DETAILS, LOCAL AND START CODES.

I HEREBY CERTIFY THAT THIS DRAWING MEETS LOCAL CODES, 2012
INTERNATIONAL BUILDING CODES

THIS IS FOR THE CONSTRUCTION OF ONE HOUSE ON A SINGLE

OF ONE HOUSE ON A SINGLI LOT, NOT TO BE REUSED

PLAN NUMBER

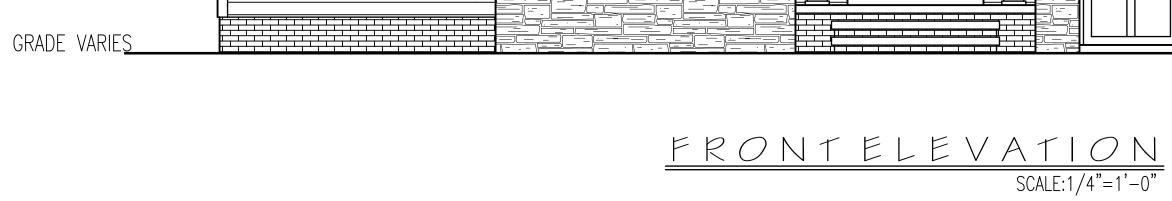
OPTION #

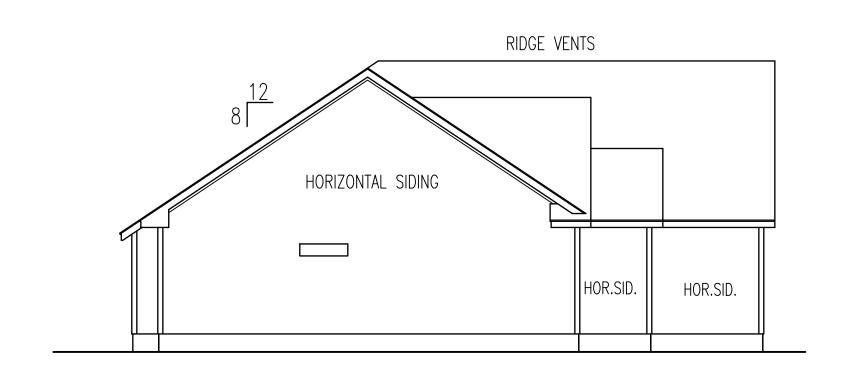
GARAGE F R

DATE:

6/19/22







- ASPHALT SHINGLES

SPECIFICATION

— 3/4"X7I/4" FASCIA

— SOFFIT WITH VENTS

- EXTERIOR FINISH

SEE BUILDER'S SPECS

- EXTERIOR SHEATHING OR INSULATION BD.

SEE BUILDER'S SPECS.

HEEL OF TRUSS
VARY WITH PITCH

VARIES

INSULATION SEE BUILDER'S SPECS MIN. (R-38)

2X4 51UD5

@ 16"O.C. —

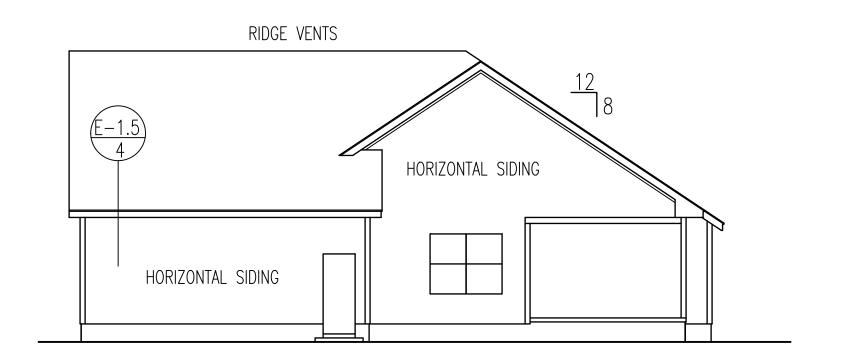
#15 FELT (SEE SPEC.)

TRUSS OR RAFTERS PER

PROVIDE GLITTERS AND DOWNSPOLITS PER CODE

— ROOF SHEATHING (SEE SPEC.)



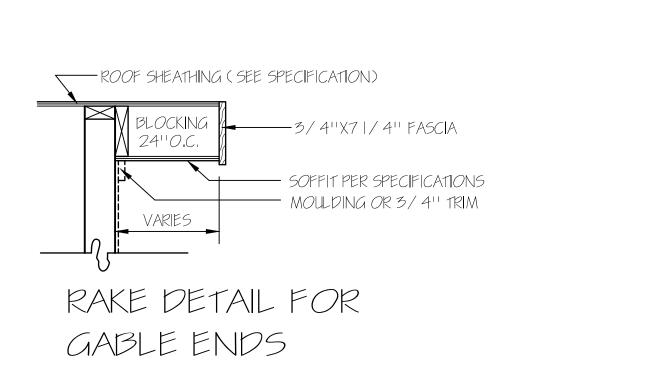


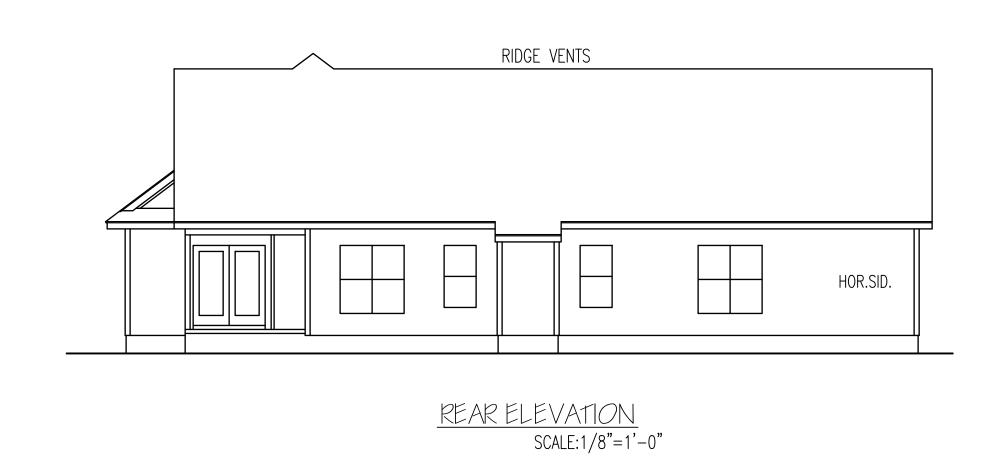
RIGHT ELEVATION

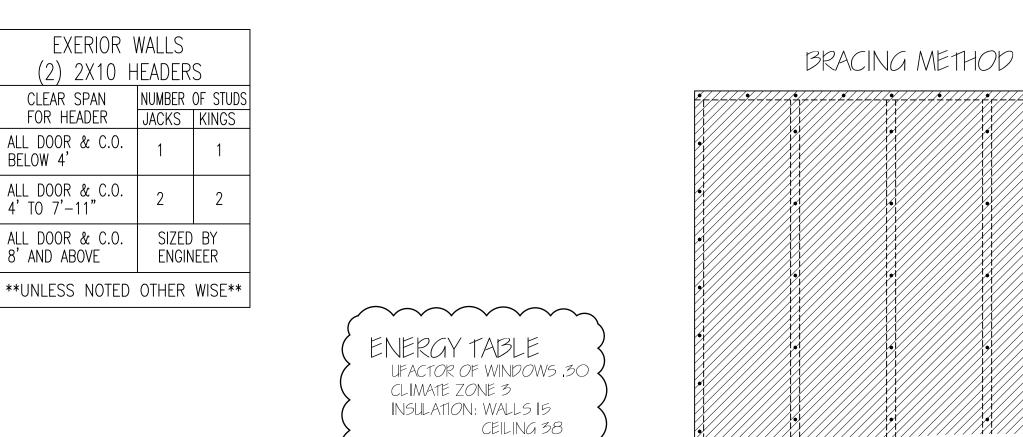
ATTIC VENTILATION CALCULATIONS

ATTIC AREA 2976 SQ.FT. (AREA VENTILATION REQUIRED 19.8 SQ.FT.)

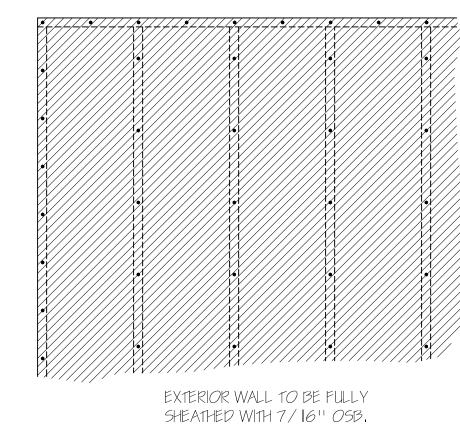
EACH FT. BASE GABLE LOUVER © SQ.FT. NET FREE AREA
EACH FT. BASE GABLE LOUVER © SQ.FT. NET FREE AREA
LOUVER © SQ.FT. NET FREE AREA
90 LIN.FT. EAVE VENT © 11 SQ.IN./FT.= 6.9 SQ.FT.NET FREE AREA
101 LIN.FT. RIDGE VENT © 18 SQ.IN./FT.= 12.6 SQ.FT.NET FREE AREA







FLOORS 19



NAILING PATTERN TO BE

IN FIELD, WITH 8d NAILS.

8" ON ALL EDGES AND 12"

WALL STUDS BOTTOM PLATE SUBFLOOR ~ SILL PLATE ~ EXTERIOR BAND CRIPPLE WALL ANCHOR BOLTS FOUNDATION WALL

> FOUNDATION CRIPPLE WALLS SHALL BE FRAMED OF STUDS NOT SMALLER THAN THE STUDDING ABOVE. WHEN EXCEEDING 4 FT. IN HEIGHT, SUCH WALLS SHALL BE FRAMED OF STUDS HAVING THE SIZE REQUIRED FOR AN ADDITIONAL STORY. CRIPPLE WALLS WITH A STUD HEIGHT LESS THAN 14 INCHES SHALL BE CONTINUOUSLY SHEATHED ON ONE SIDE WITH WOOD STRUCTURAL PANELS FASTENED TO BOTH THE TOP AND BOTTOM PLATES IN ACCORDANCE WITH TABLE R602.3(1), OR CRIPPLE WALLS SHALL BE CONSTRUCTED OF SOLID BLOCKING.

> > NOTE:
> > CEILINGS ARE 9'-0"
> > UNLESS NOTED.

FIRST FLOOR PLAN

HEATED AREA 15TFL 1921 SQFT 2NDFL 367 SQFT 10TAL 2288 SQFT

OTHER AREAS

GARAGE <u>858</u> SQ FT STORAGE 181 SQ FT F.PORCH 60 SQFT R.PORCH <u>137</u> SQ FT

GARAGE PANEL WALL

GARAGE PANEL WALLS UNDER 24" WIDE SHOULD BE EITHER PORTAL FRAMED OR 7/16" OSB ON BOTH SIDES WITH A NAILING PATTERN OF 3" ON ALL PANEL EDGES AND 6" IN THE FIELD.

ER SET WINDOWS @ 7'-4" UNLESS NOTED.

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ATTENTION PRIOR TO THE START OF CONSTRUCTION, WHILE EVERY EFFORT WAS MADE IN THE PREPARATION OF THESE DRAWINGS AND DIMENSIONS TO AVOID ERRORS THE OWNER AND / OR BUILDER SHALL VERIFY ALL DIMENSIONS
DETAILS, LOCAL AND START CODES.

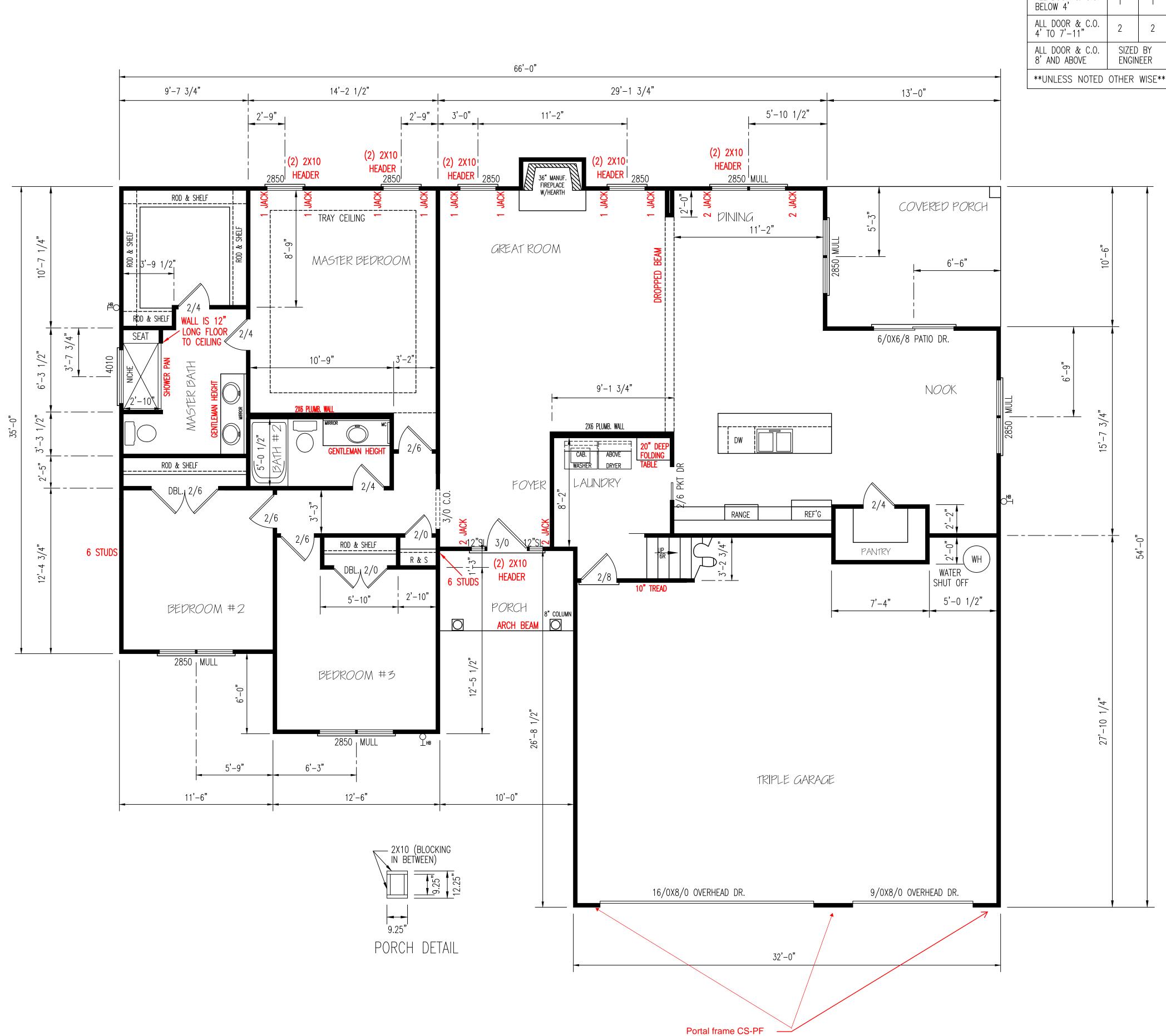
I HEREBY CERTIFY THAT THIS DRAWING MEETS LOCAL CODES, 2018 INTERNATIONAL BUILDING CODES

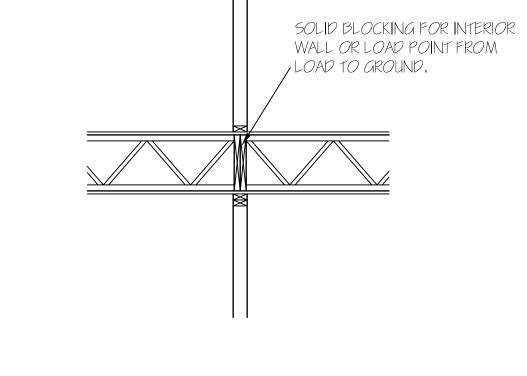
THIS IS FOR THE CONSTRUCTION OF ONE HOUSE ON A SINGLE LOT, NOT TO BE REUSED

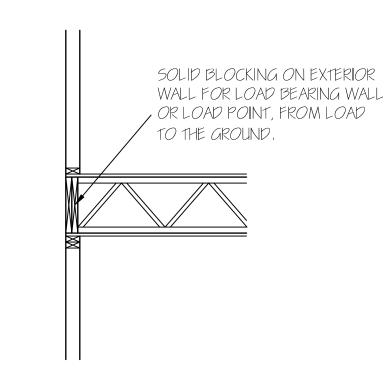
PLAN NUMBER

RG22-A06

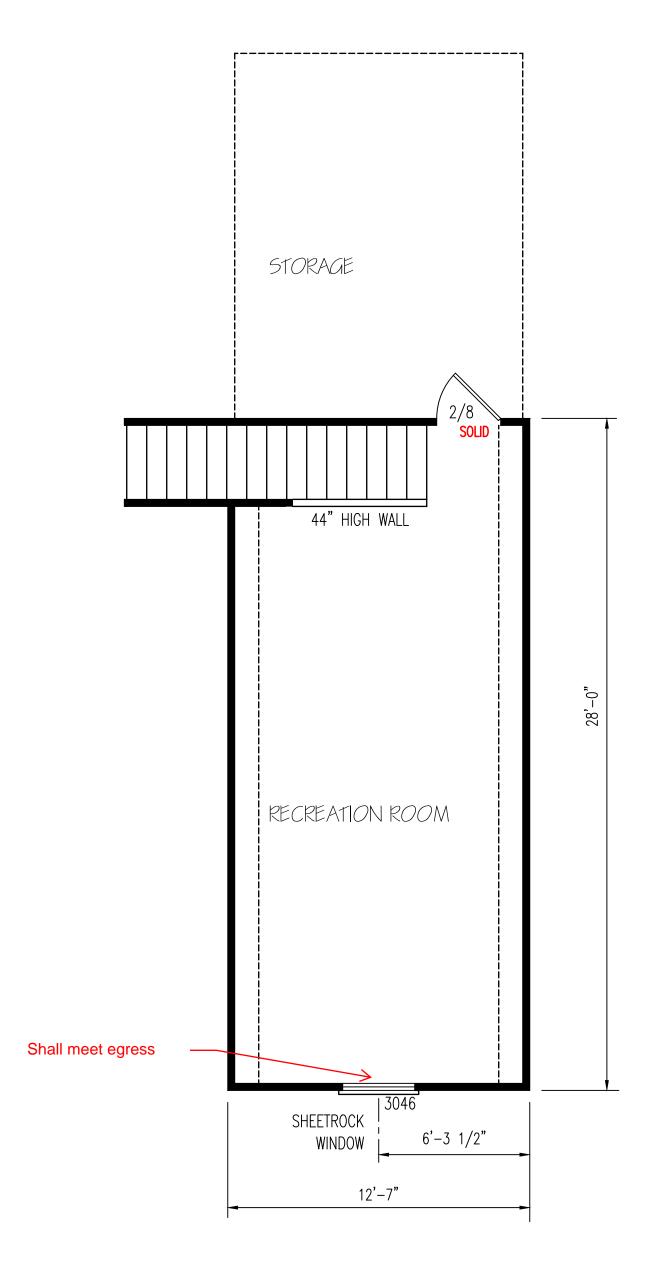
DATE: 6/19/22







EXERIOR V	VALLS			
(2) 2X10 H	EADER	S		
CLEAR SPAN		OF STUDS		
FOR HEADER	JACKS	KINGS		
ALL DOOR & C.O. BELOW 4'	1	1		
ALL DOOR & C.O. 4' TO 7'-11"	2	2		
ALL DOOR & C.O. 8' AND ABOVE	SIZED BY ENGINEER			
UNLESS NOTED	OTHER	WISE		



SECOND FLOOR PLAN

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I HEREBY CERTIFY THAT THIS DRAWING MEETS LOCAL CODES, 2018 INTERNATIONAL BUILDING CODES

1HIS IS FOR THE CONSTRUCTION OF ONE HOUSE ON A SINGLE LOT, NOT TO BE REUSED

PLAN NUMBER

WALL ANCHOR OPTIONS

USE ANCHOR BOLTS

ANCHOR BOLTS: I/2" DIA, BOLTS AT 6'-O" O.C.

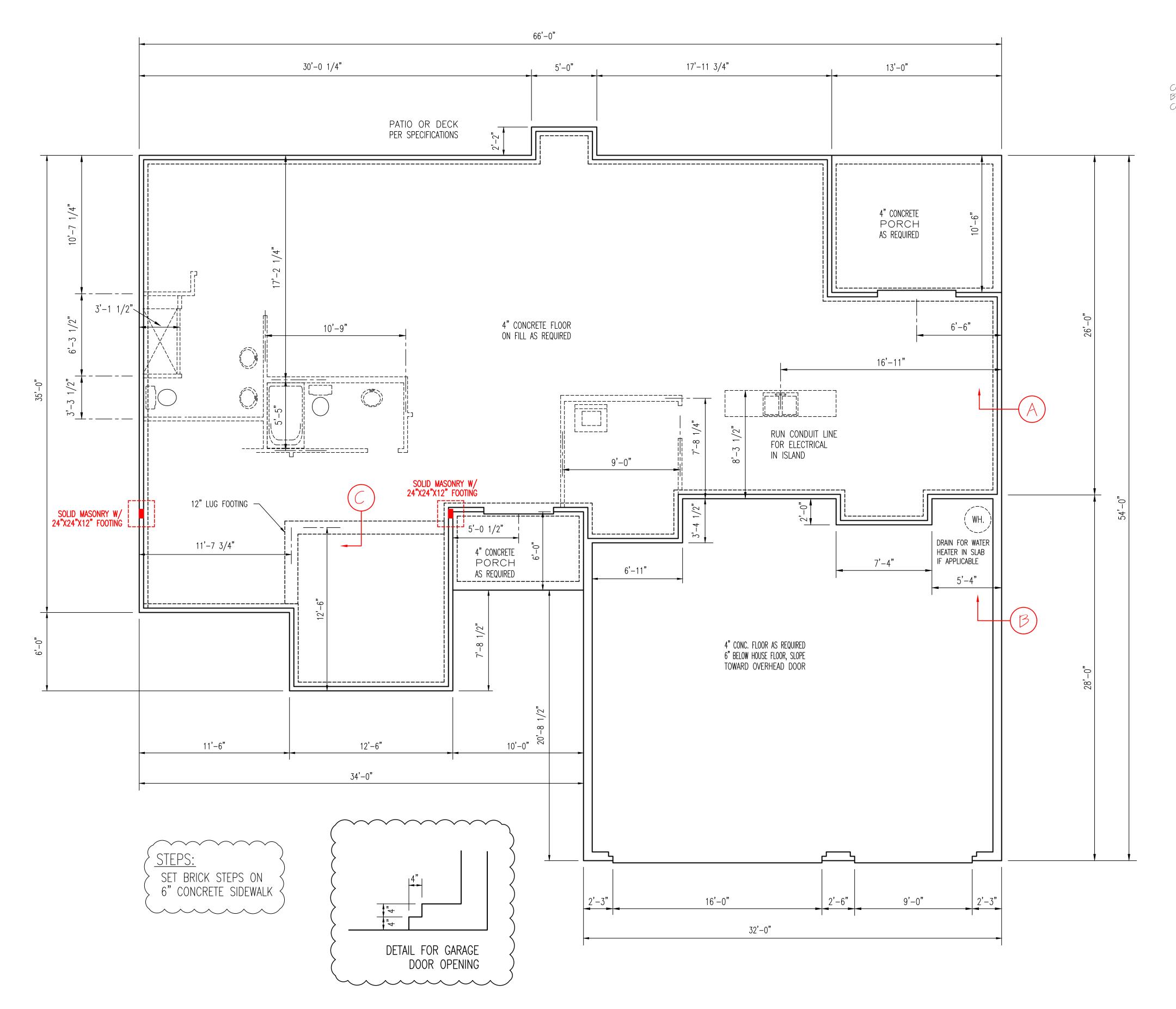
AND NOT MORE THAT I2" FROM CORNERS, EMBEDDED

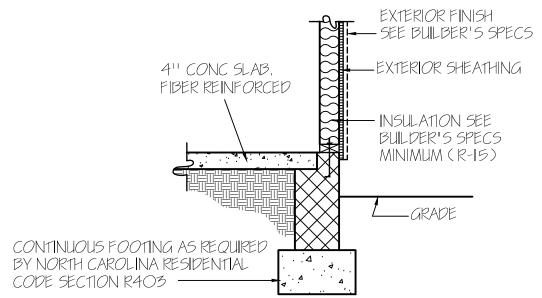
MIN. 7" INTO FOUNDATION, USE A MIN. OF 2 BOLTS

PER EACH STUD WALL

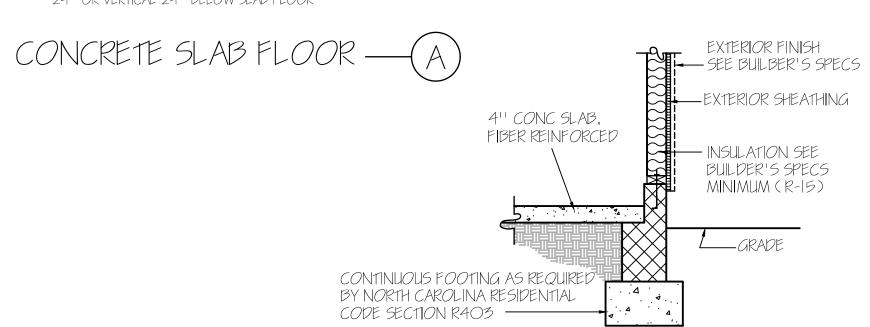
NOTE:

FOUNDATION DETAILS SHOWN ARE BASED ON ASSUMED SOIL BEARING CAPACITY OF 2000 PSF. LOCAL SITE CONDITIONS MUST BE INVESTIGATED. ALL FOOTING TO BE LOCATED BELOW FROST DEPTH.



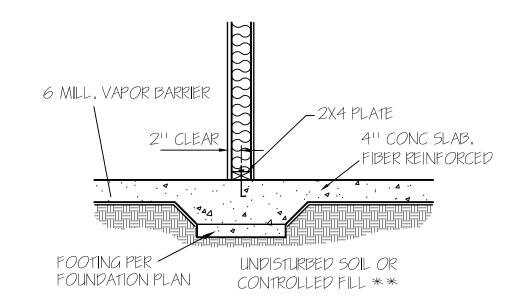


NOTE: PERIMETER INSUL, MAY EXTEND HORIZ, UNDER SLAB 24'' OR VERTICAL 24'' BELOW SLAB FLOOR



NOTE: PERIMETER INSUL, MAY EXTEND HORIZ, UNDER SLAB 24'' OR VERTICAL 24'' BELOW SLAB FLOOR

GARAGE WALL—(B)



LOAD BEARING WALL THICKENED SLAB—(C)

FOUNDATION PLAN
SCALE:1/4"=1'-0"

STEPS:

SET BRICK STEPS ON

4" CONCRETE SIDEWALK

ESIDENTIAL PLANS BY TINA MCFADDEN

TONER REPORT

WATERMAF

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CONSTRUCTION, WHILE EVERY EFFORT
WAS MADE IN THE PREPARATION OF
THESE DRAWINGS AND DIMENSIONS TO
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I HEREBY CERTIFY THAT THIS DRAWING MEETS LOCAL CODES, 2018
INTERNATIONAL BUILDING CODES

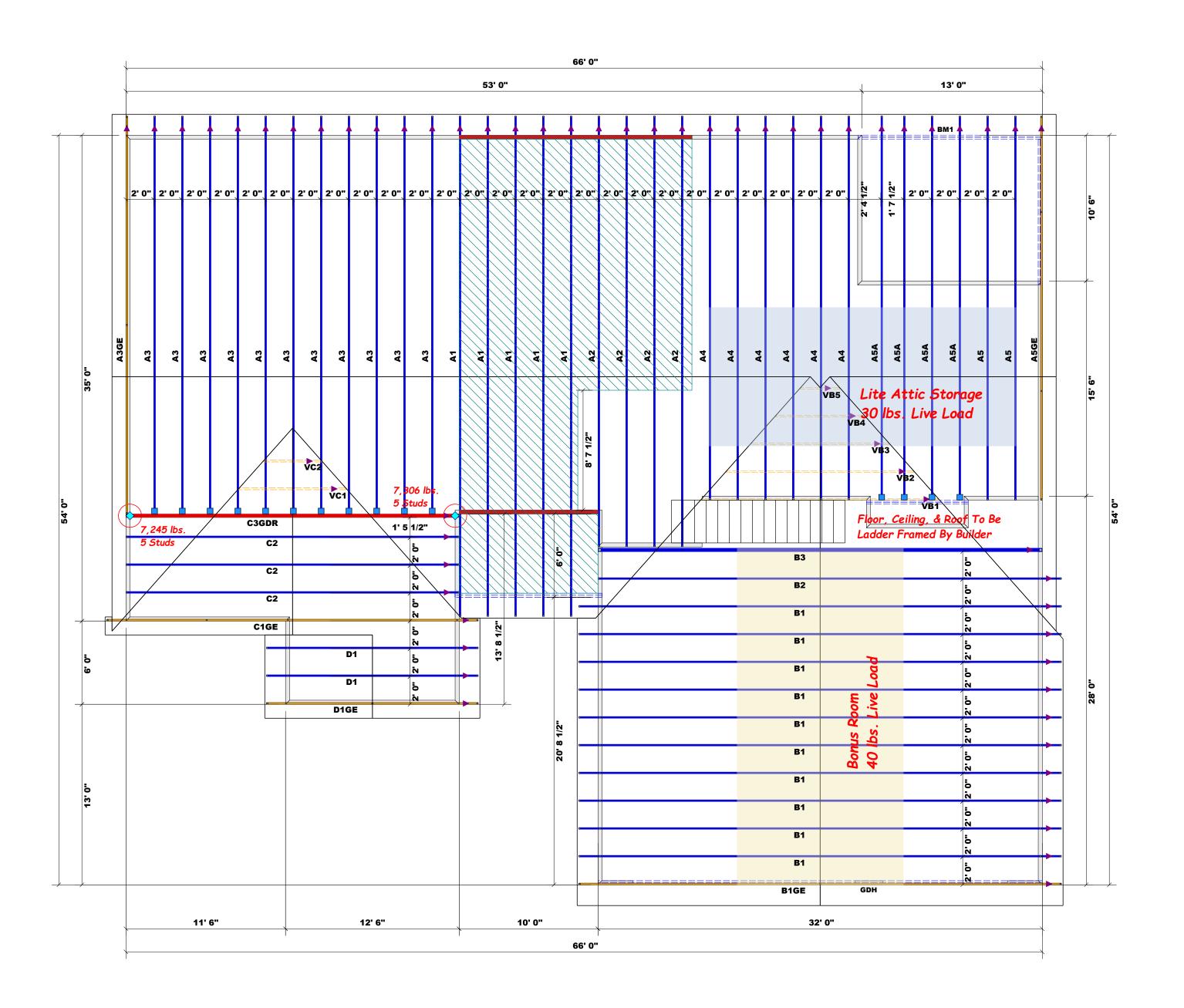
THIS IS FOR THE CONSTRUCTION OF ONE HOUSE ON A SINGLE LOT, NOT TO BE REUSED

PLAN NUMBER RG22-A06

OPTION #1

GARAGE R F

DATE:
6/19/22



Hatch Legend
Ceiling Height @ 10' 1-1/2"

1st Floor Bearing Walls @ 10' 1-1/2"

	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	15	NA	16d/3-1/2"	16d/3-1/2"
\Diamond	HTW20	USP	2	NA	10d/1-1/2"	10d/3"

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan SCALE: 3/16" = 1'

	Beam Legend								
PlotID	Length	Product	Plies	Net Qty	Fab Type				
BM1	14' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF				
GDH	32' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF				

ROOF & FLOOR
TRUSSES & BEAMS

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attachec Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

uture Curtis Quick

Curtis Quick

cui lis Quick

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

ADDRESS Lot 89 South Creek
MODEL Roof

DATE REV. 03/31/23

SALES REP. Anthony Williams

BUILDER Watermark Homes

JOB NAME Lot 89 South Creek

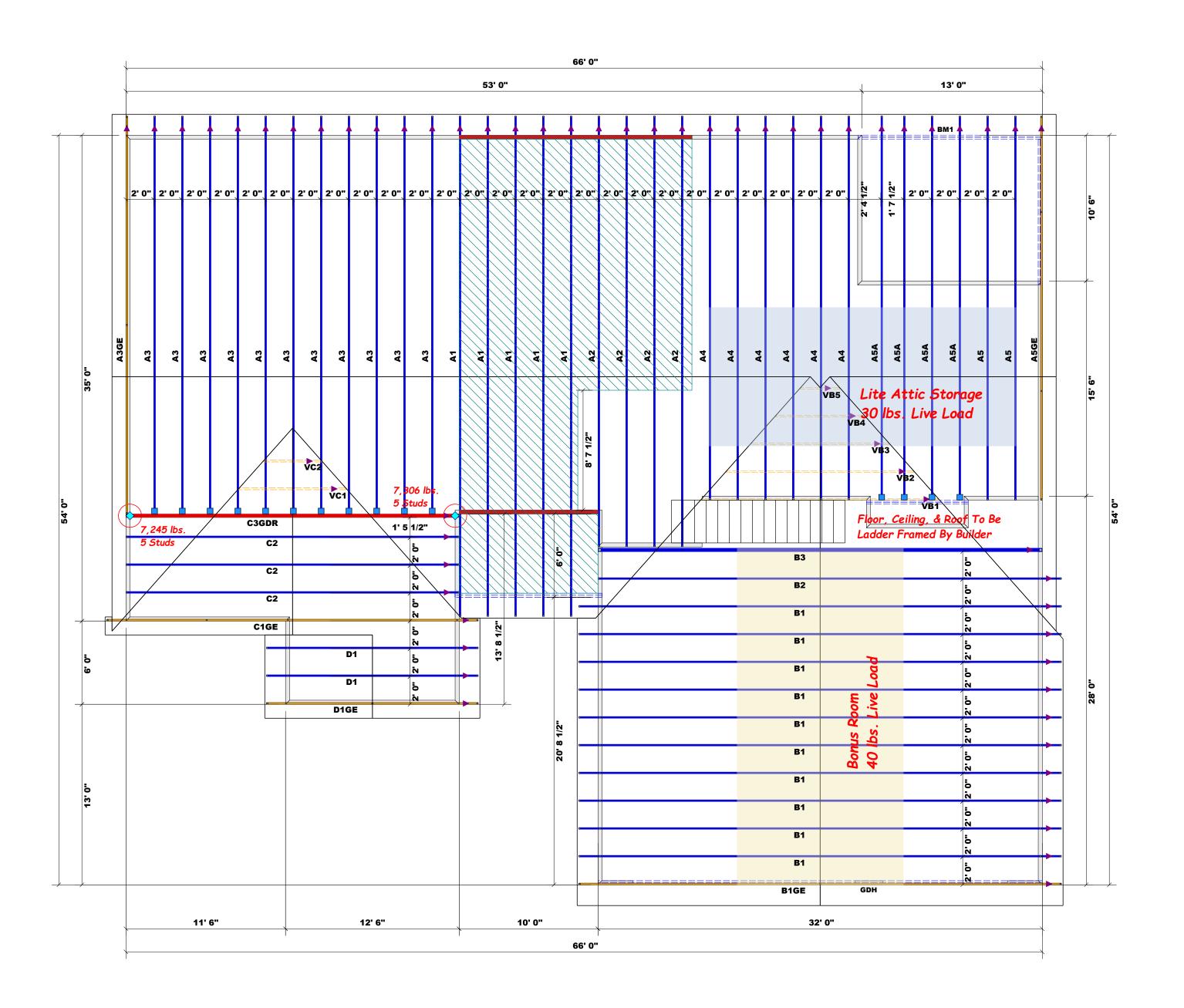
PLAN Pinyon Pine

SEAL DATE Seal Date

QUOTE # Quote #

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

▲ = Denotes Left End of Truss(Reference Engineered Truss Drawing)Do Not Erect Trusses Backwards



Hatch Legend
Ceiling Height @ 10' 1-1/2"

1st Floor Bearing Walls @ 10' 1-1/2"

	Conne	Nail Information				
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	15	NA	16d/3-1/2"	16d/3-1/2"
\Diamond	HTW20	USP	2	NA	10d/1-1/2"	10d/3"

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

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Truss Placement Plan SCALE: 3/16" = 1'

	Beam Legend								
PlotID	Length	Product	Plies	Net Qty	Fab Type				
BM1	14' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF				
GDH	32' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF				

ROOF & FLOOR
TRUSSES & BEAMS

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(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

ADDRESS Lot 89 South Creek
MODEL Roof

DATE REV. 03/31/23

SALES REP. Anthony Williams

BUILDER Watermark Homes

JOB NAME Lot 89 South Creek

PLAN Pinyon Pine

SEAL DATE Seal Date

QUOTE # Quote #

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

▲ = Denotes Left End of Truss(Reference Engineered Truss Drawing)Do Not Erect Trusses Backwards



Client: Watermark Homes

Project: Address: Date: 8/3/2022 Input by: Curtis Quick

Job Name: The Pinyon Pine Beams

Page 1 of 7

11 7/8'

Project #:

Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED GDH (PT 1)

Application:

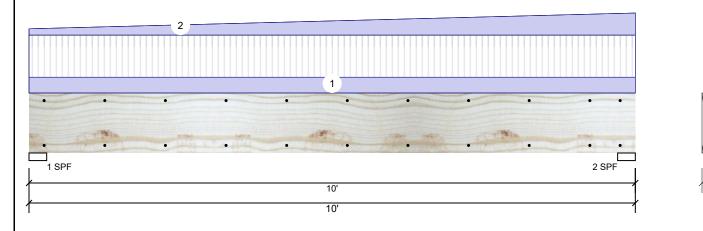
Design Method:

Building Code:

Load Sharing:

Deck:

Level: Level



Floor

ASD

No

IBC 2012

Not Checked

Member	Information
Type:	Girder

Girder Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance:

Normal - II

Temperature: Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	2000	1341	0	0	0
2	Vertical	2000	1602	0	0	0

Bearings

Bearing	Length	Dir.	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	64%	1341 / 2000	3341	L	D+L
2 - SPF	3.500"	Vert	69%	1602 / 2000	3602	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	7903 ft-lb	5'1"	19911 ft-lb	0.397 (40%)	D+L	L
Unbraced	7903 ft-lb	5'1"	9628 ft-lb	0.821 (82%)	D+L	L
Shear	2638 lb	8'8 5/8"	8867 lb	0.297 (30%)	D+L	L
LL Defl inch	0.089 (L/1287)	5'	0.239 (L/480)	0.373 (37%)	L	L
TL Defl inch	0.154 (L/741)	5' 5/16"	0.318 (L/360)	0.486 (49%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

o Eateral Sichaerness ratio based on single pry width.										
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Tie-In	0-0-0 to 10-0-0	10-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	0 PSF	Roof
2	Tapered Start	0-0-0		Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	B1GE
	End	10-0-0			210 PLF	0 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				9 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 2 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info





Client: Watermark Homes

Project: Address:

Date: 8/3/2022 Input by: Curtis Quick

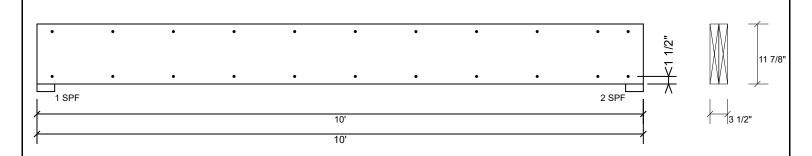
Job Name: The Pinyon Pine Beams

Page 2 of 7

Project #:

2-Ply - PASSED GDH (PT 1) Kerto-S LVL 1.750" X 11.875"

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c., Maximum end distance not to exceed 6".

, ,		,	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	163.7 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info







Client:

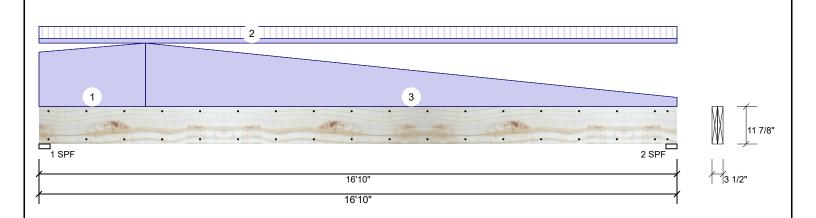
Project: Address: Watermark Homes

Date: 8/3/2022

Input by: Curtis Quick Job Name: The Pinyon Pine Beams Page 3 of 7

Project #:

Kerto-S LVL 1.750" X 11.875" GDH (PT 2) 2-Ply - PASSED Level: Level



Member Infor	rmation			Rea	ctions UNP	ATTERN	NED Ib	(Uplift)			
Type:	Girder	Application:	Floor	Brg	Direction	Live	;	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	Vertical	337	7	1588	0	0	0
Moisture Condition	on: Dry	Building Code:	IBC 2012	2	Vertical	337	7	1051	0	0	0
Deflection LL:	480	Load Sharing:	No								
Deflection TL:	360	Deck:	Not Checked								
Importance:	Normal - II										
Temperature:	Temp <= 100°F										
				Bea	rings						
				Bea	aring Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 -	SPF 3.500"	Vert	37%	1588 / 337	1925	L	D+L
		ļ.		2 -	SPF 3.500"	Vert	27%	1051 / 337	1387	L	D+L

Analysis Results

ĺ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	6794 ft-lb	7'8 7/8"	19911 ft-lb	0.341 (34%)	D+L	L
	Unbraced	6794 ft-lb	7'8 7/8"	6796 ft-lb	1.000 (100%)	D+L	L
l	Shear	1613 lb	1'3 3/8"	8867 lb	0.182 (18%)	D+L	L
l	LL Defl inch	0.070 (L/2809)	8'5 1/16"	0.409 (L/480)	0.171 (17%)	L	L
I	TL Defl inch	0.352 (L/558)	8'2 13/16"	0.546 (L/360)	0.645 (64%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 14'6 5/8" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

D Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
Tapered Start	0-0-0		Тор	180 PLF	0 PLF	0 PLF	0 PLF	0 PLF	B1GE
End	2-9-12			210 PLF	0 PLF	0 PLF	0 PLF	0 PLF	
Tie-In (0-0-0 to 16-10-0	1-0-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	0 PSF	Roof
Tapered Start	2-9-12		Тор	210 PLF	0 PLF	0 PLF	0 PLF	0 PLF	B1GE
	Tapered Start End Tie-In	D Load Type Location Tapered Start 0-0-0 End 2-9-12 Tie-In 0-0-0 to 16-10-0	Load Type Location Trib Width Tapered Start 0-0-0 End 2-9-12 Tie-In 0-0-0 to 16-10-0 1-0-0	Load Type Location Trib Width Side Tapered Start 0-0-0 Top End 2-9-12 Tie-In 0-0-0 to 16-10-0 1-0-0 Top	Dead Type Location Trib Width Side Dead 0.9 Tapered Start 0-0-0 Top 180 PLF End 2-9-12 210 PLF Tie-In 0-0-0 to 16-10-0 1-0-0 Top 15 PSF	Dead Type Location Trib Width Side Dead 0.9 Live 1 Tapered Start 0-0-0 Top 180 PLF 0 PLF End 2-9-12 210 PLF 0 PLF Tie-In 0-0-0 to 16-10-0 1-0-0 Top 15 PSF 40 PSF	Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Tapered Start 0-0-0 Top 180 PLF 0 PLF 0 PLF 0 PLF End 2-9-12 210 PLF 0 PLF 0 PLF 0 PLF Tie-In 0-0-0 to 16-10-0 1-0-0 Top 15 PSF 40 PSF 0 PSF	Decided Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Tapered Start 0-0-0 Top 180 PLF 0 PLF 0 PLF 0 PLF 0 PLF End 2-9-12 210 PLF 0 PLF 0 PLF 0 PLF Tie-In 0-0-0 to 16-10-0 1-0-0 Top 15 PSF 40 PSF 0 PSF 0 PSF	Dead Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Tapered Start 0-0-0 Top 180 PLF 0 PLF

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 2 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info





Client:

Project: Address: Watermark Homes

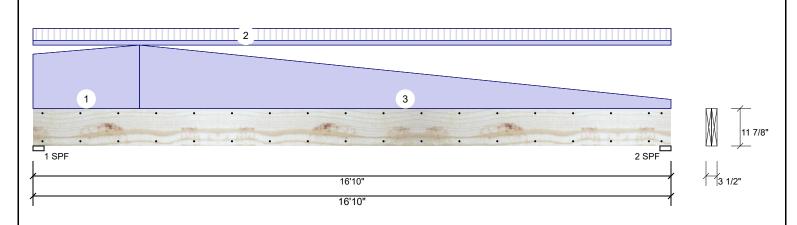
Date: 8/3/2022

Input by: Curtis Quick Job Name: The Pinyon Pine Beams Page 4 of 7

Project #:

2-Ply - PASSED Kerto-S LVL 1.750" X 11.875" GDH (PT 2)

Level: Level



.Continued from page 1

Load Type ID Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments End 16-10-0 30 PLF 0 PLF 0 PLF 0 PLF 0 PLF

> 9 PLF Self Weight

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

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- 6. For flat roofs provide proper drainage to prevent ponding

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Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



CSD DESIGN

Client:

Project: Address: Watermark Homes

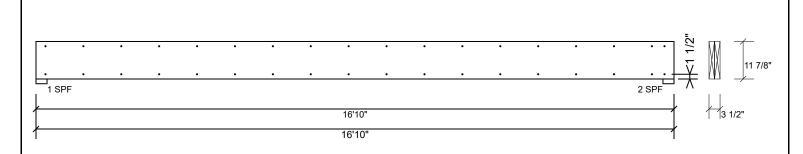
Date: 8/3/2022 Input by: Curtis Quick

Job Name: The Pinyon Pine Beams

Page 5 of 7

Project #:

Kerto-S LVL 1.750" X 11.875" GDH (PT 2) 2-Ply - PASSED Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

1 3		`	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	163.7 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes
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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

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This design is valid until 11/3/2024



Client: Watermark Homes

Project: Address: Date: 8/3/2022 Input by: Curtis Quick

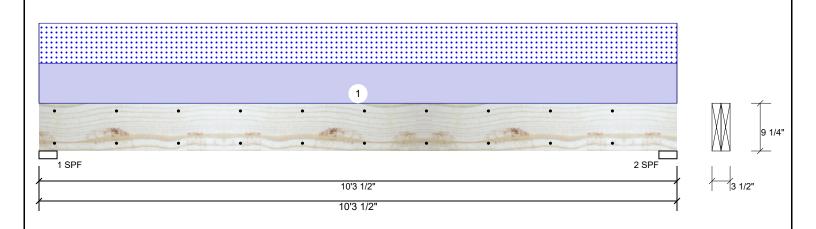
Job Name: The Pinyon Pine Beams

Page 6 of 7

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM1

Level: Level



Member Info	Rea	Reactions UNPATTERNED lb (Uplift)									
Type:	Girder	Application:	Floor	Brg	Direction	Live	· [Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	Vertical	C)	1534	1497	0	0
Moisture Condition	n: Dry	Building Code:	IBC 2012	2	Vertical	C)	1534	1497	0	0
Deflection LL:	480	Load Sharing:	No								
Deflection TL:	360	Deck:	Not Checked								
Importance:	Normal - II										
Temperature:	Temp <= 100°F										
				Bea	rings						
				Bea	aring Length	Dir.	Cap. R	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 -	SPF 3.500"	Vert	58%	1534 / 1497	3032	L	D+S
					SDE 3.500"	Vort	58%	1534 / 1407	3032	1	D+S

Analysis Results

ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
ı	Moment	7121 ft-lb	5'1 3/4"	14423 ft-lb	0.494 (49%)	D+S	L
ı	Unbraced	7121 ft-lb	5'1 3/4"	7519 ft-lb	0.947 (95%)	D+S	L
I	Shear	2411 lb	1' 3/4"	7943 lb	0.304 (30%)	D+S	L
I	LL Defl inch	0.145 (L/813)	5'1 3/4"	0.246 (L/480)	0.590 (59%)	S	L
ı	TL Defl inch	0.294 (L/402)	5'1 3/4"	0.328 (L/360)	0.896 (90%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	291 PLF	0 PLF	291 PLF	0 PLF	0 PLF	A4
	Self Weight				7 PLF					

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- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
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This design is valid until 11/3/2024 CSD DESIGN

Client: Watermark Homes

Project: Address: Date:

8/3/2022 Input by: Curtis Quick

Job Name: The Pinyon Pine Beams

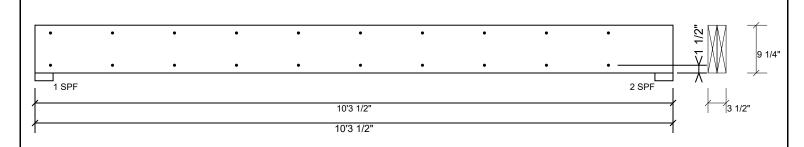
Page 7 of 7

Project #:

Kerto-S LVL BM1

1.750" X 9.250" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

		. ,
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Yield Limit per Foot	163.7 PLF	
Yield Limit per Fastener	81.9 lb.	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

Notes

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Handling & Installation

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