

RESIDENCE

PRIVATE

DINA NUNEZ ON 8 HEINER

#1248

368 576 140 FIRST FLOOR FRONT PORCH DBL CARPORT REAR DECK

HEATHER or JOHNATHAN HALL 185 HEATHERSTONE CT BENSON NC 27504 (89) 207-1403

H SQUARED HOME DESIGN, INC.

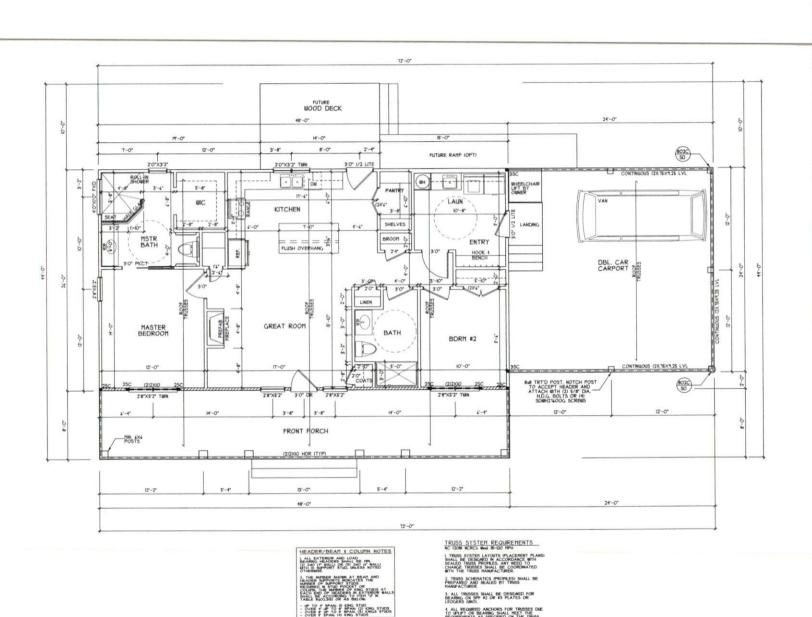
honeouner block only.

ANY DEVANTOR OF THIS PLAN DIFFERSIONS
OF OFFICER SEASON FOR HORE DESIGN.
NO. IS NOT LABELE.
This plan is to be built by the homeomer!
This plan is to be built by the homeomer!
No builder as crited in this title block only.
Not refersed for multiple builds.

DATE:

03/08/2023 1 STORY

012423



ONDINA NUNEZ RESIDENCE PRIVATE 8 HEINER

#1248

FRONT DBL CA PEAR DI

H SQUARED HOME DESIGN, INC.



honeouner block only. ANY DENANTON OF THIS PLAN DIFFICIONS
OF CHINESISE, HOLINEED HOTE DESIGN.
NOT LABLE.
This plan is to be built by the homeomer
This plan is to the built by the homeomer
This plan is to a built by the homeomer
This plan is to the builds.

DATE: 03/08/2023 I STORY

FIRST FLOOR PLAN SCALE V4" = 1-0"

012423

H SQUARED HOME DESIGN, INC.

ON THE PLAN. DIFERSORS

ANY DEVALUES IN SOLARD HOW DESCA.

ANY OF THE PLAN. DIFERSORS.

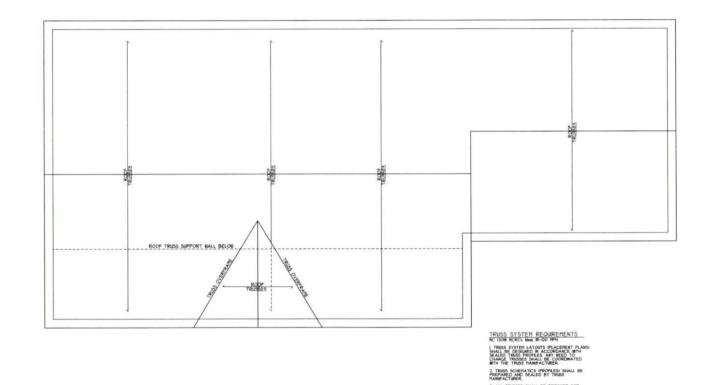
The part to the bull bull the homeomer

or bulder as cred on the tule bold.

Not referred for multiple builds.

REFER TO BASIC DETAILS SHEET FOR STANDARD DETAILS BRACING DETAILS, AND STRUCTURAL NOTES ROOF PLAN

I STORY 012423



STRUCTURAL NOTES

B ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE NORTH CARCOLAN STATE RESIDENTIAL CODE - 200E EDITION, FULS ALL LOCAL NORTH CARCOLAN STATE RESIDENTIAL CODE - 200E EDITION, FULS ALL LOCAL NORTH CARCOLAND STATES OF A CONTROL OF A CONTROL OF CONSTRUCTION BIASA, HETHODS, TECHNOLORS, SEQUENCES OF PROCEDURES, OF FOR SAFETY PRECAUTIONS, TECHNOLORS, SEQUENCES OF PROCEDURES, OF FOR SAFETY PRECAUTIONS, AND PROCARGAS IN CONSTRUCTION BIT THE CONTRACT CONTROL OF THE CONTRACT CONTROL OF THE CONTRACT CONTROL OF THE CONTRACT CALL RETRIERES SHALL BE FRAMED, ANCHORED, TED AND BRACED IN ACCORDANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BILDING COOK SACCOMBANCE WITH COOK SACCOMBANCE WIT

2)	DESIGN LOADS (R3OL4)	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION
	ROOMS OTHER THAN SLEEPING R	00H5 40	10	L/340
	SLEEPING ROOMS	30	10	L/340
	ATTIC WITH PERMANENT STAIR	40	10	L/340
	ATTIC WITH OUT PERMANENT STA	IR 20	10	L/340
_	ATTIC WITH OUT STORAGE	10	10	EY240
	STAIRS	40		1/340
	EXTERIOR BALCONES	40	10	L/340
	DECK5	40	10	L/340
	GUARDRAILS AND HANDRAILS	200		
	PASSENGER VEHICLE GARAGES	50	10	L/340
	FIRE ESCAPES	10	10	1./340
	SNOW	20	**	****

MIND LOAD (BASED ON 15/120 MPH MIND VELOCITY & EXPOSURE B)

3) BALL BRACING: BRACED BALL PANELS SHALL BE CONSTRUCTED ACCORDING TO BALL BRACING: BRACED BALL PANELS SHALL BE CONSTRUCTED ACCORDING TO SECTION BRACED ACCORDING THE BRACING SHALL COMPLY WITH TABLE RACESSO. THE AMOUNT AND LOCATION OF BRACED PANELS SHALL BE DETERMINED BY SECTION RACESSO. LATERAL BRACING SHALL BE SATISHED PER RETHOD S BY CONTRIBUSION. SHEATHING WALLS WITH STRUCTURAL SHEATHING PER SECTION RACESSO.

SHEARING THAT SPECIFIC BRACED BALL DETAIL SHALD HE BY SECTION ROJZIO.3.

NECENSIA TO SPECIFIC BRACED BALL DETAIL SHALL DE BAS SPECIFIED.

10 CONCRETE SHALL HAVE A THIRDIN 28 DAY STRENGTH OF 3000 PSI AND A DAY STRENGTH SHALL DE TAIRDIN SHALD SHALL DE TAIRDIN SHALL DE TAI

4) ALL FRAMMG LUMBER SHALL BE SPF #2 (Fb = 815 PSI) UNLESS NOTED OTHERWISE (UNO), ALL TREATED LUMBER SHALL BE STP # 2 (Fb=415 PSI). PLATE MATERIAL MAY BE SPF # 3 OR SYP #3 (Fd/pmp) = 425 PSI - 19N).

b) ALL BOODEN BEARS AND HEADERS SHALL HAVE THE FOLLOWING END SUPPORTS: (0) 2nd STUD COLUMN FOR 1-0" MAX. BEAR SPAN (BNO), (2) 2X4 STUDS FOR BEAR SPAN GREATER THAN 1-0" (BNO).

SPAN GREATER THAN 1-CT DINO.

81 LVL. SHALL BE LARMATED VENEER LUMBER: FN-2400 PSI, Fv-285 PSI, E-17N0 PSI.
PSI. SHALL BE PARALLE STRAND LUMBER: FN-2700 PSI, Fv-700 PSI, E-15N0 PSI.
LSI. SHALL BE LARMATED STRAND LUMBER: FN-2700 PSI, FV-670 PSI, E-15N0 PSI.

BY ALLIE STRAND LUMBER: FN-2700 PSI, FV-670 PSI, E-15N0 PSI.

BY ALLIE STRAND LUMBER: FN-2700 PSI, FV-670 PSI, E-15N0 PSI. INSTALL ALL CONNECTIONS PER HANUFACTURERS INSTRUCTIONS.

INSTALL ALL CORRECTIONS PER PRAIPACTURERS INSTRUCTIONS.

1 ALL ROOF FRUISS AND I-JOIST LATOUTS SHALL BE PREPARED IN ACCORDANCE
18TH ANY SEALED STRUCTURAL DRAWINGS TRUSSES AND I-JOISTS SHALL BE
INSTALLED ACCORDING TO THE HANIFACTURE'S SPECIFICATIONS. ANY CHANGE IN
TRUSS OR I-JOIST LATOUT SHALL BE COORDINATED 18TH DESIGNER OR ENGINEER.

TRIBS ON F-JOST LATOUT SHALL BE COMMUNATED BITH JESSIMEN ON FRIGHT ON ALL STRUCTURAL STEELS SHALL BE ASTIM A-25. STEEL BEARS SHALL BE SUPPORTED AT EACH END BITH A TRIBUILD BEARMS, LENCTH OF 3 LYZ TRO-JESSIMEN SHALL BE STAND FALL FROM SHALL BE SHALL BE A TACKED TO SACH SUPPORT SHITH TWO LAG SCREEN BYZ DIAMETER x * LONGL LATERAL SUPPORT IS CONSIDERED ADDIQUATE REPOYIDED THE JOST ARE TOO FALL SUPPORT IS CONSIDERED ADDIQUATE REPOYIDED THE JOST ARE TOO THE BEAN FLANGE * 49° O.C. ALL STEEL TRIBUNG SHALL BE A STAT ASOO.

B REBAR SHALL BE DEFORMED STEEL, ASTMAS, GRADE 40.

2) FLTCH BEARS SHALL BE BOLTED TOGETHER USING (2) ROWS OF 1/2" DIAHETER BOLTS (ASTH A3/21) WITH MASHERS PLACED WADER THE THREADED IND OF BOLT. BOLTS OF BOLT IS OF BOLTS OF BOLTS

B) BROCK LINTELS SHALL BE 3 1/2'x3 1/2'x4/4' STEEL ANGLE FOR UP TO 4'-0" SPAN AND 4'x1's/4" STEEL ANGLE WITH 4" LEG VERTICAL FOR SPANS UP TO 4'-0" (UNO).

H) THE POSITIVE AND NEGATIVE DESIGN PRESSURE FOR DOORS AND MINDOWS FOR A HEAN ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSF.

6) THE POSITIVE AND NEGATIVE DESIGN PRESSURES REQUIRED FOR ANY ROOF OR BALL CLADDING APPLICATION NOT SPECIFICALLY ADDRESSED IN THE NORTH CARCUMA STATE RESIDENTIAL CODE - 2018 EDITION SHALL BE AS FOLLOWS.

45.4 P5F - 2.25/2 PITCH OR LESS 34.8 P5F - 2.25/2 TO 1/2 PITCH 21 P5F - 1/2 TO 12/2 PITCH

MALLS: 24J PSF - MALLS

FOUNDATION STRUCTURAL NOTES:

(1) (3) 240 SYP #2 OR SPF#2 GIRDER, TYPICAL UNO.

© CONCRETE BLOCK PIER SZE SHALL BE.
SZE HOLLOR HASORRY SOLID HASORY
8 × 8 UP TO 32" HIGH UP TO 5"-0" HIGH
12 × 8 UP TO 46" HIGH UP TO 7"-0" HIGH
14 × 8 UP TO 46" HIGH UP TO 7"-0" HIGH
14 × 24" UP TO 4" HIGH UP TO 7"-0" HIGH
24 × 34" UP TO 4" HIGH UP TO 7"-0" HIGH
24 × 36" X 0" CONCRETE FOOTING, UNO.

3 WALL FOOTING AS FOLLOWS: DEPTH: 8" - UP TO 2-1/2 STORY IO* - 3 STORY

BRICK VENEER VENEER
- 1 STORY
- 20" - 2 STORY
- 24" - 3 STORY

FOR FOUNDATION BALL HEGHT AND BACKFEL REGISERENTS, REFER TO NORTH CAROLINA RESIDENTIAL CODE TABLE REGULE THAY ON OFFICE OF THE RESIDENTIAL CODE TABLE REGISER OF THE CONDITIONS AND CONTRACTOR HIST VERFEY STEE CONDITIONS AND CONTACT SOLS ENGINEER IF THARMAL OR UNSTABLE SOLS ARE ENCORMITERED.

(4) (4) 240 SYP#2 OR SPF#2 GIRDER.

(5) (2) LTEX 9.25 LVL OR LSL GIRDER

() (3) LTSX9.25 LVL OR LSL GIRDER

1, "M" DESIGNATES A SIGNIFICANT POINT LOAD TO HAVE SOLID BLOCKING TO PIER SOLID BLOCK ALL BEAM BEARING POINTS NOTED TO HAVE THREE OR MORE STUDS TO FID, TYPICAL

8. ABBREVIATIONS: "SJ" = SINGLE JOIST "DJ" = DOUBLE JOIST "TJ" = TRIPLE JOIST

TRUSS SYSTEM REQUIREMENTS

L TRUSS SYSTEM LAYOUTS (PLACEMENT PLANS)
SHALL BE DESIGNED IN ACCORDANCE WITH
SEALED TRUSS PROFILES. ANY NEED TO
CHANGE TRUSSES SHALL BE COORDINATED
WITH THE TRUSS MANUFACTURER.

2. TRUSS SCHEMATICS (PROFILES) SHALL BE PREPARED AND SEALED BY TRUSS MANUFACTURER

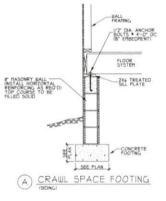
3. ALL TRUSSES SHALL BE DESIGNED FOR BEARING ON SPF #2 OR #3 PLATES OR LEDGERS (UNO).

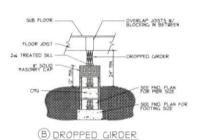
4 ALL REQUIRED ANCHORS FOR TRUSSES DIE TO UPLET OR BEARING SHALL HEET THE REQUIREMENTS AS SPECIFIED ON THE TRUSS SCHEMATICS.

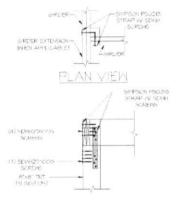
HEADER/BEAM (COLUMN NOTES L ALL EXTERIOR AND LOAD BEARING HEADERS SHALL BE MIN. (2) 2x0 (4" WALL) OR (3) 2x10 (4" WALL) WITH (1) SUPPORT STUD, UNLESS NOTED OTHERWISE.

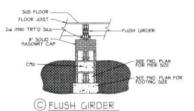
2. THE NUMBER SHOWN AT BEAT AND HEEDER SUPPORTS INDICATES THE NUMBER SHOWN IN STUDY OF THE NUMBER OF KING STUDS AT COLUMN. THE NUMBER OF KING STUDS AT EACH BID OF HEADERS IN EXTERIOR WALLS SHALL BE ACCORDING TO ITEM "4" IN TABLE RIO2365 OR AS BELOW.

UP TO 4' SPAN: (I) KING STUD OVER 4' UP TO 8' SPAN: (2) KING STUDS OVER 8' UP TO II' SPAN: (3) KINGS STUDS OVER II' SPAN: (4) KING STUDS













MPH) (115-120)BUILDING SHE

BASIC DETAIL

50 THAT NOTE ALS A PLAN

PLEASE ALL DET.

TONE C 27504 403 HEATHER H S HEATHERST BENSON NC.2 (9)8) 207-14

H SQUARED HOME DESIGN, INC.



ANT DEVANTOR OF THE SECTION SECTION SECTION SECTION OF DESCRIPTION FOR DESCRIPTION RCS LANGITH.

RCS LANGITH.

IN ACCORDANCE BITM NORTH IN ACCORDANCE BITM NORTH IN ACCORDANCE BITM NORTH IN ACCORDANCE BITM SECTION S

PLE

Non-Itemized QUOTE Estimate UFP Mid-Atlantic, LLC



REQ. QUOTE DATE //	ORDER #	
ORDER DATE //	QUOTE #	23032053
DELIVERY DATE //	CUSTOMER ACCT #	PHDC3652
DATE OF INVOICE //	CUSTOMER PO#	3
ORDERED BY	INVOICE #	
	TERMS	W0
SUPERINTENDENT	SALES REP	798 Mike Solomor
JOBSITE PHONE #	SALES AREA	282 Burlington 1

	HD COMPONENTS # 3652	JOB NAME: ODINA NUN	EZ	LOT # 699	SUBDIV:
1406	HD COMPONENTS # 3652 901	MODEL:T242469	TAG: 130W	JOB CATEG	
TO	FUQUAY VARINA, NC 27526	DELIVERY INSTRUCTIONS:	Note: Custom	ner Signature Require	d on order confirming
Ц			counts, spans	s and all other truss pr	rofile specifications prior
SH		SDECIAL INSTRUCTIONS	to order being	released to production	on. Please fax signed
-	699 DENNING ROAD	SPECIAL INSTRUCTIONS:	order to 360-	604-7476 or send PDI	F to
ô	ANGIER, NC 27501		hdc_truss@h	omedepot.com X:	
					DV DATE

										01	DAIL
BUILDING DEPARTMENT	OVERHA	ING INFO	HEEL HEIGHT	00-04-03	REQ. LAYOUTS		REQ. ENGINEERING		QUOTE	af2	03/21/23
SELECT CODE	END CUT	RETURN							LAYOUT	af2	03/21/23
	PLUMB		GABLE STUDS	24 IN. OC	JOBSITE	2	JOBSITE	1	CUTTING		11

ROOF TRUSSES LOADING INFORMATION				TCLL-TCDL-BCLL-BCDL STRESS INCR. 20.0,10.0,0.0,10.0 1.00				RO	OF TRUS	S SPACI	NG:				
PROFILE	QTY	PIT	CH	TYPE	BASE	O/A	LUN	MBER	OVER	OVERHANG		ILEVER	STUB		
	PLY	TOP	вот	ID	SPAN	SPAN	TOP	BOT	LEFT	RIGHT	LEFT	RIGHT	LEFT	RIGHT	
	2	6.00	0.00	COMMON A00	36-00-00	36-00-00	2 X 4	2 X 4							
	23	6.00	0.00	COMMON A01	36-00-00	36-00-00	2 X 4	2 X 4			01-00-00	01-00-00			
MAN	1	6.00	0.00	COMMON B00	26-00-00	26-00-00	2 X 4	2 X 4			01-00-00	01-00-00			
	11	6.00	0.00	COMMON B01	26-00-00	26-00-00	2 X 4	2 X 4			01-00-00	01-00-00			
	1	6.00	0.00	VALLEY V1	05-04-00	05-04-00	2 X 4	2 X 4							
	1	6.00	0.00	VALLEY V2	09-04-00	09-04-00	2 X 4	2 X 4							
	1	6.00	0.00	VALLEY V3	13-04-00	13-04-00	2 X 4	2 X 4							

ITEMS

QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES	
2	Truss Literature	11X17 LAYOUT				A SHAREST CO.
2	Truss Literature	PRINT TRUSS DRAWINGS				
74	New USP Hangers	RT7A - USP HANGER			USP HANGER UPDATED 1-25-23	

ACCEPTED BY SELLER	ACCEPTED BY BUYER PURCHASER:				
	BY:	TITLE:			
BY:	ADDRESS:				
TITLE:					
DATE OF ACCEPTANCE:	PHONE:	DATE:			

Retail	\$6,414.29

Pricing provided is effective for 15 days from the quote date. Delivery of items listed from the time of quote should not exceed 60 days Quote is based on current design values at the time of quote (lumber, EWP, hardware, etc). Should any of these val sell price accordingly.

If any truss in this system exceeds 60' in length UFP will require the builder, framer or installer to sign an acknowledgement of risk for installation of long span trusses. UFP will provide this document to the customer.

A INDICATES LEFT END OF TRUSS SCALE, N.T.S DESIGNER TAYOUT DATE STAG DARE STAG DATE PLACEMENT PLAN 9475 48.0. 15.5. .t.S 13.0. 15.5. .7.9 2088 WED 3 1/2" x 10" Generic Material 3 1/2" x 10" Generic Material 8803 3 1/2" x 10" Generic Material WED 8802 MED 1 3/4" x 11 7/8" 2.0E Microllam® LVL 59.0. BM3 8' 0" 1 3/4" x 11 7/8" 2.0E Microllam® LVL WED Plies Net Qty Fab Type Products UFP 54.0. 48.0. HID SINES OU SITE AVETEA FINES 33'2 # # E8.18 3NLJ 30019

BUILT

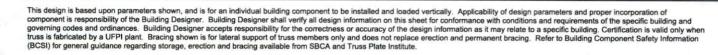
THESE VALUES ARE APPROXIMATE ONLY

TO PART OF 17 IT.OFTS AREA TOOR

. [Job ,	Truss	Truss Type		Qty	Ply					
	23032053	A00	Truss		2	1	Job Referen	ce (optional)			
U	FP Mid Atlantic LLC, 5631 S. N	NC 62, Burlington, NC, Ang	ela Fogleman	Run: 8.62 S	S Sep 22 2022 Print: 8.620 S Sep 22 2022 MiTek Industries, Inc. Tue Mar 21 12:42:54 Page: 1 ID:Omr/3Q?WZSPTNZ8SbvKQuRzYjsq-u2OyS9D6AH6N?XCW7yRkixVT574VJ5dGy3tytvzYjbl						
Γ	1		18-0-0		iD:Omr	I3Q/WZSP1	NZOSDVKQUKZ		36-0-0		
	ł		18-0-0		+			18-0-0			─ ┤
					3x6						
	†			10	11						
				9		_ 13					
1			8			1	14				
			612 7 12				15	5			
1	7		6	ST	в зтв			16			
	94-3	5	STB	ST7 🛭	⊠	STIZ	STE		17		
		3x6 4	SIT4				ST5	ST4	18	3x6	
		2 STD ST	3]						S113	20	
	m 1	Sin							ST2	STITE	21
1	1 3± 1 €		B1	***************************************		 **********	***************************************	I	B1	***************************************	× '
	3x5	39 38 37	36 35 34	33 31	lb/ 120 lb	28	27 26	25	24 23	22	3x5
1	1-0-0			3x6 ³³⁰	3x	6					
	100				36-0-0 35-0-0						+
L	Odaic = 1.00.5	400054-1									
-	1000 Care 1200 Care 1200 F.	1:0-3-0,Edge]									
- 1	.oading 'CLL (roof)	(psf) Spacing 20.0 Plate Grip DOL	2-0-0 1.15	TC	0.23 Vert		in (loc) n/a -	l/defl L/d n/a 999	PLATES MT20	GRIP 244/190	
	CDL BCLL	10.0 Lumber DOL 0.0* Rep Stress Incr	1.15 YES	BC WB	0.21 Vert 0.12 Hori	č	n/a - 0.01 22	n/a 999 n/a n/a	1900		
- 1	BCDL	10.0 Code	IRC2015/TPI2014	Matrix-MSH	0.72	_(/	0.01	100 100	Weight: 228 lb	FT = 20%	
	LUMBER				BRACING						
-	TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2	2			TOP CHORD BOT CHORD		ructural wood sh gid ceiling direct	Contrate of Contrate	applied or 10-0-0 or -0 oc bracing.	oc purlins.	
- 1	OTHERS 2x4 SP No.3 REACTIONS All be	3 earings 34-0-0.			WEBS	1 1	Row at midpt		10-31, 12-30		
	(lb) - Max i	Horiz 39=-157 (LC 11)									
1	Max (37, 39 except 23=-	less at joint(s) 22, 24, 25, 26, 27, 121 (LC 11), 38=-129 (LC 10)								
	IVIAX (o) or less at joint(s) 23, 24, 25, 26 2=359 (LC 22), 30=256 (LC 1), 3	1=256 (LC 1),							
	FORCES	(lb) - Max. Comp./Max. Te	n All forces 250 (lb) or less exce								
- 1	TOP CHORD NOTES	8-9=0/255, 9-10=-6/314, 1	0-11=-20/276, 11-12=-20/276, 12	-13=-6/314							
1	1) Unbalanced roof live	loads have been considered	lered for this design. just) Vasd=103mph; TCDL=6	S Oper PCDI -6 O	nof: h=3Eft: Ca	t II: Evn B	: England:				
-	MWFRS (envelope) e	exterior zone and C-C E	xterior (2) zone; cantilever le	ft and right expos	ed; end vertic	al left and					
	Truss designed for wi	ind loads in the plane of		mber DOL=1.60 p	plate grip DOL:	=1.60					
-	Gable studs spaced a										
			oottom chord live load nonco				tall by				
			d and any other members. f truss to bearing plate capat	ole of withstanding	a 100 lb uplift a	at ioint(s) 3	3. 34. 35. 36.				
1		, 24, 22 except (jt=lb) 3 g condition. Review red			,						
1		d in accordance with the	2015 International Resident	tial Code sections	R502.11.1 an	d R802.10	.2 and				
ı	OAD CASE(S) Standard										



Job ,		Truss		Truss Type		Qty	Ply							
23032053		A01		Truss		2	3	1	lah Dafa		4111			
LIED Mid Atlantic	II.C 5621 S N/	C 62 Pui	lington, NC, Angela Fo		D 9 62	0 0 22 20	22 Drint: 0.6		Job Refe			Inc. T Mar. 24	40.40.54	Do not
OFF WIII Allamiic	LLC, 3631 S. NO	62, Bui	nington, NC, Angela Fo	gleman	Run; 8.62							Inc. Tue Mar 21	12:42:54 yRkixVLW7xrJ1k0	Page: 1
		6-4-5		10.0.0	49.00			200			Salari Alaria		1000 00 000	3,00,002.7,00
	1	6-4-5	+	12-2-3 5-9-13	18-0-0 5-9-13	+	23-9- 5-9-1		+		7-11 9-13	+	36-0-0 6-4-5	→
					00.0	FC					, 10		040	
						5x6 4								- 1
+						<u></u>								
														- 1
				3x4					3x4					
				3 7	2			13	2 5					
				612					1					
m						W4			///					- 1
94-3			5x6					//		\		5x6		- 1
0,				wz.	W3		,	W3		NS.				- 1
				//	18		//			18		>		- 1
		71	WA	//			//					who	Th	
	5x8	//	.//	\ //			//				/	//		3x10
9	1 HW1					W//				//	$\sqrt{/}$		HW1	7
T 2=	NSH NSH		В	0.0000		B2					⊗ 8	B1	1100	X
	Δ.			12		10					8			
1	1070 lb/-161 lb			3x4	3x6	3x8		3x6		1546 lb/	376 lb		345	lb/-67 lb _{3x5}
	1-0-0		0.00											36-0-0
-	+ +		9-8-9 8-8-9		8-0-0 8-3-7	+		26-1 8-10			+	35-0 8-1-		++1
Scale = 1:60.3	1-0-0		0-0-0		5-5-7			0-10) - +			0-1-	-12	1-0-0
Plate Offsets (X, Y	Y): [1:0-	-8-0,0-0-	2], [1:0-0-8,0-11-5], [2:0	-3-0,0-3-0], [6:0-3-0,0-3-0],	[7:0-0-12,0-0-2], [7:0-0-2]	7:0-0-4,Edge]							
Loading		(nef)	Cassina	200	001							T		
TCLL (roof)		(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.15	TC TC	0.72	DEFL Vert(LL)	-0	in (loc).15 10-1		L/d 240	PLATES MT20	GRIP 244/190	- 1
TCDL		10.0	Lumber DOL	1.15	BC	0.83	Vert(CT)		0.27 10-1		180	WIIZO	244/190	- 1
BCLL		0.0*	Rep Stress Incr	YES	WB	0.37	Horz(CT)	(.04	8 n/a	n/a			- 1
BCDL		10.0	Code	IRC2015/TPI2014	Matrix-MSH							Weight: 187 II	b FT = 20%	
LUMBER						BRACING	1							
TOP CHORD	2x4 SP No.2					TOP CHO		Stru	ictural wood	sheathing	directly	applied or 3-7-	14 oc purlins.	- 1
BOT CHORD WEBS	2x4 SP No.2 2x4 SP No.3					вот сно	RD		d ceiling dir 0 oc bracin		ed or 10	-0-0 oc bracing,	Except:	- 1
WEDGE	Left: 2x6 SP I					WEBS			ow at midpt	g. 1-0.		3-10, 5-8		- 1
	Right: 2x6 SP													- 1
REACTIONS	(lb/size		=1070/0-3-8, (min. 0-1-8 1-13)), 7=265/0-3-8, (min. 0-1-8)	, 8=1546/0-3-8, (n	nin.								- 1
	Max H	oriz 1=	157 (LC 10)											- 1
	Max U _l Max G		:-161 (LC 10), 7=-67 (LC :1070 (LC 1), 7=345 (LC	것이 이번 어짓이는 그런 생생님이 되었다.										- 1
FORCES				forces 250 (lb) or less exce	ent when shown									- 1
TOP CHORD				-4=-761/328, 4-5=-759/327										- 1
BOT CHORD				8, 11-23=-153/1008, 10-11				=0/304						
WEBS	3	3-12=-28/	/385, 3-10=-601/284, 4-	10=-112/394, 5-10=-47/489), 5-8=-1227/386, (6-8=-346/231								
NOTES 1) Unbalance	and roof live to	ade ha	ve been considered	for this design										- 1
	CE 7-10; Vult	t=130m	ph (3-second aust) V	/asd=103mph; TCDL=6	.0psf: BCDL=6.	0psf: h=35	ft: Cat. II: E	Exp B:	Enclosed					
MWFRS	(envelope) ex	terior z	one and C-C Exterio	r (2) zone; cantilever let	ft and right expo	sed; end	vertical left	and ri						
				or reactions shown; Lur n chord live load noncor										
4) * This tru	ss has been o	designe	d for a live load of 20	0.0psf on the bottom che	ord in all areas	where a rec	ctangle 3-0	06-00 t	all by					
2-00-00 v	wide will fit bet	tween th	ne bottom chord and	any other members, w	ith BCDL = 10.0	psf.			110020-5	101				
	necnanicai co d 67 lb uplift a			to bearing plate capab	le of withstandii	ng 1/6 lb u	plift at join	t 8, 16	1 lb uplift	at				
6) This truss	s is designed	in accor	dance with the 2015	International Residenti	ial Code section	s R502.11	.1 and R80	02.10.2	2 and					
reference LOAD CASE(S)	ed standard Al Standard		1.											
LOND CHOC(3)	Standard	•												
														- 1
														- 1
														- 1
														- 1

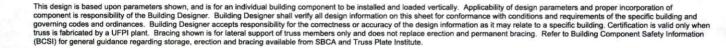




Qty Ply Job Truss Truss Type B00 23032053 1 1 Truss Job Reference (optional) UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Angela Fogleman Run: 8.62 S Sep 22 2022 Print: 8.620 S Sep 22 2022 MiTek Industries, Inc. Tue Mar 21 12:42:55 Page: 1 ID: szP8Gl?8KmXK?jje9crfRfzYjsp-MEyKgUEkwbEEdhmihfyzE82ZrXHm2PLPAjdWQMzYjbkryflower.6-9-4 13-0-0 19-2-12 26-0-0 6-9-4 6-2-12 6-2-12 6-9-4 5x6 6 5 612 8 5x4 3 9 5x4 2 10 6-10-3 S 4 **X** 24 23 22 20 18 21 19 16 15 13 25 12 5x6 1040 lb/-145 lb_{3x6} 1040 Jb/-145 lb 3x4 1-1-12 26-0-0 1-0-0 25-0-0 9-0-9 16-11-7 24-10-4 7-10-13 7-10-13 7-10-13 -0-0 0-1-12 0-1-12 Scale = 1:54.1 1-0-0 Plate Offsets (X, Y): [17:0-3-0,0-3-0] Loading 2-0-0 CSI DEFL (psf) Spacing PLATES in (loc) l/defl L/d GRIP TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.55 Vert(LL) -0.0617-18 >999 240 MT20 244/190 TCDL 10.0 Lumber DOI 1.15 BC. 0.78 Vert(CT) -0.13 17-18 >999 180 BCLL 0.0 Rep Stress Inci YES WB 0.66 0.05 Horz(CT) 12 n/a n/a BCDI 10.0 Code IRC2015/TPI2014 Matrix-MSH Weight: 189 lb FT = 20% LUMBER BRACING TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 4-9-9 oc purlins. **BOT CHORD** 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 2x4 SP No.3 **JOINTS** 1 Brace at Jt(s): 26, 27, 31, 35 OTHERS 2x4 SP No.3 REACTIONS (lb/size) 12=1040/0-3-8, (min. 0-1-8), 25=1040/0-3-8, (min. 0-1-8) Max Horiz 25=113 (LC 10) Max Uplift 12=-145 (LC 11), 25=-145 (LC 10) **FORCES** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD BOT CHORD 15-16=-275/1289, 14-15=-275/1289, 13-14=-275/1289, 12-13=-275/1289, 11-12=-124/773 6-27=-202/525, 27-32=-190/485, 16-32=-188/489, 16-33=-268/144, 21-28=-188/489, 26-28=-190/485, 6-26=-202/525, 21-29=-268/144, 25-31=-584/170, 30-31=-614/175, WEBS 2-30=-579/207, 10-34=-579/207, 34-35=-614/175, 12-35=-584/170 NOTES Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; 2) MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Truss designed for wind loads in the plane of the truss only. All plates are 2x3 MT20 unless otherwise indicated. 5) Gable studs spaced at 2-0-0 oc.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 25 and 145 lb uplift at joint 12.
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard





Job Truss Truss Type Qty Ply B01 23032053 11 1 Truss Job Reference (optional) UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Angela Fogleman Run: 8.62 S Sep 22 2022 Print: 8.620 S Sep 22 2022 MiTek Industries, Inc. Tue Mar 21 12:42:55 Page: 1 ID: szP8GI?8KmXK? jje9crfRfzYjsp-MEyKgUEkwbEEdhmihfyzE82XZXHP2XqPAjdWQMzYjbkryline for the standard property of the sta6-9-4 13-0-0 19-2-12 26-0-0 6-9-4 6-2-12 6-2-12 6-9-4 3 6 2x3 2x3 2 5x8 5x8 HW1 HW1 8 6 1040 lb/-146 lb 3x8 3x4 3x6 3x4 1040 jb/-146 lb 9-0-9 16-11-7 25-0-0 8-0-9 7-10-13 8-0-9 Scale = 1:48.3 1-0-0 1-0-0 Plate Offsets (X, Y): [1:0-8-0,0-0-2], [1:0-0-8,0-11-5], [5:0-8-0,0-0-2], [5:0-0-8,0-11-5] DEFL Loading (psf) Spacing 2-0-0 CSI (loc) I/defl 1/d PLATES GRIP TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.70 Vert(LL) -0.246-8 >999 240 MT20 244/190 TCDL 10.0 Lumber DOL Vert(CT) -0.40 180 1.15 BC 0.81 6-8 >783 BCLL 0.0 Rep Stress Incr YES WB 0.18 Horz(CT) 0.04 5 n/a n/a BCDL IRC2015/TPI2014 FT = 20% 10.0 Matrix-MSH Weight: 120 lb Code LUMBER BRACING 2x4 SP No.2 TOP CHORD TOP CHORD Structural wood sheathing directly applied or 3-7-13 oc purlins. BOT CHORD 2x4 SP No 2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.3 WEBS WEDGE Left: 2x6 SP No.2 Right: 2x6 SP No.2 REACTIONS (lb/size) 1=1040/0-3-8, (min. 0-1-8), 5=1040/0-3-8, (min. 0-1-8) Max Horiz 1=-113 (LC 11) Max Uplift 1=-146 (LC 10), 5=-146 (LC 11) **FORCES** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-1486/432, 2-3=-1306/433, 3-4=-1306/433, 4-5=-1486/432 BOT CHORD 1-8=-282/1250, 8-19=-98/881, 7-19=-98/881, 7-20=-98/881, 6-20=-98/881, 5-6=-282/1250 3-6=-102/445, 4-6=-299/230, 3-8=-102/445, 2-8=-299/230 WEBS NOTES Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 1 and 146 lb uplift at joint 5. This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. LOAD CASE(S) Standard



Job ,	Truss	Truss Type	Qty	Ply	
23032053	V1	Truss	1	1	Job Reference (optional)

UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Angela Fogleman

Run: 8.62 S Sep 22 2022 Print: 8.620 S Sep 22 2022 MiTek Industries, Inc. Tue Mar 21 12:42:55

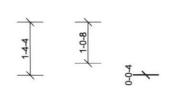
3x4

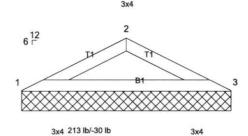
Structural wood sheathing directly applied or 5-4-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Page: 1 ID:szP8Gl?8KmXK?jje9crfRfzYjsp-MEyKgUEkwbEEdhmihfyzE82eJXRQ2ZiPAjdWQMzYjbk







Scale = 1:28.3

T.		9
	5-4-0	L
1		

Plate Offsets (X, Y): [2:0-2-0,Edge]													
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.16	Vert(TL)	n/a		n/a	999			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.01	3	n/a	n/a			
BCDL	10.0	Code	IRC2015/TPI2014	Matrix-MSH							Weight: 15 lb	FT = 20%	

BRACING

TOP CHORD

BOT CHORD

LUMBER TOP CHORD

REACTIONS

2x4 SP No.2

BOT CHORD 2x4 SP No.2

1=213/5-4-0, (min. 0-1-8), 3=213/5-4-0, (min. 0-1-8)

(lb/size) Max Horiz 1=-21 (LC 11)

Max Uplift 1=-30 (LC 10), 3=-30 (LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-399/178, 2-3=-289/151

BOT CHORD 1-3=-145/345

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 5) 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 1 and 30 lb uplift at joint 3.
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S)

Standard



Ply Job Truss Truss Type Qty V2 23032053 1 Truss Job Reference (optional) Run: 8.62 S Sep 22 2022 Print: 8.620 S Sep 22 2022 MiTek Industries, Inc. Tue Mar 21 12:42:56 UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Angela Fogleman Page: 1 $ID: K9zWU50m53fBcslqjKMu_szYjso-qRWitqFMhuM5FrLvFNTCnMbobxlen? OZPNM3yozYjbjushaning to the property of the$ 4-8-0 8-9-1 4-8-0 4-1-1 2 ST 6 12 641 lb/-78 lb 1.5x3 3x4 3x4 9-4-0 Scale = 1:27.7 DEFL PLATES GRIP 2-0-0 CSI in I/defl L/d Loading (psf) Spacing (loc) TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.23 Vert(LL) n/a n/a 999 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 RC. 0.23 Vert(TL) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.10 Horiz(TL) 0.00 n/a n/a BCDL IRC2015/TPI2014 Matrix-MSH Weight: 30 lb FT = 20% 10.0 LUMBER BRACING TOP CHORD TOP CHORD 2x4 SP No.2 Structural wood sheathing directly applied or 9-4-0 oc purlins. BOT CHORD 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing 2x4 SP No.3 **OTHERS** REACTIONS (lb/size) 1=53/9-4-0, (min. 0-1-8), 3=53/9-4-0, (min. 0-1-8), 4=641/9-4-0, (min. Max Horiz 1=-38 (LC 15) Max Uplift 1=-13 (LC 10), 3=-21 (LC 11), 4=-78 (LC 10) 1=86 (LC 21), 3=86 (LC 22), 4=641 (LC 1) Max Grav (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. **FORCES** TOP CHORD 1-2=-109/315, 2-3=-109/315 WEBS 2-4=-476/241 NOTES Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Gable requires continuous bottom chord bearing. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 5) 2-00-00 wide will fit between the bottom chord and any other members. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 1, 21 lb uplift at joint 3 and 78 lb uplift at joint 4. This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. LOAD CASE(S)



Job,	Truss		Truss Type		Qty	Ply	/								
23032053	V3		Truss		1		1	lob Ref	erence	e (optio	onal)				
FP Mid Atlantic LL	LC, 5631 S. NC 62, Bu	urlington, NC, Angela	Fogleman	Run: 8.62 S Se	ep 22 202	2 Print: 8	.620 S Se	p 22 202	2 MiTe	k Indus	tries, Ir	nc. Tue Mar	21 12:42	:56	
					ID	:K9ZWU:	OUMDSTBC	siqjKMu_	SZYJSO	-qRWiti	q-Mhu	M5FrLvFNT	CnMbpG	ixnNn05ZF	PNM
			k	6-8-0		L			12	-9-1			13-	4-0	
			1	6-8-0		1			6-	1-1			0-6	-15	
													0.0	10	
						5x6									
						3									
_															
					//	\bigcap									
_	- φ		1.5x	3 11		ST2		T	_		1.5x3	3			
34	3-0-8		6 7 2	///					/	\	4				
			STIT							\	7				
										5	1			-	
_		\$ -	××××××××××××××××××××××××××××××××××××××	**********	· · · · · · · · · · · · · · · · · · ·	···	B1	···	·	·	<u></u>	····	$\overline{\sim}$	5 ⋈	
		0	8	***********	****	$\overset{\sim}{\overset{\sim}{\overset{\sim}}{\overset{\sim}}}$	***	XXX	∞	∞	$\stackrel{8}{\underset{6}{\otimes}}$	*****	XXX	\boxtimes	
			3x4 1.5x	31	8 lb/-107	lb 1.5x3							24		
			384 1.38			1.583					1.5x3	3	3x4		
Scale = 1:32.4			<u></u>			13-4-0								1	
			1											1	
oading CLL (roof)	(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.15	TC		DEFL Vert(LL)		in (la /a	oc)	l/defl n/a	L/d 999	PLATES MT20		GRIP 244/190	
CDL	10.0	Lumber DOL	1.15	BC	0.12	Vert(TL)		/a	-	n/a	999			2111100	
BCDL	0.0* 10.0	Rep Stress Incr Code	YES IRC2015/TPI2014	WB Matrix-MSH	0.06	Horiz(TL)) 0.	00	5	n/a	n/a	Weight: 47	lb	FT = 20%	
BOT CHORD	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 All bearings 13 (lb) - Max Horiz 1 Max Uplift A	1=56 (LC 10)	ess at joint(s) 1, 5 except 6=-106	ВО	OP CHOR							applied or 6- -0 oc bracinç		urlins.	
	Max Grav A		or less at joint(s) 1, 5 except 6=	318 (LC 22), 7=300											
FORCES			- All forces 250 (lb) or less exce	ept when shown.											
NOTES 1) Unbalance	ed roof live loads ha	ave been conside	red for this design												
2) Wind: ASC MWFRS (e	CE 7-10; Vult=130n envelope) exterior a	nph (3-second gus zone and C-C Ext	st) Vasd=103mph; TCDL=6 terior (2) zone; cantilever le RS for reactions shown; Lui	ft and right exposed	; end ve	ertical le	ft and rig	Enclose ht	d;						
Gable requ	uires continuous bo	ottom chord bearing		g. 1990 A. (14,000 A. 1990 Million (1,000 A. 1990 A. 19			en P								
5) * This truss	s has been designed	ed for a live load of	of 20.0psf on the bottom che and any other members.				06-00 ta	ll by							
Provide me	echanical connection	on (by others) of	russ to bearing plate capab	le of withstanding 1	00 lb up	lift at joi	nt(s) 1, 5	excep	t						
	is designed in acco		2015 International Resident	ial Code sections R	502.11.1	and R8	802.10.2	and							
referenced LOAD CASE(S)	I standard ANSI/TF Standard	4 1.													
12.50															



\$ 26°

23032053