prescriptive Code equirements. The contractor shall refer to the ttached Tables (derived from the prescriptive Code equirements) to determine the minimum foundatic ize and number of wood studs required to support eactions greater than 3000# but not greater than 5000#. A registered design professional shall be etained to design the support system for any eaction that exceeds those specified in the attach Tables. A registered design professional shall be etained to design the support system for all eactions that exceed 15000#.

Tanathan Landry

Jonathan Landry

LO	LOAD CHART FOR JACK STUDS												
(BASED ON TABLES R502.5(1) & (b))													
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER													
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER						
1700	1		2550	1		3400	1						
3400	2		5100	2		6800	2						
5100	3		7650	3		10200	3						
6800	4		10200	4		13600	4						
8500	5		12750	5		17000	5						
10200	6		15300	6									
11900	7												
13600	8												
15300	9												

LDER	Glover Design Build	CITY / CO.	CITY / CO. Fuquay Varina / Harnett	15300
NAME	NAME Lot 19 Purfoy Place	ADDRESS	438 Lambert Lane	9
z	Paxton / 3GRF, CP	MODEL	Floor	
IL DATE N/A	N/A	DATE REV . 03/29/23	03/29/23	
эте #		DRAWN BY	DRAWN BY Jonathan Landry	
# #	J0922-4570	SALES REP.	Lenny Norris	

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Client: Glover Design Build

Project:

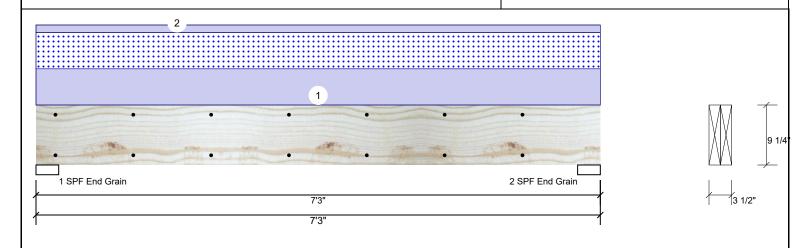
Address: 438 Lambert Lane

Fuquay Varina, NC 27526

Date: 3/29/2023 Input by: Jonathan Landry Job Name: Lot 19 Purfoy Place Project #: J0922-4570

Kerto-S LVL 2-Ply - PASSED 1.750" X 9.250" BM₁

Level: Level



Member Info	rmation			Rea	ctions UNP	ALLERN	NED Ib (Uplift)	
Туре:	Girder	Application:	Floor	Brg	Direction	Live	Dead	Snow
Plies:	2	Design Method:	ASD	1	Vertical	0	3158	2588
Moisture Condition	on: Dry	Building Code:	IBC/IRC 2015	2	Vertical	0	3158	2588
Deflection LL:	480	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal - II							
Temperature:	Temp <= 100°F							
				Bea	rings			
				Be	aring Length	Dir.	Cap. React D/L I	b Total Ld.

Ana	lysis	Resu	lts
-----	-------	------	-----

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9140 ft-lb	3'7 1/2"	14423 ft-lb	0.634 (63%)		L
Unbraced	9140 ft-lb	3'7 1/2"	9819 ft-lb	0.931 (93%)	D+S	L
Shear	4068 lb	1' 3/4"	7943 lb	0.512 (51%)	D+S	L
LL Defl inch	0.089 (L/919)	3'7 9/16"	0.170 (L/480)	0.522 (52%)	S	L
TL Defl inch	0.197 (L/414)	3'7 9/16"	0.340 (L/240)	0.580 (58%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.

Self Weight

- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

Reactions U	NPATTERNED	lb (Uplift)
-------------	------------	-------------

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	3158	2588	0	0
2	Vertical	0	3158	2588	0	0

Page 1 of 10

	Bearing	Length	Dir.	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
1	1 - SPF End Grain	3.500"	Vert	56%	3158 / 2588	5746	L	D+S
	2 - SPF End Grain	3.500"	Vert	56%	3158 / 2588	5746	L	D+S

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	714 PLF	0 PLF	714 PLF	0 PLF	0 PLF	A3
2	Uniform			Тор	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	V1GE

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- I. LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damagee Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

7 PI F

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







Project:

Address: 438 Lambert Lane

Fuquay Varina, NC 27526

Date: 3/29/2023 Input by: Jonathan Landry Job Name: Lot 19 Purfoy Place

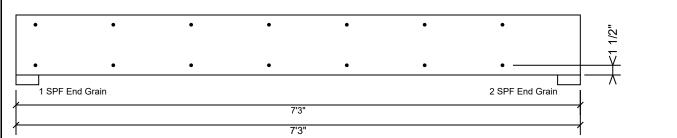
Project #: J0922-4570

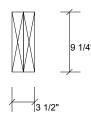
Kerto-S LVL BM1

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 2 of 10

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

rasterrain pries asing E rows	or roa box rians (. 120x3) at
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

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Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







Project:

Address: 438 Lambert Lane

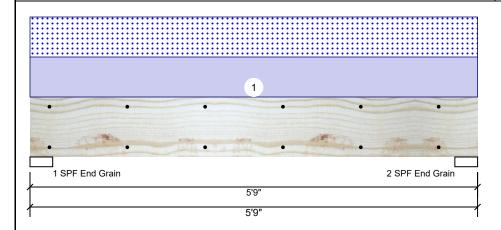
Fuquay Varina, NC 27526

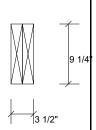
Date: 3/29/2023 Input by:

Jonathan Landry Job Name: Lot 19 Purfoy Place Project #: J0922-4570

Kerto-S LVL 2-Ply - PASSED 1.750" X 9.250" BM₂







Page 3 of 10

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	240
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015**

Load Sharing: No Deck:

Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1737	1716	0	0
2	Vertical	0	1737	1716	0	0

Bearings

Grain

I	Bearing	Length	Dir.	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
1	1 - SPF End Grain	3.500"	Vert	34%	1737 / 1716	3453	L	D+S
ı	2 - SPF End	3.500"	Vert	34%	1737 / 1716	3453	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4204 ft-lb	2'10 1/2"	14423 ft-lb	0.292 (29%)	D+S	L
Unbraced	4204 ft-lb	2'10 1/2"	11298 ft-lb	0.372 (37%)	D+S	L
Shear	2183 lb	1' 3/4"	7943 lb	0.275 (27%)	D+S	L
LL Defl inch	0.030 (L/2099)	2'10 1/2"	0.132 (L/480)	0.229 (23%)	S	L
TL Defl inch	0.061 (L/1043)	2'10 1/2"	0.265 (L/240)	0.230 (23%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	597 PLF	0 PLF	597 PLF	0 PLF	0 PLF	A3
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 11/3/2024 CSD DESIGN



Project:

5'9' 5'9'

Address: 438 Lambert Lane

Fuquay Varina, NC 27526

Input by: Jonathan Landry Job Name: Lot 19 Purfoy Place Project #: J0922-4570

1/2

3/29/2023

Level: Level

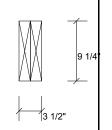
Date:

Kerto-S LVL BM₂

1.750" X 9.250"

2-Ply - PASSED

2 SPF End Grain



Page 4 of 10

Multi-Ply Analysis

1 SPF End Grain

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

rasteri ali piles asirig 2 rows	or 100 box 110113 (.120x3) at
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

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Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





BM₃

Client: Glover Design Build

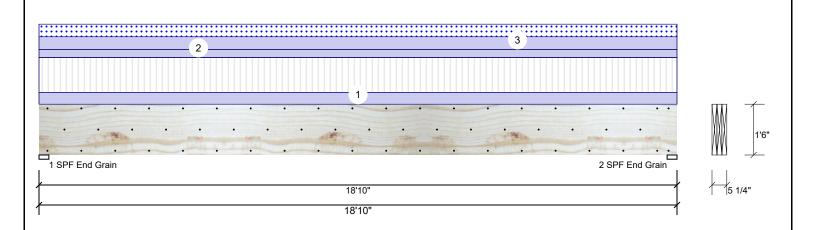
Project:

Address: 438 Lambert Lane

Fuquay Varina, NC 27526

Date: 3/29/2023 Input by: Jonathan Landry Job Name: Lot 19 Purfoy Place Project #: J0922-4570

Kerto-S LVL 1.750" X 18.000" 3-Ply - PASSED Level: Level



Member Information Application: Type: Floor Plies: 3 Design Method: ASD Moisture Condition: Dry **Building Code: IBC/IRC 2015** Deflection LL: 480 Load Sharing: Yes Deflection TL: 360 Not Checked Deck: Importance: Normal - II Ceiling: Gypsum 1/2" Temp <= 100°F Temperature: **Bearings**

Rea	Reactions UNPATTERNED lb (Uplift)										
Brg	Direction	Live	Dead	Snow	Wind	Const					
1	Vertical	4897	4727	1761	0	0					
2	Vertical	4897	4727	1761	0	0					

Page 5 of 10

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+0.75(L+S) 1 - SPF 3.500" Vert 4727 / 4993 9720 L End Grain 2 - SPF 3.500" 4727 / 4993 D+0.75(L+S) Vert 9720 L End Grain

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	43231 ft-lb	9'5"	67051 ft-lb	0.645 (64%)	D+L	L
Unbraced	43231 ft-lb	9'5"	43277 ft-lb	0.999 (100%)	D+L	L
Shear	7824 lb	17' 1/2"	20160 lb	0.388 (39%)	D+L	L
LL Defl inch	0.295 (L/748)	9'5 1/16"	0.460 (L/480)	0.642 (64%)	0.75(L+S)	L
TL Defl inch	0.574 (L/384)	9'5 1/16"	0.613 (L/360)	0.937 (94%)	D+0.75(L+S)	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 4'5 1/16" o.c.

7 Lateral slenderness ratio based on single bly width

/ Lateral	sienuenness ratio baseu on									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	174 PLF	520 PLF	0 PLF	0 PLF	0 PLF	F1
2	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
3	Uniform			Тор	187 PLF	0 PLF	187 PLF	0 PLF	0 PLF	C1
	Self Weight				21 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 11/3/2024



BM₃

Kerto-S LVL

Client: Glover Design Build

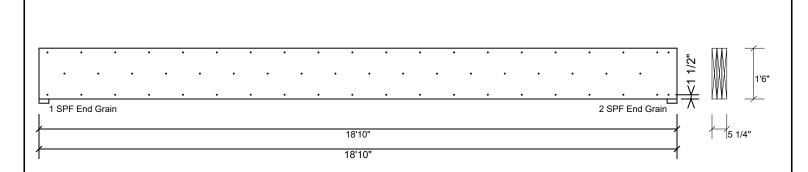
Project:

Address: 438 Lambert Lane

3/29/2023 Input by: Jonathan Landry Job Name: Lot 19 Purfoy Place J0922-4570

Fuquay Varina, NC 27526 Project #: 3-Ply - PASSED 1.750" X 18.000"

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed 6".

Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1 00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information
 requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
- Danaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
 - This design is valid until 11/3/2024

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 6 of 10





Project:

Address: 438 Lambert Lane

Fuquay Varina, NC 27526

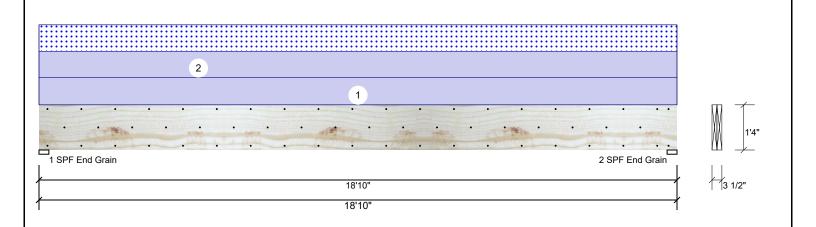
Date: 3/29/2023 Input by: Jonathan Landry

Job Name: Lot 19 Purfoy Place Project #: J0922-4570

Kerto-S LVL 2-Ply - PASSED 1.750" X 16.000" **GDH**

Level: Level

Reactions UNPATTERNED Ib (Uplift)



Application: Live Type: Floor Brg Direction Plies: 2 Design Method: ASD 0 Vertical 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 Vertical 0 Deflection LL: 480 Load Sharing: No Deflection TL: 240 Deck: Not Checked Importance: Normal - II Temp <= 100°F Temperature:

Bearings											
Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.				
1 - SPF End Grain	3.500"	Vert	34%	2349 / 1102	3451	L	D+S				
2 - SPF End Grain	3.500"	Vert	34%	2349 / 1102	3451	L	D+S				

Dead

2349

2349

Snow

1102

1102

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	15501 ft-lb	9'5"	39750 ft-lb	0.390 (39%)	D+S	L
Unbraced	15501 ft-lb	9'5"	15563 ft-lb	0.996 (100%)	D+S	L
Shear	2872 lb	1'7 1/2"	13739 lb	0.209 (21%)	D+S	L
LL Defl inch	0.136 (L/1619)	9'5 1/16"	0.460 (L/480)	0.296 (30%)	S	L
TL Defl inch	0.427 (L/517)	9'5 1/16"	0.920 (L/240)	0.464 (46%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 7'7 3/8" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

o Editoral Sichlethiess ratio based on single pry width.											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall	
2	Uniform			Тор	117 PLF	0 PLF	117 PLF	0 PLF	0 PLF	C2GE	
	Self Weight				12 PLF						

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood

301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

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Page 7 of 10

Wind

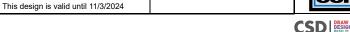
0

0

Const

0

0





GDH

Kerto-S LVL

Client: Glover Design Build

Project:

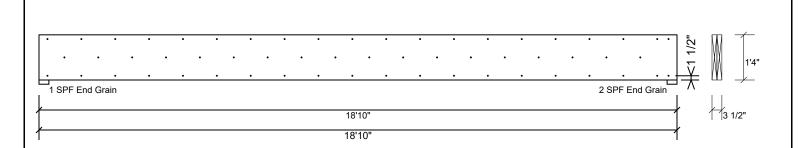
Address: 438 Lambert Lane

Fuquay Varina, NC 27526

Date: 3/29/2023 Input by: Jonathan Landry Job Name: Lot 19 Purfoy Place Project #: J0922-4570

2-Ply - PASSED 1.750" X 16.000"

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

		,	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	245.6 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 8 of 10

This design is valid until 11/3/2024 CSD DESIGN



Project:

Address: 438 Lambert Lane

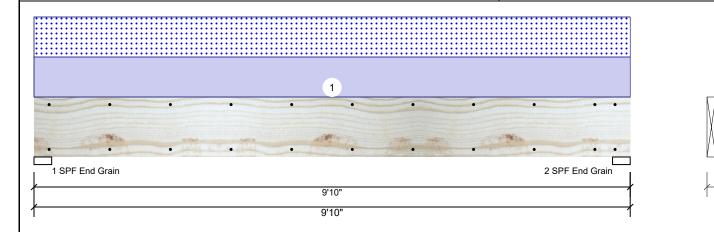
Fuquay Varina, NC 27526

Date: 3/29/2023 Input by:

Jonathan Landry Job Name: Lot 19 Purfoy Place Project #: J0922-4570

Kerto-S LVL 2-Ply - PASSED 1.750" X 11.875" GDH2

Level: Level



Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 240 Importance:

Normal - II Temp <= 100°F Temperature:

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015**

Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1225	1180	0	0
2	Vertical	0	1225	1180	0	0

Page 9 of 10

11 7/8'

Bearings

Grain

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 2405 L D+S 1 - SPF 3.500" Vert 1225 / 1180 End Grain 1225 / 1180 2405 L D+S 2 - SPF 3.500" Vert 23% End

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5375 ft-lb	4'11"	22897 ft-lb	0.235 (23%)	D+S	L
Unbraced	5375 ft-lb	4'11"	9857 ft-lb	0.545 (55%)	D+S	L
Shear	1788 lb	1'3 3/8"	10197 lb	0.175 (18%)	D+S	L
LL Defl inch	0.050 (L/2249)	4'11"	0.234 (L/480)	0.213 (21%)	S	L
TL Defl inch	0.102 (L/1104)	4'11"	0.469 (L/240)	0.217 (22%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	240 PLF	0 PLF	240 PLF	0 PLF	0 PLF	G1
	Self Weight				9 PLF					

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

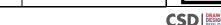
This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





isDesign

Client: Glover Design Build

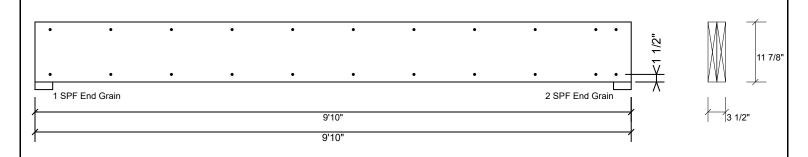
Project: Address:

438 Lambert Lane Fuquay Varina, NC 27526 Date: 3/29/2023 Input by: Jonathan Landry

Job Name: Lot 19 Purfoy Place Project #: J0922-4570

Kerto-S LVL 2-Ply - PASSED GDH₂ 1.750" X 11.875"

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c., Maximum end distance not to exceed 6".

, ,		`	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	163.7 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 10 of 10

CSD DESIGN



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0922-4570 Lot 19 Purfoy Place

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I57456101 thru I57456107

My license renewal date for the state of North Carolina is December 31, 2023.

North Carolina COA: C-0844



March 29,2023

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	Lot 19 Purfoy Place	
					I57456101	
J0922-4570	ET1	GABLE	2	1		
					Job Reference (optional)	

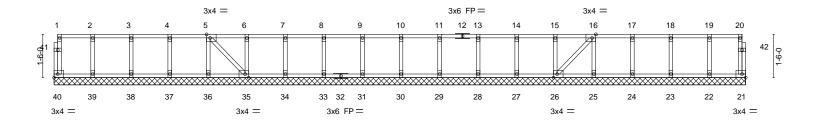
Comtech, Inc, Fayetteville, NC - 28314,

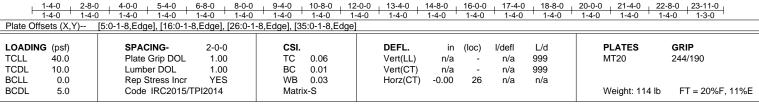
8.430 s Jan 6 2022 MiTek Industries, Inc. Wed Mar 29 09:16:12 2023 Page 1 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-LfEVb3WtSdtMvTFrCI_5xtMTJva_JMiH7jaSt5zW7?H

Structural wood sheathing directly applied or 6-0-0 oc purlins,

0-<u>1</u>-8

0-<u>1</u>1-8 Scale = 1:39.8





TOP CHORD

LUMBER-**BRACING-**

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

except end verticals. **WEBS** 2x4 SP No.3(flat) **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. **OTHERS** 2x4 SP No.3(flat)

REACTIONS. All bearings 23-11-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 40, 21, 39, 38, 37, 36, 35, 34, 33, 31, 30, 29, 28, 27, 26, 25, 24, 23, 22

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





Job Truss Truss Type Qty Lot 19 Purfoy Place 157456102 J0922-4570 F1 **FLOOR** 14 Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.430 s Jan 6 2022 MiTek Industries, Inc. Wed Mar 29 09:16:13 2023 Page 1 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-prnuoPXVDw?DXdp1mTVKT4vYwJm12fgQMNJ?PXzW7?G

Structural wood sheathing directly applied or 6-0-0 oc purlins,

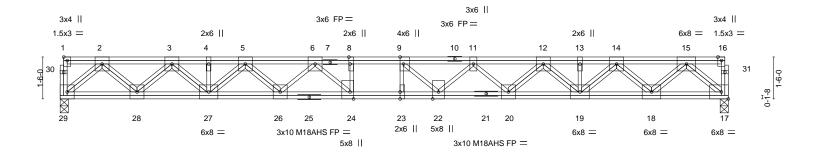
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

0-1-8 H 1-3-0

1-8-0

0-1-8 Scale = 1:41.2



23-11-0 Plate Offsets (X,Y)--[1:Edge,0-1-8], [8:0-3-0,Edge], [9:0-3-0,Edge], [23:0-3-0,0-0-0], [24:0-3-0,Edge] LOADING (psf) SPACINGin (loc) I/defl L/d **PLATES GRIP** -0.38 22-23 TCLL 40.0 Plate Grip DOL 1.00 TC 0.39 Vert(LL) >756 480 MT20 244/190 TCDL 10.0 Lumber DOL 1.00 BC 0.66 Vert(CT) -0.52 22-23 >550 360 M18AHS 186/179 **BCLL** 0.0 Rep Stress Incr NO WB 0.69 Horz(CT) 0.08 17 n/a n/a BCDL Code IRC2015/TPI2014 5.0 FT = 20%F, 11%E Matrix-S Weight: 222 lb

TOP CHORD

BOT CHORD

LUMBER-**BRACING-**

2x4 SP 2400F 2.0E(flat) TOP CHORD BOT CHORD 2x4 SP 2400F 2.0E(flat)

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 29=0-3-8, 17=0-3-8

Max Grav 29=1393(LC 1), 17=1979(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2650/0, 3-4=-4820/0, 4-5=-4820/0, 5-6=-6418/0, 6-8=-8034/0, 8-9=-8034/0,

9-11=-8559/0, 11-12=-8790/0, 12-13=-7231/0, 13-14=-7231/0, 14-15=-3872/0

BOT CHORD 28-29=0/1480, 27-28=0/3803, 26-27=0/5761, 24-26=0/7207, 23-24=0/8034, 22-23=0/8034,

20-22=0/8839, 19-20=0/8751, 18-19=0/5601, 17-18=0/2120

WFBS 2-29=-1997/0, 2-28=0/1655, 3-28=-1631/0, 3-27=0/1404, 15-17=-2862/0, 15-18=0/2478,

14-18=-2445/0, 14-19=0/2251, 12-19=-2099/0, 12-20=-133/289, 5-27=-1300/0,

5-26=0/948, 6-26=-1098/0, 11-20=-269/119, 11-22=-629/217, 6-24=0/1446, 8-24=-593/0,

9-22=-214/1061, 9-23=-521/33

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 6x6 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1300 lb down at 17-4-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 17-29=-8. 1-16=-80 Concentrated Loads (lb) Vert: 12=-1300(F)



March 29,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157456103 J0922-4570 F1A FLOOR GIRDER 2 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Wed Mar 29 09:16:16 2023 Page 1

Fayetteville, NC - 28314, Comtech, Inc.

ID:X5az_D23vLwLuiTNLuG6bHyGfxb-DQT0QRZNWrOoO5YcRb315jXyFWmaFyzt2LYg0szW7?D

Structural wood sheathing directly applied or 5-7-10 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 28-30,27-28,26-27,25-26.

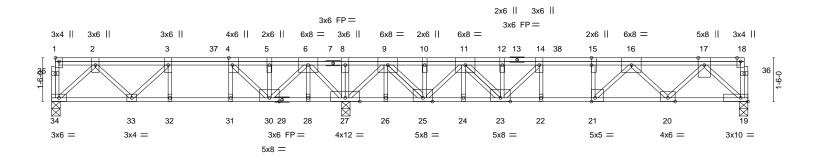
except end verticals.

0-1-8

H|-1-3-0

1-11-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8 | 1-2-8

0-1-8 Scale = 1:39.6



<u> </u>		9-11-8			10 ₁ 1-0	12-9-0				23-11-0		
		9-11-8			0-1'-8	2-8-0	'			11-2-0)	ı ı
Plate Offs	sets (X,Y)	[1:Edge,0-1-8], [4:0-3-0,E	Edge], [15:0-3	-0,0-0-0], [21:	0-1-8,Ed	ge]						
LOADING	G (psf)	SPACING-	1-7-3	CSI.			DEFL.	in (loc)	I/defI	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.89	\	/ert(LL)	-0.14 22-23	>999	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.73	\	/ert(CT)	-0.19 22-23	>862	360		
BCLL	0.0	Rep Stress Incr	NO	WB	0.91	H	Horz(CT)	0.03 19	n/a	n/a		
BCDL	5.0	Code IRC2015/TI	PI2014	Matrix	<-S						Weight: 174 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1(flat) **BOT CHORD** 2x4 SP 2400F 2.0E(flat)

2x4 SP No.3(flat) **WEBS**

(size) 34=0-3-8, 19=0-3-8, 27=0-3-8

Max Grav 34=521(LC 3), 19=1302(LC 7), 27=3526(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-795/0, 3-4=-1043/0, 4-5=0/1471, 5-6=0/1471, 6-8=0/3631, 8-9=0/3631,

9-10=-553/144, 10-11=-561/141, 11-12=-4047/0, 12-14=-4086/0, 14-15=-4606/0, 15-16=-4606/0, 16-17=-2322/0

BOT CHORD 33-34=0/512, 32-33=0/1043, 31-32=0/1043, 30-31=0/1043, 28-30=-2487/0,

27-28=-2487/0, 26-27=-1535/0, 25-26=-1533/0, 24-25=0/2411, 23-24=0/2412,

22-23=0/4606, 21-22=0/4606, 20-21=0/3324, 19-20=0/1329

WEBS 8-27=-287/0, 2-34=-706/0, 2-33=-16/411, 3-33=-350/127, 6-27=-1769/0, 6-30=0/1660,

5-30=0/461, 4-30=-2274/0, 9-25=0/2771, 11-25=-2548/0, 11-23=0/2403, 12-23=-1155/0,

14-23=-854/149, 17-19=-1836/0, 17-20=0/1440, 16-20=-1452/0, 16-21=0/1910,

15-21=-1218/0, 9-27=-2837/0

NOTES-

REACTIONS.

- 1) Unbalanced floor live loads have been considered for this design.
- All plates are 1.5x3 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1280 lb down at 17-4-12, and 976 lb down at 5-7-0, and 976 lb down at 15-7-8 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 19-34=-8, 1-18=-80

Concentrated Loads (lb)

Vert: 12=-912(B) 37=-912(B) 38=-1280(F)



March 29,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property danage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Lot 19 Purfoy Place
10000 4570	F0	FLOOR	40		157456104
J0922-4570	F2	FLOOR	13	1	Joh Reference (entional)
					Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.430 s Jan 6 2022 MiTek Industries, Inc. Wed Mar 29 09:16:18 2023 Page 1 ID:X5az_D23vLwLuiTNLuG6bHyGfxb-9pbmr6be2TeWdOi?Z05VA8cNEKRxjtx9Vf1m5kzW7?B

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



2-5-4

0-1-8 Scale = 1:41.2

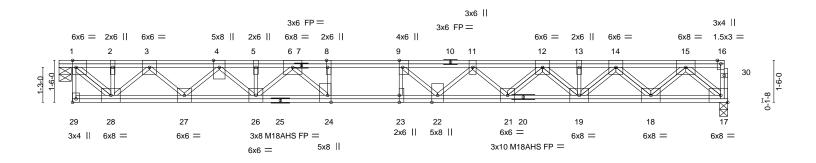


Plate Offsets (X,Y)--[1:0-1-8,Edge], [8:0-3-0,Edge], [9:0-3-0,Edge], [23:0-3-0,0-0-0], [24:0-3-0,Edge], [29:Edge,0-1-8] LOADING (psf) SPACING-(loc) I/defl L/d **PLATES GRIP** -0.40 22-23 TCLL 40.0 Plate Grip DOL 1.00 TC 0.54 Vert(LL) >706 480 MT20 244/190 TCDL 10.0 Lumber DOL 1.00 BC 0.80 Vert(CT) -0.54 22-23 >514 360 M18AHS 186/179 **BCLL** 0.0 Rep Stress Incr NO WB 0.88 0.02 Horz(CT) 17 n/a n/a Code IRC2015/TPI2014 Weight: 207 lb **BCDL** 5.0 FT = 20%F, 11%E Matrix-S

TOP CHORD

BOT CHORD

LUMBER-**BRACING-**

TOP CHORD 2x4 SP 2400F 2.0E(flat) BOT CHORD 2x4 SP 2400F 2.0E(flat)

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 1=0-5-4, 17=0-3-8

Max Grav 1=1386(LC 1), 17=1954(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1423/0, 2-3=-1428/0, 3-4=-3776/0, 4-5=-5637/0, 5-6=-5637/0, 6-8=-7637/0,

8-9=-7637/0, 9-11=-8344/0, 11-12=-8642/0, 12-13=-7131/0, 13-14=-7131/0,

14-15=-3820/0

BOT CHORD $27 - 28 = 0/2729,\ 26 - 27 = 0/4768,\ 24 - 26 = 0/6493,\ 23 - 24 = 0/7637,\ 22 - 23 = 0/7637,\ 21 - 22 = 0/8688,\ 24 - 26 = 0/6493,\ 23 - 24 = 0/7637,\ 24 - 23 = 0/7637,\ 21 - 22 = 0/8688,\ 24 - 26 = 0$

19-21=0/8616. 18-19=0/5525. 17-18=0/2093

WEBS $1-28=0/1956,\ 3-28=-1767/0,\ 3-27=0/1481,\ 15-17=-2826/0,\ 15-18=0/2443,\ 14-18=-2411/0,$ 14-19=0/2219, 12-19=-2049/0, 12-21=-111/267, 11-22=-683/190, 9-22=-152/1289, 9-23=-636/0, 4-27=-1402/0, 4-26=0/1201, 6-26=-1181/0, 6-24=0/1847, 8-24=-799/0

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 6) CAUTION, Do not erect truss backwards.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1300 lb down at 17-4-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 17-29=-8, 1-16=-80

Concentrated Loads (lb)

Vert: 12=-1300(F)



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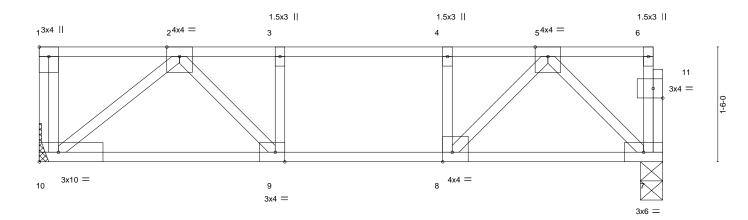


Job Truss Truss Type Qty Lot 19 Purfoy Place 157456105 J0922-4570 F3 **FLOOR** 2 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Wed Mar 29 09:16:19 2023 Page 1 Fayetteville, NC - 28314, Comtech, Inc. ID:X5az_D23vLwLuiTNLuG6bHyGfxb-e?993SbGpmmNFYHB7jckjL9TUjiQSQ1JkJmKdBzW7?A

2-0-12

Scale = 1:15.1

0-1-8



8-1-12 Plate Offsets (X,Y)--[1:Edge,0-1-8], [8:0-1-8,Edge], [9:0-1-8,Edge], [11:0-1-8,0-1-8] LOADING (psf) SPACING-DEFL. in (loc) I/defl L/d **PLATES** GRIP TCLL 40.0 Plate Grip DOL 1.00 TC 0.89 Vert(LL) -0.14 9-10 >679 480 244/190 MT20 TCDL 10.0 Lumber DOL 1.00 BC 0.92 Vert(CT) -0.19 9-10 >498 360 **BCLL** 0.0 Rep Stress Incr NO WB 0.50 Horz(CT) 0.01 n/a n/a Code IRC2015/TPI2014 FT = 20%F, 11%E **BCDL** 5.0 Matrix-S Weight: 46 lb

TOP CHORD

LUMBER-**BRACING-**

2x4 SP 2400F 2.0E(flat) TOP CHORD BOT CHORD 2x4 SP No.1(flat)

1-7-0

1-3-0

WEBS 2x4 SP No.3(flat) **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 10=Mechanical, 7=0-3-8 Max Grav 10=1366(LC 1), 7=623(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

2-3=-1253/0, 3-4=-1253/0, 4-5=-1253/0

BOT CHORD 9-10=0/1593, 8-9=0/1253, 7-8=0/571

2-10=-2045/0, 2-9=-565/59, 5-7=-802/0, 5-8=0/1016, 4-8=-566/0, 3-9=-43/289 WEBS

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1300 lb down at 1-10-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 7-10=-8, 1-6=-80

Concentrated Loads (lb) Vert: 2=-1300(F)



Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

March 29,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157456106 J0922-4570 F4 **FLOOR** 2 Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Wed Mar 29 09:16:20 2023 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:X5az_D23vLwLuiTNLuG6bHyGfxb-6BiXGocua4uEtisNgR7zFZipO7lLBzuSzzWt9dzW7?9 0-1-8 2-2-4 1-3-0 \vdash Scale = 1:11.6 3x4 = 11.5x3 || 3x4 = 4 3x4 II 2 9 3x4 =1.5x3 || 6 1.5x3 || 7 5 3x6 = 3x6 =Plate Offsets (X,Y)--[2:0-1-8,Edge], [3:0-1-8,Edge], [9:0-1-8,0-1-8] SPACING-**PLATES** GRIP LOADING (psf) CSI. DEFL. in (loc) I/defI L/d 244/190 **TCLL** 40.0 Plate Grip DOL 1.00 TC 0.18 Vert(LL) -0.01 6 >999 480 MT20 TCDL 10.0 Lumber DOL 1.00 BC 0.10 Vert(CT) -0.01 6 >999 360 **BCLL** 0.0 Rep Stress Incr YES WB 0.08 Horz(CT) 0.00 5 n/a n/a **BCDL** Code IRC2015/TPI2014 FT = 20%F, 11%E 5.0 Weight: 32 lb Matrix-S

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

2x4 SP No.1(flat) TOP CHORD **BOT CHORD** 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)

> (size) 8=0-3-8, 5=Mechanical Max Grav 8=279(LC 1), 5=285(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-8=-336/0, 3-5=-339/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.



Structural wood sheathing directly applied or 5-5-4 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

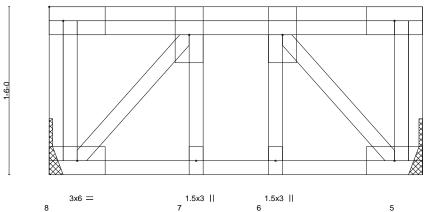


Job Truss Truss Type Qty Ply Lot 19 Purfoy Place 157456107 J0922-4570 FG1 FLOOR GIRDER 2 | Job Reference (optional) 8.430 s Jan 6 2022 MiTek Industries, Inc. Wed Mar 29 09:16:20 2023 Page 1

Comtech, Inc, Fayetteville, NC - 28314, ID:X5az_D23vLwLuiTNLuG6bHyGfxb-6BiXGocua4uEtisNgR7zFZik?7GeBv9SzzWt9dzW7?9

Scale = 1:10.3





3x6 =

except end verticals.

Structural wood sheathing directly applied or 3-4-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

ı	ı	3-4-0	1
		3-4-0	1

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.46	Vert(LL) -0.01 7 >999 480	MT20 244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.21	Vert(CT) -0.01 7 >999 360	
BCLL 0.0	Rep Stress Incr NO	WB 0.32	Horz(CT) 0.00 5 n/a n/a	
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S		Weight: 29 lb FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat) BOT CHORD

WEBS 2x4 SP No.3(flat)

> 8=Mechanical, 5=Mechanical (size) Max Grav 8=1470(LC 1), 5=1447(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-8=-448/0, 4-5=-424/0, 2-3=-916/0 TOP CHORD **BOT CHORD** 7-8=0/916, 6-7=0/916, 5-6=0/916 WEBS 2-8=-1366/0, 3-5=-1366/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1337 lb down at 0-10-4, and 1336 lb down at 2-5-7 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 5-8=-10, 1-4=-100 Concentrated Loads (lb)

Vert: 9=-1289(F) 10=-1288(F)



March 29,2023

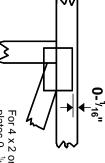


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- ¹/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE



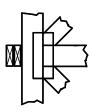
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



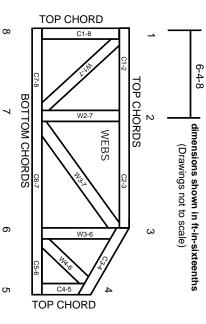
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

Industry Standards:

National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing. Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

ANSI/TPI1: DSB-89:

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

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- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.

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- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

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- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.