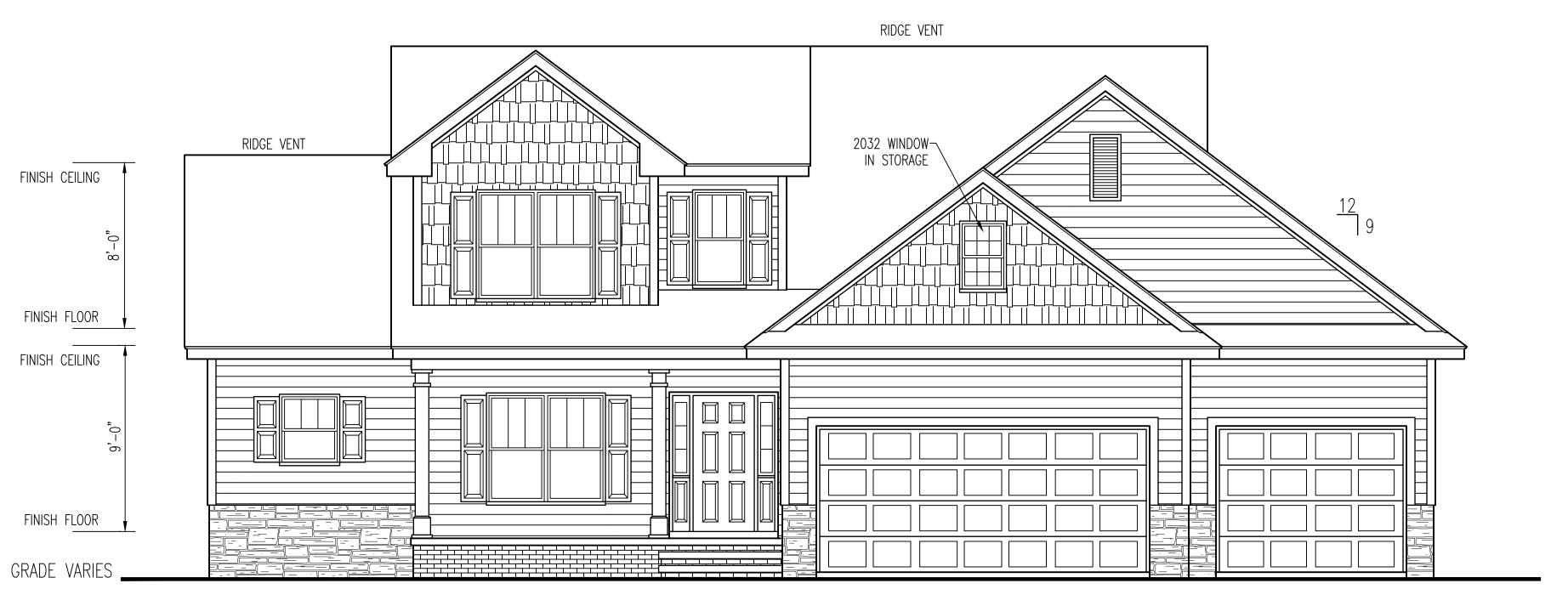
I HEREBY CERTIFY THAT THIS DRAWING MEETS LOCAL CODES, 2012 INTERNATIONAL BUILDING CODES

1HIS IS FOR THE CONSTRUCTION OF ONE HOUSE ON A SINGLE LOT, NOT TO BE REUSED

PLAN NUMBER

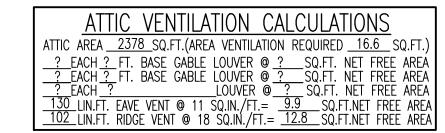
OPTION #6 GARAGE R F

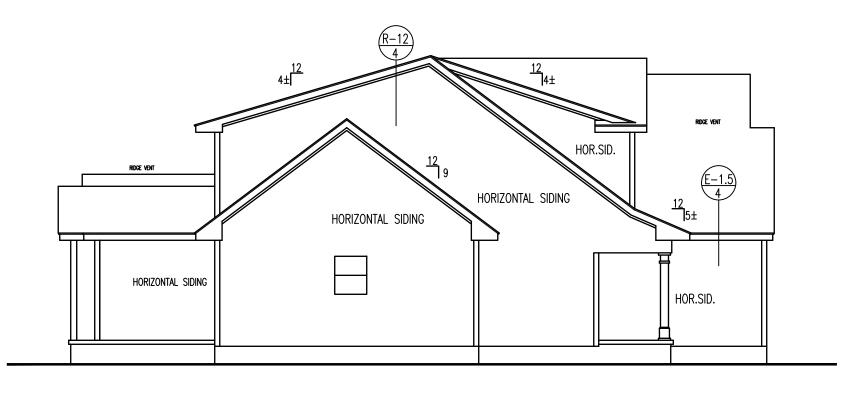
DATE:
3/15/23

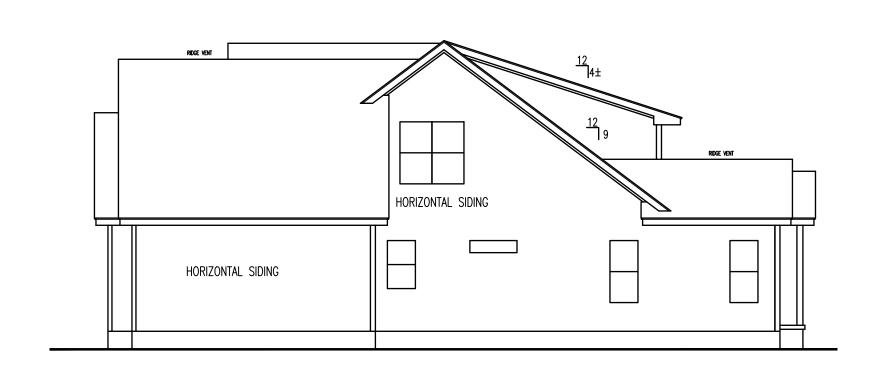




<u>FRONT ELEVATION</u> SCALE:1/4"=1'-0"

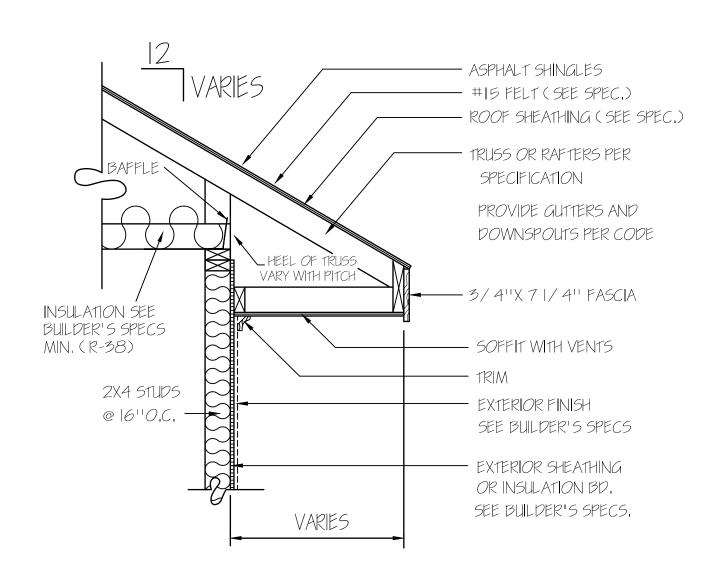


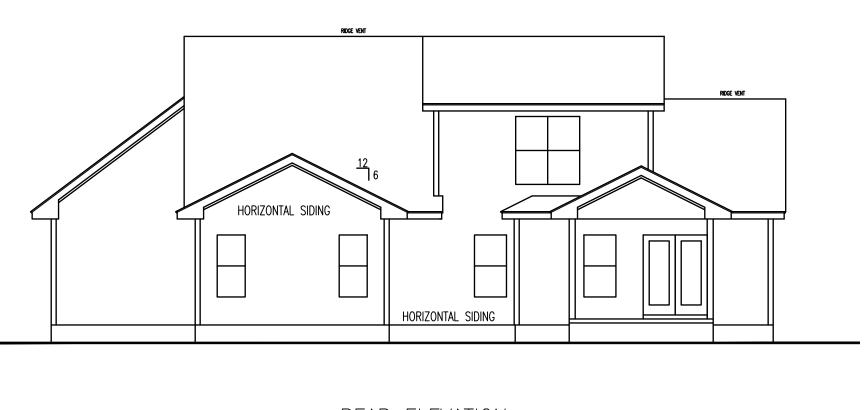




LEFT ELEVATION

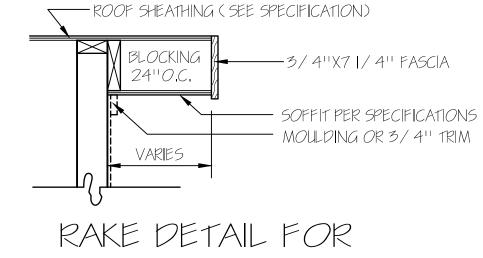
RIGHT ELEVATION





REAR ELEVATION

SCALE:1/8"=1'-0"



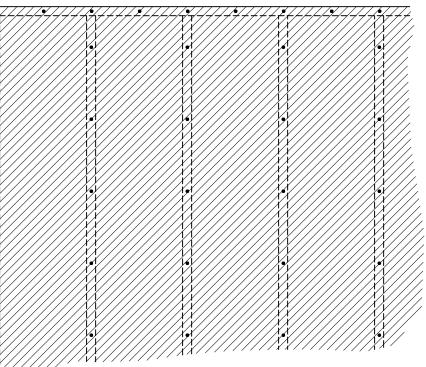
GABLE ENDS

16/0X8/0 OVERHEAD DR.

20'-0"

12'-0"

BRACING METHOD

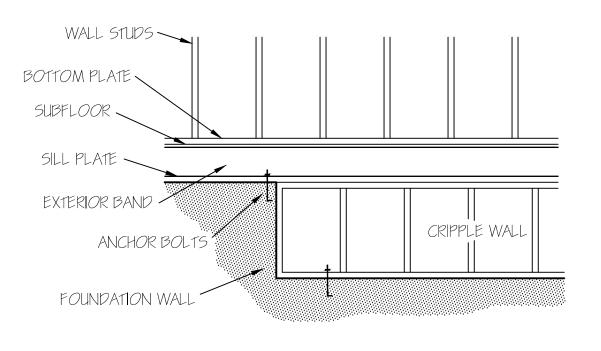


EXTERIOR WALL TO BE FULLY SHEATHED WITH 7/16" OSB. NAILING PATTERN TO BE 8" ON ALL EDGES AND 12" IN FIELD, WITH 8d NAILS.

ENERGY TABLE UFACTOR OF WINDOWS 30 CLIMATE ZONE 3 INSULATION: WALLS 15 CEILING 38 FL00R5 19

GARAGE PANEL WALL

GARAGE PANEL WALLS UNDER 24" WIDE SHOULD BE EITHER PORTAL FRAMED OR 7/16" OSB ON BOTH SIDES WITH A NAILING PATTERN OF 311 ON ALL PANEL EDGES AND 6" IN THE FIELD.



FOUNDATION CRIPPLE WALLS SHALL BE FRAMED OF STUDS NOT SMALLER THAN THE STUDDING ABOVE. WHEN EXCEEDING 4 FT. IN HEIGHT, SUCH WALLS SHALL BE FRAMED OF STUDS HAVING THE SIZE REQUIRED FOR CRIPPLE WALLS WITH A STUD HEIGHT LESS THAN 14 INCHES SHALL BE CONTINUOUSLY SHEATHED ON ONE SIDE WITH WOOD STRUCTURAL PANELS FASTENED TO BOTH THE TOP AND BOTTOM PLATES IN ACCORDANCE WITH TABLE R602.3(1),

OR CRIPPLE WALLS SHALL BE CONSTRUCTED OF SOLID BLOCKING.

NOTE: CEILINGS ARE 9'-0" UNLESS NOTED. SET WINDOWS @ 7'-4" UNLESS NOTED.

EXERIOR \	WALLS	
(2) 2X10 H	HEADER	S
CLEAR SPAN	NUMBER	
FOR HEADER	JACKS	KINGS
ALL DOOR & C.O. BELOW 4'	1	1
ALL DOOR & C.O. 4' TO 7'-11"	2	2
ALL DOOR & C.O. 8' AND ABOVE	SIZED ENGIN	
**UNLESS NOTED	OTHER	WISE*

FIRST FLOOR PLAN

SCALE:1/4"=1'-0" HEATED AREA

1ST FL 1421 SQ FT 2ND FL 910 SQ FT TOTAL 2331 SQ FT

GARAGE 683 SQ FT F.PORCH 131 SQ FT R.PORCH 143 SQ FT

TOTAL <u>957</u> SQ FT

OTHER AREAS

RESIDE (910) 3

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I HEREBY CERTIFY THAT THIS DRAWING MEETS LOCAL CODES, 2012 INTERNATIONAL BUILDING CODES

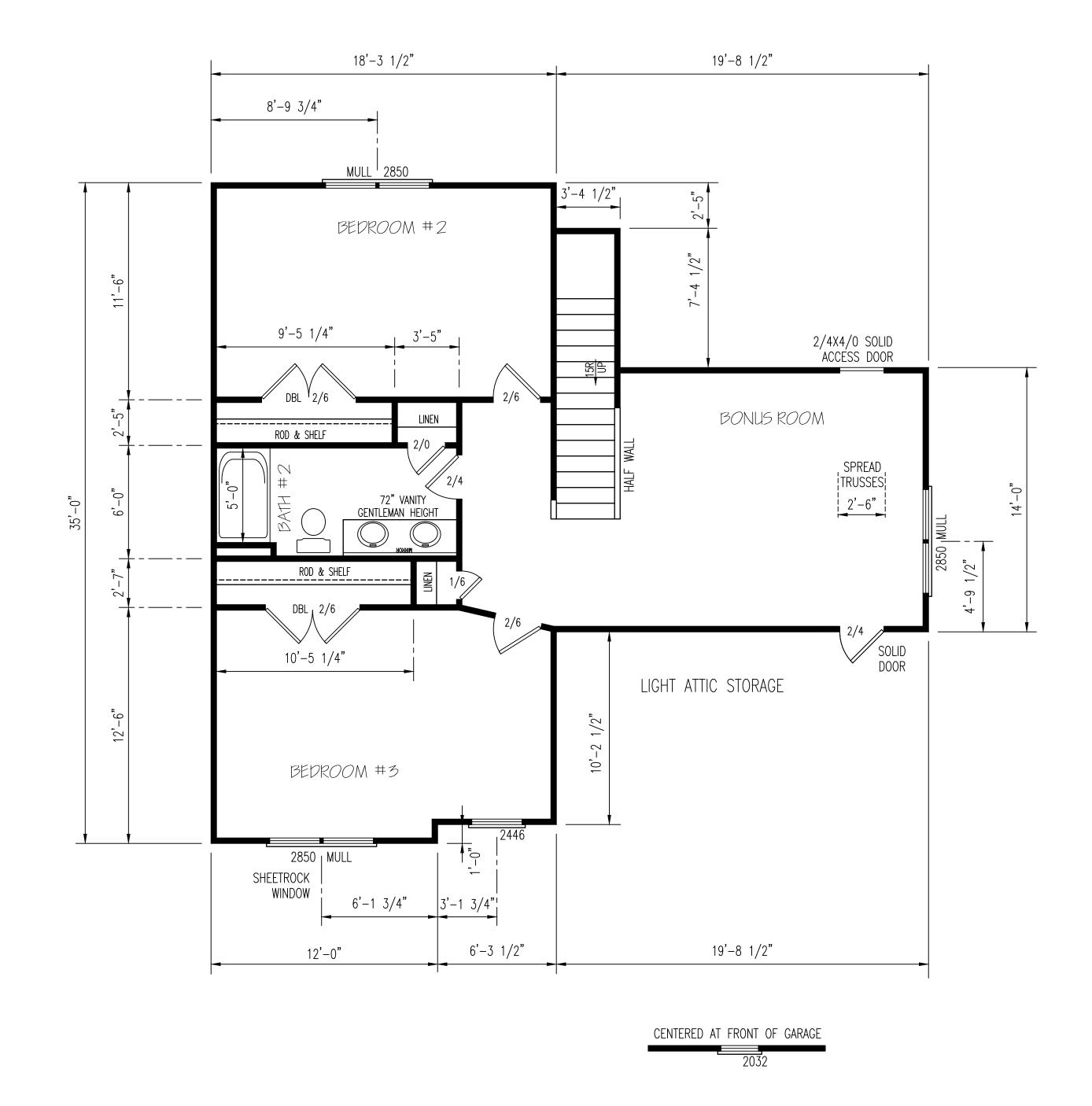
1HIS IS FOR THE CONSTRUCTION OF ONE HOUSE ON A SINGLE LOT, NOT TO BE REUSED

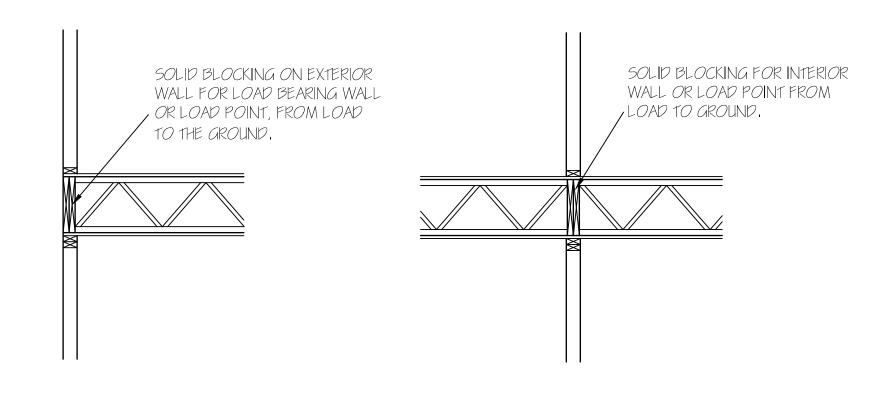
PLAN NUMBER BG22-B19

OPTION #6 GARAGE R F

DATE:

3/15/23





EXERIOR \((2) 2X10 H		S
CLEAR SPAN FOR HEADER	NUMBER JACKS	
ALL DOOR & C.O. BELOW 4'	1	1
ALL DOOR & C.O. 4' TO 7'-11"	2	2
ALL DOOR & C.O. 8' AND ABOVE	SIZED ENGIN	
UNLESS NOTED	OTHER	WISE

SECOND FLOOR PLAN

SCALE:1/4"=1'-0"

RESIDENTIAL PLANS BY TINA MCFADDE

K HOLOS OF ANDREW

NATER MA

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CONSTRUCTION, WHILE EVERY EFFORT
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THESE DRAWINGS AND DIMENSIONS TO
AVOID ERRORS THE OWNER AND / OR
BUILDER SHALL VERIFY ALL DIMENSIONS
DETAILS, LOCAL AND START CODES.

I HEREBY CERTIFY THAT THIS DRAWING
MEETS LOCAL CODES, 2012
INTERNATIONAL BUILDING CODES

1HIS IS FOR THE CONSTRUCTION
OF ONE HOUSE ON A SINGLE

PLAN NUMBER
BG22-B19

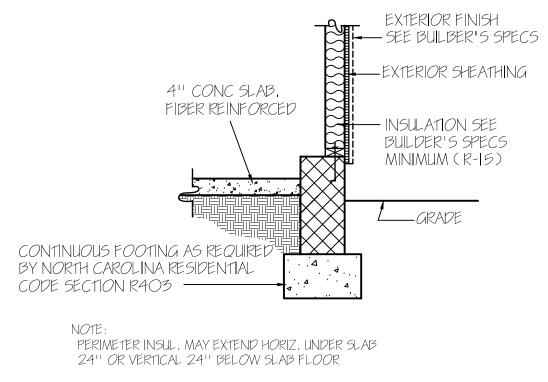
OPTION #6

GARAGE R DATE: 3/15/23

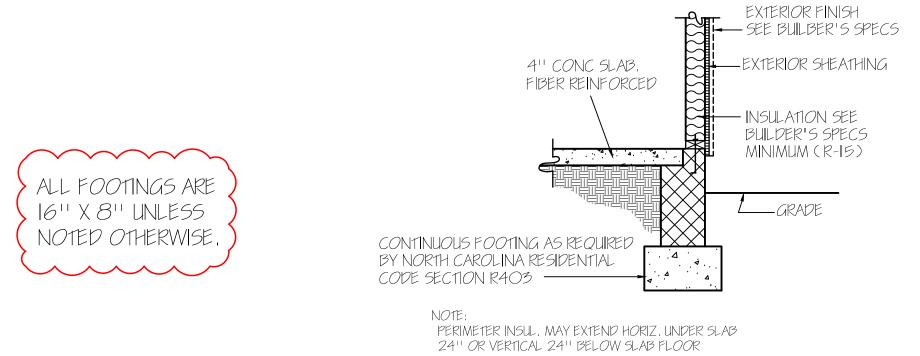


6 MILL, VAPOR BARRIER

2<u>'' CLEAR</u>



GARAGE WALL—(B)



CONCRETE SLAB FLOOR —(A)

ALL FOUNDATION WALLS HAVE A 16" X 8" FOOTING UNLESS NOTED OTHERWISE.

NOTE: FOUNDATION DETAILS SHOWN ARE BASED ON ASSUMED SOIL BEARING CAPACITY OF 2000 PSF. LOCAL SITE CONDITIONS MUST BE INVESTIGATED, ALL FOOTING TO BE LOCATED

BELOW FROST DEPTH.

FOUNDATION PLAN

ER

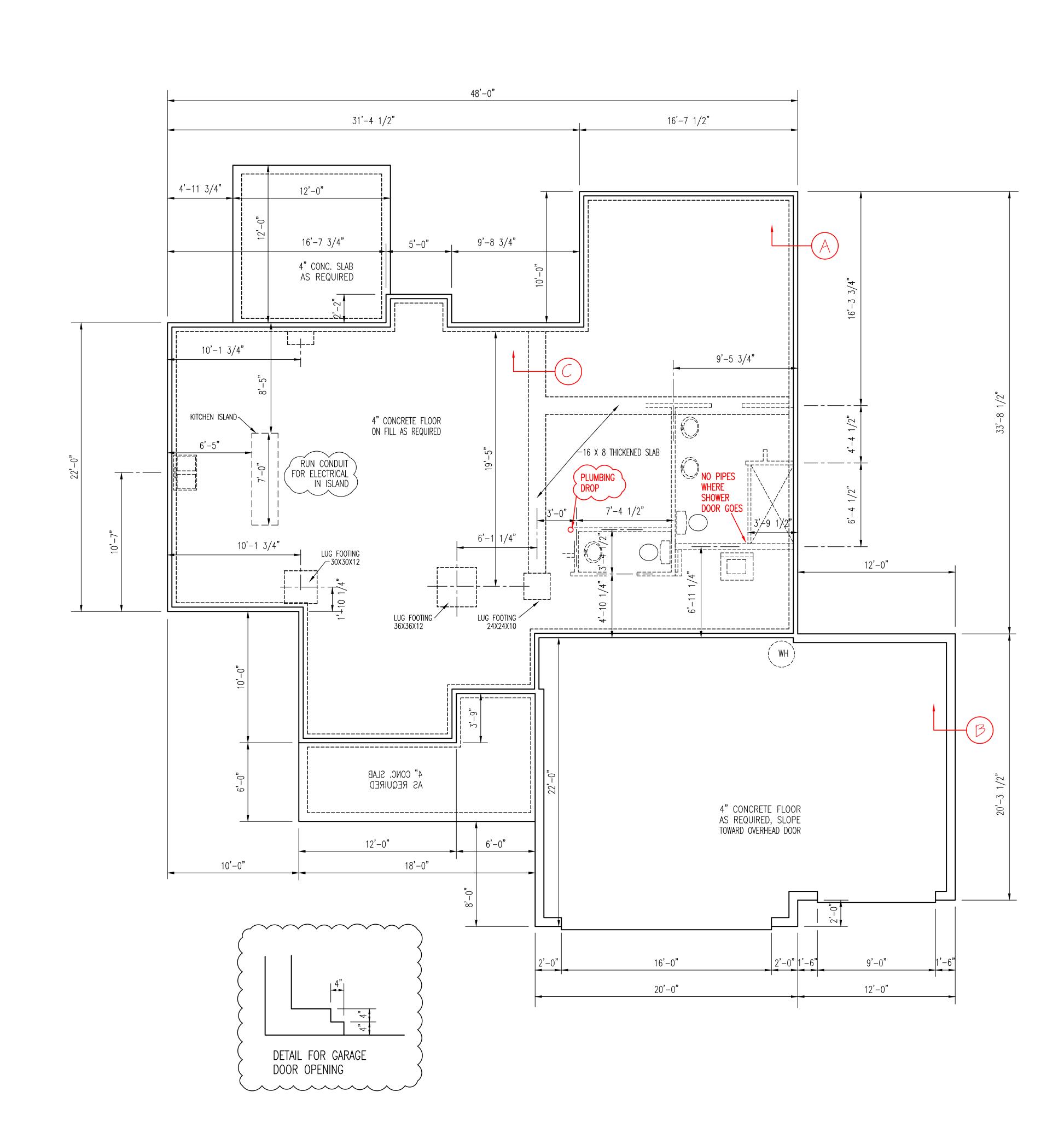
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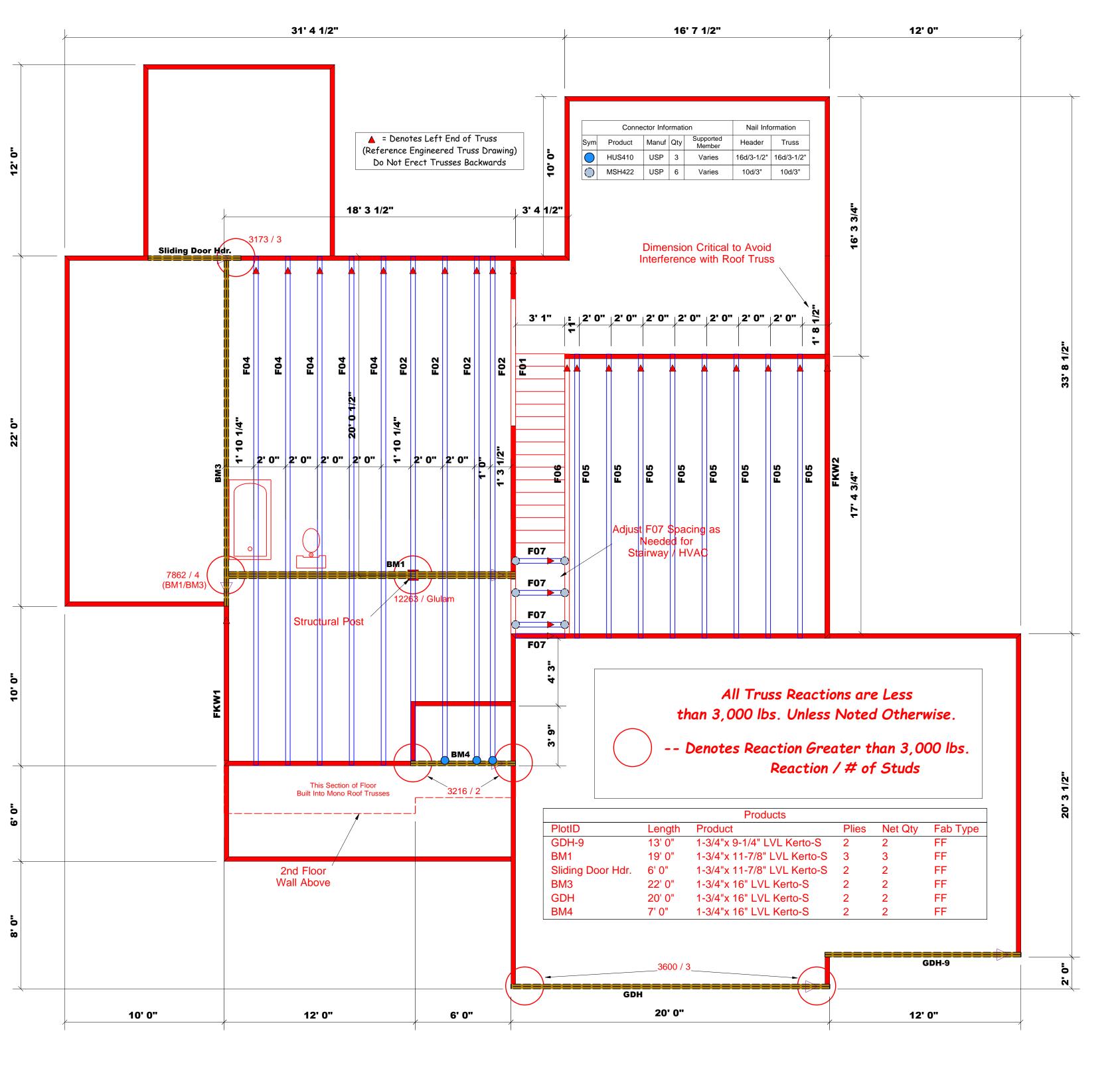
I HEREBY CERTIFY THAT THIS DRAWING MEETS LOCAL CODES, 2012 INTERNATIONAL BUILDING CODES

THIS IS FOR THE CONSTRUCTION OF ONE HOUSE ON A SINGLE LOT, NOT TO BE REUSED

PLAN NUMBER BG22-B19

OPTION #6





ROOF & FLOOR TRUSSES & BEAMS

> Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Anthony Williams

LOAD CHART FOR JACK STUDS
(BASED ON TABLES R502.5(1) & (b))

ADDRESS Lot 90 South Creek

MODEL Floor

DATE REV. 4/4/23

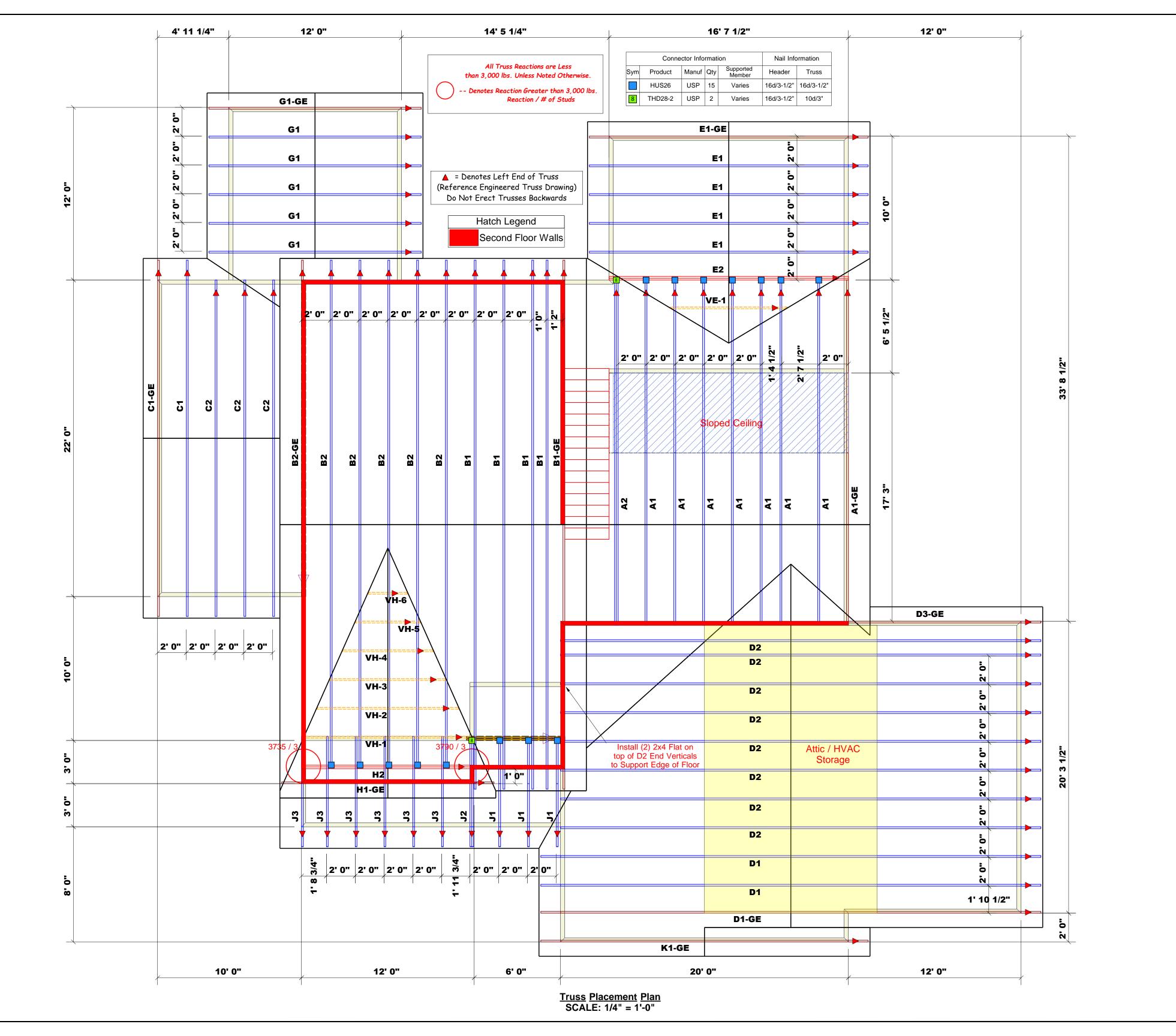
DRAWN BY Anthony Williams

SALESMAN Anthony Williams

BUILDERWatermark HomesJOB NAMELot 90 South CreekPLANThe GinkgoSEAL DATEPlan Date: 3/15/23QUOTE#Plan Date: 3/15/23

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

<u>Truss Placement Plan</u> SCALE: 1/4" = 1'-0"





Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Anthony Williams

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

-	COUNTY	Harnett
	ADDRESS	Lot 90 South Creek / Lillington, NC
	MODEL	Roof
	DATE REV . 4/4/23	4/4/23
	DRAWN BY	DRAWN BY Anthony Williams
	SALESMAN	SALESMAN Anthony Williams

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

3/15/23

J0423-1517

South Creek

Lot

Watermark Homes



Watermark Homes

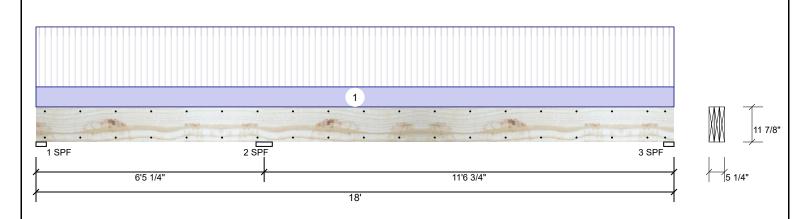
Project:

Address: Lot 90 South Creek 4/4/2023

Input by: Anthony Williams Job Name: The Ginkgo Project #: J0423-1517 & 1518

Kerto-S LVL 3-Ply - PASSED 1.750" X 11.875" BM1

evel: Level



Member	Information
T	0:1

Type.	Gildei
Plies:	3
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Slope: 0/12 Design Method: ASD **Building Code:** IBC 2012 Load Sharing: Yes Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	1162	409	0	0	0
2	Vertical	8927	3140	0	0	0
3	Vertical	3771	1326	0	0	0

Page 1 of 13

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-12572 ft-lb	6'5 1/4"	31060 ft-lb	0.405 (40%)	D+L	LL
Unbraced	-12572 ft-lb	6'5 1/4"	12575 ft-lb	1.000 (100%)	D+L	LL
Pos Moment	11556 ft-lb	13' 11/16"	31060 ft-lb	0.372 (37%)	D+L	_L
Unbraced	11556 ft-lb	13' 11/16"	11563 ft-lb	0.999 (100%)	D+L	_L
Shear	5753 lb	7'7 7/8"	13300 lb	0.433 (43%)	D+L	LL
LL Defl inch	0.148 (L/920)	12'5 1/2"	0.283 (L/480)	0.522 (52%)	L	_L
TL Defl inch	0.196 (L/695)	12'5 13/16"	0.378 (L/360)	0.518 (52%)	D+L	L

Neg Moment	-125/2 π-ID	65 1/4"	31060 IT-ID	0.405 (40%)	D+L	LL
Unbraced	-12572 ft-lb	6'5 1/4"	12575 ft-lb	1.000 (100%)	D+L	LL
Pos Moment	11556 ft-lb	13' 11/16"	31060 ft-lb	0.372 (37%)	D+L	_L
Unbraced	11556 ft-lb	13' 11/16"	11563 ft-lb	0.999 (100%)	D+L	_L
Shear	5753 lb	7'7 7/8"	13300 lb	0.433 (43%)	D+L	LL
LL Defl inch	0.148 (L/920)	12'5 1/2"	0.283 (L/480)	0.522 (52%)	L	_L
TL Defl inch	0.196 (L/695)	12'5 13/16"	0.378 (L/360)	0.518 (52%)	D+L	_L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6"
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Tie-down connection required at bearing 1 for uplift 734 lb (Combination D+L, Load Case _L).
- 7 Top must be laterally braced at a maximum of 12'7 5/16" o.c.
- 8 Bottom must be laterally braced at a maximum of 11'4 11/16" o.c.
- 9 Lateral slenderness ratio based on single ply width.

	Bearings	S						
I	Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
l	1 - SPF	3.500"	Vert	35%	376 / 2355	2731 (-734)	L_	D+L(D+L)
	2 - SPF	5.500"	Vert	100%	3191 / 9072	12263	LL	D+L
ı	3 - SPF	3.500"	Vert	66%	1308 / 3835	5143	_L	D+L

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
 - LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

 - Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 11/3/2024



Project: Address: Watermark Homes

Lot 90 South Creek

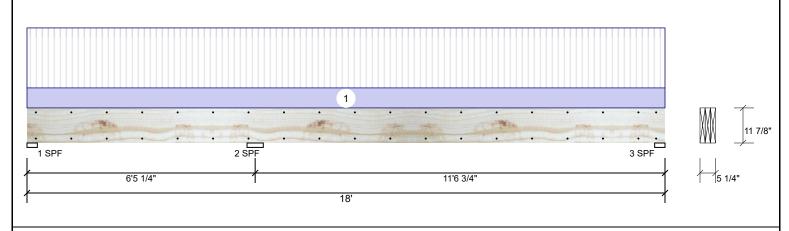
Date: Input by:

4/4/2023 Anthony Williams Page 2 of 13

Job Name: The Ginkgo Project #: J0423-1517 & 1518

1.750" X 11.875" **Kerto-S LVL** 3-Ply - PASSED BM1

evel: Level



ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	257 PLF	770 PLF	0 PLF	0 PLF	0 PLF	F04	
	Self Weight				14 PLF						

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- Handling & Installation

 1. IVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



This design is valid until 11/3/2024 CSD BUILD



Client: Address:

Watermark Homes

Project:

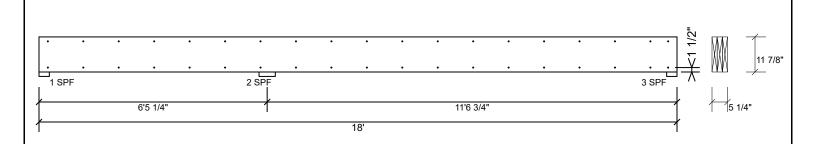
Lot 90 South Creek

4/4/2023

Input by: Anthony Williams Job Name: The Ginkgo Project #: J0423-1517 & 1518

1.750" X 11.875" **Kerto-S LVL** 3-Ply - PASSED BM₁

evel: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed 6".

Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	163.7 PLF	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Informing & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation
- - - This design is valid until 11/3/2024

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



Page 3 of 13





Watermark Homes

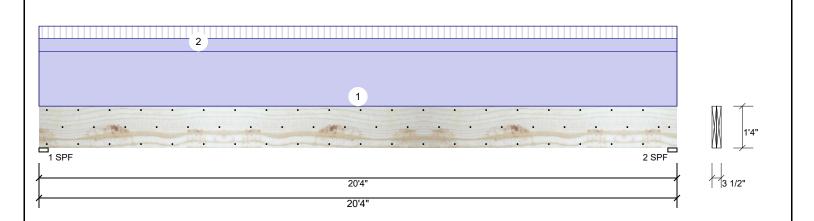
Project:

Address: Lot 90 South Creek Date: 4/4/2023

Input by: Anthony Williams Job Name: The Ginkgo Project #: J0423-1517 & 1518

2-Ply - PASSED Kerto-S LVL 1.750" X 16.000" BM₃

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Snow Wind Type: Application: Brg Direction Live Dead Const Plies: 2 Slope: 0/12 407 2312 Vertical n 0 0 1 Moisture Condition: Dry Design Method: ASD 2 Vertical 407 2312 0 0 0 Deflection LL: 480 **Building Code:** IBC 2012 Deflection TL: 360 Load Sharing: No Importance: Normal - II Deck: Not Checked Temp <= 100°F Temperature: **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+L 1-SPF 3.500" Vert 52% 2312 / 407 2719 L 2 - SPF 3.500" Vert 52% 2312 / 407 2719 L D+I

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	13233 ft-lb	10'2"	34565 ft-lb	0.383 (38%)	D+L	L
Unbraced	13233 ft-lb	10'2"	13276 ft-lb	0.997 (100%)	D+L	L
Shear	2301 lb	1'7 1/2"	11947 lb	0.193 (19%)	D+L	L
LL Defl inch	0.063 (L/3784)	10'2 1/16"	0.497 (L/480)	0.127 (13%)	L	L
TL Defl inch	0.422 (L/566)	10'2 1/16"	0.663 (L/360)	0.636 (64%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 8'11 1/4" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

o Eatoral dichacimode ratio bacca on chigle ply water.												
	ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
	1	Uniform			Тор	175 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL	
	2	Uniform			Тор	40 PLF	40 PLF	0 PLF	0 PLF	0 PLF	ROOF	
		Self Weight				12 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

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Page 4 of 13

This design is valid until 11/3/2024



BM₃

Kerto-S LVL

Client:

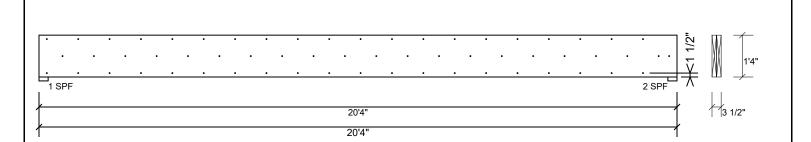
Watermark Homes

Project:

Address: Lot 90 South Creek Date: 4/4/2023

Input by: Anthony Williams Job Name: The Ginkgo Project #: J0423-1517 & 1518 Page 5 of 13

1.750" X 16.000" 2-Ply - PASSED Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

, ,		`	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	245.6 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Metsä Wood

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS







Watermark Homes

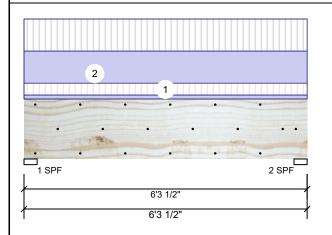
Project:

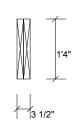
Address: Lot 90 South Creek Date: 4/4/2023

Input by: Anthony Williams Job Name: The Ginkgo Project #: J0423-1517 & 1518

1.750" X 16.000" **Kerto-S LVL** 2-Ply - PASSED BM4

Level: Level





Page 6 of 13

Member Information

Type:	Giraer
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Slope: 0/12 Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No

Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Wind	Const
0	0
0	0
	0

Bearings

Bearing	Length	Dir.	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	62%	1471 / 1746	3216	L	D+L
2 - SPF	3.500"	Vert	62%	1471 / 1746	3216	L	D+L

Analysis Results

Analysi	is	Actual	Location	Allowed	Capacity	Comb.	Case
Momen	nt	4380 ft-lb	3'1 3/4"	34565 ft-lb	0.127 (13%)	D+L	L
Unbrac	ed	4380 ft-lb	3'1 3/4"	19678 ft-lb	0.223 (22%)	D+L	L
Shear		1572 lb	1'7 1/2"	11947 lb	0.132 (13%)	D+L	L
LL Defl	inch	0.011 (L/6369)	3'1 3/4"	0.146 (L/480)	0.075 (8%)	L	L
TL Defl	inch	0.020 (L/3457)	3'1 3/4"	0.195 (L/360)	0.104 (10%)	D+L	L
Unbrac Shear LL Defl	ed	4380 ft-lb 1572 lb 0.011 (L/6369)	3'1 3/4" 1'7 1/2" 3'1 3/4"	19678 ft-lb 11947 lb 0.146 (L/480)	0.223 (22%) 0.132 (13%) 0.075 (8%)	D+L D+L L	L L L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	50 PLF	150 PLF	0 PLF	0 PLF	0 PLF	FLOOR
2	Uniform			Тор	405 PLF	405 PLF	0 PLF	0 PLF	0 PLF	J1
	Self Weight				12 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 2 Damaged Beams must not be used

 - Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

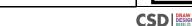
This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





BM4

Client:

Project: Address:

Watermark Homes

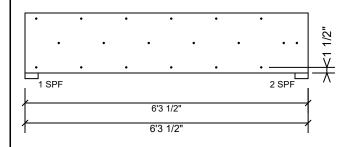
Lot 90 South Creek

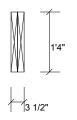
Date: 4/4/2023

Input by: Anthony Williams Job Name: The Ginkgo Project #: J0423-1517 & 1518

1.750" X 16.000" 2-Ply - PASSED **Kerto-S LVL**

Level: Level





Page 7 of 13

Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

rasteri dii pires darrig a rows or roa box rians (: 120x3)							
Capacity	0.0 %						
Load	0.0 PLF						
Yield Limit per Foot	245.6 PLF						
Yield Limit per Fastener	81.9 lb.						
Yield Mode	IV						
Edge Distance	1 1/2"						
Min. End Distance	3"						
Load Combination							
Duration Factor	1.00						

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

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This design is valid until 11/3/2024



Watermark Homes

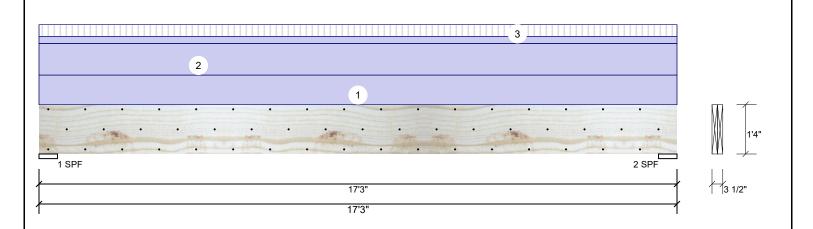
Project:

Address: Lot 90 South Creek Date: 4/4/2023

Input by: Anthony Williams Job Name: The Ginkgo Project #: J0423-1517 & 1518

2-Ply - PASSED **Kerto-S LVL** 1.750" X 16.000" **GDH**

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Wind Type: Application: Brg Direction Live Dead Snow Const Plies: 2 Slope: 0/12 518 3083 0 Vertical n 0 1 Moisture Condition: Dry Design Method: ASD 2 Vertical 518 3083 0 0 0 Deflection LL: 480 **Building Code:** IBC 2012 Deflection TL: 600 Load Sharing: No Importance: Normal - II Deck: Not Checked Temp <= 100°F Temperature: **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+L 1-SPF 6.000" Vert 40% 3083 / 518 3600 L

2 - SPF 6.000"

Vert

40%

3083 / 518

3600 L

D+I

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	14027 ft-lb	8'7 1/2"	34565 ft-lb	0.406 (41%)	D+L	L
Unbraced	14027 ft-lb	8'7 1/2"	14050 ft-lb	0.998 (100%)	D+L	L
Shear	2852 lb	1'10"	11947 lb	0.239 (24%)	D+L	L
LL Defl inch	0.045 (L/4374)	8'7 9/16"	0.410 (L/480)	0.110 (11%)	L	L
TL Defl inch	0.313 (L/629)	8'7 9/16"	0.328 (L/600)	0.954 (95%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 8'5 1/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

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ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
2	Uniform			Тор	160 PLF	0 PLF	0 PLF	0 PLF	0 PLF	BRICK
3	Uniform			Тор	35 PLF	60 PLF	0 PLF	0 PLF	0 PLF	Roof/Floor
	Self Weight				12 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

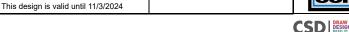
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GDH

Client:

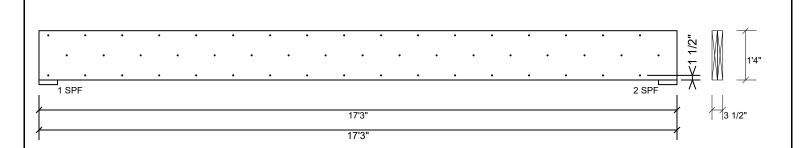
Watermark Homes

Project:

Address: Lot 90 South Creek 4/4/2023

Input by: Anthony Williams Job Name: The Ginkgo Project #: J0423-1517 & 1518

Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

1 3		•	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	245.6 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

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Manufacturer Info

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CSD DESIGN



Watermark Homes

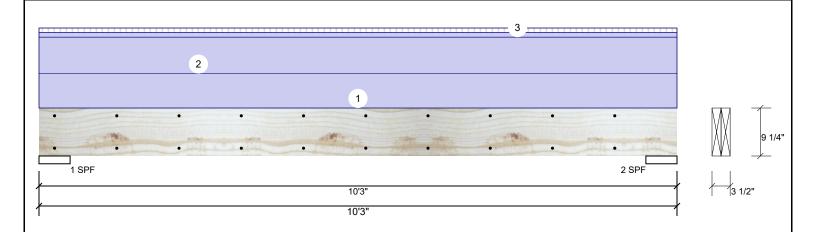
Project:

Address: Lot 90 South Creek Date: 4/4/2023

Input by: Anthony Williams Job Name: The Ginkgo Project #: J0423-1517 & 1518

2-Ply - PASSED GDH-9 Kerto-S LVL 1.750" X 9.250"

Level: Level



Member Info	rmation			Rea	ctions UNP	ATTERN	IED lb (Uplift	:)		
Туре:	Girder	Application:	Roof	Brg	Direction	Live	Dead	Snow	Wind	Const
Plies:	2	Slope:	0/12	1	Vertical	103	1728	0	0	0
Moisture Condition	on: Dry	Design Method:	ASD	2	Vertical	103	1728	0	0	0
Deflection LL:	480	Building Code:	IBC 2012							
Deflection TL:	600	Load Sharing:	No							
Importance:	Normal - II	Deck:	Not Checked							
Temperature:	Temp <= 100°F			<u> </u>						
				Bea	rings					
				Bea	aring Length	Dir.	Cap. React D/	∟lb Tota	l Ld. Case	Ld. Comb.
				1 -	SPF 6.000"	Vert	21% 1728 /	103 1831	l L	D+L
					SDE 6,000"	\/ort	21% 1728 /	103 1831		D±I

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3705 ft-lb	5'1 1/2"	11288 ft-lb	0.328 (33%)	D	Uniform
Unbraced	3924 ft-lb	5'1 1/2"	7663 ft-lb	0.512 (51%)	D+L	L
Shear	1305 lb	8'11 3/4"	6216 lb	0.210 (21%)	D	Uniform
LL Defl inch	0.008 (L/13536)	5'1 1/2"	0.234 (L/480)	0.035 (4%)	L	L
TL Defl inch	0.148 (L/758)	5'1 1/2"	0.188 (L/600)	0.792 (79%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

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ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
2	Uniform			Тор	160 PLF	0 PLF	0 PLF	0 PLF	0 PLF	BRICK
3	Uniform			Тор	20 PLF	20 PLF	0 PLF	0 PLF	0 PLF	Roof
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 2 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood

Manufacturer Info

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Page 10 of 13



Client: Watermark Homes

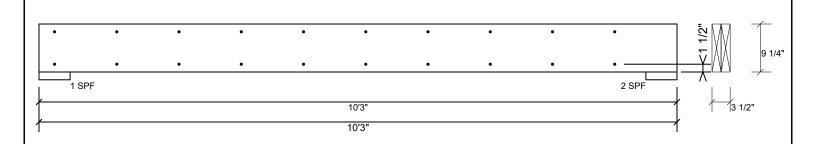
Project:

Address: Lot 90 South Creek Date: 4/4/2023

Input by: Anthony Williams Job Name: The Ginkgo Project #: J0423-1517 & 1518 Page 11 of 13

Kerto-S LVL 2-Ply - PASSED GDH-9 1.750" X 9.250"

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

, ,		,	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	163.7 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

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Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS





Sliding Door Header

Client:

nt: Watermark Homes

Project: Address:

Lot 90 South Creek

Date: 4/4/2023

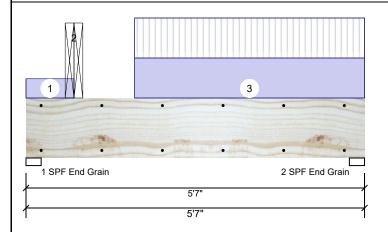
Project #:

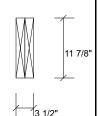
Input by: Anthony Williams
Job Name: The Ginkgo

J0423-1517 & 1518

Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level





Page 12 of 13

iviellibel illiolillatio	Member Inf	ormatio	n
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Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 240
Importance: Normal -

Importance: Normal - II
Temperature: Temp <= 100°F

Application: Floor
Design Method: ASD
Building Code: IBC 2012
Load Sharing: No
Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	684	2489	0	0	0
2	Vertical	709	960	0	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2327 ft-lb	2'5 3/16"	19911 ft-lb	0.117 (12%)	D+L	L
Unbraced	2327 ft-lb	2'5 3/16"	15061 ft-lb	0.155 (15%)	D+L	L
Shear	1840 lb	1'2 7/8"	8867 lb	0.208 (21%)	D+L	L
LL Defl inch	0.007 (L/9597)	2'9 7/8"	0.174 (L/360)	0.038 (4%)	L	L
TL Defl inch	0.018 (L/3391)	2'7 1/2"	0.260 (L/240)	0.071 (7%)	D+L	L

Bearings

Bearing	Length	Dir.	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.000"	Vert	36%	2489 / 684	3173	L	D+L
2 - SPF End Grain	3.000"	Vert	19%	960 / 709	1669	L	D+L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

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ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Part. Uniform	0-0-0 to 0-9-8		Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall Load	
2	Point	0-9-8		Тор	2312 lb	407 lb	0 lb	0 lb	0 lb	BM3 Brg 1	
	Bearing Length	0-3-8									
3	Part. Uniform	1-9-8 to 5-7-0		Тор	260 PLF	260 PLF	0 PLF	0 PLF	0 PLF	C2	
	Self Weight				9 PLF						

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumbe

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- 1. I.V.I. beams must not be cut or drilled
 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 3. Damaged Beams must not be used
- Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

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This design is valid until 11/3/2024

Client:

Watermark Homes

Project: Address:

Lot 90 South Creek

Date: 4/4/2023

Project #:

Input by: Anthony Williams Job Name: The Ginkgo

J0423-1517 & 1518

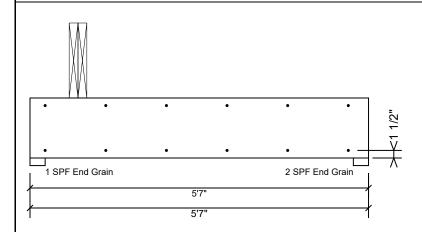
Level: Level

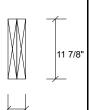
Sliding Door Header

Kerto-S LVL

1.750" X 11.875"

2-Ply - PASSED





Page 13 of 13

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

rasterrain piles asing 2 rows	or roa box rians (. 120x5) at
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



CSD DESIGN

ROOF & FLOOR ComTech| TRUSSES & BEAMS Reilly Road Industrial Park P.O. Box 40408

Fayetteville, N.C. 28309 (910) 864-TRUS

REQ. QUOTE DATE	11	ORDER#	J0423-1517
ORDER DATE	04/04/23	QUOTE #	
DELIVERY DATE	11	CUSTOMER ACCT#	000030
DATE OF INVOICE	11	CUSTOMER PO#	
ORDERED BY	Brady Rufenacht	INVOICE #	
COUNTY	Harnett	TERMS	Net 10 Days
SUPERINTENDANT	Josh Thompson	SALES REP	Anthony Williams
JOBSITE PHONE #	(910) 988-3421	SALES AREA	Anthony Williams

Watermark Homes, Inc. 196 Annettte Drive Benson, NC 27504 (919) 938-8194

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JOB NAME: Lot 90 South Creek

LOT # 90 **SUBDIV:** South Creek

JOB CATEGORY:

DELIVERY INSTRUCTIONS:

MODEL: Roof

LOADING

Watermark Homes Lot 90 South Creek Benson, NC

ROOF TRUSSES

SPECIAL INSTRUCTIONS:

Copied from Lot 50 SB (J0822-3932)

TCLL-TCDL-BCLL-BCDL

PLAN SEAL DATE:

DATE 04/04/23 PAGE 1

DATE BUILDING DEPARTMENT OVERHANG INFO HEEL HEIGHT 00-04-05 REQ. LAYOUTS REQ. ENGINEERING QUOTE END CUT RETURN LAYOUT // Roof Order PLUMB **GABLE STUDS** 24 IN. OC JOBSITE JOBSITE 1 CUTTING

STRESS INCR.

TAG: The Ginkgo III

KUUF						TCLL-TCDL-BCLL-BCDL STRESS INCR. 20.0,10.0,0.0,10.0 1.15				ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)					
	QTY	DIT	СН	TYPE	BASE	_	_	=						1	
PROFILE	PLY	TOP	ВОТ	ID	O/A	TOP	BER BOT	LEFT	HANG RIGHT	REACTIO	NS				
	7	10.00		ROOF	24-00-00 24-00-00				RIGHT	Joint 1 376.7 lbs. -255.4 lbs.	Joint 7 725.5 lbs. -29.5 lbs.	Joint 9 1531.1 lbs. -539.1 lbs.			
a a	1	10.00	0.00	GABLE A1-GE	24-00-00 24-00-00	2 X 6	2 X 6			Joint 1 917.9 lbs. -545.8 lbs.	Joint 6 355.9 lbs. -198.5 lbs.	Joint 13 118.3 lbs. 6.4 lbs.	Joint 14 262.0 lbs. -162.6 lbs.	Joint 15 161.7 lbs. -87.1 lbs.	
	1 2 Ply	10.00	0.00	ROOF A2	24-00-00 24-00-00	2 X 6	2 X 6			Joint 1 645.8 lbs. -437.9 lbs.	Joint 7 1243.8 lbs. -50.6 lbs.	Joint 9 2624.8 lbs. -924.2 lbs.			
	4	4.00	0.00	COMMON B1	34-00-00 34-00-00	2 X 6	2 X 6	01-04-08	01-04-08	Joint 2 1426.1 lbs. -153.5 lbs.	Joint 8 1426.1 lbs. -153.5 lbs.				
	1	4.00	0.00	COMMON B1-GE	34-00-00 34-00-00	2 X 6	2 X 6	01-04-08	01-04-08	Joint 2 187.9 lbs. -67.0 lbs.	Joint 20 187.9 lbs. -81.8 lbs.	Joint 22 220.6 lbs. -73.9 lbs.	Joint 23 140.2 lbs. -55.0 lbs.	Joint 24 164.3 lbs. -56.7 lbs.	
	5	4.00	0.00	COMMON B2	34-00-00 33-08-08	2 X 6	2 X 6	01-04-08		Joint 2 1418.4 lbs. -153.3 lbs.	Joint 8 1337.7 lbs. -104.4 lbs.				
	1	4.00	0.00	COMMON B2-GE	34-00-00 33-08-08	2 X 6	2 X 6	01-04-08		Joint 2 188.0 lbs. -65.6 lbs.	Joint 20 92.3 lbs. 0.8 lbs.	Joint 21 224.5 lbs. -83.7 lbs.	Joint 22 139.7 lbs. -49.1 lbs.	Joint 23 164.3 lbs. -57.2 lbs.	
	1	9.00	0.00	COMMON C1	22-00-00 22-00-00	2 X 6	2 X 6	01-04-08	01-04-08	Joint 2 984.6 lbs. -56.5 lbs.	Joint 8 984.6 lbs. -56.5 lbs.				
	1	9.00	0.00	GABLE C1-GE	22-00-00 22-00-00	2 X 6	2 X 6	01-04-08	01-04-08	Joint 2 284.2 lbs. -88.6 lbs.	Joint 14 239.4 lbs. -29.1 lbs.	Joint 16 264.9 lbs. -215.0 lbs.	Joint 17 152.1 lbs. -69.4 lbs.	Joint 18 218.3 lbs. -116.5 lbs.	
	3	9.00	0.00	COMMON C2	22-00-00 22-00-00	2 X 6	2 X 6		01-04-08	Joint 1 912.7 lbs. -38.7 lbs.	Joint 7 985.9 lbs. -56.6 lbs.				
	2	9.00	0.00	SPECIAL D1	32-00-00 32-00-00	2 X 6	2 X 6	01-04-08	01-04-08	Joint 2 1517.1 lbs. -76.0 lbs.	Joint 12 1517.1 lbs. -76.0 lbs.				

Reaction Summary of Order

ROOF & FLOOR

LOADING

ComTech| TRUSSES & BEAMS Reilly Road Industrial Park P.O. Box 40408 Fayetteville, N.C. 28309 (910) 864-TRUS

REQ. QUOTE DATE	11	ORDER#	J0423-1517
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JOBSITE PHONE #	(910) 988-3421	SALES AREA	Anthony Williams

Watermark Homes, Inc. 196 Annettte Drive Benson, NC 27504 (919) 938-8194

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JOB NAME: Lot 90 South Creek **LOT #** 90 **SUBDIV:** South Creek

MODEL: Roof TAG: The Ginkgo III **DELIVERY INSTRUCTIONS:**

JOB CATEGORY:

Watermark Homes Lot 90 South Creek Benson, NC

ROOF TRUSSES

SPECIAL INSTRUCTIONS:

Copied from Lot 50 SB (J0822-3932)

TCLL-TCDL-BCLL-BCDL

PLAN SEAL DATE:

DATE BUILDING DEPARTMENT OVERHANG INFO HEEL HEIGHT 00-04-05 REQ. LAYOUTS REQ. ENGINEERING QUOTE END CUT RETURN LAYOUT // Roof Order PLUMB **GABLE STUDS** 24 IN. OC JOBSITE JOBSITE 1 CUTTING

STRESS INCR.

ROOF TRUSSES LOADING INFORMATION				TCLL-TCDL-BCLL-BCDL STRESS INCR. 20.0,10.0,0.0,10.0 1.15				ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)						
PROFILE	QTY	PIT	СН		TYPE BASE LUMBER OVERHANG									
PROFILE	PLY	TOP	ВОТ	ID	O/A	TOP	вот	LEFT	RIGHT	REACTIO	NS			
	1	9.00	0.00	QUEENPOST D1-GE	32-00-00 32-00-00			01-04-08	01-04-08	Joint 2 446.4 lbs. -28.2 lbs.	Joint 19 984.1 lbs. -173.5 lbs.	Joint 23 1094.5 lbs. -1.2 lbs.	Joint 24 0.7 lbs. -472.3 lbs.	Joint 25 230.7 lbs. -60.0 lbs.
	8	9.00	0.00	SPECIAL D2	32-00-00 32-00-00	2 X 6	2 X 6	01-04-08		Joint 2 1488.1 lbs. -74.8 lbs.	Joint 12 1421.6 lbs. -54.7 lbs.			
<u> </u>	1	9.00	0.00	MONOPITCH D3-GE	12-00-00 12-00-00	2 X 6	2 X 6	01-04-08		Joint 2 431.7 lbs. -81.7 lbs.	Joint 11 92.8 lbs. -44.5 lbs.	Joint 12 232.8 lbs. -85.7 lbs.	Joint 13 248.3 lbs. -103.8 lbs.	Joint 14 210.1 lbs. -100.2 lbs.
	4	6.00	0.00	COMMON E1	16-07-08 16-07-08	2 X 6	2 X 6	01-04-08	01-04-08	Joint 2 738.0 lbs. -55.9 lbs.	Joint 6 738.0 lbs. -55.9 lbs.			
	1	6.00	0.00	COMMON E1-GE	16-07-08 16-07-08	2 X 6	2 X 6	01-04-08	01-04-08	Joint 2 190.3 lbs. -37.9 lbs.	Joint 12 190.3 lbs. -28.4 lbs.	Joint 14 161.6 lbs. -92.7 lbs.	Joint 15 160.0 lbs. -69.8 lbs.	Joint 16 165.6 lbs. -62.3 lbs.
	1 2 Ply	6.00	0.00	COMMON E2	16-07-08 16-07-08	2 X 6	2 X 10			Joint 1 1642.4 lbs. -1060.4 lbs.	Joint 5 2206.8 lbs. -1474.2 lbs.			
	5	6.00	0.00	COMMON G1	12-00-00 12-00-00	2 X 6	2 X 6	01-04-08	01-04-08	Joint 1 97.7 lbs. -15.4 lbs.	Joint 2 553.0 lbs. -44.9 lbs.	Joint 3 97.8 lbs. -19.0 lbs.	Joint 4 164.5 lbs. 11.7 lbs.	Joint 6 553.0 lbs. -44.9 lbs.
	1	6.00	0.00	GABLE G1-GE	12-00-00 12-00-00	2 X 6	2 X 6	01-04-08	01-04-08	Joint 2 181.2 lbs. -36.0 lbs.	Joint 8 1453.6 lbs. -143.7 lbs.	Joint 10 181.2 lbs. -39.8 lbs.	Joint 12 141.4 lbs. -81.4 lbs.	Joint 13 170.1 lbs. -66.6 lbs.
	1	9.00	0.00	COMMON H1-GE	12-00-00 12-00-00	2 X 6	2 X 6	01-04-08	01-04-08	Joint 10 185.9 lbs. -39.4 lbs.	Joint 11 168.6 lbs. -136.9 lbs.	Joint 12 188.0 lbs. -79.9 lbs.	Joint 13 169.9 lbs. 55.1 lbs.	Joint 14 188.2 lbs. -80.0 lbs.
	1 2 Ply	9.00	0.00	COMMON H2	12-00-00 12-00-00	2 X 6	2 X 8			Joint 1 3790.6 lbs. -313.2 lbs.	Joint 3 3734.4 lbs. -308.2 lbs.			
	3	5.00	0.00	ROOF J1	03-00-00 03-00-00	2 X 6	2 X 6	01-04-08	03-00-00	Joint 2 826.1 lbs. -85.7 lbs.	Joint 11 1617.8 lbs. -175.3 lbs.			

Reaction Summary of Order

ROOF & FLOOR ComTech TRUSSES & BEAMS Reilly Road Industrial Park P.O. Box 40408 Fayetteville, N.C. 28309 (910) 864-TRUS

		DATE	04/04/23 PAGE 3
REQ. QUOTE DATE	11	ORDER#	J0423-1517
ORDER DATE	04/04/23	QUOTE #	
DELIVERY DATE	11	CUSTOMER ACCT#	000030
DATE OF INVOICE	11	CUSTOMER PO#	
ORDERED BY	Brady Rufenacht	INVOICE #	
COUNTY	Harnett	TERMS	Net 10 Days
SUPERINTENDANT	Josh Thompson	SALES REP	Anthony Williams
JOBSITE PHONE #	(910) 988-3421	SALES AREA	Anthony Williams

Watermark Homes, Inc. 196 Annettte Drive Benson, NC 27504 (919) 938-8194

JOB NAME: Lot 90 South Creek **LOT #** 90 **SUBDIV:** South Creek MODEL: Roof TAG: The Ginkgo III JOB CATEGORY:

DELIVERY INSTRUCTIONS:

Watermark Homes Lot 90 South Creek Benson, NC

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S H I P

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SPECIAL INSTRUCTIONS:

Copied from Lot 50 SB (J0822-3932)

PLAN SEAL DATE:

DATE BUILDING DEPARTMENT OVERHANG INFO HEEL HEIGHT 00-04-05 REQ. LAYOUTS QUOTE **REQ. ENGINEERING** END CUT RETURN LAYOUT // Roof Order PLUMB **GABLE STUDS** 24 IN. OC JOBSITE JOBSITE 1 CUTTING

ROOF T	RUS	SES		DADING FORMATION	TCLL-TCDL-B0		_	RESS INCR.	ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)							
PROFILE	QTY	PIT	CH	TYPE	BASE			OVER	HANG	REACTIO	NS					
	PLY	TOP	BOT	ID	O/A	TOP	BOT	LEFT	RIGHT							
	1 2 Ply	5.00	0.00	ROOF J2	03-00-00 03-00-00	2 X 6	2 X 6	01-04-08	03-00-00	Joint 2 1357.2 lbs. -162.7 lbs.	Joint 9 3010.7 lbs. -331.8 lbs.					
	6	5.00	0.00	ROOF J3	03-00-00 03-00-00	2 X 6	2 X 6	01-04-08	03-00-00	Joint 2 631.9 lbs. -89.5 lbs.	Joint 7 595.4 lbs. -85.8 lbs.					
	1	9.00	0.00	COMMON K1-GE	20-00-00 20-00-00	2 X 6	2 X 6	01-04-08	01-04-08	Joint 2 1051.8 lbs. -72.2 lbs.	Joint 8 1255.8 lbs. -63.5 lbs.	Joint 14 211.4 lbs. -36.5 lbs.	Joint 16 195.5 lbs. -191.0 lbs.	Joint 17 174.4 lbs. -89.2 lbs.		
	1	6.00	0.00	VALLEY VE-1	08-02-04 08-02-04	2 X 4	2 X 4			Joint 1 24.7 lbs. -6.4 lbs.	Joint 3 141.5 lbs. -26.5 lbs.	Joint 4 273.3 lbs. 11.5 lbs.	Joint 5 20.1 lbs. 0.4 lbs.	Joint 6 295.6 lbs. -69.6 lbs.		
	1	9.00	0.00	GABLE VH-1	11-07-11 11-07-11	2 X 4	2 X 4			Joint 1 221.9 lbs. -26.2 lbs.	Joint 3 222.0 lbs. -35.4 lbs.	Joint 4 417.3 lbs. 10.3 lbs.				
	1	9.00	0.00	VALLEY VH-2	09-10-05 09-10-05	2 X 4	2 X 4			Joint 1 185.3 lbs. -21.9 lbs.	Joint 2 175.8 lbs. -10.7 lbs.	Joint 3 185.3 lbs. -29.6 lbs.	Joint 4 348.3 lbs. 8.6 lbs.	Joint 7 20.2 lbs. 6.1 lbs.		
	1	9.00	0.00	VALLEY VH-3	08-01-00 08-01-00	2 X 4	2 X 4			Joint 1 161.8 lbs. -25.1 lbs.	Joint 3 161.8 lbs. -31.3 lbs.	Joint 4 253.1 lbs. 22.0 lbs.				
	1	9.00	0.00	VALLEY VH-4	06-03-11 06-03-11	2 X 4	2 X 4			Joint 1 121.9 lbs. -18.9 lbs.	Joint 3 121.9 lbs. -23.6 lbs.	Joint 4 190.7 lbs. 16.5 lbs.				
	1	9.00	0.00	VALLEY VH-5	04-06-05 04-06-05	2 X 4	2 X 4			Joint 1 82.0 lbs. -12.7 lbs.	Joint 3 82.0 lbs. -15.8 lbs.	Joint 4 128.2 lbs. 11.1 lbs.				
	1	9.00	0.00	VALLEY VH-6	02-09-00 02-09-00	2 X 4	2 X 4			Joint 1 75.0 lbs. -3.7 lbs.	Joint 3 75.0 lbs. -3.7 lbs.					

ITEMS

	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
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Reaction Summary of Order

ROOF & FLOOR
TRUSSES & BEAMS
Reilly Road Industrial Park P.O. Box 40408

Fayetteville, N.C. 28309 (910) 864-TRUS

		DATE	04/04/23 PAGE 4
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Watermark Homes, Inc.							
196 Annettte Drive							
Benson, NC 27504							
Benson, NC 27504 (919) 938-8194							

JOB NAME: Lot 90 South Creek

MODEL: Roof TAG: The Ginkgo III

LOT # 90 SUBDIV: South Creek

JOB CATEGORY:

DELIVERY INSTRUCTIONS:

Watermark Homes Lot 90 South Creek Benson, NC

SPECIAL INSTRUCTIONS:

Copied from Lot 50 SB (J0822-3932)

PLAN SEAL DATE:

BUILDING DEPARTMENT	OVERHA	NG INFO	HEEL HEIGHT	00-04-05	R	EQ. I	LAYOUTS		REQ.	ENG	GINEERING		QUOTE	11
Roof Order	END CUT	RETURN											LAYOUT	/ /
	PLUMB		GABLE STUDS	24 IN. OC			JOBSITE	1			JOBSITE	1	CUTTING	11

ITEMS

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QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
15	Hangers, USP	HUS 26			SIMPSON (HUS26)
2	Hangers, USP	THD28-2			SIMPSON (HHUS28-2)