

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 150\_1910\_C\_10x10CP 150.1910 C\_10x10CP

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I57773963 thru I57774002

My license renewal date for the state of North Carolina is December 31, 2023.

North Carolina COA: C-0844



April 14,2023

Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 April 14,2023





# April 14,2023



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 16, 17, 18, 14, 13, 12

Max Grav All reactions 250 lb or less at joint(s) 2, 10, 15, 16, 17, 18, 14, 13, 12

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 2-1-8, Exterior(2N) 2-1-8 to 7-2-4, Corner(3R) 7-2-4 to 10-2-4, Exterior(2N) 10-2-4 to 15-3-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) N/A

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						157773966
150_1910_C_10X10CP	AG	Common Girder	1	2		
				5	Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8	.630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:05:49 2023	Page 2

8.630 s Nov 19 2022 Mi Lek Industries, Inc. Thu Apr 13 16:05:49 2023 Page 2 ID:VMD62rz1yiHD\_OqRtbnrlFztQ8K-1QgxPCDujzARzaVmNLzo27AWRWPREpVfLxz5LxzR4bG

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-43, 4-6=-43, 1-6=-20

Concentrated Loads (lb)

Vert: 1=-1619(B) 7=-2120(B) 10=-1544(B) 11=-1541(B) 12=-1608(B) 13=-1657(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						157773969
150_1910_C_10X10CP	BE	Common Supported Gable	1	1		
					Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8	.630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:05:54 2023	Page 2
		ID:VMD	62rz1yiHD	_OqRtbnr	IFztQ8K-ONTqSwH1YVok3LNk9vYzIAtN0X96vD5OUDgs0	9zR4bB
NOTES						

#### NOTES 10) N/A

11) N/A
12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						57773970
150_1910_C_10X10CP	BG	Common Girder	1	2		
				3	Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8	630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:05:56 2023 F	Page 2

ID:VMD62rz1yiHD\_OqRtbnrlFztQ8K-KmbatbIH462RIfX7HJbRqbyq6L?UN9KhyX9z51zR4b9

# NOTES-

14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-43, 5-6=-43, 2-7=-20 Concentrated Loads (lb)

Vert: 8=-106(B) 10=96(B) 11=88(B) 12=90(B) 13=-8(B) 14=-19(B) 15=-117(B) 16=-117(B) 17=-123(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





					8-0-	-0						
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         11.6/15.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACI Plate G Lumbe Rep St Code	NG- Grip DOL er DOL tress Incr IRC2018/TF	2-0-0 1.15 1.15 YES Pl2014	<b>CSI.</b> TC BC WB Matri	0.06 0.02 0.05 x-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.00	(loc) 6 7 6	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 44 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-					в	RACING-						

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. All bearings 8-0-0.

Max Horz 2=76(LC 13) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 6, 10, 8

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 2-0-0, Exterior(2N) 2-0-0 to 4-0-0, Corner(3R) 4-0-0 to 7-0-0, Exterior(2N) 7-0-0 to 8-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) N/A

- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	8-3-0	) 16-0-10	24-0-0	1	31-11-6	1	39-9-0 42-2-	8
	8-3-0	7-9-10	7-11-6	1	7-11-6	I	7-9-10 2-5-8	3
Plate Offsets (	X,Y) [2:0-0-0,0	)-1-1]						
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	<b>CSI.</b> TC 0.56 BC 0.81 WB 1.00 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.34 16-18 -0.72 16-18 0.21 12	l/defl L/d >999 240 >696 180 n/a n/a	PLATES MT20 M18AHS Weight: 251 lb	<b>GRIP</b> 244/190 186/179 FT = 20%
LUMBER-				RACING-				
TOP CHORD	2x6 SP No 2 *E	(cept*	1		Structural wood	sheathing direct	tly applied or 3-0-5 oc purlir	IS
	1-4,10-11: 2x4 S	SP No.2			except end vertic	cals, and 2-0-0	oc purlins (3-3-14 max.): 4-	10.
BOT CHORD	2x4 SP No.1		E	BOT CHORD	Rigid ceiling dire	ctly applied or 7	7-10-9 oc bracing.	
WEBS	2x4 SP No.3 *Ex	<pre><cept*< pre=""></cept*<></pre>	N	VEBS	1 Row at midpt	5-20	0, 7-18, 7-14, 8-13	
	5-20,7-18,7-14,8	3-13: 2x4 SP No.2						
REACTIONS.	(size) 2=0- Max Horz 2=14 Max Uplift 2=-1 Max Grav 2=17	3-8, 12=Mechanical 0(LC 16) 62(LC 13), 12=-214(LC 12) 39(LC 2), 12=1677(LC 40)						
FORCES. (II: TOP CHORD BOT CHORD WEBS	<ul> <li>b) - Max. Comp./M</li> <li>2-3=-3089/364</li> <li>8-10=-895/128</li> <li>2-20=-362/265</li> <li>4-20=-51/953,</li> <li>8-13=-2715/42</li> </ul>	ax. Ten All forces 250 (lb) or la , 3-4=-2985/370, 4-5=-2623/346 , 10-11=-977/128, 11-12=-1692 2, 18-20=-604/4150, 16-18=-63 5-20=-1795/332, 5-18=0/373, 7- 4, 11-13=-186/1585	ess except when shown. , 5-7=-4150/612, 7-8=-3308/49 201 3/4275, 14-16=-633/4275, 13-1 16=0/317, 7-14=-1169/168, 8-7	2, 4=-484/3308 14=0/728,				
<ul> <li>NOTES-</li> <li>1) Unbalanced</li> <li>2) Wind: ASCC</li> <li>MWFRS (et Interior(1) 1 Lumber DO</li> <li>3) TCLL: ASC</li> <li>DOL=1.15); surcharge</li> <li>4) Unbalanced</li> <li>5) This truss h non-concur</li> <li>6) Provide add</li> <li>7) All plates at</li> <li>8) This truss h</li> <li>9) * This truss will fit betwee</li> <li>10) Refer to g</li> <li>11) Provide m 12=214.</li> <li>12) One H2.5/ connection</li> </ul>	d roof live loads ha E 7-16; Vult=130rr nvelope) gable en 2-5-15 to 39-9-0, JL=1.60 plate grip E 7-16; Pr=20.0 p Is=1.0; Rough Ca applied to all expose d snow loads have has been designed rent with other live equate drainage to re MT20 plates un has been designed has been designed	we been considered for this desi ph (3-second gust) Vasd=103m d zone and C-C Exterior(2E) -0- Exterior(2E) 39-9-0 to 42-0-12 zo DOL=1.60 sf (roof LL: Lum DOL=1.15 Plate at B; Partially Exp.; Ce=1.0; Cs= been considered for this design for greater of min roof live load loads. prevent water ponding. less otherwise indicated. for a 10.0 psf bottom chord live d for a live load of 20.0psf on the ord and any other members. truss connections. ion (by others) of truss to bearin -Tie connectors recommended to and does not consider lateral force	gn. ph; TCDL=6.0psf; BCDL=6.0ps 10-8 to 2-1-8, Interior(1) 2-1-8 to pne;C-C for members and force DOL=1.15); Pg=15.0 psf; Pf=1 1.00; Ct=1.10, Lu=50-0-0; Min. In 0.500/12 in accordance with of 12.0 psf or 1.00 times flat root load nonconcurrent with any of a bottom chord in all areas whe g plate capable of withstanding to connect truss to bearing walls pes.	f; h=25ft; Cat. II; E b 8-3-0, Exterior(2 is & MWFRS for re 6.5 psf (Lum DOL flat roof snow load IBC 1608.3.4. bf load of 11.6 psf her live loads. re a rectangle 3-6 100 lb uplift at joi due to UPLIFT at	Exp B; Enclosed; R) 8-3-0 to 12-5- eactions shown; .=1.15 Plate d governs. Rain on overhangs -0 tall by 2-0-0 wi nt(s) except (jt=lb t jt(s) 2. This	15, ide	SEAL 036322	023
Continued on p	age 2							
Design valid	6 - Verify design parame for use only with MiTek	ters and READ NOTES ON THIS AND IN( ® connectors. This design is based only	CLUDED MITEK REFERENCE PAGE MI	I-7473 rev. 5/19/2020 B ndividual building comp	EFORE USE.		ENGINEERING BY	חי

818 Soundside Road Edenton, NC 27932

RENUU

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						157773972
150_1910_C_10X10CP	H1	Hip	1	1		
					Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8	.630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:06:00 2023	Page 2
		ID:V	MD62rz1	iHD OgR	tbnrlFztQ8K-DXq5jzLn7LYtnGruW9fN R7SwyBVJltHt97AE	ozR4b5

# NOTES-

13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







5-6-15	10-11-0	20-8-12	27-3-4	37-1-0	42-2-8
Plate Offsets (X,Y) [2:0-0-0	<u>5-4-1</u> .0-1-1], [15:0-4-0,0-3-0]	9-9-12	6-6-8	9-9-12	5-1-8
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 <b>CSI.</b> 1.15 TC 0.97 1.15 BC 0.90 YES WB 0.74	DEFL. Vert(LL) -( Vert(CT) -( Horz(CT) (	in (loc) l/defl L/d 0.43 15-17 >999 240 0.79 15-17 >640 180 0.14 11 n/a n/a	PLATES         GRIP           MT20         244/190
BCDL 10.0	Code IRC2018/TF	Pl2014 Matrix-S			Weight: 234 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP DSS *E 13-15: 2x4 SP WEBS 2x4 SP No.2 *E 3-18,3-17,4-17 OTHERS 2x4 SP No.3	xcept* No.1 :xcept* 9-12,10-11,10-12: 2x4 SF	No.3	BRACING- TOP CHORD Stru- exc BOT CHORD Rig WEBS 1 R	uctural wood sheathing directly cept end verticals, and 2-0-0 oc gid ceiling directly applied or 9-6 Row at midpt 5-17,	<sup>,</sup> applied or 2-8-1 oc purlins, ; purlins (2-2-0 max.): 4-9. 3-8 oc bracing. 8-12
REACTIONS. (size) 2=0 Max Horz 2=1 Max Uplift 2=- Max Grav 2=1	-3-8, 11=Mechanical 62(LC 16) I24(LC 13), 11=-162(LC 1 870(LC 3), 11=1829(LC 3	2)			
FORCES.         (lb)         Max. Comp.//           TOP CHORD         2-3=-3400/27         8-9=-1499/17           BOT CHORD         2-18=-297/29         9           WEBS         3-17=-453/16         8-14=-11/843	Ax. Ten All forces 250 9, 3-4=-3058/314, 4-5=-26 7, 9-10=-1705/178, 10-11: 31, 17-18=-297/2931, 15- 9, 4-17=-37/1070, 5-17=-1 , 8-12=-1791/296, 9-12=0	(lb) or less except when shown. 667/296, 5-6=-3571/419, 6-8=-3287/3 =-1803/171 17=-425/3444, 14-15=-423/3514, 12 060/243, 5-15=0/298, 6-14=-501/12 /505, 10-12=-150/1797	384, -14=-354/2878 4,		
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads h</li> <li>2) Wind: ASCE 7-16; Vult=1300 MWFRS (envelope) gable et 15-1-15, Interior(1) 15-1-15 f forces &amp; MWFRS for reactio</li> <li>3) TCLL: ASCE 7-16; Pr=20.0 J DOL=1.15); Is=1.0; Rough C surcharge applied to all expc</li> <li>4) Unbalanced snow loads hav</li> <li>5) This truss has been designe non-concurrent with other liv</li> <li>6) Provide adequate drainage 1</li> <li>7) This truss has been designe</li> <li>8) * This truss has been designe</li> <li>8) * This truss has been designe</li> <li>8) * This truss has been designe</li> <li>9) Refer to girder(s) for truss to 10) Provide mechanical conner 11=162.</li> <li>11) One H2.5A Simpson Stron connection is for uplift only</li> <li>Continued on page 2</li> </ul>	ave been considered for the nph (3-second gust) Vasd ad zone and C-C Exterior(2R) 37- s shown; Lumber DOL=1.1 at B; Partially Exp.; Ce=1. sed surfaces with slopes label considered for this d for greater of min roof live loads. o prevent water ponding. d for a 10.0 psf bottom child ed for a live load of 20.0ps iord and any other member truss connections. tion (by others) of truss to g-Tie connectors recommendation and does not consider later and BEAD NOTES CHILTER	his design. =103mph; TCDL=6.0psf; BCDL=6.0 2E) -0-10-8 to 2-1-8, Interior(1) 2-1-6 1-0 to 41-3-15, Interior(1) 41-3-15 to .60 plate grip DOL=1.60 5 Plate DOL=1.15); Pg=15.0 psf; Pf 0; Cs=1.00; Ct=1.10, Lu=50-0-0; Mir less than 0.500/12 in accordance wit design. the load of 12.0 psf or 1.00 times flat r prd live load nonconcurrent with any sf on the bottom chord in all areas where, with BCDL = 10.0psf. the bearing plate capable of withstanding and forces.	psf; h=25ft; Cat. II; Exp I 3 to 10-11-0, Exterior(2R 42-0-12 zone;C-C for m =16.5 psf (Lum DOL=1.' 1. flat roof snow load gov th IBC 1608.3.4. roof load of 11.6 psf on c other live loads. here a rectangle 3-6-0 ta mg 100 lb uplift at joint(s) Ills due to UPLIFT at jt(s)	B; Enclosed; A) 10-11-0 to nembers and 15 Plate verns. Rain overhangs all by 2-0-0 wide b) except (jt=lb) s) 2. This	SEAL 036322 MGINEER A. GILBH

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 ev. 5/19/2/2/2/ BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						157773973
150_1910_C_10X10CP	H2	Hip	1	1		
					Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8.	630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:06:05 2023	Page 2
		ID:	VMD62rz1	yiHD_OqF	RtbnrlFztQ8K-ZVe_mgPwytBAu1jrJiFYhUqBFzsI_3400Rrxv	v0zR4b0

# NOTES-

This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Scale = 1:77.2

818 Soundside Road Edenton, NC 27932



	6-10-2	15	<u>13-7-0</u> 6-8-1		<u>19-10-5</u> 6-3-5			28-1-11			34-5-0		42-2-8	
Plate Offsets (X,	Y) [2:0-0-0,0	)-1-1], [5:0-4-0,0	-0-12], [8:0-4-0,	)-0-12],	[13:0-2-12,	0-3-4]		000			000		100	
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 10 TCDL BCLL BCDL	20.0 6.5/15.0 10.0 0.0 * 10.0	SPACING Plate Grip Lumber D Rep Stres Code IRC	DOL 1.15 OL 1.15 OL 1.15 is Incr YES C2018/TPI2014		<b>CSI.</b> TC BC WB Matri	0.95 0.85 0.91 x-S		DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.28 -0.50 0.14	(loc) 13-15 13-15 11	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 256 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-				I			BF	RACING-						
TOP CHORD 2	2x4 SP No.2 *E>	<cept*< td=""><th></th><td></td><td></td><td></td><td>TC</td><th>OP CHORD</th><td>Structur</td><td>al wood</td><td>sheathin</td><td>g directly</td><td>applied or 2-2-0 oc purlin</td><td>s,</td></cept*<>					TC	OP CHORD	Structur	al wood	sheathin	g directly	applied or 2-2-0 oc purlin	s,
50701055	5-7,7-8: 2x6 SP	No.2, 1-4: 2x4 S	P No.1				-		except e	end verti	icals, and	2-0-0 oc	purlins (2-2-0 max.): 5-8.	
WEBS 2	2x4 SP No.1 2x4 SP No.2 *Ex	(cent*					BC		Rigid ce	iling dire	ectly appl	ed or 10-0	0-0 oc bracing.	
WEBS 2	3-18.10-11.9-12	.9-11: 2x4 SP No	0.3				vvi	EBS	TROWA	it mupt		9-11		
DEADTIONO	(													
REACTIONS.	(SIZE) 2=0- Max Horz 2=18	3-8, 11=Iviechan 3(I C 16)	cal											
	Max Uplift 2=-12	21(LC 16), 11=-1	07(LC 12)											
	Max Grav 2=18	91(LC 3), 11=18	57(LC 3)											
FORCES (Ib)	Max Comp /M	av Ten - All for	ces 250 (lb) or l		nt when sh	0000								
TOP CHORD	2-3=-3421/254	. 3-5=-2875/268	. 5-6=-2906/284	. 6-8=-2	615/246. 8-	9=-1951/	/209							
BOT CHORD	2-18=-253/296	4, 17-18=-253/2	964, 15-17=-21	5/2506, <sup>-</sup>	13-15=-321	/3011, 12	2-13=	=-115/1752,						
	11-12=-99/12	47		F 400/	070 0 45	044/000								
WEB2	3-18=0/294, 3- 6-13=-824/220	·17=-650/187, 5- 8-13=-136/127	17=-19/553, 5-1 8 8-12=-494/14	5=-106/ 4 9-12=	670,6-15≕ -70/894 9-	344/203, 11=-202	, 8/17:	2						
	0 10- 02 1/220	, 0 10- 100, 121	0,012-101/11	1, 0 12-	10,001,0	11-202	0, 111	-						
NOTES-														
1) Unbalanced ro	oof live loads ha	ive been conside	ered for this desi	gn.	I -6 Opef: E		Onof.	h_25ft: Cat II: E		olocod:				
MWFRS (env	elope) gable en	d zone and C-C	Exterior(2E) -0-	0-8 to 2	-1-8, Interio	or(1) 2-1-	·8 to	13-7-0, Exterior(	2R) 13-7	-0 to 17	-9-15,			
Interior(1) 17-	9-15 to 34-5-0,	Exterior(2R) 34-8	5-0 to 38-7-15, I	nterior(1	) 38-7-15 to	à 42-0-12	2 zon	e;C-C for membe	ers and f	orces &	,			
MWFRS for re	eactions shown;	Lumber DOL=1	.60 plate grip D	DL=1.60						- 4 -			N'ATH UARO	111
3) TOLL: ASCE	7-16; Pr=20.0 ps s=1.0. Rough Ca	st (root LL: Lum at B: Partially Ext	DOL=1.15 Plate	DOL=1	.15); Pg=15 =1 10   u=5	5.0 pst; P 60-0-0' Mi	in fl	at roof snow load	=1.15 Pl	ate Rain		13.	OFESSIO	N'a
surcharge app	blied to all expos	sed surfaces with	slopes less that	n 0.500	/12 in acco	dance w	ith IE	3C 1608.3.4.	gorome			25		Sin
<ol><li>Unbalanced s</li></ol>	now loads have	been considere	d for this design	•								Y		
5) This truss has	been designed	for greater of m	in roof live load	of 12.0 p	osf or 1.00 t	imes flat	roof	load of 11.6 psf	on overh	angs		Ξ.	SEAL	E E
6) Provide adequ	uate drainage to	prevent water p	ondina.									= :	036322	; =
7) This truss has	been designed	for a 10.0 psf bo	ottom chord live	load nor	nconcurren	t with any	oth	er live loads.				Ξ.	. 030322	1 E -
8) * This truss ha	as been designe	d for a live load	of 20.0psf on th	e bottom	chord in a	ll areas w	vhere	e a rectangle 3-6	-0 tall by	2-0-0 w	vide	E		1 3
9) Refer to girde	n the bottom cho r(s) for truss to t	russ connection	r members, witr	BCDL	= 10.0pst.							1.5	A ANGINEER.	13
10) Provide mec	hanical connect	ion (by others) o	f truss to bearin	g plate o	apable of v	vithstand	ling 1	100 lb uplift at joir	nt(s) exce	ept (jt=ll	o)	14	210	N. I.
11=107.		-											MA. GILD	111
11) One H2.5A S	Simpson Strong	- I le connectors l	ecommended to	connection Connection	ct truss to b	earing wa	alls c	due to UPLIFT at	jt(s) 2. T	nis			- mininini,	
12) This truss is	designed in acc	ordance with the	2018 Internatio	nal Res	idential Co	de sectio	ns R	502.11.1 and R8	02.10.2	and			April 14,20	023
Contileterengelag	and ANSI/T	PI 1.												
🛕 WARNING - V	Verify design parame	ters and READ NOTE	S ON THIS AND IN		IITEK REFERE	NCE PAGE	E MII-7	7473 rev. 5/19/2020 Bl	EFORE US	Е.			ENGINEERING BY	<u> </u>

ARKING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-7473 rev. 5/19/2/02/ BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						157773974
150_1910_C_10X10CP	H3	Hip	1	1		
					Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		. 8.	630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:06:07 2023	Page 2
		ID:VMD6	2rz1viHD	OaRthnrlF	ZtO8K-WtlkBMRAUVRu7LtEQ7H0mvwYvmYYSw?JUJK2	uzR4b

NOTES-

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	3-2-15	16-3-0	24-0-0		31-9-0		36-10-0	42-2-8	-1
Plate Offsets (X,Y) [2:0-0-0	),0-1-1], [7:0-5-0,0-4-8], [12:0-	3-8,0-1-8]	7-9-0		7-9-0		5-1-0	5-4-6	
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         16.5/15.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2 Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TPI20	2-0-0 <b>CSI.</b> 1.15 TC 1.15 BC YES WB 014 Matrix	0.67 0.83 0.72 (-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.22 15-17 -0.40 15-17 0.13 11	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 270 lb	<b>GRIP</b> 244/190 FT = 20%
			BP						
TOP CHORD 2x4 SP No.2* 6-7,7-8: 2x6 S	Except* P No.2		TO	P CHORD	Structural wood except end verti	sheathing cals, and 2	directly app 2-0-0 oc purl	blied or 2-4-9 oc purlins lins (4-6-10 max.): 6-8.	,
BOT CHORD         2x4 SP No.1           WEBS         2x4 SP No.2 *           3-19,5-19,9-12	Except* ,10-11,10-12: 2x4 SP No.3		BO WE	T CHORD EBS	Rigid ceiling dire 1 Row at midpt	ectly applie	ed or 10-0-0 7-17, 7-13	oc bracing.	
REACTIONS. (size) 2=( Max Horz 2=2 Max Uplift 2=- Max Grav 2=1	)-3-8, 11=Mechanical )04(LC 16) 145(LC 16), 11=-69(LC 17) )912(LC 3), 11=1861(LC 3)								
FORCES.         (lb) - Max. Comp./           TOP CHORD         2-3=-3430/2t           8-9=-2067/2t         8-9=-2067/2t           BOT CHORD         2-19=-342/2t           WEBS         5-19=-23/47t           7-13=-1040/r	Vax. Ten All forces 250 (lb) i7, 3-5=-3277/266, 5-6=-2643, i1, 9-10=-1746/169, 10-11=-1 i71, 17-19=-217/2676, 15-17= ), 5-17=-606/191, 6-17=-8/855 i44, 8-13=0/608, 9-13=-65/54	or less except when sho /278, 6-7=-2317/279, 7-{ 771/174 162/2488, 13-15=-162/ 5, 7-17=-421/140, 7-15= 7, 9-12=-738/119, 10-12	own. 3=-1804/237, 2488, 12-13= 0/460, =-113/1735	-99/1515					
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads I</li> <li>2) Wind: ASCE 7-16; Vult=130 MWFRS (envelope) gable envelope) gable enterior(1) 20-5-15 to 31-9-0 MWFRS for reactions show</li> <li>3) TCLL: ASCE 7-16; Pr=20.0 DOL=1.15); Is=1.0; Rough (surcharge applied to all exp 4) Unbalanced snow loads hav</li> <li>5) This truss has been designe non-concurrent with other line</li> <li>6) Provide adequate drainage</li> <li>7) This truss has been designe will fit between the bottom or</li> <li>9) Refer to girder(s) for truss to 10) Provide mechanical conner</li> <li>11) One H2.5A Simpson Stron connection is for uplift only</li> <li>12) This truss is designed in an referenced standard ANSI.</li> <li>13) Graphical purlin represent</li> </ul>	iave been considered for this mph (3-second gust) Vasd=10 nd zone and C-C Exterior(2E) , Exterior(2R) 31-9-0 to 35-11. ; Lumber DOL=1.60 plate grij psf (roof LL: Lum DOL=1.15 F 2at B; Partially Exp.; Ce=1.0; ( >sed surfaces with slopes less 'e been considered for this de: 'd for greater of min roof live lo re loads. to prevent water ponding. 'd for a 10.0 psf bottom chord ned for a live load of 20.0psf o hord and any other members, o truss connections. ction (by others) of truss to be g-Tie connectors recommend and does not consider lateral cordance with the 2018 Intern TPI 1.	design. 33mph; TCDL=6.0psf; B 1-0-10-8 to 2-1-8, Interio -15, Interior(1) 35-11-15 p DOL=1.60 Plate DOL=1.15); Pg=15 Cs=1.00; Ct=1.10, Lu=51 s than 0.500/12 in accorr sign. bad of 12.0 psf or 1.00 ti live load nonconcurrent n the bottom chord in all with BCDL = 10.0psf. earing plate capable of w ed to connect truss to be forces. national Residential Cod or the orientation of the	CDL=6.0psf; f r(1) 2-1-8 to 1 to 42-0-12 zo 0 psf; Pf=16. 0-0-0; Min. fla dance with IB/ mes flat roof l with any othe areas where ithstanding 10 earing walls du e sections R5 purlin along th	h=25ft; Cat. II; E: 16-3-0, Exterior(2 one;C-C for mem 5 psf (Lum DOL= t roof snow load C 1608.3.4. load of 11.6 psf of er live loads. a rectangle 3-6- 00 lb uplift at join ue to UPLIFT at 502.11.1 and R80 ne top and/or bot	xp B; Enclosed; 2R) 16-3-0 to 20 ibers and forces =1.15 Plate governs. Rain on overhangs 0 tall by 2-0-0 w ht(s) 11. jt(s) 2. This 02.10.2 and tom chord.	-5-15, & ide	A CHANNELLING	SEAL 036322	23
WARNING - Verify design parar	neters and READ NOTES ON THIS AN	ID INCLUDED MITEK REFERE	NCE PAGE MII-74	473 rev. 5/19/2020 BE	FORE USE.			ENGINEERING BY	

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





- 2-18=-395/3005, 16-18=-245/2589, 13-16=-106/2059, 12-13=-107/1695
- WEBS 3-18=-294/194, 5-18=-30/599, 5-16=-730/232, 6-16=-17/810, 7-16=-143/285,
  - 7-13=-575/132, 8-13=-11/663, 9-13=-46/339, 9-12=-577/123, 10-12=-109/1825

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 18-11-0, Exterior(2R) 18-11-0 to 23-1-15, Interior(1) 23-1-15 to 29-1-0, Exterior(2R) 29-1-0 to 33-3-15, Interior(1) 33-3-15 to 42-0-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						157773977
150_1910_C_10X10CP	H6	Hip	1	1		
					Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8.	630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:06:14 2023	Page 2
		ID:V	MD62rz1y	iHD_OqRt	bnrlFztQ8K-pDgOflWZqeJuTQvaK5vfZOiqYbx6b9gL5KWw	vi_zR4at

# NOTES-

14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





TOP CHORD

6-7,5-6: 2x6 SP No.2 except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-7. BOT CHORD BOT CHORD 2x4 SP No.2 Rigid ceiling directly applied or 6-0-0 oc bracing. WEBS 2x4 SP No.2 \*Except\* WEBS 5-15, 6-12, 7-12 1 Row at midpt 2-18,4-18,8-9: 2x4 SP No.3 REACTIONS. (size) 1=0-3-9, 12=(0-3-8 + bearing block) (req. 0-3-12), 9=Mechanical Max Horz 1=255(LC 16) Max Uplift 1=-79(LC 16), 12=-188(LC 13), 9=-281(LC 55) Max Grav 1=1018(LC 57), 12=2403(LC 3), 9=89(LC 16) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-1701/150, 2-4=-1527/131, 4-5=-803/104, 5-6=-350/70, 6-7=-51/626, 7-8=-45/285, 8-9=-65/343 BOT CHORD 1-18=-323/1459, 16-18=-191/1074, 15-16=-59/657 WEBS 2-18=-271/177, 4-18=-25/554, 4-16=-673/192, 5-16=-72/734, 5-15=-751/148, 6-15=-70/974, 6-12=-1439/226, 7-12=-918/129, 7-10=-16/442, 8-10=-352/86 NOTES-

1) 2x4 SP No.2 bearing block 12" long at jt. 12 attached to front face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. User Defined Bearing crushing capacity= 425psi.

2) Unbalanced roof live loads have been considered for this design.

3) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-13 to 31-13, Interior(1) 3-1-13 to 16-3-0, Exterior(2R) 16-3-0 to 20-5-15, Interior(1) 20-5-15 to 31-9-0, Exterior(2R) 31-9-0 to 35-11-15, Interior(1) 35-11-15 to 36-5-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

5) Unbalanced snow loads have been considered for this design.

6) Provide adequate drainage to prevent water ponding.

2x4 SP No.2 \*Except\*

TOP CHORD

7) All plates are 3x6 MT20 unless otherwise indicated.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

10) Refer to girder(s) for truss to truss connections.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=281.

12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 12. This connection is for uplift only and does not consider lateral forces.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **MSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 3-10-3 oc purlins,



Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						157773978
150_1910_C_10X10CP	H7	Hip	1	1		
					Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8.	630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:06:17 2023	Page 2
		ID:VMD	062rz1yiHl	D_OqRtbni	rlFztQ8K-DoMWHnYS7ZiTKte90ETMA0KLDozBoWvnnllaJ	JzR4aq

## NOTES-

13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Refer to girder(s) for truss to truss connections.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10 = 141
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 13. This connection is for uplift only and does not consider lateral forces.
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFUKE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

GILB

April 14,2023

minim

C



Scale: 3/16"=1'



	5-6-15	10-11-0	19-8-12		27-5-12		36-7-0	
Plate Offse	ets (X,Y) [1:0-0-0,0	-0-13]	0-5-12		7-5-0		3-1-4	
LOADING TCLL (roo Snow (Pf/F TCDL BCLL BCDI	(psf) f) 20.0 Pg) 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/T	2-0-0 <b>CSI.</b> 1.15 TC 1.15 BC YES WB Pl2014 Matrix-	0.84 <b>DEFL.</b> 0.79 Vert(LL) 0.68 Horz(CT) S	in (loc) l/d -0.24 9-11 >4 -0.41 9-11 >20 0.04 11 r	efl L/d 52 240 64 180 n/a n/a	PLATES MT20 Weight: 202 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHO BOT CHO WEBS REACTIO	RD 2x4 SP No.2 RD 2x4 SP No.2 2x4 SP No.3 *Ex 4-16,5-14,5-11,7 NS. (size) 1=0-3 Max Horz 1=20	cept* -9: 2x4 SP No.2 3-8, 9=Mechanical, 11=( 3(LC 16)	D-3-8 + bearing block) (req. (	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood shea except end verticals, Rigid ceiling directly 1 Row at midpt	athing directly a and 2-0-0 oc p applied or 6-0-( 5-11	pplied or 3-11-6 oc purlii urlins (4-11-5 max.): 3-8 ) oc bracing.	ns,
FORCES. TOP CHO BOT CHO WEBS	Max Opint 1=-ot Max Grav 1=10 (lb) - Max. Comp./M: RD 1-2=-1719/113 RD 1-17=-235/147 2-16=-543/167 5-11=-1452/24	03(LC 18), 9=200(LC 41) 03(LC 39), 9=38(LC 16) ax. Ten All forces 250 , 2-3=-1298/111, 3-4=-1 2, 16-17=-235/1472, 14- , 3-16=0/299, 4-16=-71/- 2, 7-11=-938/209, 7-9=-	(lb) or less except when show 103/123, 4-5=-643/71, 5-7=-1 16=-138/895, 9-11=-489/27 141, 4-14=-661/164, 5-14=-6 32/678	wn. 103/844 1/1090,				
NOTES- 1) 2x4 SP fastene 2) Unbalar 3) Wind: A MWFR 15-1-15 grip DC 4) TCLL: A DOL=1. surchar 5) Unbalar 6) Provide 7) All plate 8) This tru 9) * This tr will fit b 10) Refer 11) Provide 9=200 12) One H conne 13) This tr reference 14) Grandh	No.2 bearing block 12 rs. User Defined Beari need roof live loads ha SCE 7-16; Vult=130m S (envelope) gable end i, Interior(1) 15-1-15 to U=1.60 SCE 7-16; Pr=20.0 ps ge applied to all expos need snow loads have adequate drainage to ss are 3x6 MT20 unless ss has been designed tuss has been designed	" long at jt. 11 attached in ng crushing capacity= 4: ve been considered for i ph (3-second gust) Vasc 3 zone and C-C Exteriori 36-5-4 zone;C-C for me of (roof LL: Lum DOL=1. t B; Partially Exp.; Ce=1 ed surfaces with slopes been considered for this prevent water ponding. s otherwise indicated. for a 10.0 psf bottom ch for a 10.0 psf bottom ch	o front face with 2 rows of 10 25psi. his design. I=103mph; TCDL=6.0psf; BC 2E) 0-1-12 to 3-1-12, Interior mbers and forces & MWFRS 15 Plate DOL=1.15); Pg=15.0 0; Cs=1.00; Ct=1.10, Lu=50- less than 0.500/12 in accords a design. ord live load nonconcurrent w sf on the bottom chord in all a ers, with BCDL = 10.0psf. b bearing plate capable of with ended to connect truss to bear eral forces. tternational Residential Code	0d (0.131"x3") nails spaced CDL=6.0psf; h=25ft; Cat. II; E (1) 3-1-12 to 10-11-0, Exter for reactions shown; Lumb 0 psf; Pf=16.5 psf (Lum DOL -0-0; Min. flat roof snow load ance with IBC 1608.3.4. with any other live loads. areas where a rectangle 3-6 thstanding 100 lb uplift at joi aring walls due to UPLIFT a e sections R502.11.1 and R8	3" o.c. 8 Total Exp B; Enclosed; ior(2R) 10-11-0 to er DOL=1.60 plate L=1.15 Plate d governs. Rain 3-0 tall by 2-0-0 wide int(s) except (jt=lb) t jt(s) 1 and 11. This 302.10.2 and	and a second sec	SEAL 036322	All
WAR Design a truss s building is alway fabricati Safety I	NING - Verify design paramet valid for use only with MiTekt system. Before use, the build design. Bracing indicated is s required for stability and to on, storage, delivery, erection nformation available from	ters and READ NOTES ON THI © connectors. This design is by ing designer must verify the ap to prevent buckling of individu prevent collapse with possible n and bracing of trusses and tr Truss Plate Institute, 2670 Crai	S AND INCLUDED MITEK REFERENCE used only upon parameters shown, ar plicability of design parameters and p al truss web and/or chord members c personal injury and property damage uss systems, see <b>ANSI/TPI</b> n Highway, Suite 203 Waldorf, MD 20	CE PAGE MII-7473 rev. 5/19/2020 B nd is for an individual building comp properly incorporate this design into only. Additional temporary and per - For general guidance regarding 11 Quality Criteria, DSB-89 and B0 0601	BEFORE USE. ponent, not b the overall manent bracing the CSI Building Component		ENGINEERING BY AMITEK A 818 Soundside Road Edenton, NC 27932	<b>O</b> ffiliate



Scale: 3/16"=1'

KENLU

818 Soundside Road Edenton, NC 27932



L	8-3-0	17-0-1	2	27-5-12			36-7-0	1
	8-3-0	8-9-1	2	10-5-0			9-1-4	
Plate Offse	ets (X,Y) [5:0-4-0,	0-3-0]	1	1			Г	
LOADING TCLL (roof) Snow (Pf/P	(psf) ) 20.0 /g) 16.5/15.0 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15	<b>CSI.</b> TC 1.00 BC 0.83	DEFL. Vert(LL) - Vert(CT) -	in (loc) l 0.18 10-12 > 0.38 10-12 >	l/defl L/d >999 240 >865 180	PLATES MT20	<b>GRIP</b> 244/190
BCLL BCDL	0.0 * 10.0	Rep Stress Incr YES Code IRC2018/TPI2014	WB 0.52 Matrix-S	Horz(CT)	0.04 10	n/a n/a	Weight: 184 lb	FT = 20%
LUMBER- TOP CHOP BOT CHOP WEBS REACTION	RD 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 *E 4-14,5-12,5-10,1 VS. (size) 1=0- Max Horz 1=15 Max Uplift 1=-5 Max Gray 1=0 <sup>-</sup>	xcept* 5-9: 2x4 SP No.2 3-8, 9=Mechanical, 10=0-3-8 55(LC 16) 2(LC 16), 9=-135(LC 37), 10=-307(LC 22(LC 16), 9=-00(LC 16), 10=2156(LC 3	E T E (13) 6)	BRACING- OP CHORD Stri EXC BOT CHORD Rig 6-0 VEBS 1 R	ructural wood sh cept end vertica gid ceiling direct 0-0 oc bracing: 9 Row at midpt	neathing directly app Ils, and 2-0-0 oc pu Ily applied or 10-0-0 9-10. 5-10	plied or 4-4-6 oc purlin: rlins (2-2-0 max.): 3-8. ) oc bracing, Except:	S,
FORCES. TOP CHOF BOT CHOF WEBS	(Ib) - Max. Comp./N RD 1-2=-1604/160 RD 1-14=-228/136 2-14=-288/16' 6-10=-1049/25	lax. Ten All forces 250 (lb) or less e 0, 2-3=-1402/135, 3-4=-1203/136, 4-5 56, 12-14=-206/1340, 10-12=-98/318, 1, 3-14=0/375, 4-12=-520/164, 5-12=- 56, 6-9=-51/661	xcept when shown. =-1069/121, 5-6=-177/103 9-10=-530/44 36/1016, 5-10=-1705/346,	1				
NOTES- 1) Unbalan 2) Wind: A: MWFRS Interior( DOL=1.( 3) TCLL: A DOL=1.' surcharg 4) Unbalan 5) Provide 6) All plate: 7) This trus 8) * This trus 8) * This trus 9) Refer to 10) Provide 9=135. 11) One H: connecc 12) This trus 11) One H: connecc 12) This trus 13) Graphi	iced roof live loads has SCE 7-16; Vult=130n 6 (envelope) gable en 1) 12-5-15 to 366-54 z 60 SCE 7-16; Pr=20.0 p 15); Is=1.0; Rough C ge applied to all expo iced snow loads have adequate drainage to s has been designed uss has been designed state at a state of the state is a state of the state is a state of the state care at the bottom ch girder(s) for truss to g mechanical connec 2.5A Simpson Strong tion is for uplift only a uss is designed in act care d standard ANSI/T cal purlin represental	ave been considered for this design. nph (3-second gust) Vasd=103mph; T d zone and C-C Exterior(2E) 0-1-12 tr zone;C-C for members and forces & N sf (roof LL: Lum DOL=1.15 Plate DOL at B; Partially Exp.; Ce=1.0; Cs=1.00; sed surfaces with slopes less than 0.5 b been considered for this design. b prevent water ponding. ss otherwise indicated. If or a 10.0 psf bottom chord live load ad for a live load of 20.0psf on the bot ord and any other members. truss connections. tion (by others) of truss to bearing pla I-Tie connectors recommended to con and does not consider lateral forces. zordance with the 2018 International F TPI 1. ion does not depict the size or the original	CDL=6.0psf; BCDL=6.0ps p 3-1-12, Interior(1) 3-1-12 IWFRS for reactions show =1.15); Pg=15.0 psf; Pf=1 Ct=1.10, Lu=50-0-0; Min. 1 00/12 in accordance with nonconcurrent with any ot icom chord in all areas whe te capable of withstanding nect truss to bearing walls Residential Code sections is entation of the purlin along	f; h=25ft; Cat. II; Exp to 8-3-0, Exterior(2R) n; Lumber DOL=1.60 6.5 psf (Lum DOL=1. flat roof snow load go IBC 1608.3.4. her live loads. re a rectangle 3-6-0 ta 100 lb uplift at joint(s) due to UPLIFT at jt(s R502.11.1 and R802. the top and/or bottom	B; Enclosed; ) 8-3-0 to 12-5- ) plate grip 15 Plate werns. Rain all by 2-0-0 wide all by 2-0-0 wide () except (jt=lb) a) 1 and 10. This 10.2 and n chord.	15, e	SEAL 036322	A
WARN Design v	NING - Verify design parame	eters and READ NOTES ON THIS AND INCLUDE	D MITEK REFERENCE PAGE MI	-7473 rev. 5/19/2020 BEFOI	RE USE.		ENGINEERING BY	

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	150.1910 C 10x10CP	
				· ·		157773982
150_1910_C_10X10CP	HG1	Half Hip Girder	1	ົ		
				<b>Z</b>	Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8.	630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:06:35 2023	Page 2
		ID:VMD	62rz1yiHD	OgRtbnr	IFztQ8K-hGSK3xmku5zvUe0c30oavp4T136C0g6Rw56Xy	HzR4aY

#### NOTES-

- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=776.
- 13) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 16) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

# LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
  - Uniform Loads (plf) Vert: 1-3=-43, 3-11=-53, 2-12=-20
  - Concentrated Loads (lb)
    - Vert: 3=-26(B) 6=-21(B) 21=-17(B) 20=-17(B) 4=-21(B) 5=-21(B) 18=-17(B) 7=-21(B) 16=-17(B) 22=-34(B) 23=-1(B) 24=-21(B) 25=-21(B) 26=-21(B) 27=-21(B) 29=-21(B) 30=-21(B) 30=-21(B) 31=-21(B) 32=-21(B) 35=-21(B) 36=-21(B) 36=-21

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







	5-7-0	11-2-0	16-7-4	22-0-8	27-5-12	31-11-8	36-7-0	
	5-7-0	<u>5-7-0</u>	5-5-4 '	5-5-4	5-5-4	4-5-12	4-7-8	1
Plate Olise	(X,Y) - [1:0-0-7,0]	J-0-5], [2:0-6-0,0-2-8]						
LOADING TCLL (roof) Snow (Pf/P TCDL BCLL BCDL	(psf) 20.0 g) 16.5/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCode IRC2018/TPI2014	CSI. TC 0.79 BC 0.55 WB 0.72 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/de 0.17 18-20 >99 -0.28 18-20 >99 0.04 14 n/	fl L/d 9 240 9 180 a n/a	PLATES MT20 Weight: 219 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHOF BOT CHOF WEBS	<ul> <li>2x4 SP No.2</li> <li>2x6 SP No.2</li> <li>2x4 SP No.3 *Ex</li> <li>2-20,4-20,4-17,7</li> </ul>	ccept* 7-17: 2x4 SP No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood shea except end verticals, Rigid ceiling directly a 1 Row at midpt	thing directly app and 2-0-0 oc pur pplied or 5-0-13 4-17	blied or 3-0-5 oc purlins, lins (2-10-3 max.): 2-10 oc bracing.	
REACTION	IS. (size) 1=0-3 Max Horz 1=10 Max Uplift 1=-30 Max Grav 1=12	3-8, 11=Mechanical, 14=(0-3-8 + bear I7(LC 12) 07(LC 9), 11=-174(LC 2), 14=-954(LC 87(LC 2), 11=49(LC 9), 14=2886(LC	ing block) (req. 0-4-8) 9) 2)	)				
FORCES. TOP CHOF	(lb) - Max. Comp./M C 1-2=-2278/626 7-9=-190/608	ax. Ten All forces 250 (lb) or less ex 5, 2-3=-2643/848, 3-4=-2643/848, 4-6= 9-10=-190/608 10-11=-23/255	ccept when shown. 664/227, 6-7=-664/22	27,				
BOT CHOP	RD 1-21=-578/193 12-14=-1911/	6, 20-21=-575/1949, 18-20=-728/223 607	6, 17-18=-728/2236, 1	4-17=-1911/607,				
WEBS	2-21=0/382, 2- 4-17=-1783/56 9-12=-386/251	20=-327/835, 3-20=-490/316, 4-20=-1 9, 6-17=-445/298, 7-17=-946/2923, 7- , 10-12=-751/240	36/509, 4-18=0/330, -14=-2653/989, 7-12=-	-498/1556,				
NOTES- 1) 2x6 SP I fastener 2) Wind: AS MWFRS	No.2 bearing block 12 s. User Defined Beari SCE 7-16; Vult=130m (envelope) gable end	2" long at jt. 14 attached to front face v ing crushing capacity= 425psi. iph (3-second gust) Vasd=103mph; T( d zone; Lumber DOL=1.60 plate grip I	vith 3 rows of 10d (0.1 CDL=6.0psf; BCDL=6. DOL=1.60	31"x3") nails spaced 0psf; h=25ft; Cat. II; E	3" o.c. 12 Total Exp B; Enclosed;		TH CARO	

- 3) TCLL: ASCE 7-16; PT=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.

0-9-0

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=174.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This
  connection is for uplift only and does not consider lateral forces.
- 11) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14. This connection is for uplift only and does not consider lateral forces.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and Continue Contin

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **MSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						157773983
150_1910_C_10X10CP	HG10	HALF HIP GIRDER	1	1		
					Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8.	630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:06:39 2023	Page 2
		ID:'	VMD62rz1	viHD OgR	RtbnrlFztQ8K-a1hrvlpFxKTLzFKNIrsW3fE2RgYeyRw0rj4k52	2zR4aU

#### NOTES-

- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 15) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-43, 2-10=-53, 1-11=-20

Concentrated Loads (lb)

Vert: 2=-26(F) 5=-21(F) 21=-17(F) 7=-21(F) 8=-21(F) 22=-34(F) 23=-1(F) 24=-21(F) 25=-21(F) 26=-21(F) 27=-21(F) 28=-21(F) 30=-21(F) 30=-21(F) 31=-21(F) 32=-21(F) 33=-21(F) 33=-2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





		3	-11-4		1				
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         11.6/15.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.23 BC 0.07 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.01 -0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 18 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2

**REACTIONS.** (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=114(LC 14)

Max Uplift 3=-83(LC 14), 2=-7(LC 14)

Max Grav 3=115(LC 26), 2=219(LC 2), 4=75(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
   8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
- connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 3-11-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Fiale Olisels (	<u>, , , , , [3.0-3-12</u>	.,0-2-0]								
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.08 BC 0.11 WB 0.03 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.01 0.01	(loc) 6 6 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 20 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP No.2 2x6 SP No.2 2x4 SP No.3		B T B	RACING- DP CHORD DT CHORD	Structura except 2-0-0 oc Rigid cei	al wood purlins: ling dire	sheathin 3-4. ectly appl	g directly a ied or 10-0-	pplied or 3-11-4 oc purl •0 oc bracing.	ins,
DEACTIONS	(oizo) 4 M	achanical 2 0 2 9 F Machanical								

```
REACTIONS. (size) 4=Mechanical, 2=0-3-8, 5=Mechanical
Max Horz 2=80(LC 14)
Max Uplift 4=-18(LC 10), 2=-24(LC 14), 5=-14(LC 14)
Max Grav 4=43(LC 2), 2=219(LC 2), 5=98(LC 2)
```

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 2-5-4, Exterior(2E) 2-5-4 to 3-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
   10) N/A
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **MSIVTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BCLL BCDL	0.0 * 10.0	Code IRC2018/TPI2014	WB 0.00 Matrix-P	Horz(CT)	0.01	4	n/a	n/a	Weight: 18 lb	FT = 20%
LUMBER-			BI	RACING-						
TOP CHORD	2x4 SP No.2		тс	OP CHORD	Structural	wood s	heathing	g directly app	blied or 3-11-4 oc purlins,	
BOT CHORD	2x6 SP No.2				except					
WEBS	2x4 SP No.3				2-0-0 oc p	urlins:	3-4.			
			BC	DT CHORD	Rigid ceilir	ng direo	ctly appli	ed or 10-0-0	oc bracing.	
REACTIONS.	(size) 2=0-	-3-8 4=Mechanical 6=Mechanical			0	•			0	

REACTIONS. (size) 2=0-3-8, 4=Mechanical, 6=Mechanical Max Horz 2=44(LC 10) Max Uplift 2=-31(LC 10), 4=-42(LC 7) Max Grav 2=210(LC 2), 4=86(LC 2), 6=83(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 11) N/A
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 16) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **MSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						157773986
150_1910_C_10X10CP	J3	Jack-Open Girder	2	1		
					Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8.	630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:06:42 2023	Page 2
		ID	:VMD62rz	1yiHD_Oq	RtbnrlFztQ8KcNzXKs7EFrwqj2yz_QDhlsjRthy9ztSXhlOi	NzR4aR

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-43, 3-4=-53, 2-5=-20

Concentrated Loads (lb) Vert: 7=-1(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LOADING         (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         11.6/15.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2018/TPI2014	<b>CSI.</b> TC 0.05 BC 0.02 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 -0.00	(loc) 2 2 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 7 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-		BF	ACING-						

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No 2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=44(LC 16)

Max Uplift 3=-25(LC 16), 2=-22(LC 16)

Max Grav 3=30(LC 23), 2=132(LC 23), 4=31(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 1-7-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No 2

REACTIONS. (size) 1=0-3-8, 2=Mechanical, 3=Mechanical

Max Horz 1=36(LC 16)

Max Uplift 2=-30(LC 16)

Max Grav 1=62(LC 2), 2=46(LC 2), 3=31(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 1-7-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **MSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



# OTHERS 2x4 SP No.3

REACTIONS. All bearings 10-0-0.

(lb) - Max Horz 2=-92(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 14, 11, 10

Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 2-1-8, Exterior(2N) 2-1-8 to 5-0-0, Corner(3R) 5-0-0 to 8-0-0, Exterior(2N) 8-0-0 to 10-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) N/A

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



REACTIONS. (size) 2=8-1-12, 4=8-1-12, 6=8-1-12 Max Horz 2=-37(LC 21) Max Uplift 2=-46(LC 16), 4=-53(LC 17)

Max Grav 2=202(LC 23), 4=202(LC 24), 6=317(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-11 to 3-4-11, Interior(1) 3-4-11 to 5-0-3, Exterior(2R) 5-0-3 to 8-0-3, Interior(1) 8-0-3 to 9-7-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate
- DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 9) N/A
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



1		10-0-6							1
Γ		10-0-6							
Plate Offsets (X,Y) [3:0-3-0,	Edge], [5:0-3-0,Edge]								
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.14 BC 0.16 WB 0.05 Matrix-S	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 7 7 6	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 28 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 POT CHORD 2x4 SP No.2		BF TC	RACING- OP CHORD	Structural	wood	sheathin	g directly app	plied or 6-0-0 oc purlir	ns, except
WEBS 2x4 SP No.3		BC	T CHORD	Rigid ceili	na dire	ctlv appl	ied or 10-0-0	oc bracing.	

REACTIONS. (size) 2=8-1-12, 6=8-1-12, 8=8-1-12 Max Horz 2=-16(LC 17) Max Uplift 2=-34(LC 16), 6=-34(LC 17), 8=-23(LC 13)

Max Grav 2=215(LC 41), 6=215(LC 41), 8=342(LC 40)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-11 to 2-7-3, Exterior(2R) 2-7-3 to 6-10-2, Interior(1) 6-10-2 to 7-5-3, Exterior(2E) 7-5-3 to 9-7-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) Gable requires continuous bottom chord bearing.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) N/A

- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



TRENGINEERING BY A MiTek Affiliate 818 Soundside Road

Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



	I	9-7-5		18-11-13	1	19-3 <sub>1</sub> 1	0 27-5-	12	29	-0-3	36-7-0		
<b>D</b>		9-7-5		9-4-8	(	0-3-1	4 8-2-2	2	'1-	6-7 '	7-6-13	1	
Plate Offsets (X,Y)	[2:0-0-0,	<u>0-0-9], [6:0-3-0,0-2-0], [8:0</u>	0-9-0,0-2-0], [9	:Edge,0-1-1	2]								
LOADING (psf)		SPACING	200	661			DEEL	in	(loc)	l/dofl	L /d		CDID
TCLL (roof)	20.0	Blate Grip DOI	2-0-0	C31.	0.00		Vort(LL)	0 10	2 1 9	> 000	240	MT20	244/100
Snow (Pf/Pg) 16.5/1	15.0		1.15	PC	0.00		Vert(CT)	-0.19	2-10	>999	190	MIQAUS	196/170
TCDL	10.0	Rep Stress Incr	VES	W/B	0.73			-0.41	2-10	>/90 n/a	n/a	WITCALIS	100/179
BCLL	0.0 *	Code IRC2018/TP	12014	Matri	0.02 v-S		11012(01)	0.05	15	n/a	n/a	Weight: 272 lb	ET - 20%
BCDL	10.0		12014	Math	x-0								11 = 2078
LUMBER-						BR	ACING-						
TOP CHORD 2x4	SP No.2 *E	xcept*				TOF	P CHORD	Structura	al wood	sheathin	a directly appl	lied or 3-8-11 oc purlin	S.
8-9:	2x4 SP No.	.1						except e	nd vert	icals, and	2-0-0 oc purli	ins (6-0-0 max.): 6-8.	-,
BOT CHORD 2x6	SP No.2 *E	xcept*				BO	T CHORD	Rigid cei	ilina dir	ectly appl	ied or 6-0-0 or	c bracing.	
2-17	: 2x4 SP No	0.1, 13-16: 2x4 SP No.2				WE	BS	1 Row a	t midpt		5-16. 6-16.	7-13, 8-13, 9-11	
WEBS 2x4 S	SP No.2 *E	xcept*									, ,	,,	
3-18	.9-10: 2x4 \$	SP No.3											
	,												
REACTIONS. (s	size) 2=0-	-3-8. 13=0-3-8. 10=Mecha	nical										
Max	Horz 2=28	36(LC 16)											
Max	Uplift 2=-9	95(LC 16), 13=-224(LC 16)	, 10=-246(LC	57)									
Max	Grav 2=10	048(LC 59), 13=2233(LC 3	3), 10=137(LC	46)									
FORCES. (lb) - Ma	x. Comp./N	lax. Ten All forces 250 (	lb) or less exce	ept when sh	own.								
TOP CHORD 2-3	3=-1586/138	8, 3-5=-1375/115, 5-6=-50	3/78, 6-7=-378	8/96, 7-8=-3	7/439, 8-9	=-89	/429,						
9-1	0=-80/318												
BOT CHORD 2-1	8=-324/13	57, 16-18=-173/922, 11-13	3=-291/100										
WEBS 3-1	8=-333/202	2, 5-18=-27/611, 5-16=-77	4/237, 7-16=-1	29/993, 7-1	3=-1235/1	165,							
8-1	3=-827/108	8, 8-11=0/357, 9-11=-386/	131										
NOTES-													
<ol> <li>Unbalanced roof I</li> </ol>	ive loads ha	ave been considered for th	nis design.										
<ol><li>Wind: ASCE 7-16</li></ol>	; Vult=130n	nph (3-second gust) Vasd	=103mph; TCE	DL=6.0psf; E	BCDL=6.0p	psf; h	n=25ft; Cat. II; E	Exp B; En	closed;				
MWFRS (envelop	e) gable en	id zone and C-C Exterior(2	2E) -0-10-8 to 2	2-1-8, Interio	or(1) 2-1-8	3 to 1	8-11-13, Exterio	or(2R) 18	-11-13	to		N'I CAD''I	
23-3-10, Interior(1	) 23-3-10 to	o 29-0-3, Exterior(2R) 29-0	)-3 to 33-3-2, I	nterior(1) 33	3-3-2 to 36	6-5-4	zone;C-C for m	nembers	and for	ces &	111	TH UARO,	11,
MWFRS for reacti	ions shown	; Lumber DOL=1.60 plate	grip DOL=1.60	)							10	A STORES	IN'S
<ol><li>3) TCLL: ASCE 7-16</li></ol>	6; Pr=20.0 p	osf (roof LL: Lum DOL=1.1	5 Plate DOL=1	l.15); Pg=15	5.0 psf; Pf=	=16.5	5 psf (Lum DOL	.=1.15 Pla	ate		1.2	FEWDON	Sie
DOL=1.15); Is=1.0	0; Rough C	at B; Partially Exp.; Ce=1.0	0; Cs=1.00; Ct	=1.10, Lu=5	0-0-0; Mir	n. flat	roof snow load	l governs	. Rain		un		111
surcharge applied	l to all expo	sed surfaces with slopes le	ess than 0.500	)/12 in accor	dance wit	th IBC	C 1608.3.4.				4		16
<ol><li>Unbalanced snow</li></ol>	loads have	e been considered for this	design.								2 2	SEAL	: =
<ol><li>This truss has bee</li></ol>	en designed	d for greater of min roof live	e load of 12.0	psf or 1.00 t	imes flat r	roof lo	bad of 11.6 psf	on overh	angs		= :	JLAL	: =
non-concurrent wi	ith other live	e loads.										036322	· · · · ·
6) Provide adequate	drainage to	prevent water ponding.											- E
7) All plates are M12	20 plates un	less otherwise indicated.									- E - N		£ 3
8) This truss has bee	en designed	for a 10.0 psr bottom cho	ord live load no	nconcurren	t with any	othei	r live loads.				20	· SNOWFER.	んご
9) " This truss has be	een designe	ed for a live load of 20.0ps	t on the botton	n chord in a	li areas wr	nere	a rectangle 3-6-	-0 tall by	2-0-0 W	lide	1.8	GINE	2.5
Will fit between the	e bottom ch	ord and any other membe	rs, with BCDL	= 10.0pst.							111	CADIB	and the second
10) Refer to girder(s	) IOF TRUSS to	tion (by others) of the	heering also	aanahi'		10		at(a)		L)	11	A. GILLI	N
10-246	ical connec	suon (by others) of truss to	bearing plate	capable of v	viinstandir	ng 10	io is uplift at joir	m(s) exce	≠pt (Jt=ll	0)		annun.	
10=240.	oon Strong	Tio connectors recomme	ndod to conno	ot truce to b	ooring we	الم طر		it(c) 2 cr	d 12 T	hic		April 14,20	23
	Son Suong	- ne connectors recomme	nueu lo conne	CI ITUSS IO D	earing wa	uis di	ie to UPLIFT at	$J_{I}(S) \ge ar$	iu 13. I	1115		• •	

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						157773993
150_1910_C_10X10CP	T1	ROOF TRUSS	3	1		
					Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8	.630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:06:52 2023	Page 2
		ID:\	/MD62rz1	yiHD OqR	tbnrlFztQ8K-hXzldlzPtK6V0FptZ4bZ5PHGyvxuVMhxqFjw2i	nzR4aH

## NOTES-

- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 15) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





ŀ	9-7-5		18-11-13		24-0-0	29-0	-3		36-10-4	4	37-0-0	42-6-0	
Diata Offact	9-7-5		9-4-8		5-0-3	5-0-	3		7-10-1		0-1-12	5-6-0	
Plate Oliset	S (A, T) [0.0-3-0,0	J-2-0], [8.0-3-0,0-2-0	J, [9.0-4-0,0-3-0]			1							
LOADING ( TCLL (roof) Snow (Pf/Pg TCDL BCLL	(psf) 20.0 1) 16.5/15.0 10.0	<b>SPACING-</b> Plate Grip D0 Lumber D0L Rep Stress Ir	2-0-0 DL 1.15 . 1.15 nor YES	CSI. TC BC WB	0.80 0.74 0.40	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.15 -0.27 0.06	(loc) 18-20 18-20 12	l/defl >999 >999 n/a	L/d 240 180 n/a		PLATES MT20	<b>GRIP</b> 244/190
BCDI	10.0	Code IRC20	18/TPI2014	Matri	x-S							Weight: 305 lb	FT = 20%
LUMBER- TOP CHORI BOT CHORI	D 2x4 SP No.2 D 2x6 SP No.2 *E: 15-17: 2x4 SP N 2x4 SP No.2 *E: 3-20,10-11,10-1	kcept* lo.2 kcept* 2: 2x4 SP No.3			B T( B( W	RACING- DP CHORD DT CHORD EBS	Structura except e Rigid cei 1 Row at	al wood nd verti iling dire t midpt	sheathing cals, and ectly appli	g directly 2-0-0 or ied or 10 5-18,	y applied c purlins ( )-0-0 oc b , 7-14, 9-1	or 2-10-11 oc purl (4-6-6 max.): 6-8. racing. 2	ins,
REACTION	S. (size) 2=0- Max Horz 2=22 Max Uplift 2=-1 Max Grav 2=16	3-8, 12=0-3-8 24(LC 16) 63(LC 16), 12=-112 645(LC 3), 12=2173(	(LC 17) (LC 3)										
FORCES. TOP CHOR BOT CHOR WEBS	(lb) - Max. Comp./M D 2-3=-2901/281 8-9=-1219/192 D 2-20=-387/250 3-20=-305/195 7-14=-770/117	ax. Ten All forces , 3-5=-2684/257, 5- 2, 9-10=-26/305 16, 18-20=-240/2061 5, 5-20=-24/639, 5-1 7, 8-14=-3/315, 9-14	250 (lb) or less exc 6=-1813/237, 6-7=- I, 14-18=-79/1354 8=-783/233, 6-18=0 =-9/1151, 9-12=-17	ept when sh 1559/243, 7- /518, 7-18=- 95/226	iown. -8=-1017/207 -69/570,	, ,							
NOTES- 1) Unbalanc 2) Wind: AS MWFRS 23-2-11, 1 MWFRS 3) TCLL: AS DOL=1.1: surcharge 4) Unbalanc 5) This trus non-conc 6) Provide a 7) All plates 8) This trus 9) * This trus will fit bet 10) One H2 connect 11) This trus referenc 12) Graphic 13) ATTIC	ed roof live loads ha CE 7-16; Vult=130m (envelope) gable en Interior(1) 23-2-11 tc for reactions shown; GCE 7-16; Pr=20.0 p 5); Is=1.0; Rough Ca e applied to all expose ed snow loads have shas been designed shas been desi	ave been considered ph (3-second gust) d zone and C-C Ext 0 29-0-3, Exterior(2F Lumber DOL=1.60 sf (roof LL: Lum DO at B; Partially Exp.; been considered for for greater of min re loads. prevent water ponds so therwise indicate for a 10.0 psf botto d for a live load of 2 ord and any other m -Tie connectors recand does not conside cordance with the 20 Pl 1. ion does not depict DESIGNED AS UNIT	I for this design. Vasd=103mph; TCI erior(2E) -0-10-8 to R) 29-0-3 to 33-3-2, I plate grip DOL=1.6( L=1.15 Plate DOL= Ce=1.0; Cs=1.00; Cf opes less than 0.500 or this design. oof live load of 12.0 ding. ad. m chord live load nc 20.0psf on the bottor lembers, with BCDL ommended to conne er lateral forces. D18 International Re: the size or the orien NHABITABLE.	DL=6.0psf; E 2-1-8, Interior nterior(1) 33 1.15); Pg=15 =1.10, Lu=5 0/12 in accol psf or 1.00 t enconcurren n chord in a = 10.0psf. tot truss to b sidential Contact	3CDL=6.0psf or(1) 2-1-8 to 3-3-2 to 42-4- 5.0 psf; Pf=16 50-0-0; Min. fl dance with II times flat rool t with any oth II areas wher earing walls de sections F purlin along	; h=25ft; Cat. II; E 18-11-13, Exterii 4 zone;C-C for m 5.5 psf (Lum DOL at roof snow load BC 1608.3.4. f load of 11.6 psf her live loads. e a rectangle 3-6 due to UPLIFT at 2502.11.1 and R8 the top and/or bo	Exp B; En or(2R) 18 hembers a sembers a d governs on overha -0 tall by -0 tall by -1 tall by -0 tall by to tall by to tall by the constant to tall by the constant constant -0 tall by -0 t	closed; -11-13 ; and forc ate . Rain angs 2-0-0 w nd 12. T and rd.	to xes & ide his	A COLUMNIA AND	TRACE STRACT	SEAL 036322 A. GILBE	A.I.I.
Design val a truss sys	NG - Verify design parame lid for use only with MiTek stem. Before use, the built asign. Bracing indicated it	eters and READ NOTES O ® connectors. This design ding designer must verify s to prevent buckling of in	IN THIS AND INCLUDED In In is based only upon para the applicability of design adividual truss web and/or	MITEK REFERE ameters shown parameters an chord member	And is for an ind and properly incorp	7473 rev. 5/19/2020 B dividual building comp porate this design into al temporary and perr	EFORE USE onent, not the overall	E.				ENGINEERING BY	0

A MiTek Affiliate 818 Soundside Road Edenton, NC 27932

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Brace Offsets (X/T)         E0-0.0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0	L	9-7-5	18-11-13	24-0-0	29-0-3	35-3-3	1	42-4-4	42 <sub>1</sub> 6-0	48-0-0			
Prace Unsets       K.Y1       [ICO-94:00-240], [B0-94:00-240]       CSI.       DEFL.       in       (Ioc)       Videt       L/d       PLATES       GRIP         Show (P/Pig)       15/15.0       Iumber Dotu       1.15       BC       0.72       1.02       1.5.       958       240       MT2D       244/190         Show (P/Pig)       10.0       Rep Stress Incr       YES       WB       0.77       Uver(C1)       0.49       15.18       958       240         BCDL       10.0       Code IRC2018/TPI2014       Matrix-S       BRACINC-       TOP CHORD       24.90       95.64       74.9		9-7-5	9-4-8	5-0-3	5-0-3	6-3-0	1	7-1-1	0-1 <sup>!!</sup> 12	5-6-0	1		
LOADING (pfr)     SPACING-     2:0-0     CSI.     DEFL.     in (pc)     Veri(L)     -0.20     I/d is 5:05     PLATES     GRIP       Snow (Pfr)     10.0     10.0     Parts Gip DDL     1.15     BC     0.86     Veri(L)     -0.24     15:3     5:999     24.0       BCLL     0.0     0     Rep Stress Incr     YES     WB     0.77     Veri(C)     -0.44     15:18     5:999     16.0       BCLL     0.0     0     Rep Stress Incr     YES     WB     0.77     Veri(C)     -0.44     15:18     5:999     16.0       BCL     0.0     24.35 P No.2     TOP CHORD     24.35 P No.2     TOP CHORD     Structural wood sheathing directly applied or 2-6-3 oc purlins, except       BOT CHORD     24.35 P No.2     BOT CHORD     BOT CHORD     Rigid caling directly applied or 0-0-oc braing. Except       3.24,115 / 52.44 S P No.3     WEBS     1 Row at midpt     5:22,7:22,7:18     2 Rows at 1/3 pts     5:21,7:22,7:18       PECCES     (i) Nax form X 2=167(12.20)     Max form X 2=167(12.20)     Max form X 2=167(12.20)     Now sat 1/3 pts     5:21,7:22,7:18       VEBS     2.42-928 (158, 15-42-16)     Sinter 2=177/28, 7:48-377212     2:40-38 (156, 11-12-183/180, 11-12-183/180, 11-13 (12-18, 11-13, 12-183/180, 11-13, 12-183/180, 11-13, 12-183/180, 11-13, 12-183/180, 11-13, 12-183/180,	Plate Offsets (	(X,Y) [6:0-3-0,	0-2-0], [8:0-3-0,0-2-0]						1				
BCDL         10.0         Code IRC2018/TPI2014         Matrix-S         Weight: 326 Ib         FT = 20%           LUMBER.         TOP CHORD         2x4 SP No.2         BRACING-         Structural wood sheathing directly applied or 2-6-3 oc purlins, except           DOT CHORD         2x4 SP No.2         Except*         5-0.0 oc purlins (3-11-2 max); 6-8.         BOT CHORD           WEBS         2x4 SP No.2         Except*         BOT CHORD         Matrix-S         BOT CHORD           WEBS         2x4 SP No.2         Except*         BOT CHORD         Max Implit 2-16.         5-22, 7-22, 7-18           WEBS         2x4 SP No.3         WEBS         1 Row at midpt         5-22, 7-22, 7-18           REACTIONS.         (size)         2-0-3-8, 15=(0-3-8 + bearing block (req, 0-3-14)         Max Unplit 2-168(LC16), 16-s-190(LC 17)           Max Lopit 2-168(LC16), 16-s-190(LC 17)         Max Grav 2-1892(LC 3), 15=-2481(LC 3)         FT =-2016(2-5, 5-22-2483), 5-5-22/480(2-58), 15=-2-371/22, 12-3577/254, 13= 2-20-378, 5-32-24/280(2-58), 15=-2-371/22, 12-3577/254, 13= 2-20-378, 5-32-24/280(2-58), 15=-2-378/254, 13=-364(4-4), 9-18=-0/473, 11-15=-4080/26, 5-22-2408(2/28), 9-15-3577/212, 12-3577/21	LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL	sf) 20.0 16.5/15.0 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.73 BC 0.86 WB 0.77	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.29 15-18 -0.49 15-18 0.09 15	l/defl >999 >999 n/a	L/d 240 180 n/a	PL M1	<b>ATES</b> 20	<b>GRIP</b> 244/190		
DUMBER:         BRACING- TOP CHORD         Structural wood sheathing directly applied or 2-6-3 oc purlins, except 2-0-0 oc purlins (3-11-2 max.); 6-8.           BOT CHORD         2x4 SP No.2 "Except"         BOT CHORD         Structural wood sheathing directly applied or 2-6-3 oc purlins, except 2-0-0 oc purlins (3-11-2 max.); 6-8.           WEBS         2x4 SP No.2 "Except"         BOT CHORD         Rigid celling directly applied or 10-0-0 oc bracing. Except: 6-0-0 oc bracing: 1-25.           (size)         2-0-34; 15-(0.3-8 + bearing block) (req. 0-3-14) Max Horz 2-157(LC 20) Max Upul 2169(LC 10); 15190(LC 17) Max Grav 2-169(LC 10); 152481(LC 3)         WEBS         1 Row at midpt         5-22, 7-22, 7-18           PORCES.         (b) - Max: Comp./Max. Ten - All forces 250 (b) or less except when shown.	BCDL	10.0	Code IRC2018/TPI2014	Matrix-S					We	eight: 326 lb	FT = 20%		
<ul> <li>REACTIONS. (size) 2=0-3-8, 15=(0-3-4 + bearing block) (req. 0-3-14) Max Horz 2=157(LC 20), 15=-2457(LC 3), Max Grav 2=1592(LC 3), 15=-2451(LC 3)</li> <li>FORCES. (lb) - Max. Comp.Max. Ten - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-327/253, 3-5=-3214/269, 5-6=2349/261, 6-7=-2041/276, 7-8=-1787/254, 8-9=-2068/245, 9-11=-78/493, 11-12=-133/490</li> <li>BOT CHORD 2-24=-331/2571, 22-24=-185/2538, 18-22=-22/1979, 15-18=-39/1520, 12-15=-357/212</li> <li>WEBS 3-24=-235(195, 5-24=-226/66, 5-22=-717/756, 8-18=-8/644, 9-18=-0/473, 11-15=-408/226, 7-22=-101/281, 7-18=-532/135, 9-15=-2438/272</li> <li>NOTES-</li> <li>1) Xx6 SP No.2 bearing block 12' long at jL 15 attached to front face with 3 rows of 10d (0.131*x3') nails spaced 3' o.c. 12 Total fastenes: User Defined Bearing crushing capacity = 425psi.</li> <li>2) Unbalanced roof live loads have been considered for this design.</li> <li>3) Windr ASCE 7-16; Vull=130mph (3-second gust) Yaach 103mph; TCDL=6 (0psf; BCDL=6 (0psf; H=25f; Cat. II; Exp B; Enclosed; MWFRS for reactions shown; Lumber DOL=1.60 (0.51; 0.52)-1.60 (0.51; 0.52)-2.51, 0.52, 0.51, 0</li></ul>	LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP No.2 2x6 SP No.2 *E: 19-21: 2x4 SP No.2 *E: 3-24,11-15: 2x4	xcept* lo.2 xcept* SP No.3	B T B V	BRACING- OP CHORD BOT CHORD VEBS	Structural wood 2-0-0 oc purlins Rigid ceiling dire 6-0-0 oc bracing 1 Row at midpt 2 Rows at 1/3 pt	sheathing c (3-11-2 ma ectly appliec g: 12-15. ts	directly app x.): 6-8. d or 10-0-0 5-22, 7-22 9-15	olied or 2-6 oc bracin 2, 7-18	6-3 oc purlin: g, Except:	s, except		
<ul> <li>FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown.</li> <li>TOP CHORD 2-3=3427/293, 35=-3214/269, 5-6=2348/281, 6-7=2041/278, 7-8=-1787/254, 8-9=-2068/245, 9-11=-78/493, 11-12=-183/490</li> <li>BOT CHORD 2-24=-331/2971, 22-24=-185/2538, 18-22=-22/1979, 15-18=-39/1520, 12-15=-357/212</li> <li>WEES 3-24=-297195, 5-42-27/81/23, 6-22=-1717/58, F-18=-39/1520, 12-15=-357/212</li> <li>WEES 3-24=-297195, 5-24=-27/81/23, 6-22=-1717/58, F-18=-39/1520, 12-15=-357/212</li> <li>WETS 1-15=-408/226, 7-22=-101/281, 7-18=-532/135, 9-15=-2438/272</li> <li>NOTES</li> <li>1) Dubalanced root live loads have been considered for this design.</li> <li>3) Wind: ASCE 7-16; VIII-130mph (3-second gust) Yads-103mph: TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 18-11-13, Exterior(2R) 18-11-13 to 23-2-111, Interior(1) 23-3-2 to 48-10-8 zone; C-C for members and forces &amp; MWFRS for reactions shown; Lumb TD DL=1.160 plate grip DDL=1.60</li> <li>4) TCLL: ASCE 7-16; VIII-200 pdf (root LL: Lum DDL=-1.15; Pig=15.0 psf; Pf=16.5 psf (Lum DDL=-1.15 Plate DDL=-1.15; Is=10; R Rungh Cat B; Partially Exp; Ca=-10; CS=-10; CS=-</li></ul>	REACTIONS.	(size) 2=0- Max Horz 2=15 Max Uplift 2=-1 Max Grav 2=18	3-8, 15=(0-3-8 + bearing block) (req. 0-3 57(LC 20) 69(LC 16), 15=-190(LC 17) 392(LC 3), 15=2481(LC 3)	3-14)									
<ul> <li>NOTES-</li> <li>1) 2x6 SP No.2 bearing block 12" long at jt. 15 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity + 425psi.</li> <li>2) Unbalanced roof live loads have been considered for this design.</li> <li>3) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 2-0-10-8 to 2-1-8. Interior(1) 21-18 to 18-11-13, Exterior(2R) 18-11-13 to 232-211 to 29-0-3. Exterior(2R) 29-0-3 is 033-3-2; Interior(1) 33-3-2 to 48-10-8 zone; C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60.</li> <li>4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ch=1.0; Uhalanced snow loads have been considered for this design.</li> <li>6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.</li> <li>7) Provide adequate drainage to prevent water ponding.</li> <li>8) All plates are 4x6 MT20 unless otherwise indicated.</li> <li>9) This truss has been designed for a 1.0.0 psf bottom chord in eload nonconcurrent with any other live loads.</li> <li>10) One H2.5A Simpson Strong-Tile connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 15. This connection is for uplift only and does not consider lateral forces.</li> <li>April 14,2023</li> </ul>	FORCES. (II TOP CHORD BOT CHORD WEBS	ORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         OP CHORD       2-3=-3427/293, 3-5=-3214/269, 5-6=-2348/281, 6-7=-2041/278, 7-8=-1787/254, 8-9=-2068/245, 9-11=-78/493, 11-12=-183/490         IOT CHORD       2-24=-331/2971, 22-24=-185/2538, 18-22=-22/1979, 15-18=-39/1520, 12-15=-357/212         VEBS       3-24=-295/195, 5-24=-22/636, 5-22=-781/232, 6-22=-17/756, 8-18=-8/644, 9-18=0/473, 11-15=-408/226, 7-22=-101/281, 7-18=-532/135, 9-15=-2438/272											
Continue to special and a contractice with the 2016 international Residential Code Sections R502. 11.1 and R602. 10.2 and Continue to section and the special section of the section of th	NOTES- 1) 2x6 SP No. fasteners. U 2) Unbalanced 3) Wind: ASC MWFRS (e 23-2-11, Ini & MWFRS 4) TCLL: ASC DOL=1.15) surcharge a 5) Unbalanced 6) This truss f 10) * This truss will fit bett 11) One H2.5. connection 12) This truss Contacted one	2 bearing block 12 Jser Defined Bear d roof live loads ha E 7-16; Vult=130n nvelope) gable en terior(1) 23-2-11 to for reactions show E 7-16; Pr=20.0 p; j ls=1.0; Rough C; applied to all expo d snow loads have as been designed rent with other live equate drainage to re 4x6 MT20 unles as been designed as been designed as been designed is for uplift only a is for uplift only a	2" long at jt. 15 attached to front face with ing crushing capacity= 425psi. ave been considered for this design. nph (3-second gust) Vasd=103mph; TCD d zone and C-C Exterior(2E) -0-10-8 to 2 o 29-0-3, Exterior(2R) 29-0-3 to 33-3-2, Ir vn; Lumber DOL=1.60 plate grip DOL=1. sf (roof LL: Lum DOL=1.15 Plate DOL=1 at B; Partially Exp.; Ce=1.0; Cs=1.00; Ct sed surfaces with slopes less than 0.500 been considered for this design. If or greater of min roof live load of 12.0 p e loads. o prevent water ponding. as otherwise indicated. If or a 10.0 psf bottom chord live load non- ned for a live load of 20.0psf on the botto hord and any other members, with BCDL -Tie connectors recommended to conner and does not consider lateral forces. cordance with the 2018 International Res PI 1.	L=6.0psf; BCDL=6.0ps -1-8, Interior(1) 2-1-8 to therior(1) 33-3-2 to 48-1 30 .15); Pg=15.0 psf; Pf=1 -1.10, Lu=50-0-0; Min. 1 (12 in accordance with 1 bsf or 1.00 times flat roc nconcurrent with any oth m chord in all areas wh . = 10.0psf. t truss to bearing walls idential Code sections 1	x3") nails spaced f; h=25ft; Cat. II; f o 18-11-13, Exteri 0-8 zone;C-C for 6.5 psf (Lum DOI flat roof snow load IBC 1608.3.4. of load of 11.6 psf her live loads. ere a rectangle 3- due to UPLIFT a R502.11.1 and R	3" o.c. 12 Total Exp B; Enclosed; for(2R) 18-11-13 members and for _=1.15 Plate d governs. Rain on overhangs -6-0 tall by 2-0-0 t jt(s) 2 and 15. T 302.10.2 and	to rces wide his		SI 030 CA	EAL 5322 NEER GILB	A		

WARNING - Veniy design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-74/3 rev. 5/19/2/02/ BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	150.1910 C_10x10CP	
						157773995
150_1910_C_10X10CP	T3A	ROOF TRUSS	2	1		
					Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		8	630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:06:57 2023	Page 2
			AD62rz1vi	HD OaRth	nrlEztO8K-2\/negS1Yisko71irMdBkoS_8twd2Aabg_WRhk2	2zR4aC

## NOTES-

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

14) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





<b> </b>	9-7-5		<u>18-11-13</u> 9-4-8	24-0-0	29-0-3	35-3-3		42-4-4	42-6-0	48-0-0	
Plate Offsets (2	X,Y) [6:0-3-0,	0-2-0], [8:0-3-0,0-2-	0]						0.12	000	
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 16.5/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip D Lumber DOL Rep Stress I Code IRC20	2-0-0 DL 1.15 . 1.15 ner YES 18/TPI2014	<b>CSI.</b> TC 0.73 BC 0.87 WB 0.77 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT	in (loc) -0.30 15-18 -0.50 15-18 ) 0.10 15	l/defl >999 >999 n/a	L/d 240 180 n/a	PI M'	<b>.ATES</b> T20 eight: 314 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS REACTIONS.	2x4 SP No.2 2x6 SP No.2 2x4 SP No.2 *E 3-22,11-15: 2x4 (size) 2=0- Max Horz 2=15 Max Uplift 2=-1 Max Grav 2=18	<pre>kcept* SP No.3 3-8, 15=(0-3-8 + be i7(LC 16) 69(LC 16), 15=-190 193(LC 3), 15=2483</pre>	aring block) (req. 0- (LC 17) (LC 3)	3-14)	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood 2-0-0 oc purlins Rigid ceiling dir 6-0-0 oc bracing 1 Row at midpt 2 Rows at 1/3 p	I sheathini i (3-11-0 n ectly appli g: 12-15. ots	g directly ap nax.): 6-8. ied or 10-0- 5-20, 7-2 9-15	oplied or 2- 0 oc bracir 20, 7-18	6-2 oc purlin: g, Except:	s, except
FORCES. (Ib TOP CHORD BOT CHORD WEBS	) - Max. Comp./N 2-3=-3428/293 8-9=-2069/246 2-22=-331/297 3-22=-295/196 7-18=-534/134	lax. Ten All forces 3, 3-5=-3215/269, 5- 5, 9-11=-79/493, 11- 2, 20-22=-185/254 5, 5-22=-22/631, 5-2 4, 8-18=-9/644, 9-18	250 (lb) or less exc 6=-2353/281, 6-7=-2 12=-184/490 I, 18-20=-22/1982, 1 0=-778/232, 6-20=- <sup>2</sup> =0/473, 9-15=-2438	ept when shown. 2046/278, 7-8=-1788/ 5-18=-39/1520, 12-1 17/758, 7-20=-99/286 /274, 11-15=-408/220	/255, 5=-356/213 3, 6						
NOTES- 1) 2x6 SP No.2 fasteners. U 2) Unbalanced 3) Wind: ASCE MWFRS (er 23-2-11, Inti & MWFRS f 4) TCLL: ASCE DOL=1.15); surcharge a 5) Unbalanced 6) This truss hi non-concurr 7) Provide ade 8) This truss hi non-concurr 7) This truss hi 9) * This truss hi 9) * This truss hi 9) * This truss hi 1) Thi truss hi 1) This truss hi 1) This truss hi 1) Thi truss	2 bearing block 12 lser Defined Bear roof live loads ha 7-16; Vult=130m rovelope) gable en erior(1) 23-2-11 tt or reactions show 2 7-16; Pr=20.0 p Is=1.0; Rough C. pplied to all expo snow loads have as been designed ent with other live quate drainage tt as been designed has been designed has been designed is for uplift only a is designed in action s tastandard ANS/7 aggig represental	2" long at jt. 15 attact ing crushing capaci- ive been considered ph (3-second gust) d zone and C-C Ext o 29-0-3, Exterior(21, n; Lumber DOL=1.1, sf (roof LL: Lum DC at B; Partially Exp.; sed surfaces with sl been considered for for greater of min r beads. prevent water pon- for a 10.0 psf bottc d for a live load of 2 ord and any other m -Tie connectors rec and does not considered cordance with the 20 Pl 1.	thed to front face wit ty = 425psi. I for this design. Vasd=103mph; TCI erior(2E) -0-10-8 to : 3, 29-0-3 to 33-3-2, 1, 30 plate grip DOL=1. L=1.15 Plate DOL=2 Ce=1.0; Cs=1.00; Ct opes less than 0.500 or this design. oof live load of 12.0 ding. m chord live load nc 20.0psf on the bottor the bottor the bottor embers, with BCDL ommended to conne er lateral forces. 2018 International Res the size or the orient	h 3 rows of 10d (0.1: DL=6.0psf; BCDL=6.0 2-1-8, Interior(1) 2-1- nterior(1) 33-3-2 to 4 60 I.15); Pg=15.0 psf; P =1.10, Lu=50-0-0; MI //12 in accordance w psf or 1.00 times flat nconcurrent with any n chord in all areas w = 10.0psf. ct truss to bearing was sidential Code section	31"x3") nails spaced Opsf; h=25ft; Cat. II; 8 to 18-11-13, Exter 8-10-8 zone;C-C for f=16.5 psf (Lum DO in. flat roof snow loa ith IBC 1608.3.4. roof load of 11.6 ps v other live loads. vhere a rectangle 3-1 alls due to UPLIFT a ns R502.11.1 and R pag the top and/or b	d 3" o.c. 12 Total Exp B; Enclosed; ior(2R) 18-11-13 r members and fo L=1.15 Plate d governs. Rain f on overhangs 6-0 tall by 2-0-0 v at jt(s) 2 and 15. T 802.10.2 and ottom chord.	to rrces vide This	and the second s	S S S S S S S S S S S S S S S S S S S	CAR SOLUTION EAL 6322 INEE GILBE	A 111111111111111111111111111111111111
WARNING Design valid for a truss system	- Verify design parame or use only with MiTel n. Before use, the buil	eters and READ NOTES C ® connectors. This designed ding designer must verify	IN THIS AND INCLUDED I In is based only upon para the applicability of design	MITEK REFERENCE PAGE ameters shown, and is for a parameters and properly i	MII-7473 rev. 5/19/2020 I an individual building com incorporate this design int	BEFORE USE. ponent, not			ENG	NEERING BY	'n

a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ADS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



lah	Truce	Truce Type	Otr	Dhy	150 1010 C 10v10CP	
000	11055	Thuss Type	Quy	FIY	150.1910 C_10X10CF	
						157773996
150 1910 C 10X10CP	Τ4	ROOF TRUSS	2	1		
					Job Reference (optional)	
84 Components (Dunn, NC),	Dunn, NC - 28334,		. 8.	630 s Nov	19 2022 MiTek Industries, Inc. Thu Apr 13 16:07:00 2023	Page 2
		ID:VMD6	2rz1yiHD	OqRtbnrll	FztQ8K-S4SnJU4Q?n6M_UQQ1mkRQ5cf78eeNxL6gUfLK	KzR4a9

# NOTES-

13) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. (size) 1=8-0-8, 3=8-0-8, 4=8-0-8

Max Horz 1=-56(LC 10)

Max Uplift 1=-30(LC 14), 3=-38(LC 15)

Max Grav 1=154(LC 2), 3=154(LC 2), 4=263(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-0-10, Exterior(2R) 4-0-10 to 7-0-10 , Interior(1) 7-0-10 to 7-7-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) N/A
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	0-0-6	5-1-4							
	0-0-6	5-0-14							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.07 BC 0.04 WB 0.02 Matrix-P	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 17 lb	<b>GRIP</b> 244/190 FT = 20%
BCDL 10.0								-	
LUMBER-		BR	ACING-						

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. (size) 1=5-0-8, 3=5-0-8, 4=5-0-8

Max Horz 1=-32(LC 10)

Max Uplift 1=-18(LC 14), 3=-22(LC 15)

Max Grav 1=89(LC 2), 3=89(LC 2), 4=152(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 7) N/A
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 5-1-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





		0- <mark>0</mark> -6			3-4	-8						
	0-0-6					-2						
Plate Offsets (X,Y	′) [2:0-3-0,I	Edge]										
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 11. TCDL	20.0 .6/15.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.03 0.07 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0	Code IRC2018/TF	912014	Matri	x-P						Weight: 9 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No 2 2x4 SP No.2 BOT CHORD

REACTIONS. (size) 1=3-3-12, 3=3-3-12

Max Horz 1=-19(LC 10) Max Uplift 1=-8(LC 14), 3=-8(LC 15)

Max Grav 1=96(LC 2), 3=96(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 7) N/A
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 3-4-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. (size) 1=9-11-10, 3=9-11-10, 4=9-11-10

Max Horz 1=71(LC 11) Max Uplift 1=-29(LC 14), 3=-38(LC 15), 4=-5(LC 14)

Max Grav 1=179(LC 2), 3=179(LC 2), 4=368(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 5-0-3, Exterior(2R) 5-0-3 to 8-0-3, Interior(1) 8-0-3 to 9-6-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate

DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.

8) N/A

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





		7-0-0				7-0-6	;	
	1	7-0-0				0-0-6		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.17 BC 0.09 WB 0.03 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	<b>PLATES</b> MT20 Weight: 24 lb	<b>GRIP</b> 244/190 FT = 20%
BCDL 10.0							- <b>J</b>	
LUMBER-		BRA	ACING-					

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. (size) 1=6-11-10, 3=6-11-10, 4=6-11-10

Max Horz 1=47(LC 13)

Max Uplift 1=-26(LC 14), 3=-32(LC 15)

Max Grav 1=131(LC 2), 3=131(LC 2), 4=223(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 7) N/A
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





2x4 🥢

2x4 📎

		400					- p o	
	ſ	4-0-0					0-0-6	
Plate Offsets (X,Y) [2:0-3-0,	Edge]							
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         11.6/15.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.04 BC 0.11 WB 0.00 Matrix-P	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in (I n/a n/a 0.00	loc) l/defl - n/a - n/a 3 n/a	L/d 999 999 n/a	PLATES MT20 Weight: 11 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-		BR	ACING-					

TOP CHORD

BOT CHORD

1-0-0

LUMBER-

TOP CHORD 2x4 SP No 2 2x4 SP No.2 BOT CHORD

REACTIONS. (size) 1=3-11-10, 3=3-11-10

Max Horz 1=24(LC 11) Max Uplift 1=-11(LC 14), 3=-11(LC 15)

Max Grav 1=123(LC 2), 3=123(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 7) N/A
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



1-0-6

Structural wood sheathing directly applied or 4-0-6 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



