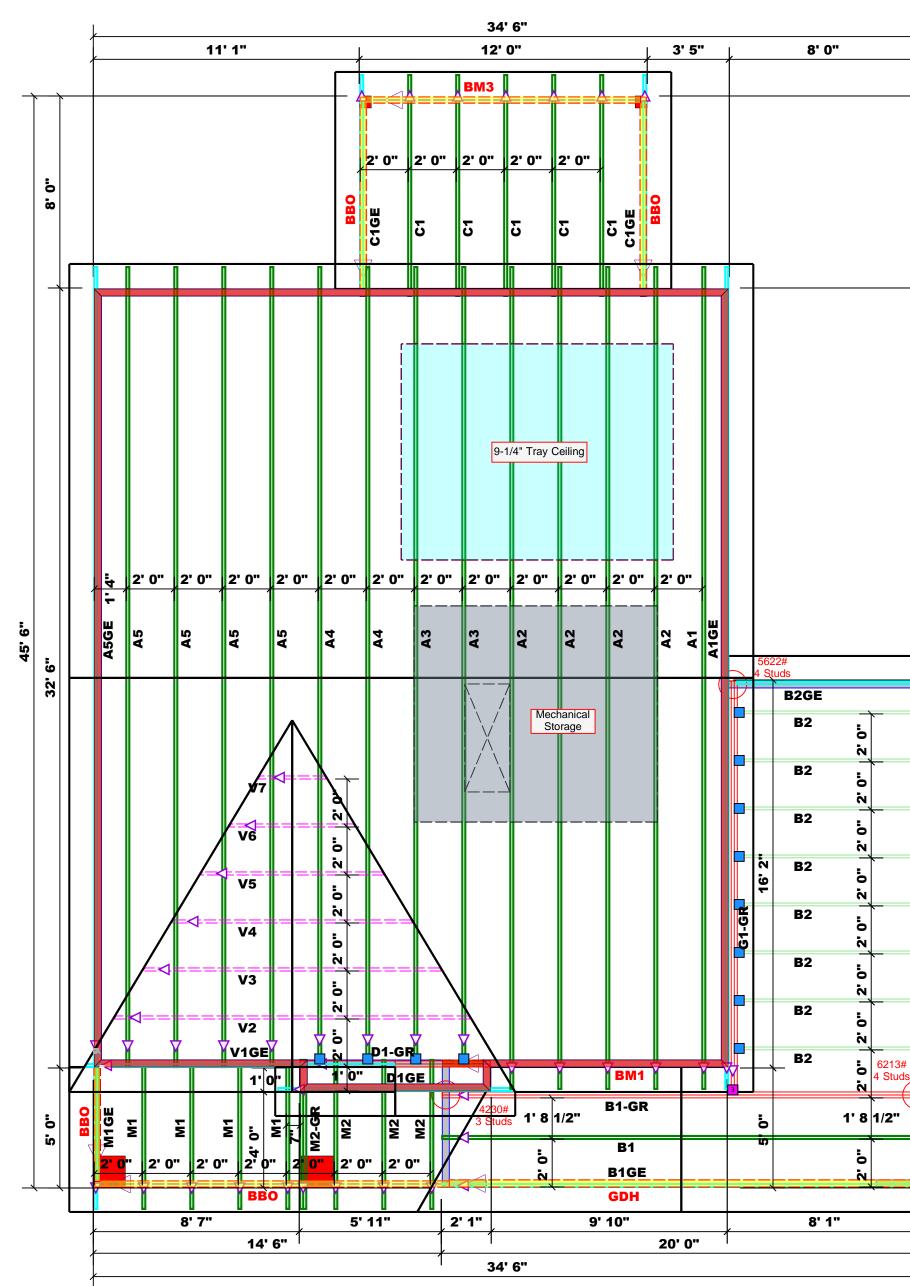


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		o Mige		A5GE 1' 4"			
	x x	BBO				*	
		2.0.	32	. 6.	8.0"	1	
						ſ	
		Sym       Product       Manuf       Qty       Supported Member       Header       Truss         HUS26       USP       12       NA       16d/3-1/2"       16d/3-1/2"         THDH210-3       USP       1       Varies       16d/3-1/2"       16d/3-1/2"         THDH210-3       USP       1       Varies       16d/3-1/2"       16d/3-1/2"         PlotID       Length       Product       Plies       Net Qty         BM1       12'0"       1-3/4"x 16"       LVL Kerto-S       2       2         BM2       15'0"       1-3/4"x 16"       LVL Kerto-S       2       2         BM3       12'0"       2x10 SPF No.2       2       2         GDH       20'0"       1-3/4"x 14"       LVL Kerto-S       2       2	All Walls Shown Are Considered Load Bearing         Roof Area = 1692.08 sq.ft. Ridge Line = 52.07 ft. Hip Line = 0 ft. Horiz. OH = 115.69 ft. Raked OH = 175.3 ft. Decking = 58 sheets         Image: Connector Information	Dimension Notes         1. All exterior wall to wall dimensions are to face of shearhing unless noted otherwise         2. All interior wall dimensions are to face of face of shearhing unless noted otherwise         3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise			
THIS	BUILDER	Weaver Development Co. Inc.	<b>CITY / CO</b> . Sanford / Har	5100 6800 10200 11900 13600 15300	Tables retaine reactio Signat	deeme require attache require size ar reactio 15000# retaine	T
IS A TRUS	JOB NAME	Lot 3 Holly Place	ADDRESS 172 Colby Farm	3 4 5 0 6 7 8 9 9	(BASE (BASE WABER OF J NO2 SCILLS (2) MBER OF J NO2 SCILLS (2) MBER OF J (2) MBER OF J	g reaction d to comp ments. Th ed Tables ments) to d number ns greate A registo d to desig	Reilly F Reilly F Fayet Phor
S PLACE	PLAN	Hickory II "A" / 2GRF, CP	MODEL Roof	7650 1020 1275 1530	Davi Davi ART FC	ly with the contract derived determine of wood than 300 ered design the sup	OF &
	SEAL DATE	N/A	<b>DATE REV</b> . 03/17/23	) 3 0 4 0 5	n profess port syst i0#.	e prescrip tor shall from the be the min studs req 0# but no in profess port syst	<b>&amp; FL</b> <b>&amp; B</b> ndustr e, N.C. 0) 864
GRAM O	QUOTE #		DRAWN BY David Landry	102 136	CK STI 1) & (b)) 0 @ EA EN 34	otive Cod refer to the prescription nimum fo puired to a of greater sional sha em for ar	<b>OO</b> <b>EA</b> 28309 -8787
NLY.	JOB #	J0223-0917	SALES REP. Lenny Norris		er (1P TO) STUDS FOR S C C C C C C C C C C C C C C C C C C	e ve Code undation support than all be	R MS rk

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



	Bearing deemed requiren attachee requiren size and reaction Tables. retained reaction Signatu	ROC RUS ceilly R Fayet Phon Fax reactions to complete to complete	OF & SES Soad Ir teville e: (910) : (9	k FL( & FL( & B dustr , N.C. 2 )) 864- 864-4 section boot specific profession or equal prescrip or shall r prescrip or shall r profession profession or equal prescrip profession or equal prescrip profession or equal profession or equal profession	OOF EAN ial Par 28309 -8787 444 tive Code efer to the rescriptivi imum foou juired to si greater t ional shall imum for any d in the a onal shall im for all More onal shall im for all CK STU ) & (b)) @ EA END	AS AS k tare e e e c c c c c c c c c c c c c
Image: State of the state	Sanford / Harnett / Harnett	Colby Farm Drive	KO (QL dr) 2550 5100 7650 12750 15300	2 3 4 5	680 1020 1360	Lenny Norris
Ridge Line = 52.07 ft.       2nd Floor Walls         Hip Line = 0 ft.       2nd Floor Walls         Horiz. OH = 115.69 ft.       Tray Ceiling         Decking = 58 sheets       Drop Beam         Nail Information       Nail Information         Sym       Product       Manuf       Qty       Supported Member       Header       Truss         HUS26       USP       12       NA       16d/3-1/2"       16d/3-1/2"	<b>CITY / CO.</b> S	ADDRESS 172	WODEL	DATE REV. 00	DRAWN BY	SALES REP. Le
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Weaver Development Co. Inc.	Lot 3 Holly Place	Hickory II "A" / 2GRF, CP	N/A		J0223-0917
	BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #



THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Trenco 818 Soundside Rd Edenton, NC 27932

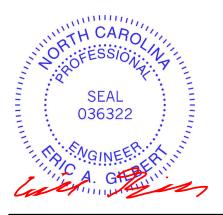
Re: J0223-0917 Lot 3 Holly Place

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I57239345 thru I57239372

My license renewal date for the state of North Carolina is December 31, 2023.

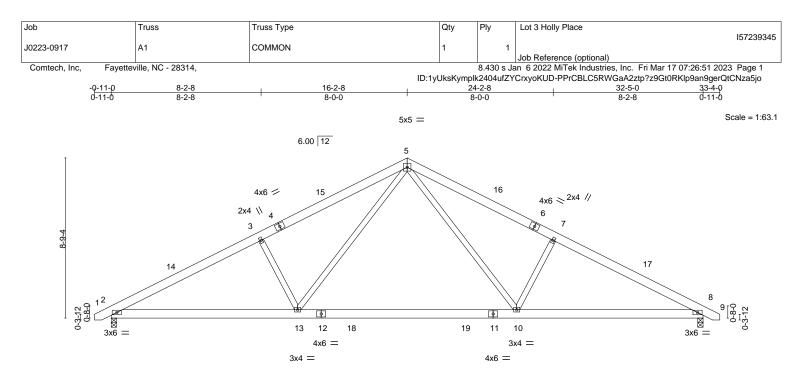
North Carolina COA: C-0844



March 17,2023

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



	10-2-8	22-2-8	<u>32-5-0</u>
	10-2-8	12-0-0	10-2-8
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.64 Vert(CT) - WB 0.27 Horz(CT)	in (loc) I/defi L/d PLATES GRIP 0.34 10-13 >999 360 MT20 244/190 0.47 10-13 >824 240 0.05 8 n/a n/a 0.05 2-13 >999 240 Weight: 208 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-110(LC 10) Max Uplift 2=-89(LC 12), 8=-89(

Max Uplift 2=-89(LC 12), 8=-89(LC 13) Max Grav 2=1337(LC 1), 8=1337(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS 5-10=-147/921, 7-10=-454/288, 5-13=-147/921, 3-13=-454/288

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2 and 89 lb uplift at joint 8.

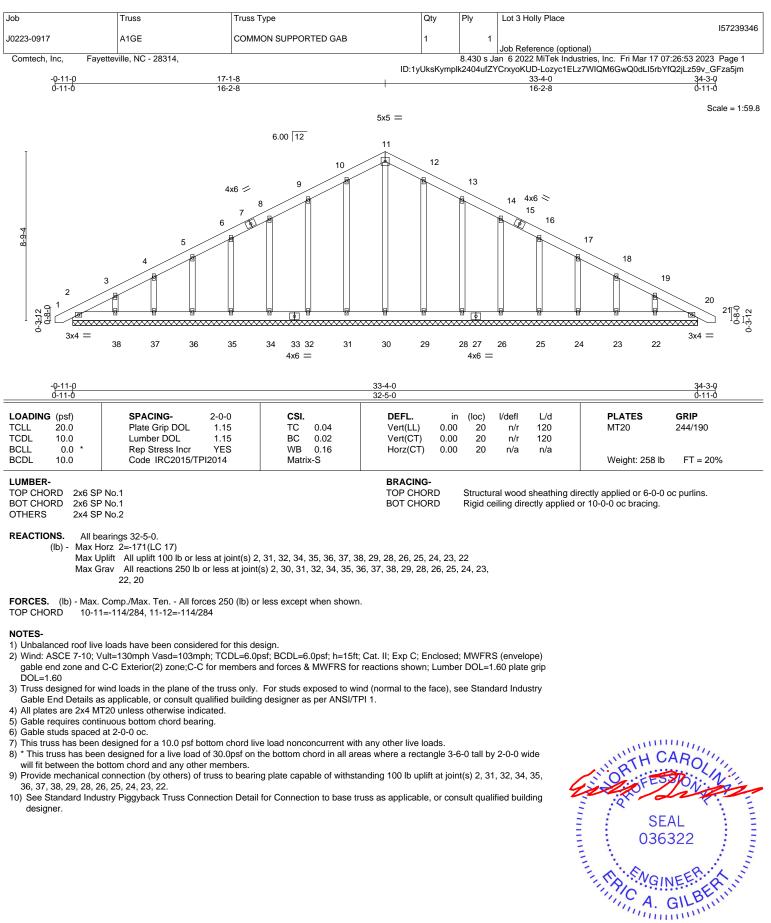
 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 4-11-7 oc purlins.

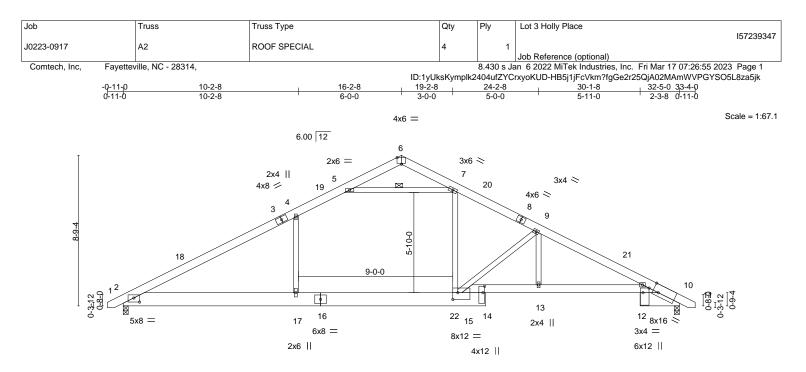
Rigid ceiling directly applied or 10-0-0 oc bracing.





March 17,2023





		16-2-8 6-0-0	<u>19-2-8</u> 21-1- <u>3-0-0</u> 1-11-	0 3-1-0	30-1-8 5-11-0	32-5-0	
Plate Offsets (X,Y)	[2:0-4-0,0-2-14], [6:0-3-0,Edge], [10:0-4-0	),Edgej, [12:0-3-4,Edgej, [1	4:0-4-8,0-1-4], [15:0-3	3-8,0-4-12]			
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.75 BC 0.67 WB 0.70 Matrix-S	DEFL.         in           Vert(LL)         -0.21           Vert(CT)         -0.38           Horz(CT)         0.09           Wind(LL)         0.17	10 n/a	L/d 360 240 n/a 240	PLATES MT20 Weight: 247 lb	<b>GRIP</b> 244/190 FT = 20%
BOT CHORD 2x10 \$ 10-15	P No.1 SP No.1 *Except* : 2x6 SP 2400F 2.0E P No.2	I	BRACING- TOP CHORD BOT CHORD WEBS		irectly applied or	ctly applied or 4-0-8 c 10-0-0 oc bracing.	oc purlins.

REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=110(LC 11)

Max Uplift 2=-90(LC 12), 10=-90(LC 13) Max Grav 2=1393(LC 2), 10=1353(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-2217/403, 4-5=-1870/483, 7-9=-2258/519, 9-10=-2889/551

BOT CHORD 2-17=-193/1848, 15-17=-195/1860, 13-15=-371/2525, 10-13=-380/2525

WEBS 4-17=-29/402, 7-15=-114/967, 9-15=-1075/232, 9-13=0/616, 5-7=-1955/459

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

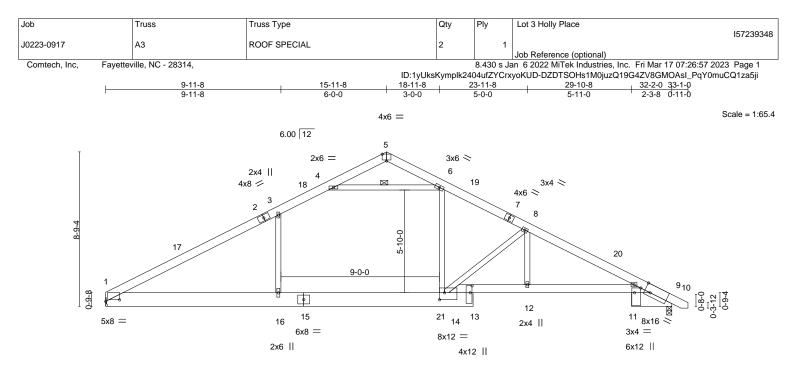
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







I.	9-11-8	18-11-8	20-10-8	23-11-8	29-10-8	32-2-0				
Г	9-11-8	9-0-0	1-11-0	3-1-0	5-11-0	2-3-8				
Plate Offsets (X,Y)	Plate Offsets (X,Y) [1:0-9-6,0-1-2], [5:0-3-0,Edge], [9:0-4-0,Edge], [11:0-3-4,Edge], [13:0-4-12,0-1-4], [14:0-3-8,0-4-12]									

TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/T	1.15 1.15 YES PI2014	TC BC WB Matrix	0.76 0.66 0.70 x-S	Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.21 -0.36 0.09 0.16	16 16 9 1-16	>999 >999 n/a >999	360 240 n/a 240	MT20 Weight: 243 lb	244/190 FT = 20%
LUMBER- TOP CHORD 2x6 S	P No.1				BRACING- TOP CHOF		Structu	ral wood	sheathing d	lirectly applied or 3-10-0	oc purlins.

BOT CHORD

WEBS

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

4-6

 BOT CHORD
 2x6 SP No.1

 BOT CHORD
 2x10 SP No.1 \*Except\*

 9-14:
 2x6 SP 2400F 2.0E

 WEBS
 2x4 SP No.2

REACTIONS. (size) 1=Mechanical, 9=0-3-8 Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 9=-90(LC 13)

Max Grav 1=1345(LC 2), 9=1347(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-3=-2189/401, 3-4=-1853/491, 6-8=-2237/517, 8-9=-2874/549

BOT CHORD 1-16=-198/1827, 14-16=-200/1839, 12-14=-375/2511, 9-12=-384/2511

WEBS 6-14=-117/966, 3-16=-53/392, 4-6=-1931/474, 8-14=-1081/228, 8-12=0/620

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

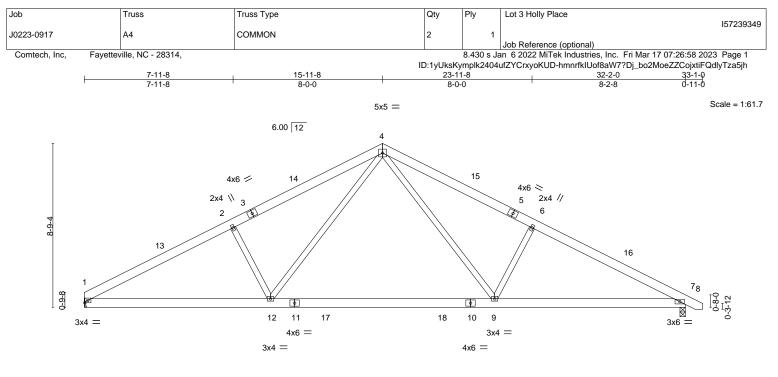
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.







	9-11-8		21-11-8		2-2-0	-
Plate Offsets (X,Y)	<u>9-11-8</u> [1:0-1-14,0-1-8]		12-0-0	10	0-2-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.28 BC 0.64 WB 0.27 Matrix-S	DEFL.         in         (loc)           Vert(LL)         -0.34         9-12           Vert(CT)         -0.47         9-12           Horz(CT)         0.05         7           Wind(LL)         0.05         12	l/defl L/d >999 360 >822 240 n/a n/a >999 240	-	<b>GRIP</b> 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 1=Mechanical, 7=0-3-8 Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 7=-89(LC 13)

Max Grav 1=1278(LC 1), 7=1331(LC 1)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 1-2=-2276/496, 2-4=-2096/546, 4-6=-2113/532, 6-7=-2294/484

BOT CHORD 1-12=-319/1973, 9-12=-109/1291, 7-9=-324/1953

WEBS 4-9=-147/922, 6-9=-454/288, 4-12=-144/897, 2-12=-437/286

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

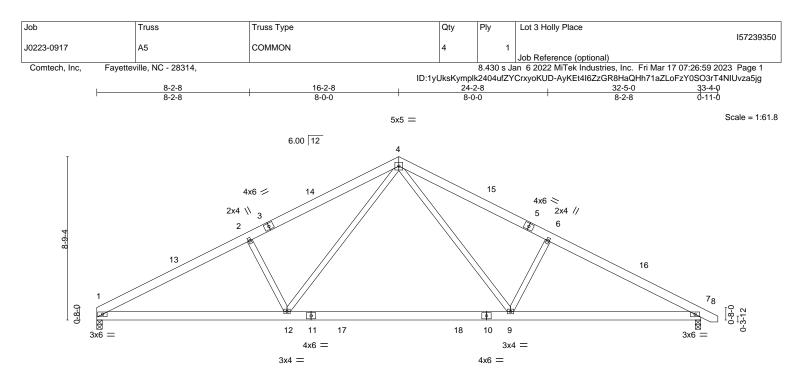
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-11-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





	10-2-8 10-2-8	<u>22-2-8</u> 12-0-0			2-5-0 0-2-8	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.65 Vert(CT) -( WB 0.27 Horz(CT) (	in (loc) 0.34 9-12 0.47 9-12 0.05 7 0.05 12	l/defl L/d >999 360 >822 240 n/a n/a >999 240		<b>GRIP</b> 244/190 FT = 20%

BRACING-TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

BOT CHORD 1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS 4-9=-147/921, 6-9=-454/288, 4-12=-149/924, 2-12=-458/292

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

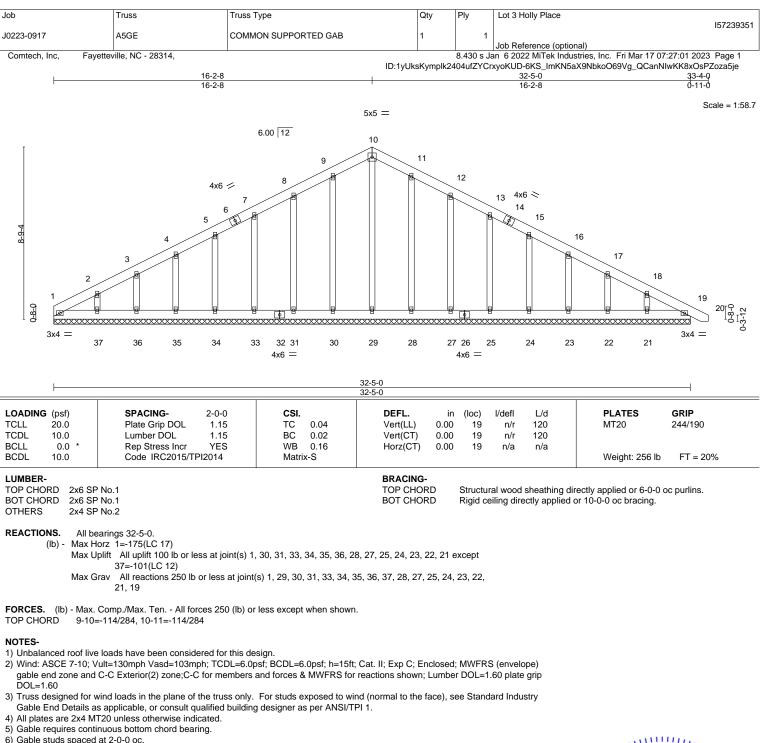
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-10-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





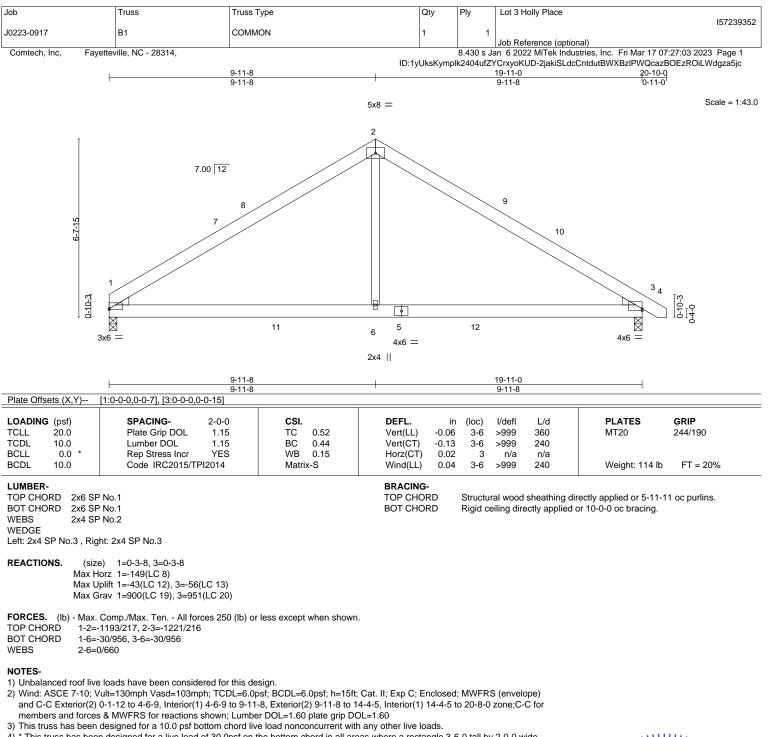
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.



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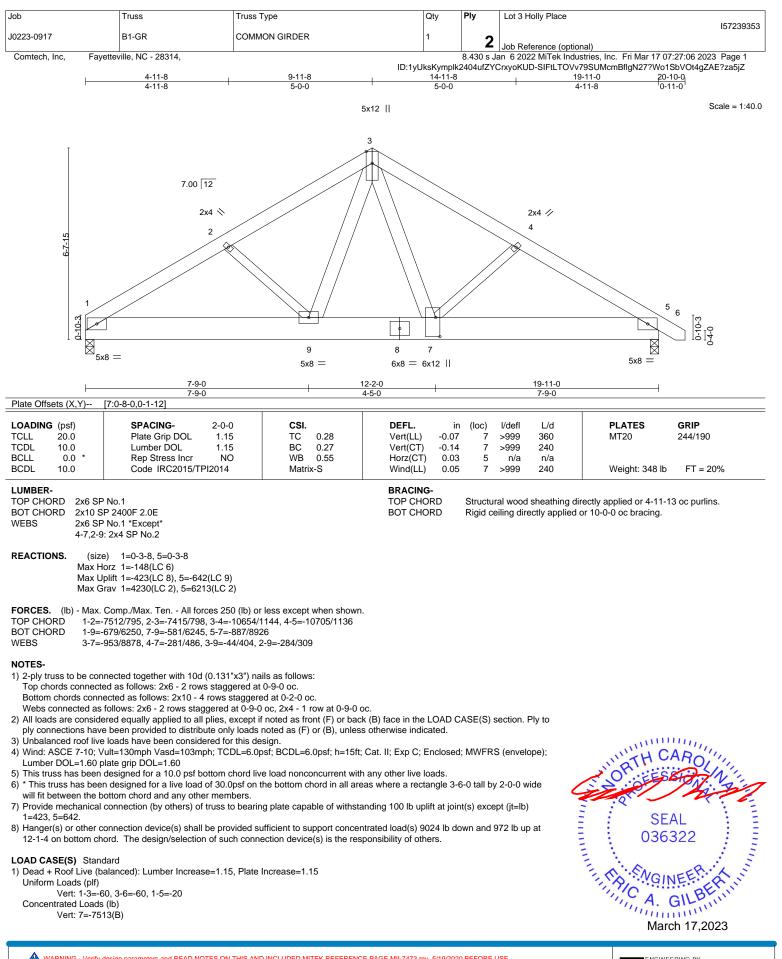


4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

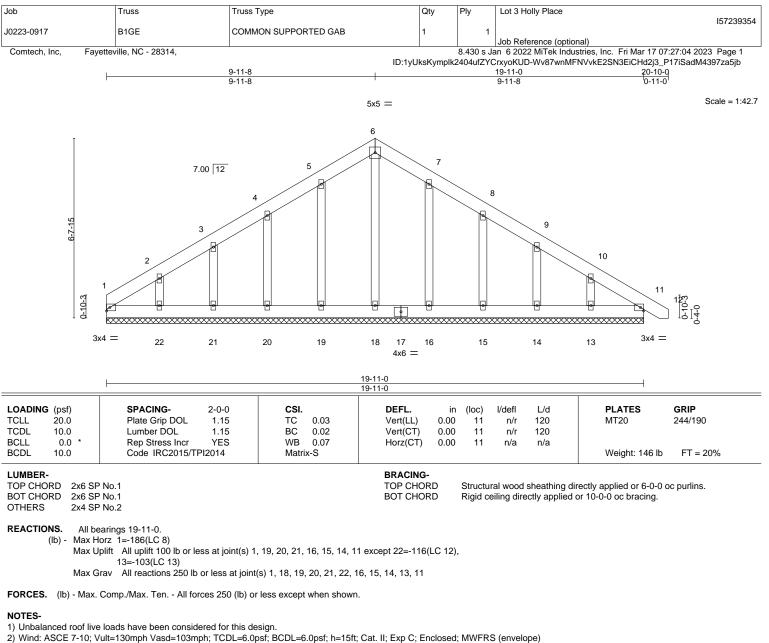






WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



<sup>2)</sup> Wind: ASCE 7-10; Vuit=130mph Vasd=103mph; TCDL=6.0pst; BCDL=6.0pst; n=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

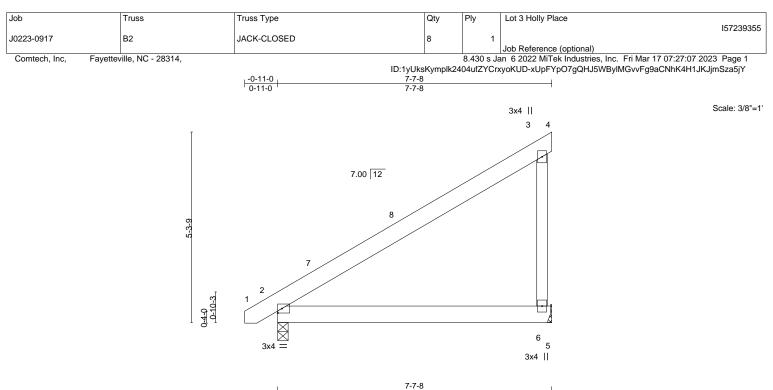
5) Gable requires continuous bottom chord bearing.6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 19, 20, 21, 16, 15, 14, 11 except (jt=lb) 22=116, 13=103.





				7-7-8				
LOADIN	· · · ·	SPACING- 2-0-0	CSI.		n (loc)	l/defl L/d	PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.33	Vert(LL) -0.04	1 2-6	>999 360	MT20 244/1	90
TCDL	10.0	Lumber DOL 1.15	BC 0.21	Vert(CT) -0.0	7 2-6	>999 240		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.0	)	n/a n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.0	) 2	**** 240	Weight: 48 lb FT	Γ = 20%

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

2x4 SP No.2

REACTIONS. 6=Mechanical, 2=0-3-8 (size) Max Horz 2=158(LC 12) Max Uplift 6=-82(LC 12)

Max Grav 6=318(LC 19), 2=345(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 3-6=-288/220

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-0 to 3-7-13, Interior(1) 3-7-13 to 7-7-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6.

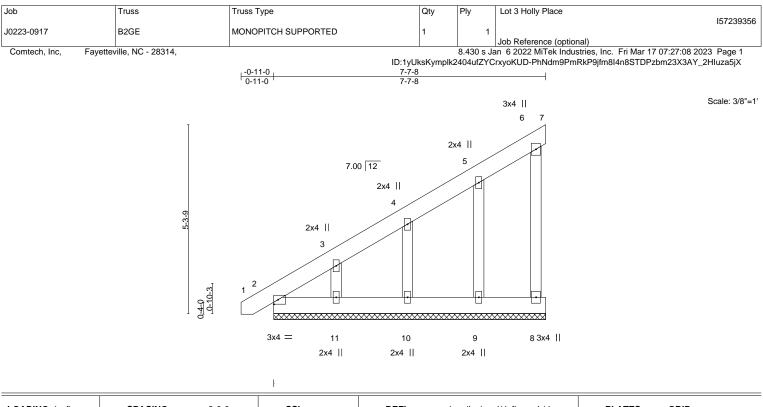


Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in	(loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.03	Vert(LL) -0.00	<u></u> 1	n/r 120	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.01	Vert(CT) -0.00	1	n/r 120	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.03	Horz(CT) -0.00	7	n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	· · /			Weight: 57 lb FT = 20%

## LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2
OTHERS	2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 7-7-8.

Max Horz 2=228(LC 12) (lb) -

2-3=-269/227

Max Uplift All uplift 100 lb or less at joint(s) 7, 8, 9, 10 except 11=-117(LC 12) Max Grav All reactions 250 lb or less at joint(s) 7, 8, 2, 9, 10, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

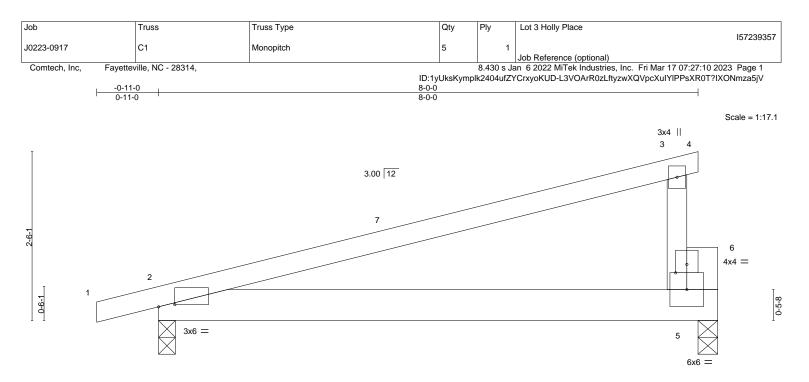
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 8, 9, 10 except (jt=lb) 11=117.







			8-3-8						
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [6:0-2-0,0-1-8]		8-3-8						
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.84	Vert(LL) -0	).05	2-5	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.24	Vert(CT) -0	).10	2-5	>969	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) (	0.00	5	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) (	0.10	2-5	>886	240	Weight: 37 lb	FT = 20%
LUMBER-			BRACING-						
TOP CHORD 2x4 SP BOT CHORD 2x6 SP			TOP CHORD			al wood and verti	0	rectly applied or 5-3-4	oc purlins,
WEBS 2x4 SP			BOT CHORD					or 10-0-0 oc bracing.	

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8) Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

2x6 SP No.1

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

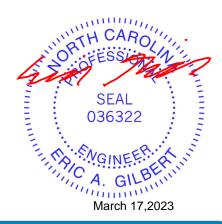
OTHERS

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

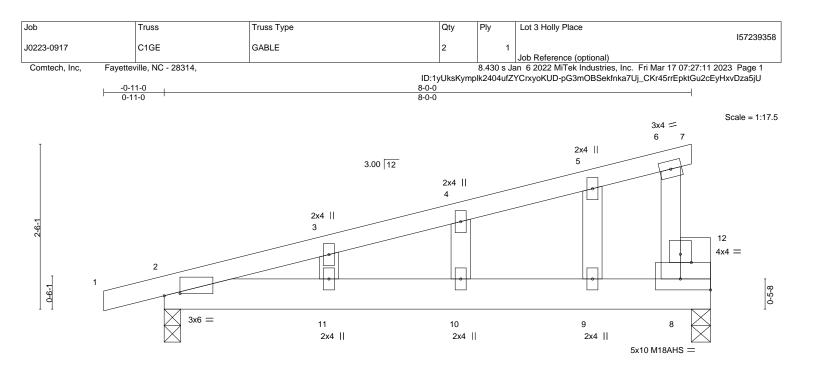
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.







			8-3-8		
			8-3-8		I
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [8:Edge,0-2-0], [12:0-2	2-0,0-1-8]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.26 WB 0.01 Matrix-S	Vert(LL) 0.0	in (loc) I/defi L/d 9 10-11 >999 240 8 10-11 >999 240 0 8 n/a n/a	PLATES         GRIP           MT20         244/190           M18AHS         186/179           Weight: 41 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP WEBS 2x4 SP	No.1	1	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing.

OTHERS 2x4 SP No.2 \*Except\* 8-12: 2x6 SP No.1

REACTIONS. (size) 2=0-3-0, 8=0-3-8 Max Horz 2=105(LC 8)

Max Uplift 2=-216(LC 8), 8=-188(LC 8) Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD 2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) All plates are MT20 plates unless otherwise indicated.

4) Gable studs spaced at 2-0-0 oc.

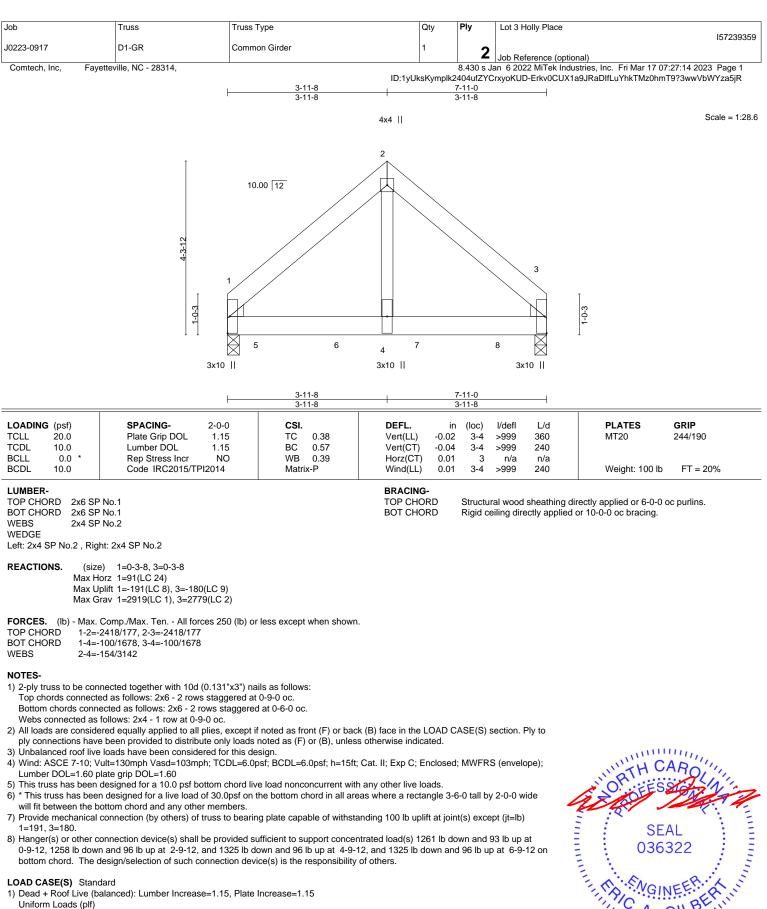
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216, 8=188.







8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1261 lb down and 93 lb up at 0-9-12, 1258 lb down and 96 lb up at 2-9-12, and 1325 lb down and 96 lb up at 4-9-12, and 1325 lb down and 96 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 1-3=-20

#### Continued on page 2

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Edenton, NC 27932

G mmm March 17,2023

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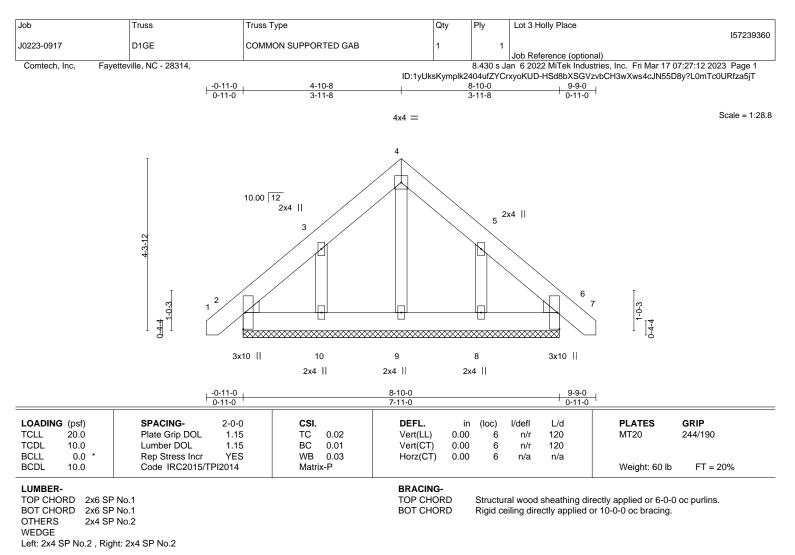
[	Job	Truss	Truss Type	Qty	Ply	Lot 3 Holly Place
						157239359
	J0223-0917	D1-GR	Common Girder	1	2	
					2	Job Reference (optional)
	Comtech, Inc, Fayettev	ille, NC - 28314,			8.430 s Ja	an 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:14 2023 Page 2

ID:1yUksKymplk2404ufZYCrxyoKUD-Erkv0CUX1a9JRaDlfLuYhkTMz0hmT9?3wwVbWYza5jR

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)





# REACTIONS. All bearings 7-11-0.

(lb) - Max Horz 2=-118(LC 10)

Max Uplit All uplift 100 lb or less at joint(s) 2, 6 except 10=-152(LC 12), 8=-148(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

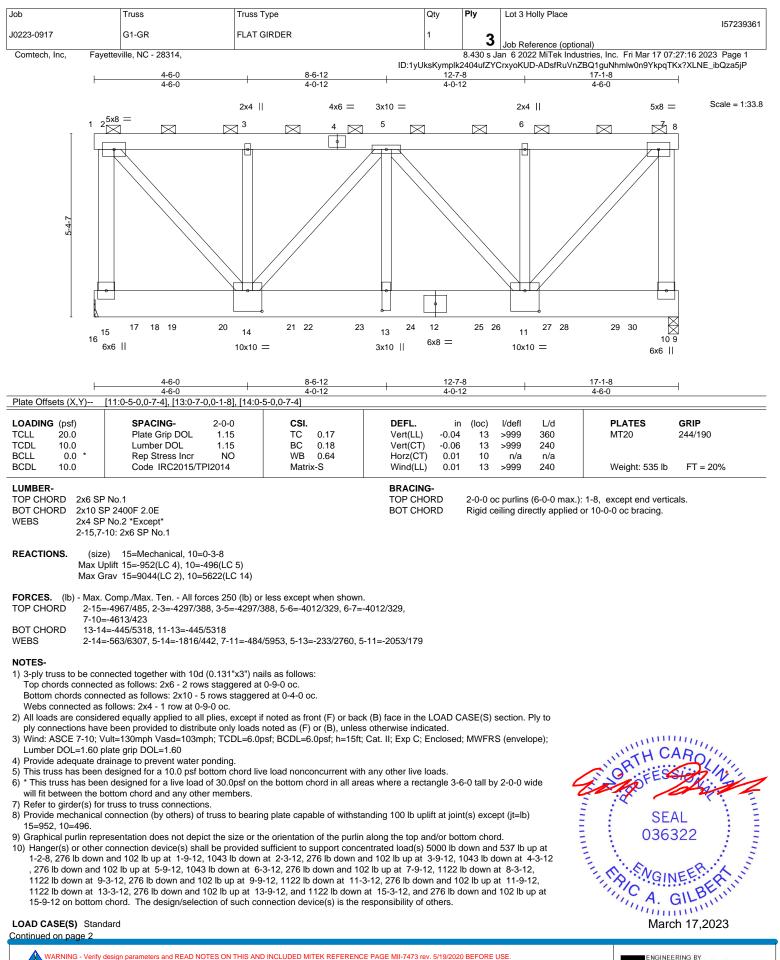
7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=152, 8=148.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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Truss	Truss Type	Qty	Ply	Lot 3 Holly Place
				157239361
G1-GR	FLAT GIRDER	1	2	
			<u>ວ</u>	Job Reference (optional)
ille, NC - 28314,			8.430 s Ja	an 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:16 2023 Page 2
	G1-GR	G1-GR FLAT GIRDER	G1-GR FLAT GIRDER 1	G1-GR FLAT GIRDER 1 3

ID:1yUksKymplk2404ufZYCrxyoKUD-ADsfRuVnZBQ1guNhmlw0n9YkpqTKx?XLNE\_ibQza5jP

LOAD CASE(S) Standard

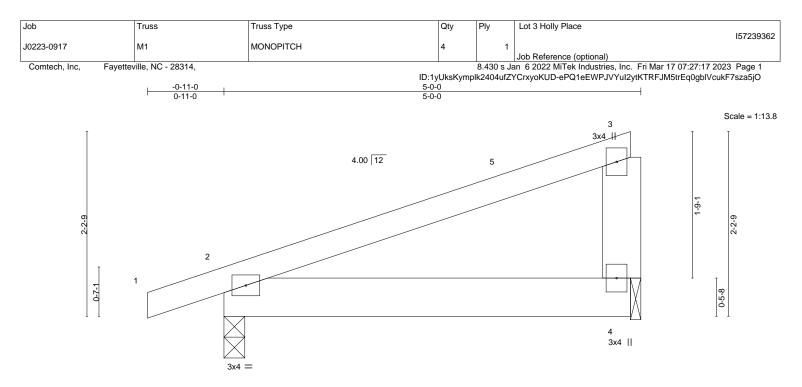
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-7=-60, 7-8=-60, 9-16=-20

Concentrated Loads (lb)

Vert: 12=-276(F) 14=-255(B) 13=-291(B) 17=-5000(B) 18=-276(F) 19=-255(B) 20=-276(F) 21=-276(F) 22=-255(B) 23=-276(F) 24=-291(B) 25=-291(B) 26=-276(F) 27=-291(B) 28=-276(F) 29=-291(B) 30=-276(F) 30=-291(B) 30=-276(F) 30=-291(B) 30=-276(F) 30=-291(B) 30=-276(F) 30=-291(B) 30=-276(F) 30=-291(B) 30=-276(F) 30=-291(B) 30=-





	5-0-0 5-0-0							
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES GRIP		
TCLL 20.0	Plate Grip DOL 1.15	TC 0.28	Vert(LL) -0	0.01 2-4	>999 360	MT20 244/190		
TCDL 10.0	Lumber DOL 1.15	BC 0.08	Vert(CT) -0	0.01 2-4	>999 240			
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) (	0.00	n/a n/a			
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) (	0.01 2-4	>999 240	Weight: 24 lb FT = 20%		

BRACING-

# LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=63(LC 8) Max Uplift 2=-102(LC 8), 4=-79(LC 8) Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

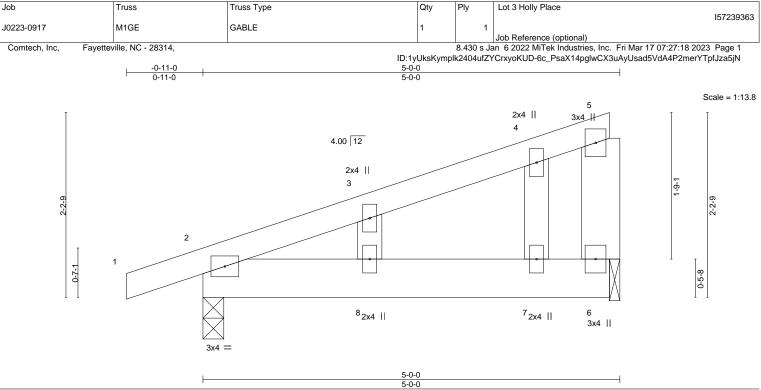
 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.







			1			5-0-0						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	0.01	8	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.09	Vert(CT)	-0.01	8	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.02	Horz(CT)	-0.00	6	n/a	n/a		
BCDL	10.0	Code IRC2015/TP	12014	Matri	x-S						Weight: 27 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No.2

**REACTIONS.** (size) 2=0-3-0, 6=0-1-8

Max Horz 2=90(LC 8) Max Uplift 2=-147(LC 8), 6=-115(LC 8) Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

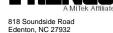
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147, 6=115.

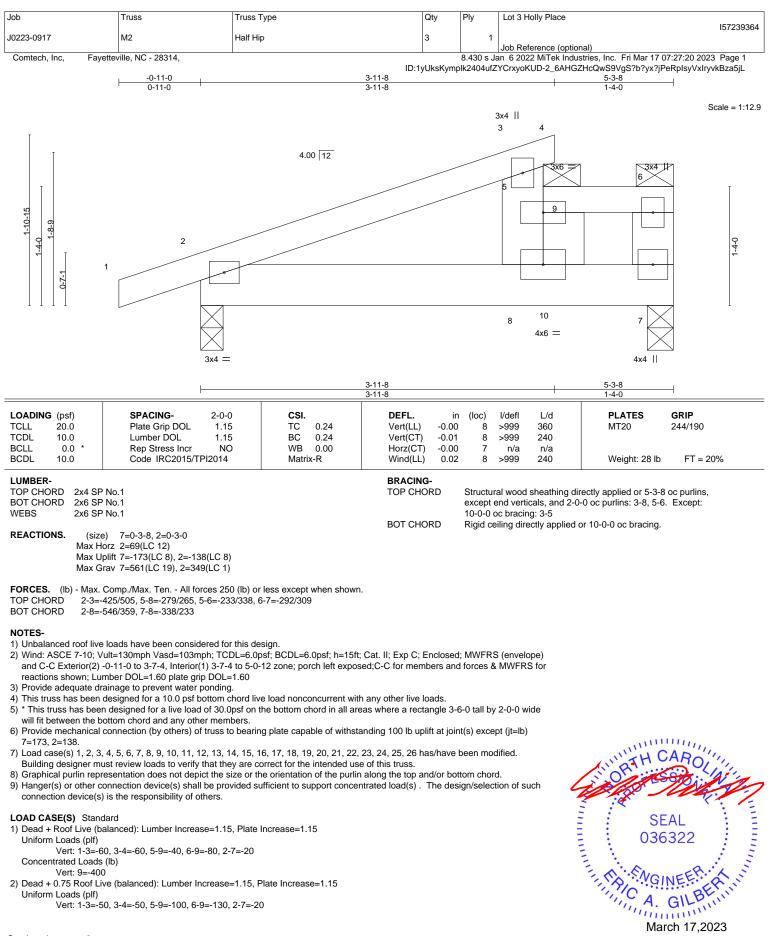


Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





## Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 3 Holly Place
					157239364
J0223-0917	M2	Half Hip	3	1	
					Job Reference (optional)
Comtech, Inc, Fayettev			8.430 s Ja	an 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:20 2023 Page 2	
		ID:1	yUksKymp	lk2404ufZ	YCrxyoKUD-2_6AHGZHcQwS9VgS?b?yx?jPeRpIsyVxIryvkBza5jL

Concentrated Loads (lb)
Vert: 9=-300
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=12, 2-8=52, 8-10=115, 7-10=52
Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55
Concentrated Loads (lb) Vert: 9=548
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=42, 2-8=52, 8-10=115, 7-10=52
Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55
Concentrated Loads (Ib)
Vert: 9=566
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=-1, 2-3=-45, 3-4=17, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51
Concentrated Loads (lb)
Vert: 9=-420
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf) Vert: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-58, 2-8=-9, 8-10=2, 7-10=-9
Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51
Concentrated Loads (Ib)
Vert: 9=-420
8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-11, 2-8=10, 8-10=33, 7-10=10
Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7
Concentrated Loads (lb)
Vert: 9=154 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=1, 2-7=-12
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
Concentrated Loads (Ib)
Vert: 9=43
Vert: 9=43 10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
<ol> <li>Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)</li> </ol>
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb)
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<ul> <li>10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339</li> <li>11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60</li> </ul>
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<ul> <li>10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-19, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34</li> <li>Concentrated Loads (lb) Vert: 9=-339</li> <li>11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0</li> <li>Concentrated Loads (lb) Vert: 9=-234</li> <li>12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39</li> <li>Concentrated Loads (lb) Vert: 9=43</li> <li>13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=4, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-26, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 9=43</li> </ul>
<ul> <li>10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34</li> <li>Concentrated Loads (lb) Vert: 9=-339</li> <li>11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234</li> <li>12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=43</li> <li>13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60</li> </ul>
<ul> <li>10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34</li> <li>Concentrated Loads (lb) Vert: 9=-339</li> <li>11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0</li> <li>Concentrated Loads (lb) Vert: 9=-234</li> <li>12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39</li> <li>Concentrated Loads (lb) Vert: 9=43</li> <li>13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-4, 2-3=-21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 9=43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)</li> <li>Vert: 9=43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)</li> </ul>
<ul> <li>10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-19, 3-4=-6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34</li> <li>Concentrated Loads (lb) Vert: 9=-339</li> <li>11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0</li> <li>Concentrated Loads (lb) Vert: 9=-234</li> <li>12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39</li> <li>Concentrated Loads (lb) Vert: 9=43</li> <li>13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-24, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-44, 2-3=-21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 9=43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 9=43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=-14, 3-5=-27</li> </ul>
<ul> <li>10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34 Concentrated Loads (lb) Vert: 9=-339</li> <li>11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234</li> <li>12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=-43</li> <li>13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-26, 2-3=-31, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=-43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 2=-43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=-14, 5-6=-71, 2-7=-12 Horz: 1-2=-14, 2-3=21, 3-4=-14, 5-6=-71, 2-7=-12 Horz: 1-2=-14, 2-3=21, 3-4=-14, 5-6=-71, 2-7=-72</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=-14, 5-6=-71, 2-7=-72 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39</li> </ul>
<ul> <li>10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-19, 3-4=-6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34</li> <li>Concentrated Loads (lb) Vert: 9=-339</li> <li>11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0</li> <li>Concentrated Loads (lb) Vert: 9=-234</li> <li>12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39</li> <li>Concentrated Loads (lb) Vert: 9=43</li> <li>13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-24, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-44, 2-3=-21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 9=43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 9=43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=-14, 3-5=-27</li> </ul>
<ul> <li>10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34</li> <li>Concentrated Loads (lb) Vert: 9=-339</li> <li>11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0</li> <li>Concentrated Loads (lb) Vert: 9=-234</li> <li>12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39</li> <li>Concentrated Loads (lb) Vert: 9=-43</li> <li>13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-14, 2-3=21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 9=-43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-24, 2-3=21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 1-2=-34, 2-3=-21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (plf) Vert: 1-2=-43, 2-3=-21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39</li> <li>Concentrated Loads (plf)</li> <li>Vert: 1-2=-4, 2-3=-21, 3-4=-4, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39</li> <li>Concentrated Loads (lb)</li> </ul>
<ul> <li>10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-8=2, 8-10=25, 7-10=2 Horz: 1-2=-26, 2-3=-10, 3-4=-26, 3-5=34</li> <li>Concentrated Loads (lb) Vert: 9=-339</li> <li>11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0</li> <li>Concentrated Loads (lb) Vert: 9=-234</li> <li>12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39</li> <li>Concentrated Loads (lb) Vert: 9=-43</li> <li>13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 9=-43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 9=-43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=-14, 3-5=-27</li> <li>Concentrated Loads (lb) Vert: 9=-43</li> <li>14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=-14, 5-6=-11, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39</li> <li>Concentrated Loads (lb) Vert: 9=-43</li> </ul>

3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25

Vert: 1-3=-20, 3-4=-20, 5-6=-40, 2-7=-40

# Continued on page 3

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 9=-350

Uniform Loads (plf)

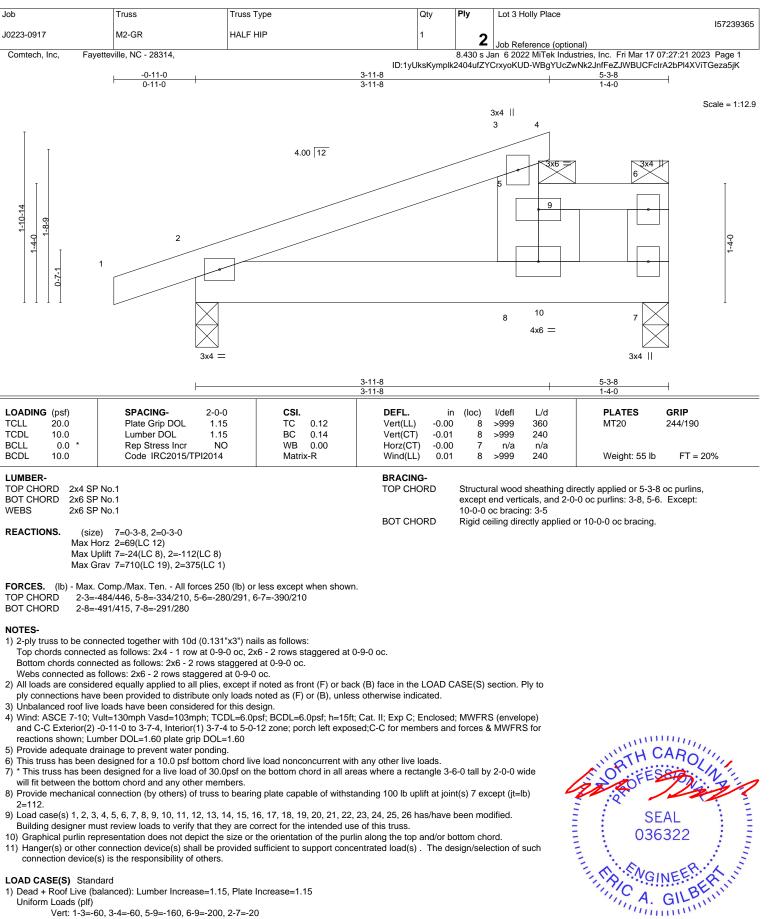


				-	
Job	Truss	Truss Type	Qty	Ply	Lot 3 Holly Place
					15723936
J0223-0917	M2	Half Hip	2	1	10120000
30223-0917		пан пр	3	· ·	
					Job Reference (optional)
Comtech, Inc, Fayettev	lle, NC - 28314,			8.430 s Ja	an 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:20 2023 Page 3

ID:1yUksKymplk2404ufZYCrxyoKUD-2\_6AHGZHcQwS9VgS?b?yx?jPeRplsyVxIryvkBza5jL

LOAD CASE(S) Standard Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-21, 2-7=-20 Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-6=-120, 2-7=-20 Concentrated Loads (lb) Vert: 9=-200 19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-8=-3, 8-10=13, 7-10=-3 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26 Concentrated Loads (lb) Vert: 9=-454 20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-86, 6-9=-116, 2-7=-20 Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0 Concentrated Loads (lb) Vert: 9=-375 21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-7=-20 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9 Concentrated Loads (lb) Vert: 9=-375 22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-86, 6-9=-116, 2-7=-20 Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0 Concentrated Loads (lb) Vert: 9=-375 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-6=-40, 2-7=-20 Concentrated Loads (lb) Vert: 9=-400 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-40, 6-9=-80, 2-7=-20 Concentrated Loads (lb) Vert: 9=-400 25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 5-6=-100, 2-7=-20 Concentrated Loads (lb) Vert: 9=-350 26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-100, 6-9=-130, 2-7=-20 Concentrated Loads (lb) Vert: 9=-350





Vert: 1-3=-60, 3-4=-60, 5-9=-160, 6-9=-200, 2-7=-20

#### Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

March 17,2023

Jo	b.	Truss	Truss Type	Qty	Ply	Lot 3 Holly Place
0		11033		QUY	,	157239365
1.10	0223-0917	M2-GR	HALF HIP	1	_	101200000
					2	Job Reference (optional)
	Comtech, Inc, Fayettev	ille, NC - 28314,			8.430 s Ja	an 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:21 2023 Page 2

	Fayetteville, NC - 28314,	8.430 s Jan 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:21 2023 Pag ID:1yUksKymplk2404ufZYCrxyoKUD-WBgYUcZwNk2JnfFeZJWBUCFcIrA2bPl4XViTGeza
	Cton dord	
LOAD CASE(S) Concentrated L		
Vert: 9		
,	oof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Incr	ease=1.15
Uniform Loads	· · · ·	
Concentrated L	1-3=-50, 3-4=-50, 5-9=-220, 6-9=-250, 2-7=-20	
Vert: 9		
	abitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.	25
Uniform Loads	; (plf)	
	1-3=-20, 3-4=-20, 5-6=-160, 2-7=-40	
Concentrated L		
Vert: 9		<b>60</b>
4) Dead + 0.6 C-C Uniform Loads	C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1	60
	1-2=70, 2-3=58, 3-4=153, 5-6=-108, 2-8=52, 8-10=115, 7-10=52	
	1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55	
Concentrated L		
Vert: 9	Ð=548	
,	C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1	60
Uniform Loads	· · · ·	
	1-2=51, 2-3=58, 3-4=51, 5-6=-78, 2-8=52, 8-10=115, 7-10=52	
Concentrated L	1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55	
Vert: 9		
	C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1	.60
Uniform Loads		
Vert: 1	1-2=-1, 2-3=-45, 3-4=17, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9	
	1-2=-19, 2-3=25, 3-4=-37, 3-5=51	
Concentrated L		
Vert: 9		<b>00</b>
Uniform Loads	C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1	.60
	1-2=-39, 2-3=-45, 3-4=-39, 5-6=-178, 2-8=-9, 8-10=2, 7-10=-9	
	1-2=19, 2-3=25, 3-4=19, 3-5=51	
Concentrated L		
Vert: 9	Ð=-420	
,	VFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=	1.60
Uniform Loads	· · · ·	
	1-2=36, 2-3=21, 3-4=14, 5-6=-131, 2-8=10, 8-10=33, 7-10=10	
Concentrated L	1-2=-48, 2-3=-33, 3-4=-26, 3-5=7	
Vert: 9		
	VFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase	=1.60
Uniform Loads	s (plf)	
	1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12	
	1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27	
Concentrated L		
Vert: 9		-1 60
Uniform Load	IWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase	=1.60
	2 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2	
	:: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34	
Concentrated		
Vert:	9=-339	
,	IWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increas	e=1.60
Uniform Load		
	: 1-2=-2, 2-3=-9, 3-4=-2, 5-6=-141, 2-7=-20	
Concentrated	:: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0	
	- 9=-234	
	WFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Ir	crease=1.60
Uniform Load	, , , , , , , , , , , , , , , , , , , ,	
Vert:	: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12	
	:: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39	
Concentrated		
	9=43	
,	WFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate I	ncrease=1.60
Uniform Load		
	: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 :: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27	
	1-2=-17, 2-3=-21, 3-4=-14, 3-3=-21	
	(dl) ads	
Concentrated	I Loads (lb) 9=43	

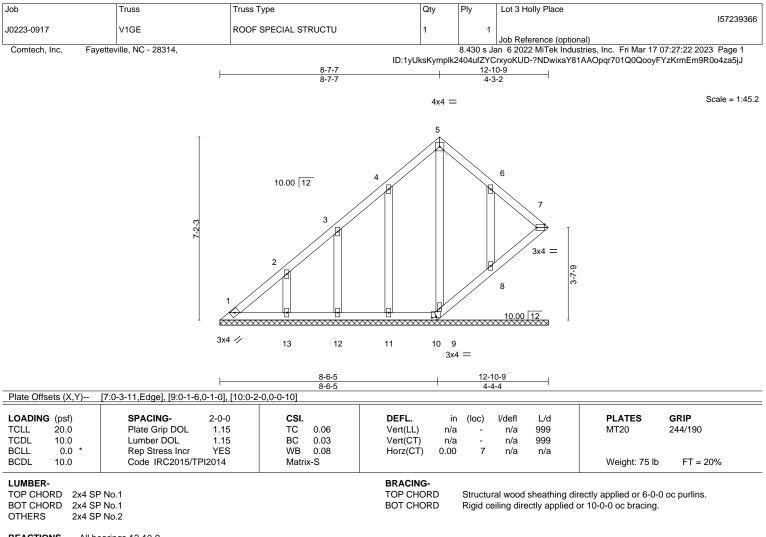
Continued on page 3



Job	Tr	russ	Truss Type	Qty	Ply	Lot 3 Holly Place
						157239365
						137239303
J0223-0917	M	2-GR	HALF HIP	1	2	
					<b></b>	Job Reference (optional)
Comtech, Inc,	Fayetteville	e, NC - 28314,			8.430 s Ja	an 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:21 2023 Page 3

ID:1yUksKymplk2404ufZYCrxyoKUD-WBgYUcZwNk2JnfFeZJWBUCFcIrA2bPl4XViTGeza5jK LOAD CASE(S) Standard Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=43 15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20 Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20 Concentrated Loads (lb) Vert: 9=-200 19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26 Concentrated Loads (lb) Vert: 9=-454 20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20 Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0 Concentrated Loads (lb) Vert: 9=-375 21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9 Concentrated Loads (lb) Vert: 9=-375 22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-206, 6-9=-236, 2-7=-20 Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0 Concentrated Loads (lb) Vert: 9=-375 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-6=-160, 2-7=-20 Concentrated Loads (lb) Vert: 9=-400 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20 Concentrated Loads (lb) Vert: 9=-400 25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 5-6=-220, 2-7=-20 Concentrated Loads (lb) Vert: 9=-350 26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20 Concentrated Loads (lb) Vert: 9=-350





**REACTIONS.** All bearings 12-10-9.

(lb) - Max Horz 1=231(LC 12)

1-2=-295/189

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10 except 11=-112(LC 12), 12=-107(LC 12), 13=-133(LC 12),

8=-126(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

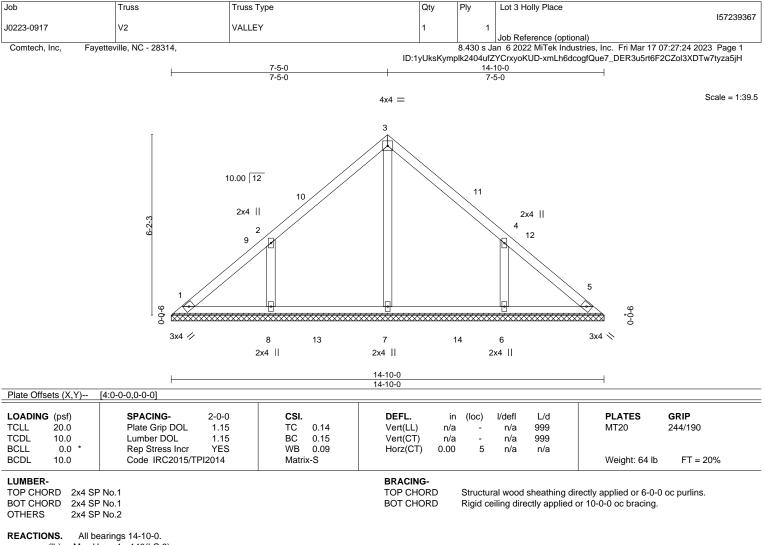
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# TOP CHORD

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.



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<sup>(</sup>lb) - Max Horz 1=-140(LC 8)

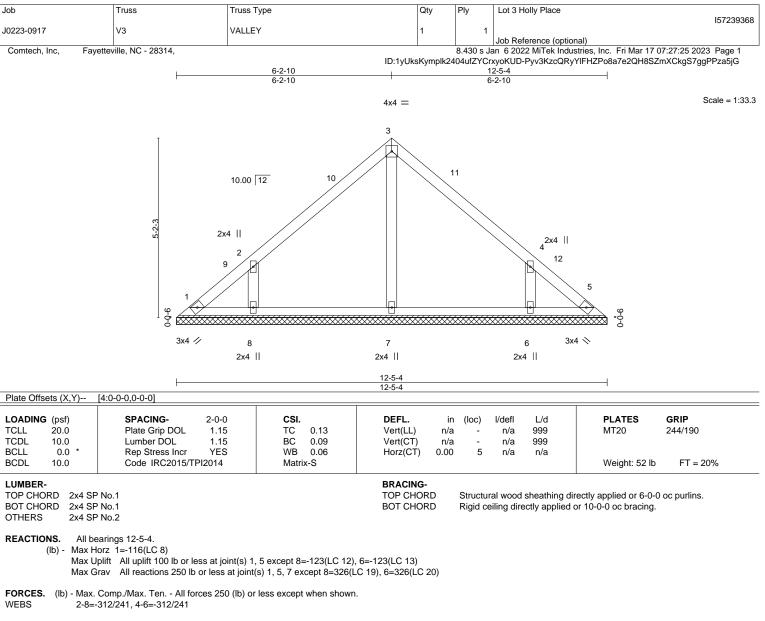
- Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-135(LC 12), 6=-135(LC 13)
- Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=400(LC 19), 8=393(LC 19), 6=393(LC 20)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- WEBS 2-8=-338/247, 4-6=-338/247

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=135, 6=135.







## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

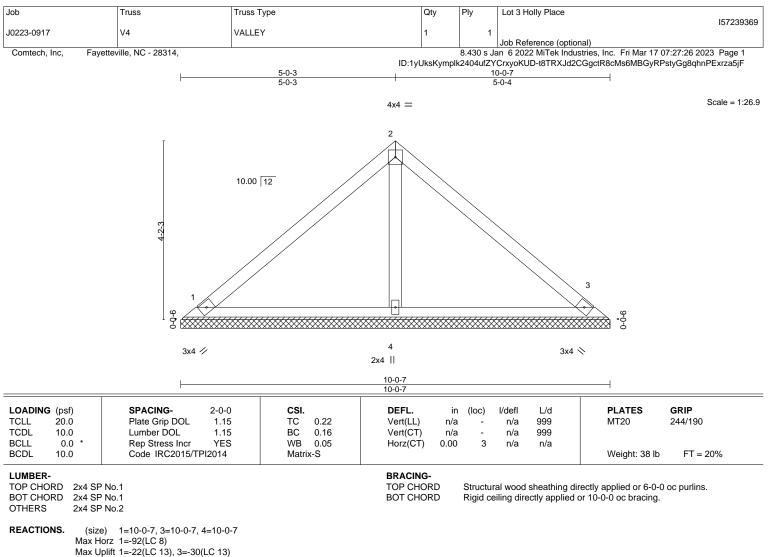
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.







Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

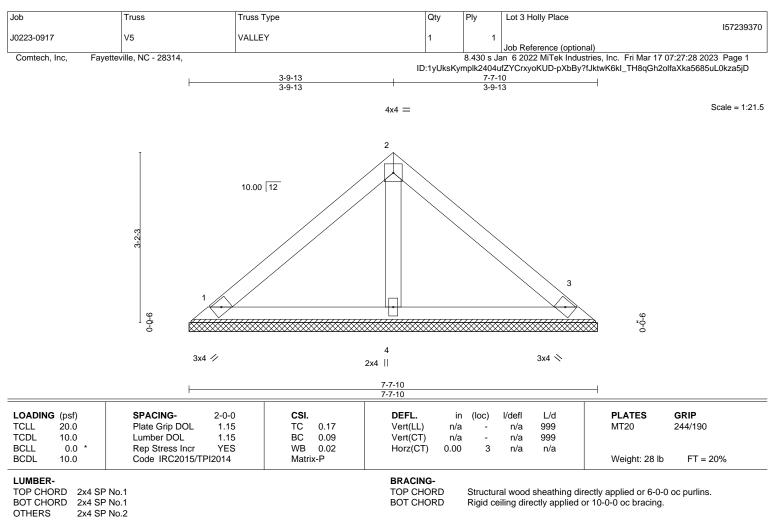
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







REACTIONS. (size) 1=7-7-10, 3=7-7-10, 4=7-7-10 Max Horz 1=68(LC 9) Max Uplift 1=-24(LC 13), 3=-30(LC 13) Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

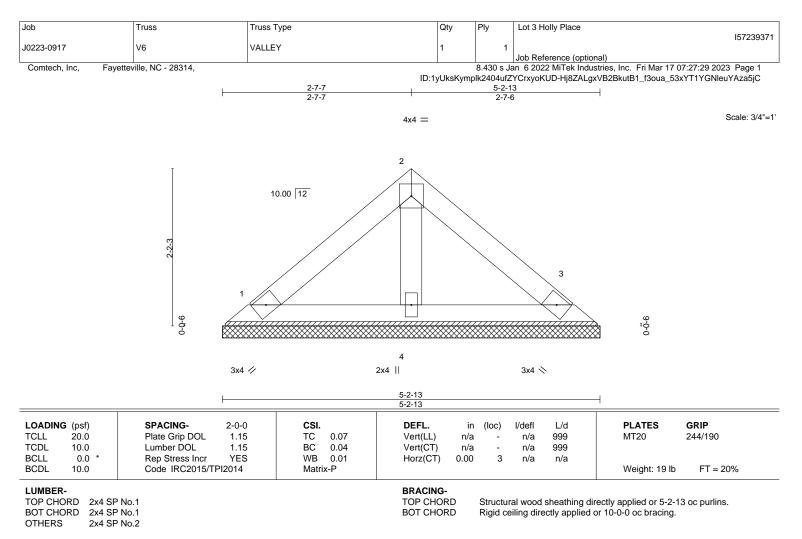
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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REACTIONS. (size) 1=5-2-13, 3=5-2-13, 4=5-2-13 Max Horz 1=-44(LC 8) Max Uplift 1=-15(LC 13), 3=-19(LC 13) Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

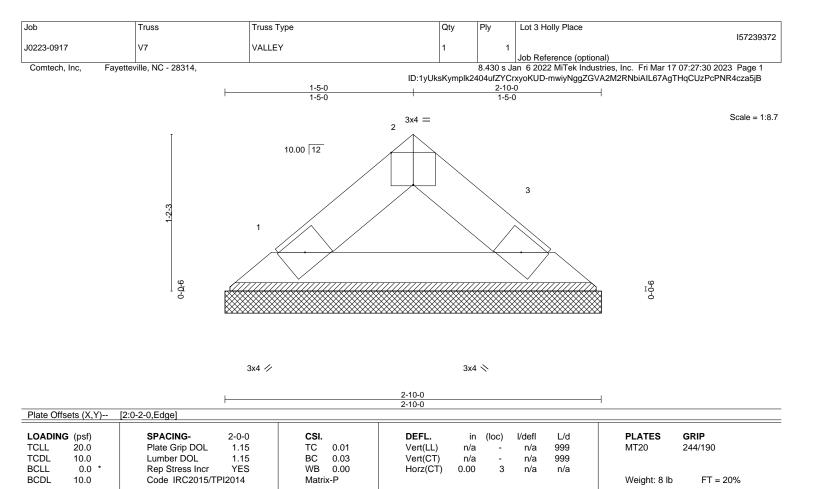
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



818 Soundside Road Edenton, NC 27932



BRACING-

TOP CHORD

BOT CHORD

LUMBER-TOP CHORD 2x4 SP No.1

BOT CHORD 2x4 SP No.1

REACTIONS. (size) 1=2-10-0, 3=2-10-0 Max Horz 1=-20(LC 8) Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



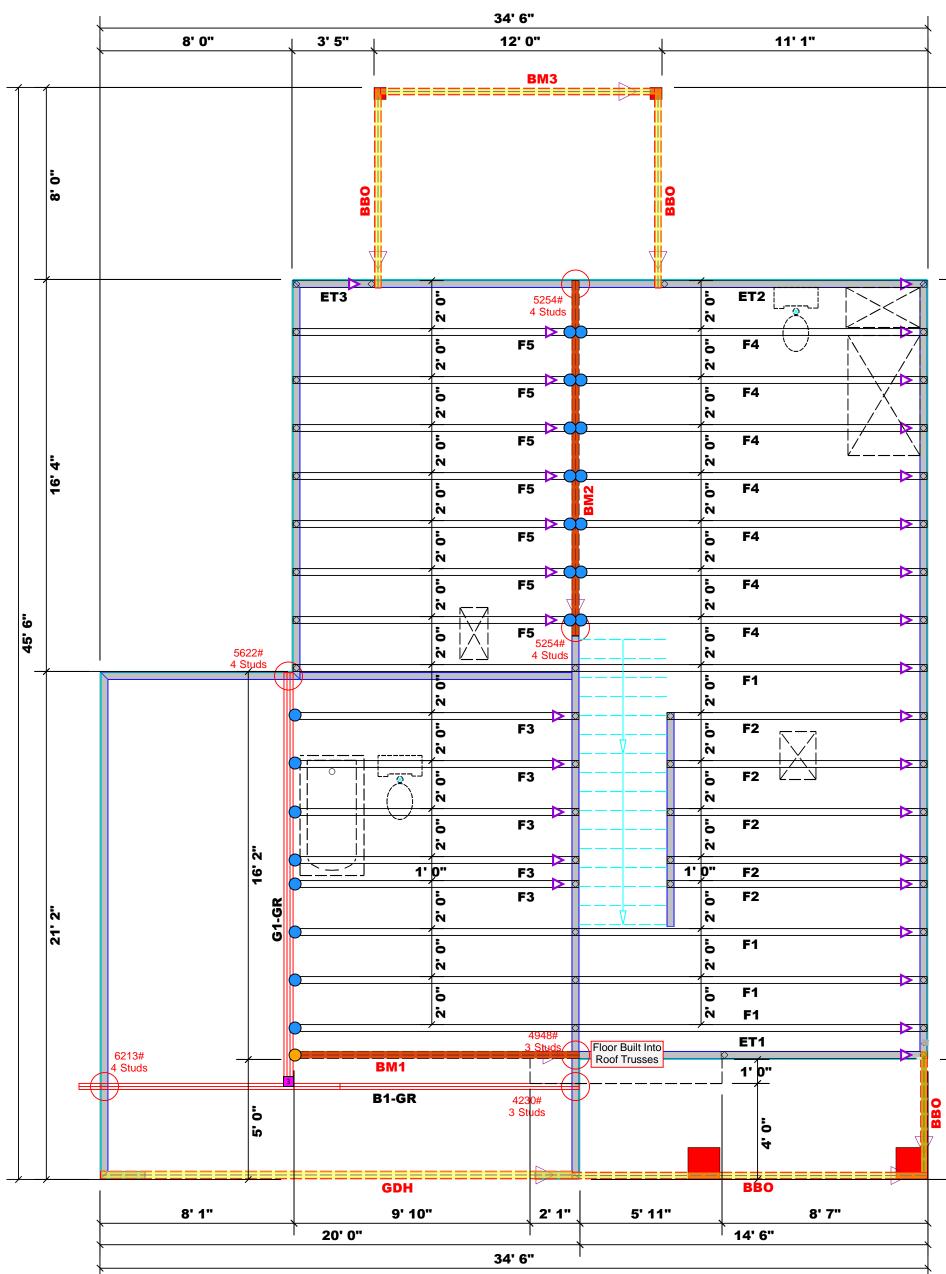
Structural wood sheathing directly applied or 2-10-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932





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These t compor design See ind identifie designe for the support and col designe consult	BUILDER	Weaver Development Co. Inc.	<b>CITY / CO</b> .	<b>CITY / CO</b> . Kenly / Johnston	NUM	reaction Tables. / retained	deemed requirem attached requirem size and reactions 15000#.	
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Dimension Notes All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes Plumbing drop locations shown are NOT exact.
 Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

	Conne	ctor Info	rmati	ion	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS410	USP	22	NA	16d/3-1/2"	16d/3-1/2"
$\bigcirc$	THDH412	USP	1	NA	16d /3-1/2"	16d /3-1/2"

		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM3	12' 0"	2x10 SPF No.2	2	2
GDH	20' 0"	1-3/4"x 14" LVL Kerto-S	2	2
		Truss Placement Plan		

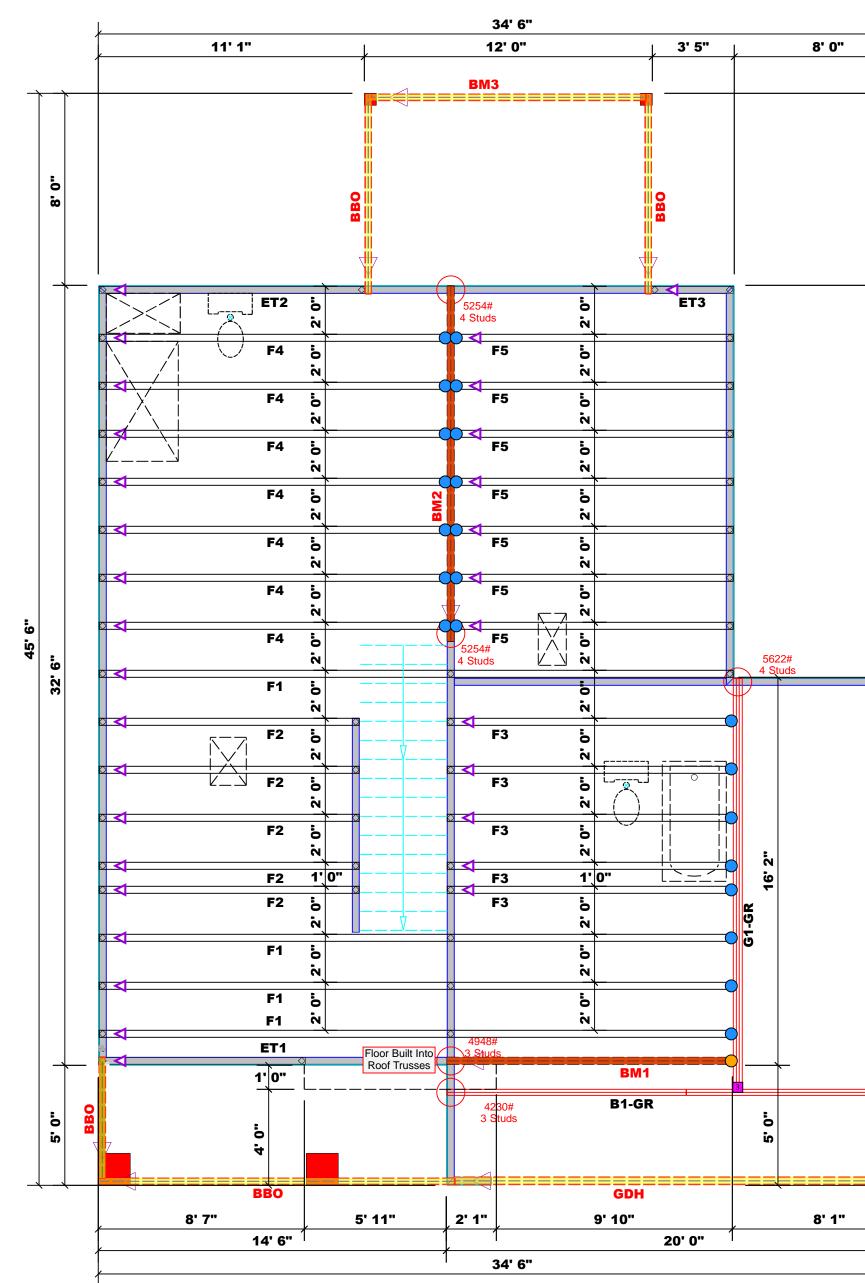
(1) Scale: 1/4"=1'

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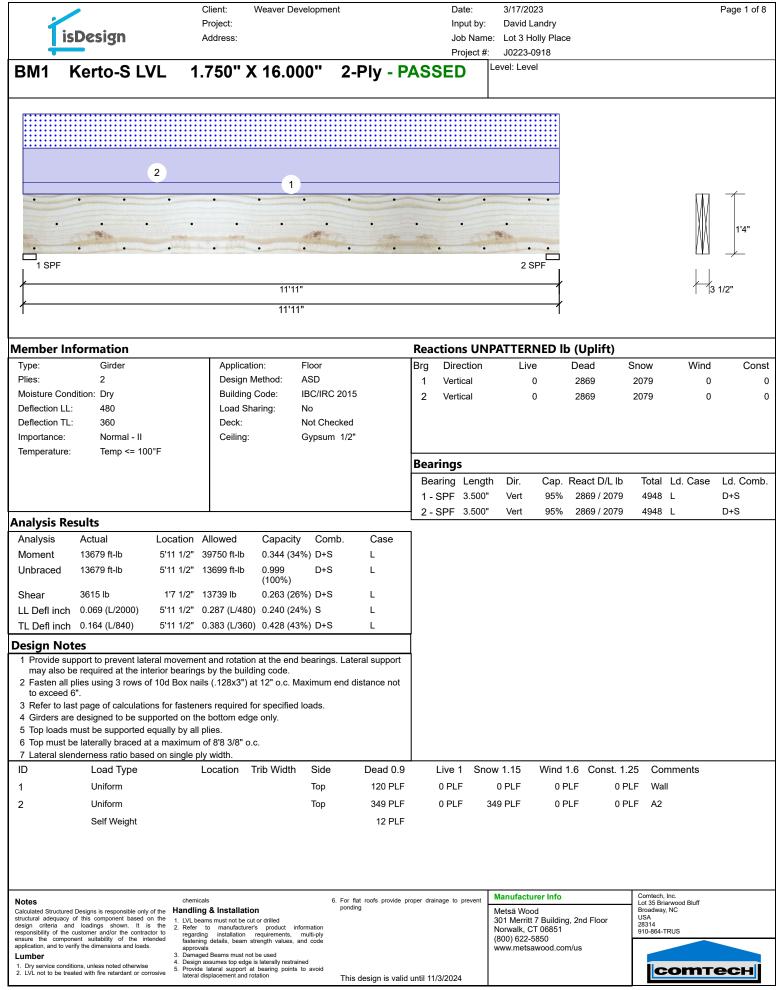
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C lio

> Indicates Left End of Truss
> (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

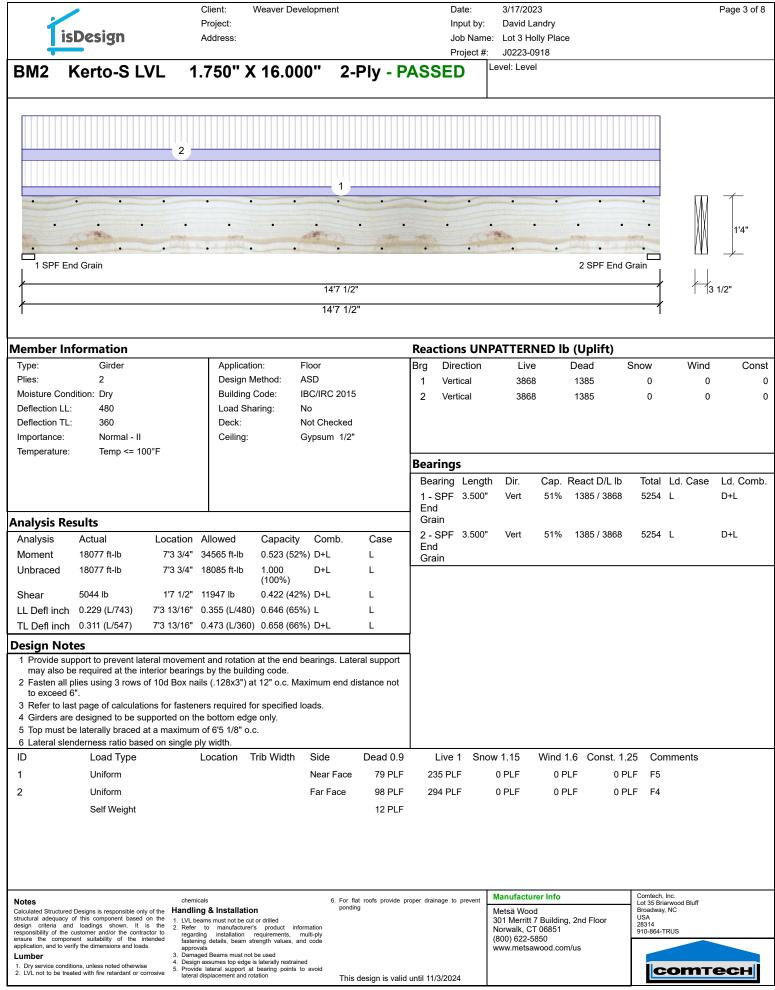


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	8. 0.			requirent attached requirent size and reaction 15000#, retained reaction Tables, retained reaction Signatu	nents. Thin I Tables ( nents) to s greater A register to design that excc re Re AD CH/ (BASED	determined determined of wood s than 3000 red design in the supp eved sthose ed design the supp eved sthose David David David ART FC o ON TABLE	or shall r om the p om the p the min tuds requ # but not professi ort syste specific professi ort syste specific d Lar d Lar PR JAC s R502.5(1	efer to the rescription imum foou irred to s igreater foonal sha m for any d in the a onal shal m for all Mdry idry K STU	e re Code indation upport than II be y titached I be
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		1 45' 6"	Dimension Notes         1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise         2. All interior wall dimensions are to face of frame wall unless noted otherwise         3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise         All Walls Shown Are Considered Load Bearing         Plumbing Drop Notes         1. Plumbing drop locations shown are NOT exact.	Kenly / Johnston	172 Colby Farm Drive	Floor	03/17/23	David Landry	Lenny Norris
	. 2"		2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.     3. Adjust spacing as needed not to exceed 24"oc.     Connector Information     Nail Information     Sym Product Manuf Qty Supported Header Truss	<b>CITY / CO</b> .	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
6213# 4 Studs	, ,		$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Weaver Development Co. Inc.	Lot 3 Holly Place	Hickory II "A" / 2GRF, CP	N/A		J0223-0918
	,			BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #
			= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards	These compo design See ind identifi design permar for the suppor and co design consult	russes an nents to b at the sp lividual de ed on the er is resp ent braci overall st t structur umns is f er. For ge BCSI-B1	S PLACEM e designe ee incorpo ecification esign shee placemen onsible fo nonsible fo nonsible fo neructure. T e includin he respon neral guid and BCS ckage or	d as indi rated into of the be to for ea t drawing tempora of and he design g headers sibility o ance reg -B3 provi	vidual bu o the buil uilding de ch truss g. The bu ary and floor sys n of the t s, beams f the buil arding br ided with	ilding ding ssigner. design ilding tem and russ , walls, ding acing, the

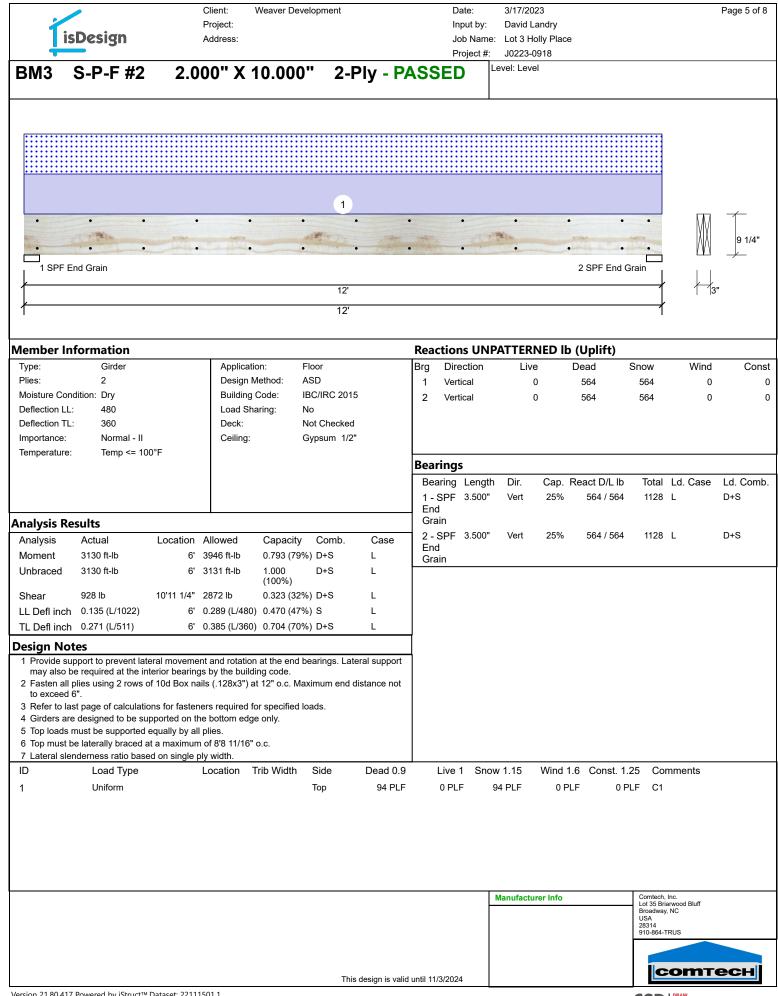


CSD DESIG

	Client: Weaver Developm	nent Date: Input by:	3/17/2023	Page 2 of 8
isDesign	Project: Address:	Job Nam	ne: Lot 3 Holly Place	
		Project #	E J0223-0918 Level: Level	
BM1 Kerto-S I	_VL 1.750" X 16.000"	2-Ply - PASSED		
• • •	• • • •	• • • •	••]	$\Pi$ $\uparrow$
				1'4"
	• • • •		<u> </u>	
1 SPF				
/	11'11"			3 1/2"
1	11'11"			
Multi-Ply Analysis				
Fasten all plies using 3 rc Capacity	ows of 10d Box nails (.128x3") at 12'	o.c Maximum end distance r	ot to exceed 6".	
Load	0.0 PLF			
Yield Limit per Foot Yield Limit per Fastener	245.6 PLF 81.9 lb.			
Yield Mode	IV			
Edge Distance Min. End Distance	1 1/2" 3"			
Load Combination				
Duration Factor	1.00			
				_
Notes	chemicals Inly of the Handling & Installation	6. For flat roofs provide proper drainage to prevent ponding	Manufacturer Info	Comtech, Inc. Lot 35 Briarwood Bluff Broadway, NC
Calculated Structured Designs is responsible of structural adequacy of this component base design criteria and loadings shown. It componentiality of the outstance and/or the com-	d on the 1. LVL beams must not be cut or drilled is the 2. Refer to manufacturer's product information		Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
responsibility of the customer and/or the con ensure the component suitability of the application, and to verify the dimensions and lo	intended fastening details, beam strength values, and code approvals		(800) 622-5850 www.metsawood.com/us	
Lumber 1. Dry service conditions, unless noted otherw 2. LVL not to be treated with fire retardant or				соттесн
Version 21.80.417 Powered by iStruct		This design is valid until 11/3/2024	1	



Project: Address:     Job Name: Lot 3 Holly Place   Project #: J0223-0918    BM2 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED   Level     Level     Level     Implete:     Job Name:   Lot 3 Holly Place   Project #:   J0223-0918     BM2 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED     Level:     Level:     Level:     Staten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c Maximum end distance not to exceed 6".     Capacity     79.9 %				Client:	Weaver Developm	oont	Da	to:	3/17/2023	Page 4 of 8
New Nume:         Control Num:         Control Nume:         Control Nume:	2				weaver Developin	lent				Fage 4 01 o
New Process         Description         Description         Description           Image: Description of the second of the	1	isDesign						-	-	
BM2         Kerto-S LVL         1.750" X 16.000"         2-Ply - PASSED         Level Lower           Image: Second Seco		Ispesign		Address.						
NMZ         NERICO-S LVL         1.700         X 10.000         2-PTy - PASSED           Image: Second Sec								-		
Image: State in the state i	BM2	Kerto-S	LVL	1.750"	X 16.000"	2-Ply ·	- PASSED	) '		
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Image: State in the state i										$\Sigma$ $M$ $1'4''$
Image: state of the state o	•	• •	•	•	• •	• •	•	•	• • •	
Math-Dy Analysis       Easten all plies using 3 rows of 10d Box nails (128x3°) at 12° o.c. Maximum end distance not to exceed 6°.       Charachy 100 100 000 100 000 100 000 100 000 100 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 0000	1 SPF	F End Grain							2 SPF	End Grain
Math-Dy Analysis       Easten all plies using 3 rows of 10d Box nails (128x3°) at 12° o.c. Maximum end distance not to exceed 6°.       Charachy 100 100 000 100 000 100 000 100 000 100 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 000 0000										
Multi-Py Analysis         Exate nal plies using a rows of 10d Box nails (128x3') at 12° o.c., Maximum end distance not to exceed 6'.         Charlow 1997 Food 100 Ford 100						14'7 1/2"				]
Multi-Py Analysis         Exate nal plies using a rows of 10d Box nails (128x3') at 12° o.c., Maximum end distance not to exceed 6'.         Charlow 1997 Food 100 Ford 100	/					14'7 1/2"				ł
Fasten all plies using 3 rows of 104 Box nails (128/3*) at 12° o.c. Maximum end distance not to exceed 6°.         Capacity       78.8 %         Viet Minger Foot       24.6 P.F.         Viet Minger Foot       21.6 P.F.         Wiel Monger Bothome Botho										
Fasten all plies using 3 rows of 104 Box nails (128/3*) at 12° o.c. Maximum end distance not to exceed 6°.         Capacity       78.8 %         Viet Minger Foot       24.6 P.F.         Viet Minger Foot       21.6 P.F.         Wiel Monger Bothome Botho	ļ									
Capacity       70.8 %         Load       70.8 %         Viel Linit per Foot       245.6 PL F         Viel Linit per Statemer       19.10.         Viel Linit per Statemer       19.10.         Viel Linit per Statemer       10.2 %         Min. End Distance       3"         Ladd Combination       D-L         Duration Factor       1.00	Multi-Ply	y Analysis								
Capacity       70.8 %         Load       70.8 %         Viel Linit per Foot       245.6 PL F         Viel Linit per Statemer       19.10.         Viel Linit per Statemer       19.10.         Viel Linit per Statemer       10.2 %         Min. End Distance       3"         Ladd Combination       D-L         Duration Factor       1.00	Fasten all	l plies usina 3	rows of	10d Box nails	; (.128x3") at 12'	' o.c Maxim	um end distar	nce no	ot to exceed 6".	
Lead       196 0 FLF         Weld Link per Fastener       81.9 Kr.         Weld Kode       V         Edge Distance       112*         M.E. dD Distance       1         M.E. dD Distance       1         Distance       1.12*         Laad Combination       0-L         Duration Factor       1.00		1 3								
Netal Mage Fastener     31.9 lb.       Veld Mage     V       Edge Distance     11/2*       Mr. Ed Distance     3*       Land Combination     D-I       Duration Factor     1.00	Load		196	.0 PLF						
Yead Mode       V         Gap Oblance       3         Edgo Oblance       3         Ducation Factor       100			245	.6 PLF						
Edge Datanoo       1 1/2*         Mic. End Datanoo       3         Load Combination       D-L         Duration Factor       1,00		er Fastener		9 lb.						
Mine. End Distance       3' Load Combination       Dividual         Duration Factor       1.00				0						
Load Combination       D-L         Duration Factor       1.00				2						
Duration Factor     100       Note        • for fait rode provide projet danage to projet danage t				_						
Notes     Chemicals     6     For flat roots provide proper dranage to proven ponding     Metrix     Construction     Lot 35 Britancod Bluff Broadway, NC       Calculated Structured Designs is responsible only of the design criteria and loadings shown. It is ensome the component based on the application, and to verify the dimensions and loads.     Handling & Installation     I. VL Deams must not be cut or drilled     Metrix 4000d     USA 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850     USA 28314       1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrorive     3. Damaged Beams must not be used 4. Design assumes top degis laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation     This design is valid until 11/3/2024     Www.metsawood.com/us     Urit or top are provide proper drainage to provide 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850     Urit or top are provide proper drainage to provide pro										
Notes     Chemicals     6     For flat roots provide proper dranage to proven ponding     Metrix     Construction     Lot 35 Britancod Bluff Broadway, NC       Calculated Structured Designs is responsible only of the design criteria and loadings shown. It is ensome the component based on the application, and to verify the dimensions and loads.     Handling & Installation     I. VL Deams must not be cut or drilled     Metrix 4000d     USA 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850     USA 28314       1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrorive     3. Damaged Beams must not be used 4. Design assumes top degis laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation     This design is valid until 11/3/2024     Www.metsawood.com/us     Urit or top are provide proper drainage to provide 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850     Urit or top are provide proper drainage to provide pro										
responsibility of the customer and/or the contractor to ensure the component subility of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. UVL not to be treated with fire retardant or corrosive 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation This design is valid until 11/3/2024	Calculated Struct structural adequidesign criteria	uacy of this component ba and loadings shown.	ased on the It is the	Handling & Installa 1. LVL beams must not b	e cut or drilled	6. For flat roofs pro ponding	vide proper drainage to p	prevent	Metsä Wood 301 Merritt 7 Building, 2nd Floor	Lot 35 Briarwood Bluff Broadway, NC USA 28314
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<ol> <li>Dry service conditions, unless noted otherwise</li> <li>Device treated with fire retardant or corrosive</li> <li>Device lateral support at bearing points to avoid lateral displacement and rotation</li> <li>This design is valid until 11/3/2024</li> </ol>		to verify the dimensions and		approvals 3. Damaged Beams mus	t not be used				www.metsawood.com/us	
This design is valid until 11/3/2024	1. Dry service of		erwise	<ol> <li>Design assumes top e</li> <li>Provide lateral support</li> </ol>	dge is laterally restrained rt at bearing points to avoid					сотесн
						This design is	valid until 11/3/2024			



	is	Design	1	Clien Proje Addre	ct:	/eaver Develop	ment		Date: Input by: Job Name:	3/17/2023 David Landry Lot 3 Holly Pla	ace			Page 6 of 8
									Project #:	J0223-0918				
BN	13	S-P-F	#2	2.000	' X 1	0.000"	2-Ply	- PASS	SED	evel: Level				
													-	
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		•	•	•	•	•	•	•	•	•	•	• • -	<u> </u>	9 1/4"
	J 1 SPF E	End Grain									2 SPF	End Grain		
							12'						3"	
<del> </del>  -							12'					1		
	-	Analysis												
		lies using			hails (.1	28x3") at 12	" o.c Maxii	mum end	distance not	to exceed	6".			
Capaci	ty			.0 %										
Load				.0 PLF										
	imit per F			57.4 PLF										
		astener		8.7 lb.										
Yield M			IN											
Edge D				1/2"										
Min. Er	nd Distar	nce	3											

Manufacturer Info	Comtech, Inc. Lot 36 Briarwood Bluff Broadway, NC USA 28314 910-864-TRUS
	соттесн

Load Combination Duration Factor

1.00

This design is valid until 11/3/2024

			Client:	Weaver Dev	/elopment		Date		3/17/20					Page 7 of 8
Tie	Design		Project:				•	it by:	David L	-				
	Design	F	Address:					Name ect #:	: Lot 3 Ho J0223-0	-				
GDH	Kerto-S L	VI 1	750"	x 1/ 00	ר ייחר		ASSED		Level: Leve					
		.VL I.	150 /	A 14.00	50 2-	- F I <b>y -</b> F	ASSLD							
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	and the second s				1. March				-				. W	
		- Miner									Mar Maria		. M	1'2"
1 SPF End	d Grain										2 SPF E	nd Grain		
ļ					20'									2 1/2"
<u> </u>					-									3 1/2
					20'								Т	
Member In	formation						Reactions	UN	PATTER	NED Ib	(Uplift)			
Туре:	Girder		Applicat		Floor		Brg Direct		Live		Dead	Snow		
Plies: Moisture Cond	2 dition: Dru		Design Building		ASD IBC/IRC 2015		1 Vertica				1804	196		0 0
Deflection LL:	•		Load Sh		No		2 Vertica	al	(	)	1750	141		0 0
Deflection TL:			Deck:	0	Not Checked									
Importance:	Normal - II		Ceiling:		Gypsum 1/2"									
Temperature:	Temp <= 10	00°F					<b>D</b>							
							Bearings		Dia	0		<u>и.</u> т.		- 1 -1 O h
							Bearing L	-	n Dir. Vert	Cap. R 19%	eact D/L 1804 / 19		tal Ld.Cas 00 L	e Ld. Comb. D+S
							End	.500	ven	1370	1004713	20	00 L	DIG
Analysis Re	sults						Grain			1001				
Analysis	Actual	Location A	Allowed	Capacity	Comb.	Case	2 - SPF 3 End	.500"	Vert	18%	1750 / 14	11 18	91 L	D+S
Moment	8592 ft-lb	9'11 11/16" 2		0.354 (35	-	Uniform	Grain							
Unbraced	9503 ft-lb	9'11 1/2" 9	9509 ft-lb	0.999 (100%)	D+S	L								
Shear	1553 lb	1'5 1/2" 9	9408 lb	0.165 (17	%) D	Uniform								
LL Defl inch	0.041 (L/5726)	9'11 1/16" C	).489 (L/480	) 0.084 (8%	5) S	L								
TL Defl inch	0.430 (L/546)	9'11 7/8" C	).651 (L/360	) 0.660 (66	%) D+S	L	]							
Design Not	es													
	oport to prevent la e required at the in				bearings. Late	ral support								
2 Fasten all p	blies using 3 rows	•		•	aximum end di	istance not								
to exceed 6	6". Ist page of calculati	ions for fastana	rs required	for specified	loade									
	designed to be si				ioaus.									
	nust be supported													
	e laterally braced nderness ratio bas			0.C.										
ID	Load Type			Trib Width	Side	Dead 0.9	Live 1	Sno	w 1.15	Wind 1.	6 Const.	1.25	Comments	
1	Uniform				Тор	60 PLF	0 PLF		0 PLF	0 PLI	F	0 PLF	Wall	
2	Uniform				Тор	90 PLF	0 PLF		0 PLF	0 PLI	F	0 PLF	B1GE	
3	Tie-In	0-0-0 to	16-10-0	1-0-0	Тор	20 PSF	0 PSF	:	20 PSF	0 PSI	F (	) PSF	Roof Load	
	Self Weight					11 PLF								
	-													
								1	Monuf4	roy lafe		Corr	tech, Inc.	
Notes Calculated Structured	Designs is responsible on	chemical ly of the <b>Handling</b>	ls I & Installatio	on	<ol><li>For flat pondin</li></ol>	at roofs provide p g	roper drainage to pre	eveni	Manufactu Metsä Woo			Lot 3	tech, Inc. 5 Briarwood Bluff dway, NC	
structural adequacy of design criteria and	of this component based I loadings shown. It	on the 1. LVL bear is the 2. Refer	ms must not be ci to manufacture	ut or drilled r's product info	ormation				301 Merritt Norwalk, C	7 Building, 2	2nd Floor	USA 2831	4	
ensure the compon	customer and/or the contr ient suitability of the i ify the dimensions and load	actor to regarding ntended fastening	g installation g details, beam :	requirements, strength values, a	multi-ply				(800) 622-5	850	10	910-1	864-TRUS	
Lumber		3. Damage	d Beams must no	t be used is laterally restrain	led				www.metsa	woou.com/l	19			
	ions, unless noted otherwis ted with fire retardant or c	5. Provide		at bearing points	to avoid	design is valid	until 11/3/2024						com	тесн
						-								

Ţ	isDesign	Client: We Project: Address:	aver Development	Date: Input by: Job Name Project #:	3/17/2023 David Landry : Lot 3 Holly Place J0223-0918	Page 8 of
GDH	Kerto-S LVL	1.750" X	14.000" 2-Ply	- PASSED	.evel: Level	
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1 SPF E	· · ·	· · ·		• • •	••••••••••••••••••••••••••••••••••••••	
<b></b>			20' 20'			13 1/2"
I			20			
apacity bad eld Limit per eld Mode Ige Distance n. End Dista bad Combina uration Facto	Fastener         81.5           IV         IV           e         1 1/           ance         3"           ation         3"	PLF 6 PLF 9 lb. 2"				
		chemicals	6. For flat roofs pro		Manufacturer Info	Comtech, Inc. Lot 35 Briarwood Bluff Broadway, NC

Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc. Lot 35 Briarwood Bluff
structural adequacy of this component based on the	1. IVI beams must not be cut or drilled	ponding This design is valid until 11/3/2024	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	Broadway, NC USA 28314 910-864-TRUS



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0223-0918 Lot 3 Holly Place

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I57239373 thru I57239380

My license renewal date for the state of North Carolina is December 31, 2023.

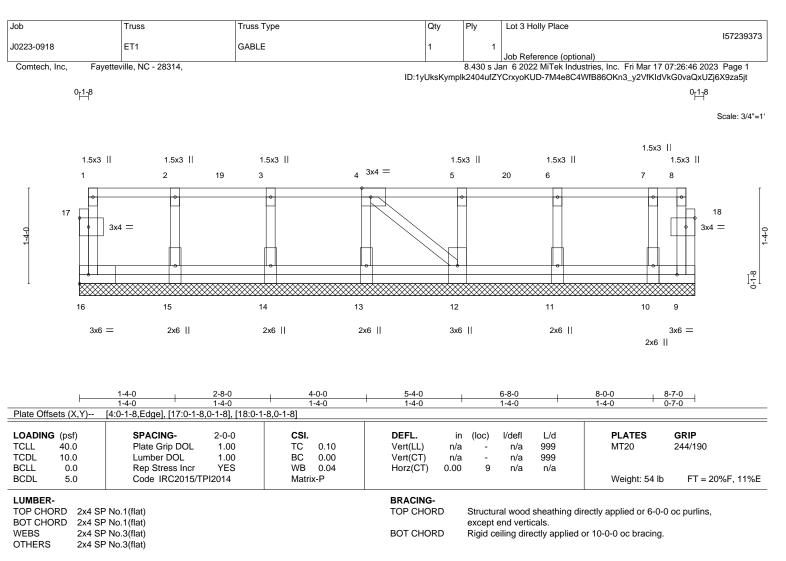
North Carolina COA: C-0844



March 17,2023

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



REACTIONS. All bearings 8-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

#### LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 9-16=-10. 1-8=-100

## Concentrated Loads (lb)

Vert: 4=-71 7=-77 19=-71 20=-71



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A Mi Tek Affiliate 818 Soundside Road

Edenton, NC 27932

Job	Truss		Truss Type		Qty	Ply	Lot 3 Holly Place		15700007
10223-0918	ET2		GABLE		1	1			157239374
							Job Reference (option	nal)	
Comtech, Inc,	Fayetteville, NC	- 28314,							07:26:47 2023 Page 1
					ID:1yUksKympl	k2404ufZY	CrxyoKUD-bYe0LX590	QVGz0UMFYgZkBXr	po8c6e1p4jDSg3cza5js
0 <sub>[1]</sub> 8									0 <sub>[1]</sub> 8
									Scale = 1:18
			4 3x4 =						
1	2	3	4 014 -	5	6		7	8	9 10
20	•	<u> </u>		0	•	-	0	0	
1-4-0									
	•	•	•				•	•	

<u> </u>	2-8-0	4-0-0 1-4-0	5-4-0	6-8-0			-0-0 -4-0		9-4-0 1-4-0		0-8-0 -4-0	+ <u>11-1-0</u> 0-5-0
Plate Offsets (X,Y)-	- [4:0-1-8,Edge], [15:0-1-8	3,Edge]										
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0	<b>SPACING-</b> Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.00 1.00 YES	<b>CSI.</b> TC 0.06 BC 0.01 WB 0.03	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 11	l/defl n/a n/a n/a	L/d 999 999 n/a		PLATES MT20	<b>GRIP</b> 244/190	
BCDL 5.0	Code IRC2015/T	PI2014	Matrix-S							Weight: 54 lb	FT = 2	20%F, 11%l
	SP No.1(flat) SP No.1(flat)	L. L		BRACING- TOP CHORE			ral wood		g directly	applied or 6-0-0	) oc purlins	,

3x4 =

TOP CHORD	2x4 SP No.1(flat)	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.1(flat)		except end verticals.
WEBS	2x4 SP No.3(flat)	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3(flat)		

REACTIONS. All bearings 11-1-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 19, 11, 18, 17, 16, 15, 14, 13, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

3x4 =

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

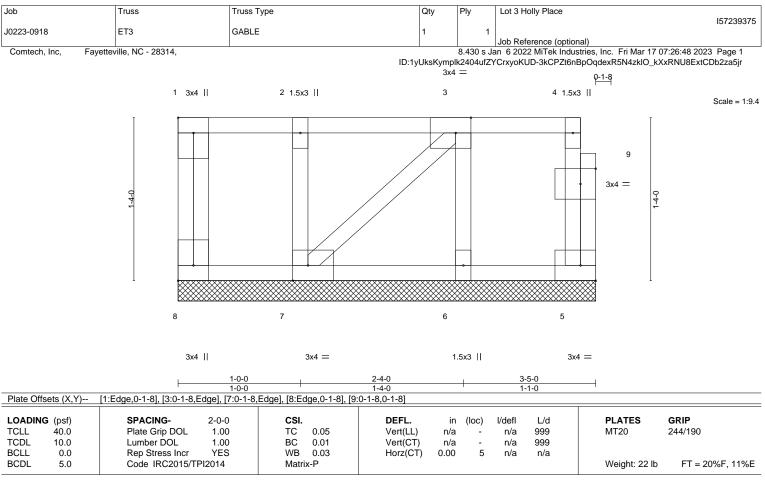
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



3x6 =

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#### LUMBER-

 TOP CHORD
 2x4 SP No.1(flat)

 BOT CHORD
 2x4 SP No.1(flat)

 WEBS
 2x4 SP No.3(flat)

 OTHERS
 2x4 SP No.3(flat)

 
 BRACING 

 TOP CHORD
 Structural wood sheathing directly applied or 3-5-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 3-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

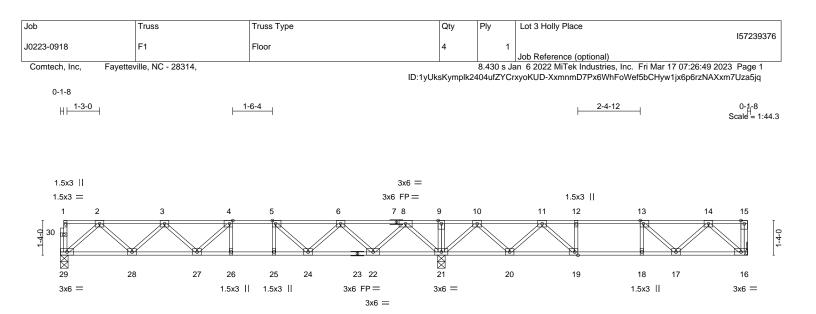
Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



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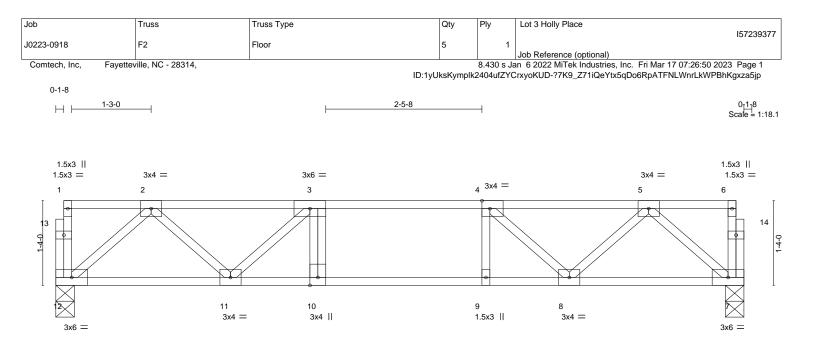


<b> </b>	14-7-12 14-7-12				26-5-0					
Plate Offsets (X,Y)	[4:0-1-8,Edge], [5:0-1-8,Edge], [13:0-1-8	3,Edge], [19:0-1-8,Edge]				11-5	- 7			
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	<b>CSI.</b> TC 0.55 BC 0.76 WB 0.44 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.10 26-27 -0.13 26-27 0.03 16		L/d 480 360 n/a	<b>PLATES</b> MT20 Weight: 136 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E		
BOT CHORD2x4 SPWEBS2x4 SP	P No.1(flat) P No.1(flat) P No.3(flat) e) 29=0-3-8, 16=Mechanical, 21=0-3-8		BRACING TOP CHOP BOT CHOP	RD Structo except	end vertica	ls.	ectly applied or 6-0-0 o	oc purlins,		
Max G FORCES. (lb) - Max. TOP CHORD 15-16 8-9=( 13-14 BOT CHORD 28-22 22-22 17-18 WEBS 2-29= 13-17	Comp./Max. Ten All forces 250 (lb) or 6=-537/0, 2-3=-1257/0, 3-4=-1883/0, 4-5 0/1401, 9-10=0/1401, 10-11=-468/367, 1 4=-884/0 9=0/771, 27-28=0/1717, 26-27=0/2002, 2 4=-34/1340, 21-22=-444/147, 20-21=-63 8=0/1158, 16-17=0/586 8=0/1158, 16-17=0/587, 3-28=-640/0, 8-21 7=-372/90, 10-21=-1064/0, 8-22=0/923, 0 0=0/687, 11-20=-690/0, 11-19=0/582, 12	=1671(LC 1) less except when shown =-2002/0, 5-6=-1658/0, 6 1-12=-1158/0, 12-13=-1 25-26=0/2002, 24-25=0/2 7/19, 19-20=-175/900, 18 =-1289/0, 14-16=-780/0, 6-22=-882/0, 6-24=0/527	i-8=-756/225, 158/0, 2002, 3-19=0/1158, 14-17=0/415,							
<ul> <li>2) All plates are 3x4 M</li> <li>3) Plates checked for a</li> <li>4) Refer to girder(s) for</li> <li>5) Recommend 2x6 str Strongbacks to be a</li> <li>6) CAUTION, Do not e</li> <li>LOAD CASE(S) Stand</li> <li>1) Dead + Floor Live (b Uniform Loads (plf)</li> </ul>	dard palanced): Lumber Increase=1.00, Plate =-10, 1-15=-100 s (lb)	is center. c and fastened to each tr strained by other means.	uss with 3-10d (0.	131" X 3") nail	5.		SEA 0363	NEER RUIN		

March 17,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			10-10-0			
Plate Offsets (X,Y)	[4:0-1-8,Edge]		10-10-0			
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYES	CSI. TC 0.36 BC 0.46 WB 0.21	DEFL. in Vert(LL) -0.07 Vert(CT) -0.09 Horz(CT) 0.01	10 >999 480 10 >999 360	PLATES MT20	<b>GRIP</b> 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 57 lb	FT = 20%F, 11%E
BOT CHORD 2x4 SP	No.1(flat) No.1(flat) No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	2 11	) oc purlins,	
REACTIONS. (size Max G	e) 12=0-3-8, 7=0-3-8 rav 12=576(LC 1), 7=576(LC 1)					

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-923/0, 3-4=-1242/0, 4-5=-925/0

BOT CHORD 11-12=0/606, 10-11=0/1242, 9-10=0/1242, 8-9=0/1242, 7-8=0/603

WEBS 2-12=-805/0, 2-11=0/441, 3-11=-469/0, 5-7=-800/0, 5-8=0/449, 4-8=-475/0

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

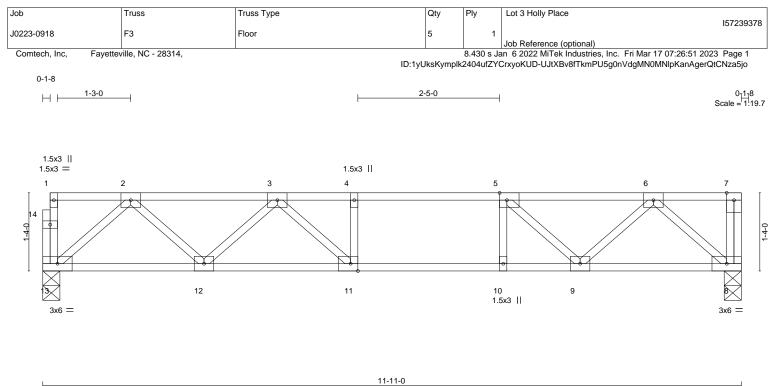
3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



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			11-11-0			
Plate Offsets (X,Y)	[5:0-1-8,Edge], [11:0-1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrNOCode IRC2015/TPI2014	CSI. TC 0.60 BC 0.70 WB 0.27 Matrix-S	Vert(LL) -0.13	in (loc) l/defl L/d 3 11-12 >999 480 6 11-12 >894 360 2 8 n/a n/a	PLATES MT20 Weight: 62 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	) oc purlins,	

REACTIONS.	(size)	13=0-3-8, 8=0-3-8				
	Max Grav	13=635(LC 1), 8=1142(LC 1)				

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 7-8=-548/0, 2-3=-1060/0, 3-4=-1495/0, 4-5=-1495/0, 5-6=-1059/0

BOT CHORD 12-13=0/676, 11-12=0/1395, 10-11=0/1495, 9-10=0/1495, 8-9=0/659

WEBS 2-13=-898/0, 2-12=0/534, 3-12=-466/0, 3-11=-19/356, 6-8=-877/0, 6-9=0/557, 5-9=-616/0

### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

### LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

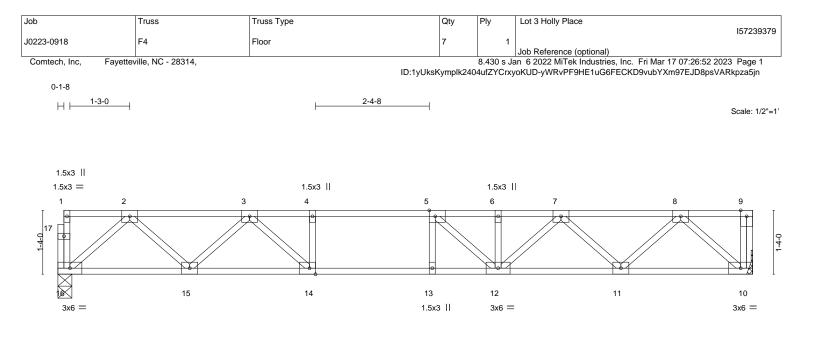
Vert: 8-13=-10, 1-7=-100

Concentrated Loads (lb) Vert: 7=-500



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			<u>14-6-0</u> 14-6-0			I
Plate Offsets (X,Y)	[5:0-1-8,Edge], [14:0-1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.62 BC 0.78 WB 0.35 Matrix-S	Vert(LL) -0.1	n (loc) l/defl L/d 7 12-13 >999 480 2 12-13 >790 360 3 10 n/a n/a	<b>PLATES</b> MT20 Weight: 76 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	,	oc purlins,	

#### REACTIONS. (size) 16=0-3-8, 10=Mechanical

Max Grav 16=778(LC 1), 10=784(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1348/0, 3-4=-2244/0, 4-5=-2244/0, 5-6=-2152/0, 6-7=-2152/0, 7-8=-1359/0 BOT CHORD

15-16=0/834, 14-15=0/1856, 13-14=0/2244, 12-13=0/2244, 11-12=0/1857, 10-11=0/835 WEBS

2-16=-1109/0, 2-15=0/714, 3-15=-707/0, 3-14=0/697, 4-14=-339/0, 8-10=-1111/0,

8-11=0/729, 7-11=-693/0, 7-12=0/401, 5-12=-438/123

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

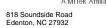
5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

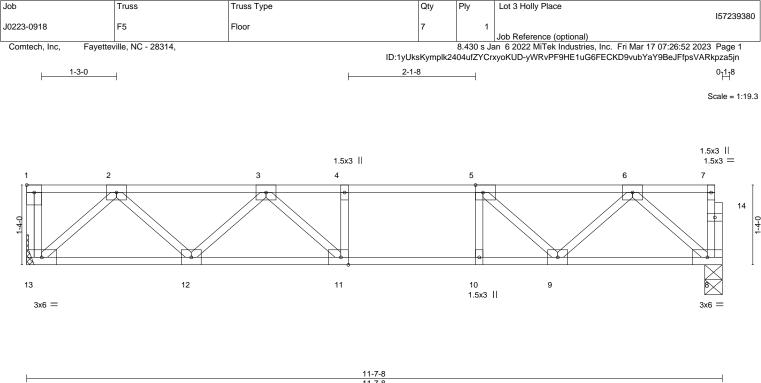
Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



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I			11-7-8			I
Plate Offsets (X,Y)	[1:Edge,0-1-8], [5:0-1-8,Edge], [11:0-1-8	3,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 DCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.44 BC 0.56 WB 0.25	Vert(LL) -0.1	in (loc) l/defl L/d 0 11-12 >999 480 3 11-12 >999 360 2 8 n/a n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S			Weight: 61 lb	FT = 20%F, 11%E
BOT CHORD 2x4 SI	P No.1(flat) P No.1(flat) P No.3(flat)	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o		) oc purlins,	

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 13=Mechanical, 8=0-3-8 Max Grav 13=626(LC 1), 8=619(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1025/0, 3-4=-1427/0, 4-5=-1427/0, 5-6=-1022/0

BOT CHORD 12-13=0/658, 11-12=0/1345, 10-11=0/1427, 9-10=0/1427, 8-9=0/644

WEBS 2-13=-876/0, 2-12=0/510, 3-12=-446/0, 3-11=-41/323, 6-8=-854/0, 6-9=0/526,

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

5-9=-570/0



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