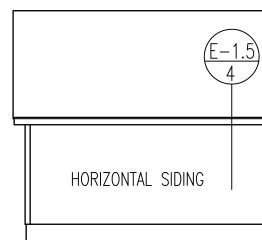
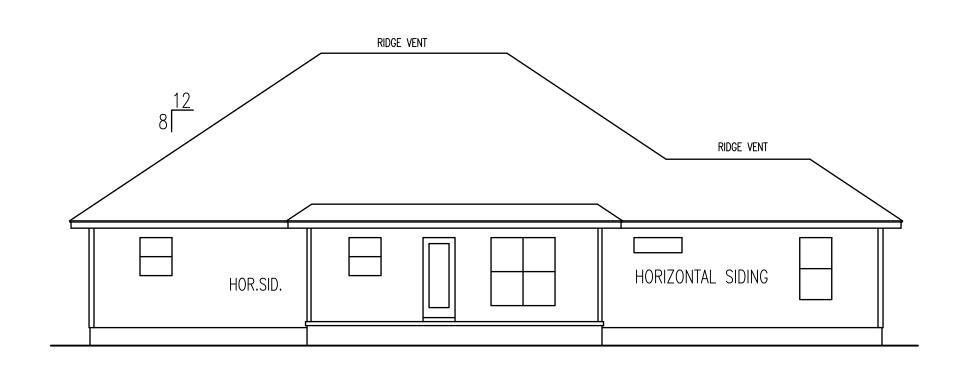


GABLE ENDS

RIDGE VENT





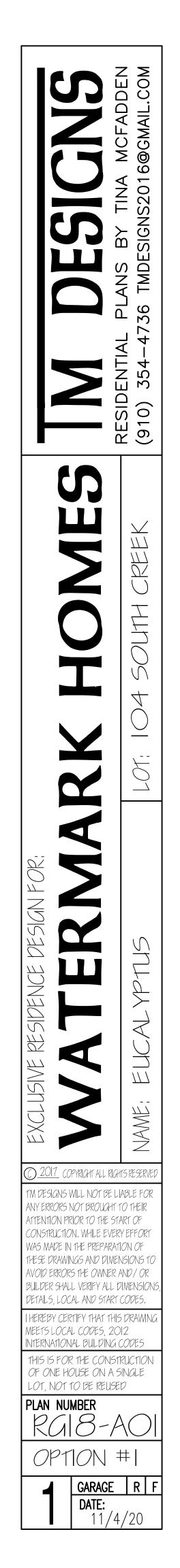
REAR ELEVATION SCALE:1/8"=1'-0"

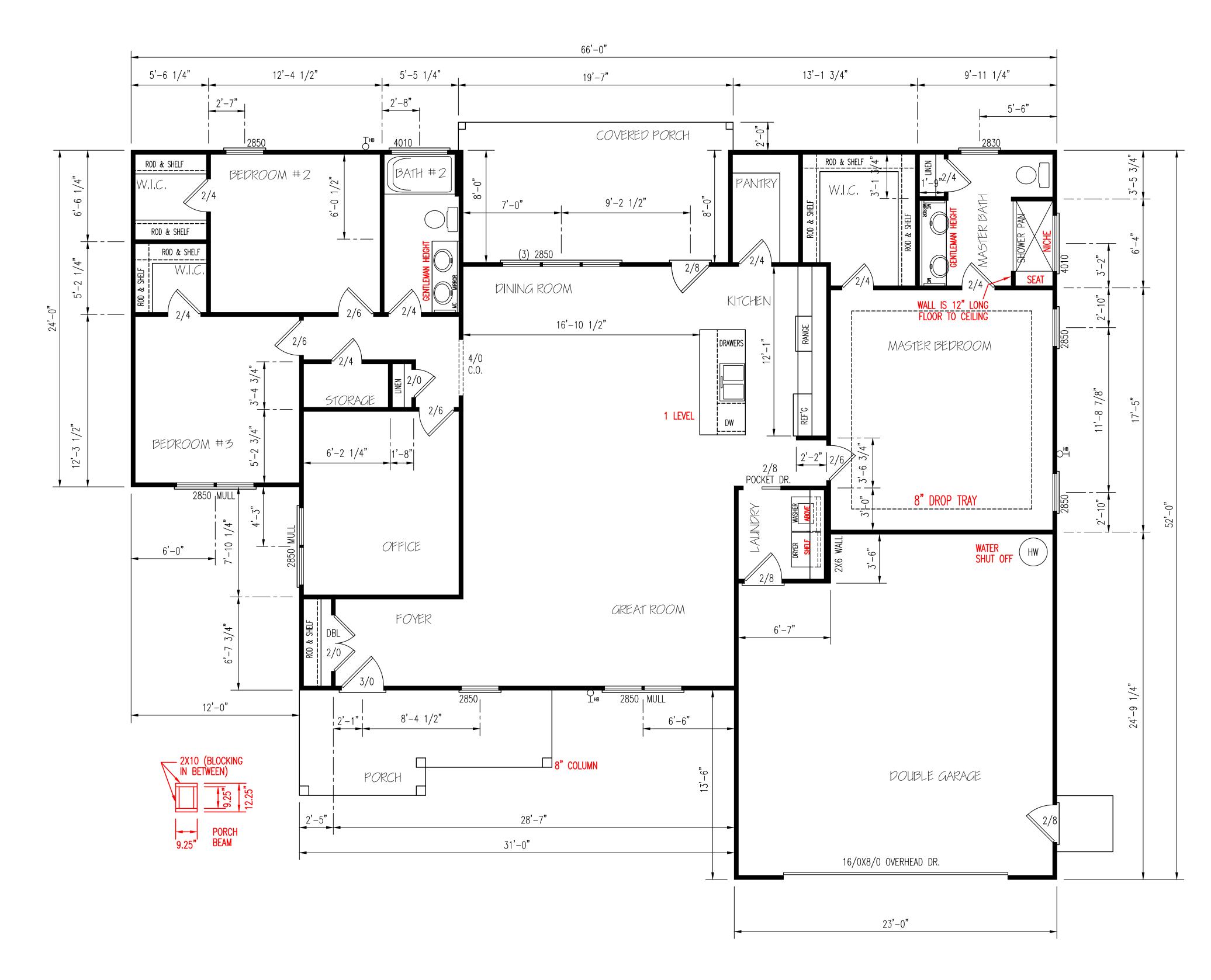


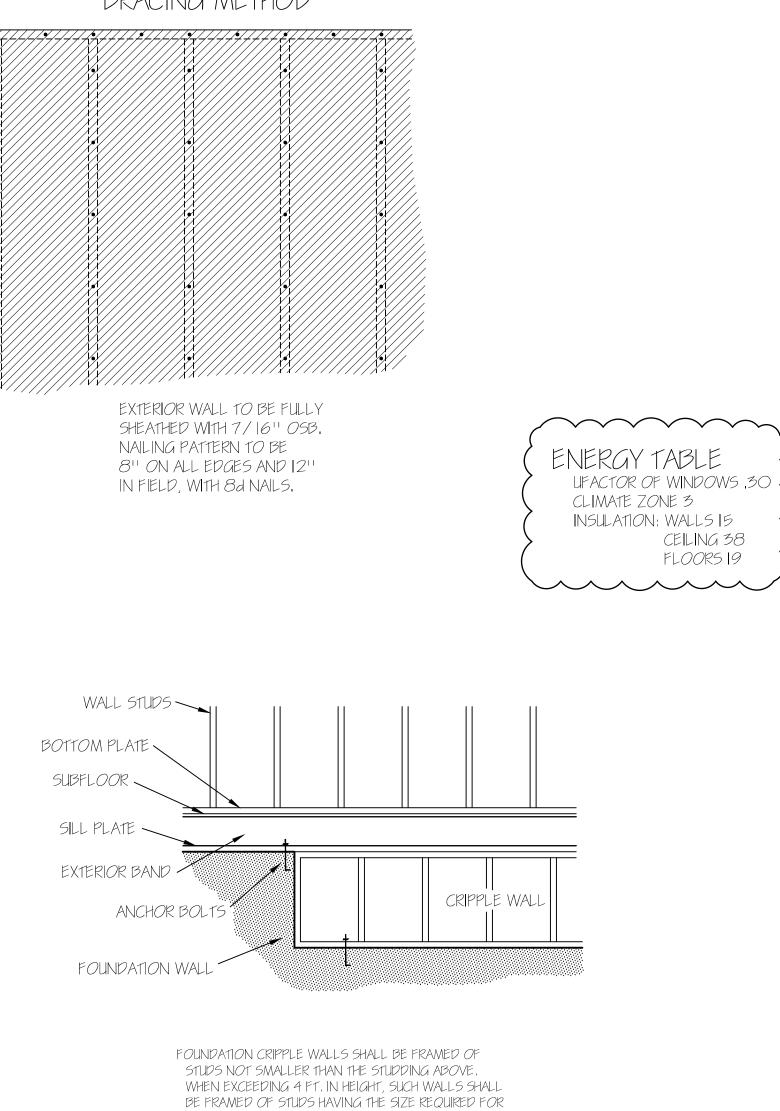
04/10/2023

HOR.SID.

NORTH CAROLINA







EXERIOR N	NALLS	
(2) 2X10 ⊢	IEADER	S
CLEAR SPAN		OF STUDS
FOR HEADER	JACKS	KINGS
ALL DOOR & C.O. BELOW 4'	1	1
ALL DOOR & C.O. 4' TO 7'-11"	2	2
ALL DOOR & C.O. 8' AND ABOVE	SIZED ENGIN	
UNLESS NOTED	OTHER	WISE

GARAGE PANEL WALL

GARAGE PANEL WALLS UNDER 24" WIDE SHOULD BE EITHER PORTAL FRAMED OR 7/16" OSB ON BOTH SIDES WITH A NAILING PATTERN OF 311 ON ALL PANEL EDGES AND 6" IN THE FIELD.

BRACING METHOD

AN ADDITIONAL STORY, CRIPPLE WALLS WITH A STUD HEIGHT LESS THAN 14 INCHES SHALL BE CONTINUOUSLY SHEATHED ON ONE SIDE WITH

WOOD STRUCTURAL PANELS FASTENED TO BOTH THE TOP AND BOTTOM PLATES IN ACCORDANCE WITH TABLE R602.3(1), OR CRIPPLE WALLS SHALL BE CONSTRUCTED OF SOLID BLOCKING,

<u>NOTE:</u> CEILINGS ARE 9'–0" UNLESS NOTED.

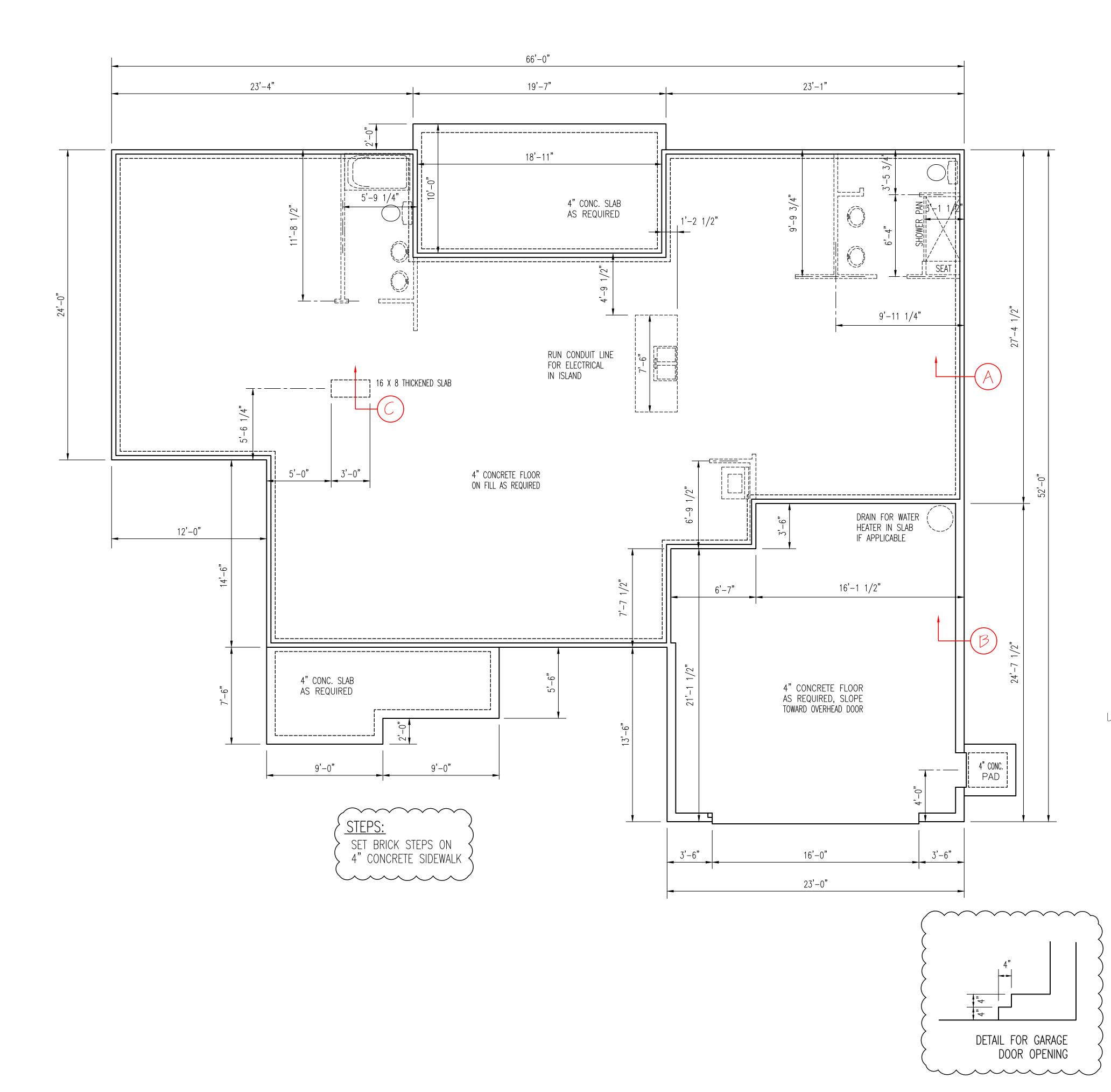
FLOOR PLAN SCALE: | / 4'' = | ' - 0''

HEATED AREA <u>1986</u> SQ FT

OTHER AREAS

GARAGE	540	SQ FT
F.PORCH	7	5Q F1
R.PORCH	9	SQ FT
TOTAL	848	SQ FT

NDESICNS	RESIDENTIAL PLANS BY TINA MCFADDEN (910) 354-4736 TMDESIGNS2016@GMAIL.COM
ARK HOMES	LOT. 104 SOUTH CREEK
EXCLUSIVE RESIDENCE DESIGN FOR:	NAME: EUCALYPTUS
© 2017 COPYRIGHT ALL TM DESIGNS WILL NOT E ANY ERRORS NOT BROUG ATTENTION PRIOR TO THE CONSTRUCTION, WHILE E WAS MADE IN THE PREPA THESE DRAWINGS AND D AVOID ERRORS THE OWN BUILDER SHALL VERIFY A DETAILS, LOCAL AND ST I HEREBY CERTIFY THAT MEETS LOCAL CODES, INTERNATIONAL BUILDII THIS IS FOR THE CON OF ONE HOUSE ON LOT, NOT TO BE RE PLAN NUMBER RGI 8-, OPTION GARAGE DATE:	BE LIABLE FOR AHT 10 THEIR START OF VERY EFFORT ARATION OF IMENSIONS TO IER AND / OR ILL DIMENSIONS, ART CODES, THIS DRAWING 2012 NG CODES NSTRUCTION A SINGLE USED

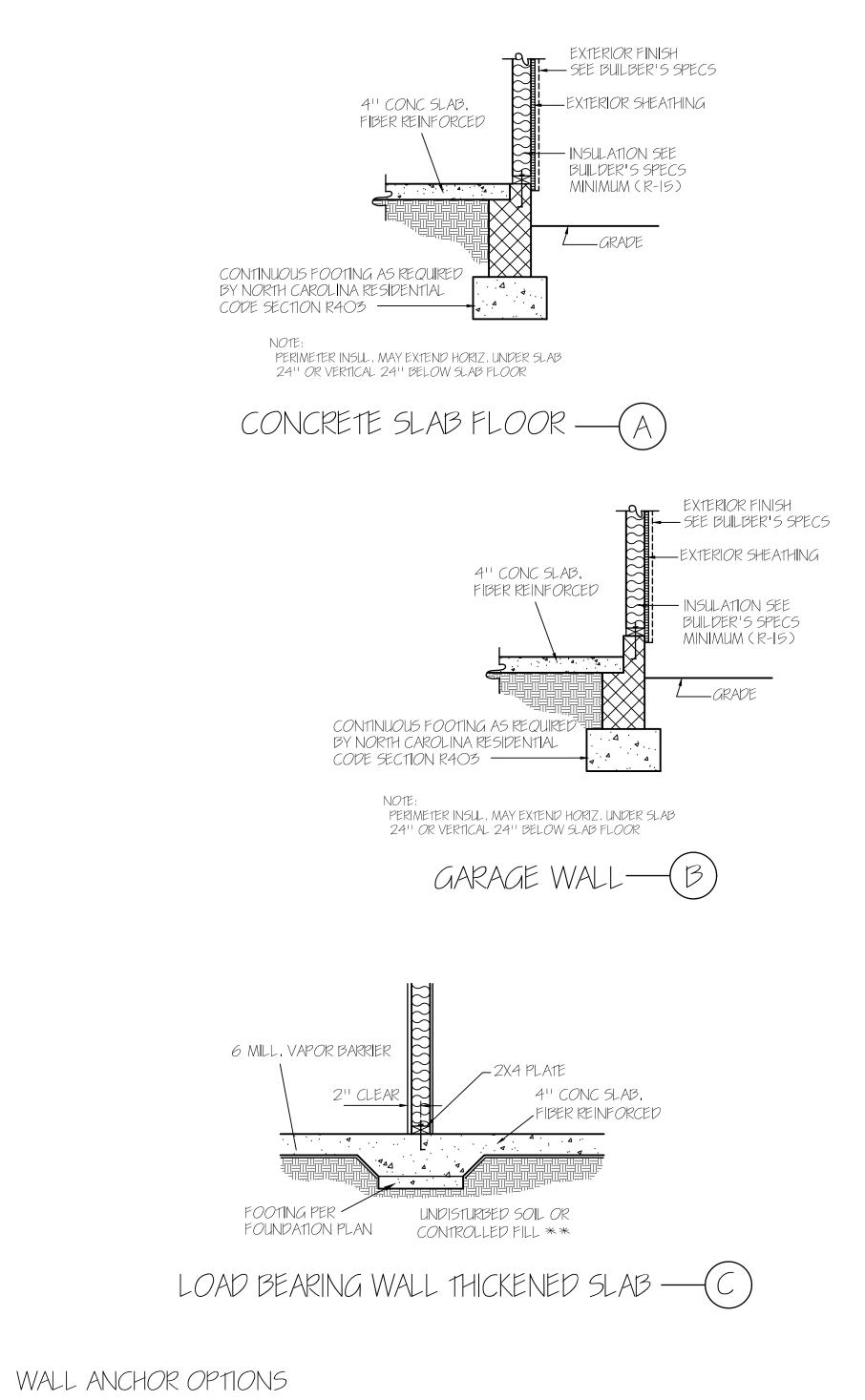




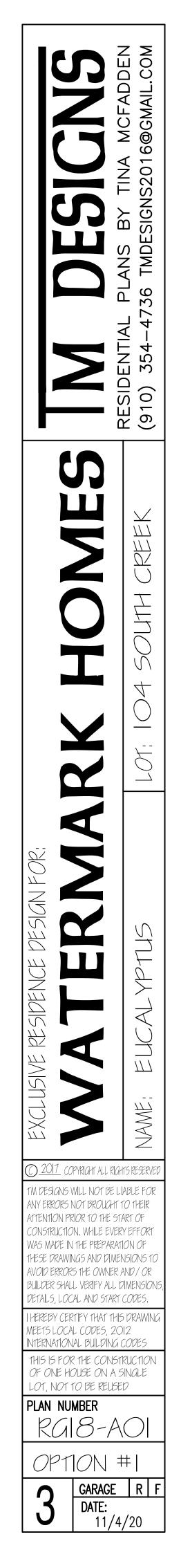
ANCHOR BOLTS: 1/2" DIA, BOLTS AT 6'-O" O.C. AND NOT MORE THAT 12" FROM CORNERS, EMBEDDED MIN, 7" INTO FOUNDATION, USE A MIN, OF 2 BOLTS PER EACH STUD WALL

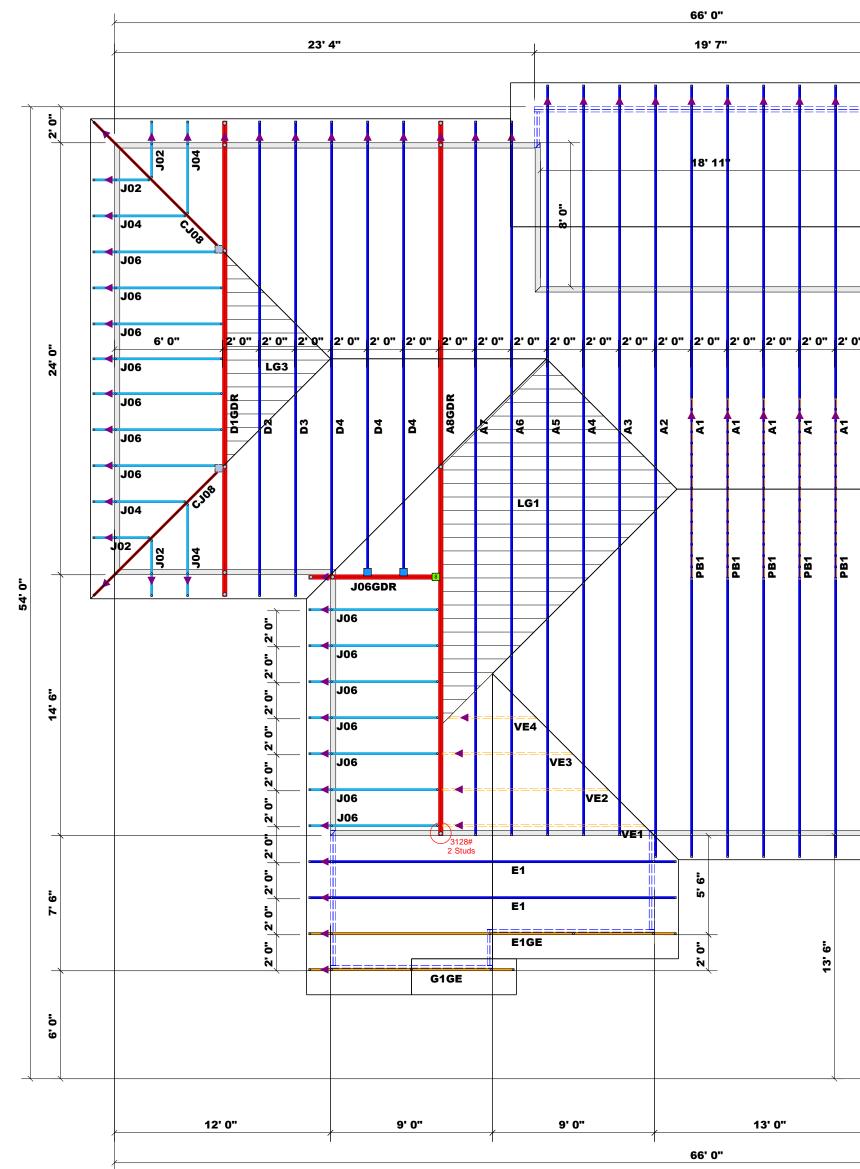
NOTE:

FOUNDATION DETAILS SHOWN ARE BASED ON ASSUMED SOIL BEARING CAPACITY OF 2000 PSF, LOCAL SITE CONDITIONS MUST BE INVESTIGATED, ALL FOOTING TO BE LOCATED BELOW FROST DEPTH.



FOUNDATION PLAN SCALE:1/4"=1'-0"





= Denotes Left End of Truss
 (Reference Engineered Truss Drawing)
 Do Not Erect Trusses Backwards

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

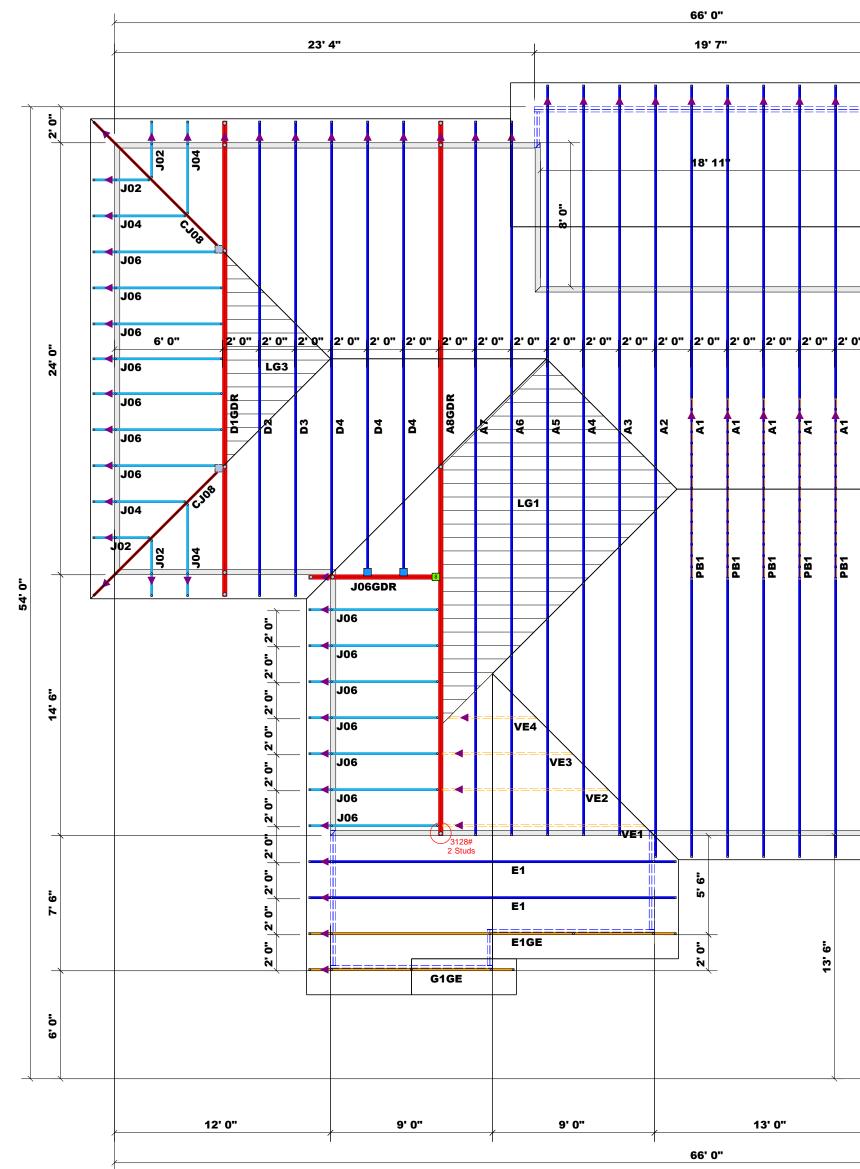
<u>Truss Placement Plan</u> SCALE: 3/16" = 1'

																							-
	<u> </u>					2	3' 1"				,							TF Re	ROC RUS eilly R Fayet ⁻ Phon	OF & SES oad Ir teville e: (91(& FL & B dustr , N.C.) 864	CH OOF EAN ial Pai 28309 -8787	F N 9
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									J06A	JO2A		1' 10 1/2' .0 .7 .0 .7	4				de rec att rec siz rec 15/ ret rea Ta ret rea	emed to quireme ached quireme te and i actions 000#. A cained t action t bles. A cained t	to comply ents. The Tables (ents) to number of s greater A register to design that exce A register to design to design that exce	y with the e contract derived f determin of wood s than 3000 red desig the suppled those ed desig the suppled the	e prescrip or shall i rom the p e the min studs req 0# but no n profess port syste e specific n profess port syste 0#.	I to 3000/ tive Code effer to the prescripti- imum for uired to s t greater - ional sha em for an d in the a tional sha em for all uired to s toral sha em for all	le heiviu sital yaali
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- 0 - 7		PB1	PB1				C3			80L 80L 80L		:0 :N :0 :N 1' 10 1/2'	54' 0"										
					3. 6.		C3		0										×				
			6' 7"	/			C2		0 0								:	rnett	Creek				
	F	-	/	/	1		C2		2.0									Lillington / Harnett	South			lick	
		7	/			8 -	C2		5.0									igton	104 S		03/31/23	Curtis Quick	
_						tic Storage Live Load	C2		5.0.			ž							Lot 1	Roof	03/3	Curt	
						te Attic 1 lbs. Liv	C1		5.0.				5					o.	S			BY	t
						1. 30 30	C1		5.0.									CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	
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	+					23	3' 0"				,							nes	Creek				
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	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
83	HJC26	USP	3	Varies	16d/3-1/2"	10d/3"
	HUS26	USP	2	Varies	16d/3-1/2"	16d/3-1/2"
8	THD28-2	USP	1	Varies	16d/3-1/2"	10d/3"

		Beam Legend			
PlotID	Length	Product	Plies	Net Qty	Fab Type
GDH	23' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF

WAVA</th



= Denotes Left End of Truss
 (Reference Engineered Truss Drawing)
 Do Not Erect Trusses Backwards

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

<u>Truss Placement Plan</u> SCALE: 3/16" = 1'

																							-
	<u> </u>					2	3' 1"				,							TF Re	ROC RUS eilly R Fayet ⁻ Phon	OF & SES oad Ir teville e: (91(& FL & B dustr , N.C.) 864	CH OOF EAN ial Pai 28309 -8787	F N 9
	1	٦								,										(910)			_
									J06A	JO2A		1' 10 1/2' .0 .7 .0 .7	↓				de rec att rec siz rec 15/ ret rea Ta ret rea	emed to quireme ached quireme te and i actions 000#. A cained t action t bles. A cained t	to comply ents. The Tables (ents) to number of s greater A register to design that exce A register to design to design that exce	y with the e contract derived f determin of wood s than 3000 red desig the suppled those ed desig the suppled the	e prescrip or shall i rom the p e the min studs req 0# but no n profess port syste e specific n profess port syste 0#.	I to 3000/ tive Code effer to the prescripti- imum for uired to s t greater - ional sha em for an d in the a tional sha em for all uired to s toral sha em for all	le heiviu sital yaali
										J06A		5. 5.											
	3							/ 		80L		1' 4" 							(BASED	ON TABLE	ES R502.5(1		
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- 0 - 7		PB1	PB1				C3			80L 80L 80L		:0 :N :0 :N 1' 10 1/2'	54' 0"										
					3. 6.		C3		0										×				
			6' 7"	/			C2		0 0								:	rnett	Creek				
	F	-	/	/	1		C2		2.0									Lillington / Harnett	South			lick	
		7	/			8 -	C2		5.0									igton	104 S		03/31/23	Curtis Quick	
_						tic Storage Live Load	C2		5.0.			ž							Lot 1	Roof	03/3	Curt	
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						1. 30 30	C1		5.0.									CITY / CO.	ADDRESS	MODEL	DATE REV.	DRAWN BY	
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						Sym	Produc	ct	Manuf Qty	Suppo Mem	rted ber	He	ader	Truss				x	МE		ATE	#	

	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
83	HJC26	USP	3	Varies	16d/3-1/2"	10d/3"
	HUS26	USP	2	Varies	16d/3-1/2"	16d/3-1/2"
8	THD28-2	USP	1	Varies	16d/3-1/2"	10d/3"

		Beam Legend			
PlotID	Length	Product	Plies	Net Qty	Fab Type
GDH	23' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF

WAVA</th

1 - SPF 3.500" Vert 232 - SPF 3.500" Vert 23AnalysisActualLocationAllowedCapacityComb.CaseMoment5365 ft-lb8'5"19911 ft-lb0.269 (27%) D+LLLUnbraced5365 ft-lb8'5"6063 ft-lb0.885 (88%) D+LLShear1087 lb15'6 5/8"8867 lb0.123 (12%) D+LLLLDefineh0.276 (L/712)8'5 1/16"0.409 (L/480)0.171 (17%) LLDesign Notes1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support to exceed 6".Stateral support at an and support of prevent lateral movement and rotation at the end bearings. Lateral support to exceed 6".SideDead 0.9Live 1Snow 1.15Wirt1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support to exceed 6".Side for specified loads.Siderian are designed to be supported on the bottom edge only.3 Refer to last page of calculations for fasteners required for specified loads.Siderian are designed to be supported on the bottom edge only.5 Top loads must be laterally braced at end bearings.LocationTrib WidthSideDead 0.9Live 1Snow 1.15Wirt1Tapered Start0-0.0Top45 PLF0 PLF0 PLF0 PLF2Tie-In0-0.0 to 16-10.01-0.0Top5 PSF40 PSF0 PLF2Tie-In0-0.0 to 16-10.01-0.0Top5 PSF0 PLF<	tus Beams
Image: Second	
1 3 1 SPF 16'10" Pasign Method: ASD Defection L: 40 Defection TL: 380 Importance: Normal - II Temperature: Temp <= 100"F Analysis Actual Moment 5365 ft-Ib 0 SS5 ft-Ib 85' 19911 ft-Ib 0.269 (27%) D+L L Definic 0.070 (L2809 B 5'116' 0.469 (27%) D+L L Libranced 335 ft-Ib 0.55 ft-Ib 0 SS5 ft-Ib 85' 19911 ft-Ib 0.269 (27%) D+L L Libranced 3356 ft-Ib 0.55 ft-Ib 0.55 ft-Ib 1 Loeft Ion 0.70 (L2809 B 5'116'' 0.452 (L280) 0.71 (T/17%) L L L L Definic 0.070 (L2800 B 5'116'' 0.454 (L280) 0.506 (51%) D+L L L Shara alpibe subgrot de opulations f	
Application: Floor Bractions UNPATTERNED Amber Information Reactions UNPATTERNED Type: Girder Application: Floor Piese: 2 Moisture Condition: Dry Design Method: ASD Building Code: IBC 2012 Data Sharing: No Deck: Not Checked Defection TL: 380 Deck: Not Checked Importance: Starting Analysis Actual Location Allowed Capacity Comb. Case Unbraced 5365 fl-lb 85' 19911 fl-lb 0.269 (27%) D+L L L Unbraced 5365 fl-lb 85' 19911 fl-lb 0.269 (27%) D+L L L Unbraced 5365 fl-lb 85' 19911 fl-lb 0.269 (27%) D+L L L Libraced 5365 fl-lb 85' 19911 fl-lb 0.269 (27%) D+L L L Libraced 5365 fl-lb 85' 1991 fl-lb 0.269 (27%) D+L L L Libraced 15'6 5/8'' 8867 lb 0.123 (12%) D+L L L Libraced 15'6 5/8'' 8867 lb 0.123 (12%) D+	
IspF I6'10" IspF I6'10" IspF I6'10" Member Information Reactions UNPATTERNED Type: Girder Piese: 2 Moisture Condition: Dry Design Method: ASD Building Code: IBC 2012 Defection TL: 360 Importance: Normal - II Temperature: Temp <= 100"F	
IspF IspF IspF Interview Information Type: Girder Piles: 2 Application: Floor Defection LL: 480 Design Method: ASD Binding Code: IBC 2012 Defection LL: Bearing Length: Dir. Call Vertical 337 Defection LL: Bearing Length: Dir. Call Vertical 337 Defection LL: Bearing Length: Dir. Call Vertical 337 Direction TL: 3600 'Vert 23 malysis Actual Location Allowed Capacity Comb. Case More: Bearing Length: Dir. Call Dir. Case Direction 11: 3600 'Vert 23 Mathysis Actual Location Allowed Capacity Comb	· · · · · · · · · · · · · · · · · · ·
I6'10" Reactions UNPATTERNED Type: Girder Appleation: Floor Brg Direction Live Deflection IL: 480 Design Method: ASD Building Code: BC 2012 Load Sharing: No Deflection TL: 300 Deflection TL: 00"F Deck: Not Checked 1 Vertical 337 Importance: Normal - II Temperature: Temp <= 100"F Deck: Not Checked Bearing Endition Dir. Case Analysis Actual Location Allowed Capacity Comb. Case Soo" Vert 23 Analysis Actual Location Allowed Capacity Comb. Case Case 2 - SPF 3.500" Vert 23 Moraced 5365 ft-lb 85" 19911 ft-lb 0.286 (28%) D+L L L L L Deflection L12 2 - SPF 3.500" Vert 23 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support L Safe ft-lb 2.567 </td <td>2 SPF</td>	2 SPF
Aember Information Reactions UNPATTERNED Type: Girder Application: Floor Brg Direction Live Piles: 2 Design Method: ASD Building Code: BC 2012 Load Sharing: No Deflection TL: 380 Beck: Not Checked 1 Vertical 337 Importance: Normal - II Deck: Not Checked Eearing Eegring Line Moment 5365 ft-lb B5° 19911 ft-lb 0.269 (27%) D+L L L Unbraced 5365 ft-lb 85° 19911 ft-lb 0.269 (27%) D+L L L Unbraced 5365 ft-lb 85° 19911 ft-lb 0.269 (27%) D+L L L Unbraced 5365 ft-lb 85° 19911 ft-lb 0.269 (27%) D+L L L Unbraced 5365 ft-lb 85° 19911 ft-lb 0.269 (27%) D+L L L Unbraced 5365 ft-lb 85° 19911 ft-lb 0.269 (27%) D+L L L Unbraced 5365 ft-lb 85° 1060 (0.500 (5.06	3 1/2"
Type: Girder Application: Floor Brg Direction Live Ples: 2 Building Code: IBC 2012 337 2 Vertical 337 Deflection TL: 360 Building Code: IBC 2012 Load Sharing: No Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F	1
Piles: 2 Design Method: ASD 1 Vertical 337 Moisture Condition: Dry Building Code: IBC 2012 1 Vertical 337 Deflection LL: 480 Deflection TL: 360 Inoportance: Not Checked 337 Deflection TL: 360 Deck: Not Checked Inoportance: Not Checked Importance: Normal - II Temperature: Temperature: Not Checked Bearing: Not Checked Analysis Actual Location Allowed Capacity Comb. Case Moment 5365 ft-lb 8'5" 19911 ft-lb 0.286 (27%) D+L L L Unbraced 5365 ft-lb 8'5" 19911 ft-lb 0.286 (27%) D+L L L Unbraced 5365 ft-lb 8'5" 19911 ft-lb 0.286 (88%) D+L L LL Deflection to 0.70 (L/2809) 8'5 1/16" 0.490 (L/480) 0.171 (17%) L L L Provide support to prevent lateral moverment and) lb (Uplift)
Moisture Condition: Dry Deflection LL: 480 Building Code: IBC 2012 Load Sharing: No 2 Vertical 337 Deflection TL: 360 Importance: Normal - II Deck: Not Checked Deck: Not Checked Eearing 2 Vertical 337 Importance: Normal - II Temp <= 100"F	Dead Snow Wind 877 0 0
Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F	877 0 0 877 0 0
Importance: Normal - II Temperature: Temp <= 100°F	
Bearing Length Dir. Ca	
Bearing Length Dir. Ca analysis Results Analysis Actual Location Allowed Capacity Comb. Case Moment 5365 ft-lb 8*5" 19911 ft-lb 0.269 (27%) D+L L Unbraced 5365 ft-lb 8*5" 6063 ft-lb 0.855 (88%) D+L L Shear 1087 lb 15*6 5/8" 8867 lb 0.123 (12%) D+L L Unbraced 5365 ft-lb 8*5 1/16" 0.409 (L/480) 0.171 (17%) L L LL Defl inch 0.070 (L/2809) 8*5 1/16" 0.506 (51%) D+L L TL Defl inch 0.276 (L/712) 8*5 1/16" 0.506 (51%) D+L L esign Notes 1 1 0.546 (L/360) 0.506 (51%) D+L L 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code. 4 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6". 3 3 Refer to last page of calculations for fasteners required for specified loads. 4 4 Girders are designed to be supported on the botom edge only. 5 5 Top loads must be laterally braced at end bearings. <td< td=""><td></td></td<>	
1 - SPF 3.500" Vert 23 2 - SPF 3.500" Vert 23 Analysis Actual Location Allowed Capacity Comb. Case Moment 5365 ft-lb 8'5" 19911 ft-lb 0.269 (27%) D+L L Uhbraced S365 ft-lb 8'5" 6063 ft-lb 0.885 (88%) D+L L L Shear 1087 lb 15'6 5/8" 8667 lb 0.123 (12%) D+L L LL Defl inch 0.070 (L/2809) 8'5 1/16" 0.409 (L/480) 0.171 (17%) L L The Defl inch 0.276 (L/712) 8'5 1/16" 0.409 (L/480) 0.506 (51%) D+L L Design Notes 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code. 2 Fasten all plies using 2 rows of 10d Box nalls (.1283'') at 12" o.c. Maximum end distance not to exceed 6". 3 Refer to last page of calculations for fasteners required for specified loads. 4 Girders are designed to be supported on the bottom edge only. 5 Top nust be laterally braced at end bearings.	ap. React D/L lb Total Ld. Case Ld. C
Analysis Actual Location Allowed Capacity Comb. Case Moment 5365 ft-lb 85" 19911 ft-lb 0.269 (27%) D+L L Unbraced 5365 ft-lb 85" 19911 ft-lb 0.269 (27%) D+L L Shear 1087 lb 15"6 5/8" 8867 lb 0.123 (12%) D+L L LL Deflinch 0.070 (L/2809) 8"5 1/16" 0.409 (L/480) 0.517 (17%) L L Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code. 2 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support to exceed 6". 3 Refer to last page of calculations for fasteners required for specified loads. 4 Girders are designed to be supported on the bottom edge only. 5 5 Top loads must be laterally braced at end bearings. 1 Pioled bearings. 5 7 Bottom must be laterally braced at end bearings. 5 15 0 PLF 0 PLF 10 Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wir <td< td=""><td>ap. React D/L lb Total Ld. Case Ld. C 3% 877 / 337 1214 L D+L</td></td<>	ap. React D/L lb Total Ld. Case Ld. C 3% 877 / 337 1214 L D+L
AnalysisActualLocationAllowedCapacityComb.CaseMoment5365 ft-lb8'5"19911 ft-lb0.269 (27%)D+LLUnbraced5365 ft-lb8'5"6063 ft-lb0.885 (88%)D+LLShear1087 lb15'6 5/8"8867 lb0.123 (12%)D+LLLL Defl inch0.070 (L/2809)8'5 1/16"0.409 (L/480)0.171 (17%)LLThe Defl inch0.276 (L/712)8'5 1/16"0.546 (L/360)0.506 (51%)D+LLDesign Notes1Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.2Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".3Refer to last page of calculations for fasteners required for specified loads.4Girders are designed to be supported on the bottom edge only.5Top loads must be supported qually by all plies.6Top must be laterally braced at end bearings.7Bottom must be laterally braced at end bearings.8Lateral senderness ratio based on single ply width.IDLoad TypeLocation1Tapered Start0-0-01Tapered Start0-0-02Tie-In0-0-0 to 16-10-03Tapered Start8-5-03Tapered Start8-5-04O PLF0 PLF4Tapered Start8-5-0<	3% 877/337 1214 L D+L
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	0 PLF 0 PLF Gable
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Manufacturer In	fo Comtech, Inc.
Notes communication of the Handling & Installation provide proper draining to prevent Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC
tructural adequacy of this component based on the 1. LVL beams must not be cut or drilled lesign criteria and loadings shown. It is the 2. Refer to manufacturer's product information sponsibility of the customer and/or the contractor to regarding installation requirements, multi-ply (200) CP	101119, 2110 F1001 28314
nsure inter component suitability of the intended pplication, and to verify the dimensions and loads. umber 3. Damaged Beams must not be used (000) 622-0630	l.com/us
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	Client: Watermark Home Project:		e: 3/31/2023 ut by: Curtis Quick	Page 2 of 2
isDesign	Address:	Job	Name: The Eucalyptus Beams	
GDH Kerto-S L	/L 1.750" X 11.875"	2-Ply - PASSED	ject #: Level: Level	
• • • •	· · · ·	· · · ·	· · · · ·	
1 SPF	• • • •	• • • •	• • • •	
		16'10"		3 1/2"
· · · · · · · · · · · · · · · · · · ·		16'10"		
				·
Multi-Ply Analysis				
	s of 10d Box nails (.128x3") at 12	" o.c Maximum end distan	ce not to exceed 6".	
Capacity Load	0.0 % 0.0 PLF			
Yield Limit per Foot Yield Limit per Fastener	163.7 PLF 81.9 lb.			
Yield Mode	IV			
Edge Distance Min. End Distance	1 1/2" 3"			
Load Combination	3			
Duration Factor	1.00			
Notes	chemicals	 For flat roofs provide proper drainage to pr ponding 		Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Designs is responsible only or structural adequacy of this component based or design criteria and loadings shown. It is responsibility of the customer and/or the contract	the 1. LVL beams must not be cut or drilled	1	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	Fayetteville, NC USA 28314 910-864-TRUS
responsibility of the customer and/or the contract ensure the component suitability of the inte application, and to verify the dimensions and loads.	approvals	y 9	(800) 622-5850 www.metsawood.com/us	
Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corr	3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoi lateral displacement and rotation	d This design is valid until 11/3/2024		соттесн
Version 21 80 417 Powered by iStruct™ [J 20100 - 17672024	L	