# PLANS DESIGNED TO THE 2018 NORTH CAROLINA STATE **RESIDENTIAL BUILDING CODE.**



PLATE HGT.

COVERED-

PORCH





TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE





TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE





TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE





# STRUCTURAL NOTES:

- 1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- INSTALL AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
   WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 5. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
- 6. ALL 4 X 4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES (OR EQUAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4 AND 6 X 6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS AT TOP (UNO.)
- FOR FIBERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/ (2) METAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/ 1/4" THROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE SIDES OF COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING COLUMN.

# **BRACE WALL PANEL NOTES:**

**EXTERIOR WALLS:** ALL EXTERIOR WALLS TO BE SHEALTHED WITH CS-WSP OR CS-SFB IN ACCORDANCE WITH SECTION R602.10.3 UNLESS NOTED OTHERWISE.

**REQUIRED LENGTH OF BRACING:** REQUIRED BRACE WALL LENGTH FOR EACH SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE INTERPOLATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB CONTRIBUTE THIER ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5 ITS ACTUAL LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS ACTUAL LENGTH.

**GYPSUM:** ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS TO HAVE 1/2" GYPSUM INSTALLED. WHEN NOT USING METHOD GB GYPSUM TO BE FASTENED PER TABLE R702.3.5. METHOD GB TO BE FASTENED PER TABLE R602.10.1.

**HD:** 800 LBS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE BRACE WALL PANEL NEAREST TO THE CORNER

METHODS: PER TABLE R602.10.1





DATE: FEBRUARY 19, 2021
REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:
SECOND FLOOR
FRAMING PLAN

S-2

SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE



## TABLE R602.7.5 MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (FEFT)	MAXIMUM STUD : (PER TABL	SPACING (INCHES) E R602.3(5)							
()	16	24							
UP TO 3'	1	1							
4'	2	1							
8'	3	2							
12'	5	3							
16'	6	4							
1									

# STRUCTURAL NOTES:

- ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
- 2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
- 3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
- 4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)
  - DSP DOUBLE STUD POCKET TSP - TRIPLE STUD POCKET



RESIDENTIAL DESIGN, INC. RALEIGH, NC 27612 (919) 649-4128 WWW.RRDCAROLINA.COM The art of transforming your vision into reality.



DITE. ILDICOINCI 17, 202
--------------------------

REV.:	
SCALE: 1/4" = 1'-0"	
DRAWN BY: WG	
ENGINEERED BY:	
REVIEWED BY:	

ATTIC FLOOR FRAMING PLAN

S-3

<u>SCALE NOTE:</u> 18x24 PRINTS ARE TO SCALE AS NOTED. **11x17 PRINTS ARE NOT TO SCALE** 





1302 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 8.7 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

## STRUCTURAL NOTES:

- ALL FRAMING LUMBER TO BE #2 SPF (UNO).
   HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
- 3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
- FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
- 5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.



SCALE NOTE: 18x24 PRINTS ARE TO SCALE AS NOTED. 11x17 PRINTS ARE NOT TO SCALE





				TI	RO RO RUS eilly R Fayet Phon	OF & SES	<b>Te</b> <b>&amp; FL</b> <b>&amp; B</b> dustr , N.C. 0) 864	CH OOI EAN 28309 -8787	R R VS rk
	4	8. 0.		Bearing deemed requirent attached requirent size and reactions Tables. / retained reactions Signatur	Fax reactions to compl ients. Thu Tables ( ients ) to number s greater A register to design that excc A register to design s that exc e	: (910) I less that y with the e contract derived f determin of wood a than 3000 red design of the sup- seed 1500 Davi Davi ART FC	864-4 n or equa e prescrip- tor shall for shall tor shall tor shall be the mini- studs req be but no n profess port syste off. d La DR JAG	I to 3000 trive Cod refer to the prescripti imum fo uired to a t greater sional sha em for al em for al em for al cod mdry CK STI	# are e 10 ve Cod undatio support than all be iV attache iII be cy
				NUM NCLOPEN (CL 40) 1700 3400 5100 6800 8500 10200 11900 13600 15300	(BASEL BER OF J/ BER OF J/ BER OF J/ BER OF J/ BER OF J/ BER OF J/ BUSSEL BER OF J/ BUSSEL BER OF J/ BUSSEL BER OF J/ BER OF J	o ON TABLI CK STUDS HEADER 7 2550 5100 7650 10200 12750	ES R502.5( REQUIREI /GIRDER D 4000000000000000000000000000000000000	1) & (b)) 0 @ EA EN 10 2 10 4 (b) 10 2 10 4 (b) 10 2 13 6 17 0 10 2 13 6 17 0 10 2 13 6 17 0 10 2 10 2 10 10	x o F         00 squts q, bay q           (01 an)         12           x 00 squts q, bay q         1           x 00 squts q, bay q         1
A5GE 1'4"		32' 6"	Dimension Notes 1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of frame wall unless noted otherwise 3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise All Walls Shown Are Considered Load Bearing Hatch Legend Red Area - 1602 Ch as fa	Sanford / Harnett	172 Colby Farm Drive	Roof	03/17/23	David Landry	Lenny Norris
			Roor Area = 1092.08 Sq.rt.         Ridge Line = 52.07 ft.         Hip Line = 0 ft.         Horiz. OH = 115.69 ft.         Raked OH = 175.3 ft.         Decking = 58 sheets         Drop Beam         Sym         Product       Manuf         Qty       Supported Member         Header       Truss         HUS26       USP         USP       12         NA       16d/3-1/2"         16d/3-1/2"	<b>СІТУ / СО</b> .	ADDRESS	MODEL	DATE REV.	DRAWN BY	SALES REP.
M1GE	BBO	<b>5.</b> 0	THDH210-3       USP       1       Varies       16d/3-1/2"       16d/3-1/2"         Products       Product       Plies       Net Qty         BM1       12' 0"       1-3/4"x 16" LVL Kerto-S       2       2         BM2       15' 0"       1-3/4"x 16" LVL Kerto-S       2       2         BM3       12' 0"       2x10 SPF No.2       2       2         GDH       20' 0"       1-3/4"x 14" LVL Kerto-S       2       2	Weaver Development Co. Inc.	Lot 3 Holly Place	Hickory II "A" / 2GRF, CP	N/A		J0223-0917
	<del>-                                    </del>			BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #

Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



						Fearing	ROC RUS RUS RUS Reilly F Fayet Phon Fax	OF 8 SES load Ir teville ie: (91( : (910)	<b>TC</b> <b>&amp; FL</b> <b>&amp; B</b> ndustr (, N.C )) 864 864-4	CH OOF EAN ial Par 28309 -8787 1444	k
	•	·				deemed require attache require	reactions to compl nents. The 1 Tables ( nents ) to	y with the contract derived f determin	or equal prescrip or shall r rom the p e the min	tive Code efer to the prescriptiv	e ve Code indation
		.0				reaction 15000#. retained reaction Tables. retained reaction	s greater A registe to design that exce A registe to design is that exc	than 3000 red design the supp eds those ed design the supp ceed 1500	ditus requ i# but not n profess port syste e specifie n professi port syste 0#.	in equation of a second	han I be / ttached I be
		8				Jighatu	Te	Davi	d Lai	ndry	<u> </u>
							AD CHI (BASEL WBER OF JI	ON TABLE	)R JAC S R502.5(1 REQUIRED /GIRDER	CK STU ) & (b)) ) @ EA END	DS
		·				NCHUSTAN (QL 40) 1700 3400 5100 6800 8500	<b>G b c b r c c c d b r d b r b b c c d c d b r d b r d b r d b r d d b r d d d r d d d r d d d d d d d d d d</b>	No LLOY 2550 2550 5100 7650 10200 12750	<b>7 1 1 1 1 1 1 1 1 1 1</b>	340 1020 1360 1700	0         0
		16' 4"				10200 11900 13600 15300	6 7 8 9		) 6		
			1. All ex face of s 2. All int frame w 3. All ex face of f	Dimension Notes erior wall to wall dimensions are to the thing unless noted otherwise rior wall dimensions are to face o Il unless noted otherwise erior wall to truss dimensions are ame wall unless noted otherwise	o f to	ett	Drive				
	<b></b>		All V Consid	/alls Shown Are ered Load Bear	ing	/ Harne	y Farm		ß	Indry	orris
F			Roof Area = 1692.08 sq.ft.		Hatch Legend Padded HVAC	anford	72 Colb	toof	3/17/2	avid La	enny No
			Ridge Line = $52.07$ ft. Hip Line = 0 ft. Horiz. OH = $115.69$ ft. Raked OH = $175.3$ ft. Decking = $58$ sheets		2nd Floor Walls Tray Ceiling	0.	SS 1		EV.	1 BY	REP. L
			Connector Infe	rmation	Drop Beam		ADDRE	MODEL	DATE R	DRAWN	SALES
		21.2"	Sym Product Manuf	Qty Supported Member H	Header Truss						
- C			3         THDH210-3         USP	1         Varies         16	id/3-1/2" 16d/3-1/2"						
-			PlotID Length Pro	Products uct	Plies Net Qty	o. Inc.		Ъ			
s	€		BM1 12' 0" 1-3/ BM2 15' 0" 1-3/ BM3 12' 0" 2x1	I"x 16" LVL Kerto-S I"x 16" LVL Kerto-S SPF No.2	2 2 2 2 2 2	ment Co		/ 26RF,			
	_		GDH 20'0" 1-3/	"x 14" LVL Kerto-S Placement Plan	2 2	evelopt	y Place	1 "A" /			17
			Scale:	/4"=1'	-	/eaver D	ot 3 Holl	ickory I.	1/A		0223-05
						~	WE L	I	ATE N	#	<u>-</u>
1						BUILDE	JOB NA	PLAN	SEAL D	QUOTE	JOB #



THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0223-0917 Lot 3 Holly Place

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I57239345 thru I57239372

My license renewal date for the state of North Carolina is December 31, 2023.

North Carolina COA: C-0844



March 17,2023

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



F	10-2-8 10-2-8	2	<u>2-2-8</u> 2-0-0	-	<u>32-5-0</u> 10-2-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.28 BC 0.64 WB 0.27 Matrix-S	DEFL.         in (loc)           Vert(LL)         -0.34 10-13           Vert(CT)         -0.47 10-13           Horz(CT)         0.05 8           Wind(LL)         0.05 2-13	l/defl L/d >999 360 >824 240 n/a n/a >999 240	PLATES MT20 Weight: 208 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-110(LC 10) Max Uplift 2=-89(LC 12), 8=-89(

Max Uplift 2=-89(LC 12), 8=-89(LC 13) Max Grav 2=1337(LC 1), 8=1337(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2307/486, 3-5=-2125/534, 5-7=-2125/534, 7-8=-2307/486

BOT CHORD 2-13=-316/2007, 10-13=-106/1303, 8-10=-320/1964

WEBS 5-10=-147/921, 7-10=-454/288, 5-13=-147/921, 3-13=-454/288

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2 and 89 lb uplift at joint 8.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 4-11-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





March 17,2023





Plate Offsets (X,Y)	10-2-8 10-2-8 [2:0-4-0,0-2-14], [6:0-3-0,Edge], [10:0-4-	16-2-8 6-0-0 0,Edge], [12:0-3-4,Edge],	<u>19-2-8</u> 21-1-4 <u>3-0-0</u> 1-11-4 [14:0-4-8,0-1-4], [15:0-3	3 24 0 3- -8,0-4-1	-2-8 1-0 2]	30-1-8 5-11-0	<u>32-5-0</u> 2-3-8	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.75 BC 0.67 WB 0.70 Matrix-S	DEFL.         in           Vert(LL)         -0.21           Vert(CT)         -0.38           Horz(CT)         0.09           Wind(LL)         0.17	(loc) 17 17 10 2-17	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	<b>PLATES</b> MT20 Weight: 247 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 S BOT CHORD 2x10 S 10-15 WEBS 2x4 S	BRACING- TOP CHORD BOT CHORD WEBS	Structu Rigid co 1 Row	ral wood eiling dire at midpt	sheathing dired ectly applied or 5-7	ctly applied or 4-0-8 o 10-0-0 oc bracing.	oc purlins.		

REACTIONS. (size) 2=0-3-8, 10=0-3-8 Max Horz 2=110(LC 11)

Max Uplift 2=-90(LC 12), 10=-90(LC 13) Max Grav 2=1393(LC 2), 10=1353(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-2217/403, 4-5=-1870/483, 7-9=-2258/519, 9-10=-2889/551

BOT CHORD 2-17=-193/1848, 15-17=-195/1860, 13-15=-371/2525, 10-13=-380/2525

WEBS 4-17=-29/402, 7-15=-114/967, 9-15=-1075/232, 9-13=0/616, 5-7=-1955/459

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







1	9-11-8	18-11-8	20-10-8	23-11-8	29-10-8	32-2-0	1
Г	9-11-8	9-0-0	1-11-0	3-1-0	5-11-0	2-3-8	I
Plate Offsets (X,Y)	[1:0-9-6,0-1-2], [5:0-3-0,Edge], [9:0-4-0,E	dge], [11:0-3-4,Edge], [	13:0-4-12,0-1-4], [14:0-3-8	8,0-4-12]			

LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	<b>CSI.</b> TC 0.76 BC 0.66 WB 0.70 Matrix-S	DEFL.         in           Vert(LL)         -0.21           Vert(CT)         -0.36           Horz(CT)         0.09           Wind(LL)         0.16	(loc) l/de 16 >99 16 >99 9 n/ 1-16 >99	efl L/d 99 360 99 240 /a n/a 99 240	<b>PLATES</b> MT20 Weight: 243 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP	No.1		BRACING- TOP CHORD	Structural we	ood sheathing dir	ectly applied or 3-10-0 o	c purlins.

BOT CHORD

WEBS

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

4-6

 BOT CHORD
 2x6 SP No.1

 BOT CHORD
 2x10 SP No.1 \*Except\*

 9-14:
 2x6 SP 2400F 2.0E

 WEBS
 2x4 SP No.2

REACTIONS. (size) 1=Mechanical, 9=0-3-8 Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 9=-90(LC 13)

Max Grav 1=1345(LC 2), 9=1347(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-3=-2189/401, 3-4=-1853/491, 6-8=-2237/517, 8-9=-2874/549

BOT CHORD 1-16=-198/1827, 14-16=-200/1839, 12-14=-375/2511, 9-12=-384/2511

WEBS 6-14=-117/966, 3-16=-53/392, 4-6=-1931/474, 8-14=-1081/228, 8-12=0/620

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.







L	9-11-8		21-11-8	32	2-2-0	
I	9-11-8	I	12-0-0	10	)-2-8	1
Plate Offsets (X,Y) [1:0-	1-14,0-1-8]					
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.28 BC 0.64 WB 0.27 Matrix-S	DEFL.         in         (loc)           Vert(LL)         -0.34         9-12           Vert(CT)         -0.47         9-12           Horz(CT)         0.05         7           Wind(LL)         0.05         12	l/defl L/d >999 360 >822 240 n/a n/a >999 240	PLATES MT20 Weight: 204 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 1=Mechanical, 7=0-3-8 Max Horz 1=-111(LC 8) Max Uplift 1=-76(LC 12), 7=-89(LC 13)

Max Grav 1=1278(LC 1), 7=1331(LC 1)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 1-2=-2276/496, 2-4=-2096/546, 4-6=-2113/532, 6-7=-2294/484

BOT CHORD 1-12=-319/1973, 9-12=-109/1291, 7-9=-324/1953

WEBS 4-9=-147/922, 6-9=-454/288, 4-12=-144/897, 2-12=-437/286

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-12 to 4-5-9, Interior(1) 4-5-9 to 15-11-8, Exterior(2) 15-11-8 to 20-4-5, Interior(1) 20-4-5 to 32-10-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



Structural wood sheathing directly applied or 4-11-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





	<u>10-2-8</u> 10-2-8	<u>22-2-8</u> 12-0-0		+ <u>3</u> : 10	2-5-0 0-2-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI.         DEFL.           TC         0.29         Vert(LL)           BC         0.65         Vert(CT)           WB         0.27         Horz(CT)           Matrix-S         Wind(LL)	in (loc) -0.34 9-12 -0.47 9-12 0.05 7 0.05 12	l/defl L/d >999 360 >822 240 n/a n/a >999 240	PLATES MT20 Weight: 206 lb	<b>GRIP</b> 244/190 FT = 20%

## LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 7=0-3-8 Max Horz 1=-111(LC 10) Max Uplift 1=-77(LC 12), 7=-89(LC 13) Max Grav 1=1284(LC 1), 7=1337(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-2310/503, 2-4=-2129/551, 4-6=-2126/535, 6-7=-2308/487

BOT CHORD 1-12=-327/2012, 9-12=-111/1304, 7-9=-326/1966

WEBS 4-9=-147/921, 6-9=-454/288, 4-12=-149/924, 2-12=-458/292

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-2-8, Exterior(2) 16-2-8 to 20-7-5, Interior(1) 20-7-5 to 33-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-10-13 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 30, 31, 33, 34, 35, 36, 28, 27, 25, 24, 23, 22, 21 except (jt=lb) 37=101.



818 Soundside Road Edenton, NC 27932



4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



2) White ASCE 7-10; Vitie 130mph Vasde 103mph, TCDL=6.0pst, BCDL=6.0pst, n=15it, Cat. II, Exp C, Enclosed, MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 19, 20, 21, 16, 15, 14, 11 except (jt=lb) 22=116, 13=103.

![](_page_22_Picture_8.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

![](_page_23_Figure_0.jpeg)

		7-7-8	1	
LOADING (psf)         SPACING-         2-0-0           TCLL         20.0         Plate Grip DOL         1.15           TCDL         10.0         Lumber DOL         1.15           BCLL         0.0 *         Rep Stress Incr         YES           BCDL         10.0         Code IRC2015/TPI2014         100	<b>CSI.</b> TC 0.33 BC 0.21 WB 0.00 Matrix-P	DEFL.         in         (loc)           Vert(LL)         -0.04         2-6         2-6           Vert(CT)         -0.07         2-6         2-6           Horz(CT)         0.00         2	l/defl L/d >999 360 >999 240 n/a n/a **** 240	PLATES         GRIP           MT20         244/190           Weight: 48 lb         FT = 20%

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

2x4 SP No.2

REACTIONS. 6=Mechanical, 2=0-3-8 (size) Max Horz 2=158(LC 12) Max Uplift 6=-82(LC 12)

Max Grav 6=318(LC 19), 2=345(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 3-6=-288/220

### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-0 to 3-7-13, Interior(1) 3-7-13 to 7-7-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6.

![](_page_23_Picture_14.jpeg)

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

![](_page_23_Picture_16.jpeg)

![](_page_24_Figure_0.jpeg)

LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.03 BC 0.01 WB 0.03 Matrix-P	DEFL. in Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) -0.00	(loc) 1 1 7	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES         GRIP           MT20         244/190           Weight: 57 lb         FT = 20%
							•

## LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2
OTHERS	2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 7-7-8.

Max Horz 2=228(LC 12) (lb) -

2-3=-269/227

Max Uplift All uplift 100 lb or less at joint(s) 7, 8, 9, 10 except 11=-117(LC 12) Max Grav All reactions 250 lb or less at joint(s) 7, 8, 2, 9, 10, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 8, 9, 10 except (jt=lb) 11=117.

![](_page_24_Figure_18.jpeg)

![](_page_24_Picture_20.jpeg)

![](_page_25_Figure_0.jpeg)

			8-3-8				1
			8-3-8				I
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [6:0-2-0,0-1-8]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.84 BC 0.24 WB 0.00 Matrix-P	DEFL.         in           Vert(LL)         -0.05           Vert(CT)         -0.10           Horz(CT)         0.00           Wind(LL)         0.10	(loc) l/d 2-5 >9 2-5 >9 5 r 2-5 >8	lefi L/d 99 360 69 240 n/a n/a 86 240	PLATES MT20 Weight: 37 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF	P No.1 P No.1 P No.2		BRACING- TOP CHORD BOT CHORD	Structural w except end Rigid ceiling	vood sheathing dire verticals. g directly applied o	ectly applied or 5-3-4 r 10-0-0 oc bracing.	oc purlins,

REACTIONS. (size) 2=0-3-0, 5=0-3-8 Max Horz 2=74(LC 8) Max Uplift 2=-150(LC 8), 5=-127(LC 8) Max Grav 2=375(LC 1), 5=314(LC 1)

2x6 SP No.1

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

OTHERS

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 8-0-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 5=127.

![](_page_25_Picture_9.jpeg)

![](_page_25_Picture_11.jpeg)

![](_page_26_Figure_0.jpeg)

			8-3-8		
			8-3-8		
Plate Offsets (X,Y)	[2:0-2-14,0-0-6], [8:Edge,0-2-0], [12:0-2	-0,0-1-8]			
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.35 BC 0.26 WB 0.01 Matrix-S	<b>DEFL.</b> ir Vert(LL) 0.09 Vert(CT) -0.08 Horz(CT) -0.00	n (loc) I/defl L/d 10-11 >999 240 10-11 >999 240 10-11 >999 240 8 n/a n/a	PLATES         GRIP           MT20         244/190           M18AHS         186/179           Weight: 41 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF	P No.1 P No.1 P No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 oc purlins, r 10-0-0 oc bracing.

OTHERS 2x4 SP No.2 \*Except\* 8-12: 2x6 SP No.1

REACTIONS. (size) 2=0-3-0, 8=0-3-8 Max Horz 2=105(LC 8)

Max Uplift 2=-216(LC 8), 8=-188(LC 8) Max Grav 2=375(LC 1), 8=314(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD 2-11=-284/207, 10-11=-284/207, 9-10=-284/207, 8-9=-284/207

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) All plates are MT20 plates unless otherwise indicated.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216, 8=188.

![](_page_26_Picture_15.jpeg)

![](_page_26_Picture_17.jpeg)

![](_page_27_Figure_0.jpeg)

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1261 lb down and 93 lb up at 0-9-12, 1258 lb down and 96 lb up at 2-9-12, and 1325 lb down and 96 lb up at 4-9-12, and 1325 lb down and 96 lb up at 6-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 1-3=-20

## Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_27_Picture_7.jpeg)

Edenton, NC 27932

G mmm March 17,2023

036322

Job	Truss	Truss Type	Qty	Ply	Lot 3 Holly Place	
					15	7239359
J0223-0917	D1-GR	Common Girder	1	2		
				~	Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,			8.430 s Ja	an 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:14 2023 Pa	age 2

ID:1yUksKymplk2404ufZYCrxyoKUD-Erkv0CUX1a9JRaDlfLuYhkTMz0hmT9?3wwVbWYza5jR

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 5=-1261(B) 6=-1258(B) 7=-1258(B) 8=-1258(B)

![](_page_28_Picture_4.jpeg)

![](_page_29_Figure_0.jpeg)

## REACTIONS. All bearings 7-11-0.

(lb) - Max Horz 2=-118(LC 10)

Max Uplit All uplift 100 lb or less at joint(s) 2, 6 except 10=-152(LC 12), 8=-148(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=152, 8=148.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_29_Picture_15.jpeg)

ENGINEERING BY EREPACED A MITEK Affiliate 818 Soundside Road Edenton, NC 27932

![](_page_30_Figure_0.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-74/3 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 3 Holly Place	
						157239361
J0223-0917	G1-GR	FLAT GIRDER	1	3		
				v	Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,			8.430 s Ja	an 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:16 2023	Page 2

ID:1yUksKymplk2404ufZYCrxyoKUD-ADsfRuVnZBQ1guNhmlw0n9YkpqTKx?XLNE\_ibQza5jP

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-7=-60, 7-8=-60, 9-16=-20

Concentrated Loads (lb)

Vert: 12=-276(F) 14=-255(B) 13=-291(B) 17=-5000(B) 18=-276(F) 19=-255(B) 20=-276(F) 21=-276(F) 22=-255(B) 23=-276(F) 24=-291(B) 25=-291(B) 26=-276(F) 27=-291(B) 28=-276(F) 29=-291(B) 30=-276(F) 30=-

![](_page_31_Picture_7.jpeg)

![](_page_32_Figure_0.jpeg)

		F		5-0-0	
				000	
LOADING	G (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L	/d PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.28	Vert(LL) -0.01 2-4 >999 36	30 MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.08	Vert(CT) -0.01 2-4 >999 24	40
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 n/a n	la
BCDL	10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.01 2-4 >999 24	40 Weight: 24 lb FT = 20%

BRACING-

## LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=63(LC 8) Max Uplift 2=-102(LC 8), 4=-79(LC 8) Max Grav 2=255(LC 1), 4=179(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-0 to 3-5-13, Interior(1) 3-5-13 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=102.

![](_page_32_Picture_16.jpeg)

![](_page_32_Picture_18.jpeg)

![](_page_33_Figure_0.jpeg)

		1		5-0-0	
LOADING TCLL	(psf) 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.09	<b>DEFL.</b> in (loc) //defl L/d Vert(LL) 0.01 8 >999 240	PLATES         GRIP           MT20         244/190
BCLL BCDL	10.0 0.0 * 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.02 Matrix-S	Ven(C1) -0.01 8 >999 240 Horz(CT) -0.00 6 n/a n/a	Weight: 27 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No.2

**REACTIONS.** (size) 2=0-3-0, 6=0-1-8

Max Horz 2=90(LC 8) Max Uplift 2=-147(LC 8), 6=-115(LC 8) Max Grav 2=255(LC 1), 6=179(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=147, 6=115.

![](_page_33_Picture_16.jpeg)

Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

![](_page_33_Picture_18.jpeg)

![](_page_34_Figure_0.jpeg)

## Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Lot 3 Holly Place	
			-	-		157239364
J0223-0917	M2	Half Hip	3	1		
					Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,			8.430 s Ja	an 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:20 2023	Page 2
		ID:1	yUksKymp	lk2404ufZ	YCrxyoKUD-2_6AHGZHcQwS9VgS?b?yx?jPeRpIsyVxIryv	kBza5jL

	Concentrated Loads (lb)
	Vert: 9=-300
4)	Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
	Vert 1-2=70 2-3=58 3-4=153 5-6=12 2-8=52 8-10=115 7-10=52
	Horz: 1-2=-82, 2-3=-70, 3-4=-165, 3-5=-55
	Concentrated Loads (lb)
	Vert: 9=548
5)	Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (pit)
	Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55
	Concentrated Loads (lb)
	Vert: 9=566
6)	Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Ven: 1-2=-1, 2-3=-40, 3-4=17, 5-6=-56, 2-6=-9, 6-10=2, 7-10=-9 Horz: 1-219, 2-3-25, 3-437, 3-5-51
	Concentrated Loads (lb)
	Vert: 9=-420
7)	Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Veil: 1-2=-39, 2-3=-45, 3-4=-39, 5-6=-56, 2-6=-9, 6-10=2, 7-10=-9 Horz: 1-2-19, 2-3-25, 3-4-19, 3-5-51
	Concentrated Loads (lb)
	Vert: 9=-420
8)	Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Ven: 1-2=30, 2-3=21, 3-4=14, 5-6=-11, 2-8=10, 8-10=33, 7-10=10 Horz: 1-248, 2-333, 3-426, 3-5-7
	Concentrated Loads (lb)
	Vert: 9=154
9)	Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vent. 1-2=0, 2-3=12, 3-4=20, 3-0=1, 2-7=-12 Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27
	Concentrated Loads (lb)
	Vert: 9=43
10	) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (pit)
	Horz: 1-2=-0, 2-3=-19, 3-4=-26, 3-5=34
	Concentrated Loads (lb)
	Vert: 9=-339
11	) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Horz: 1-2=-2, 2-3=-3, 3-4=-2, 3-6=-21, 2-7=-20 Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0
	Concentrated Loads (lb)
	Vert: 9=-234
12	) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (pit) Vert: 1-2-14, 2-3-21, 3-4-14, 5-611, 2-712
	Horz: 1-2=-26. 2-3=-33. 3-4=-26. 3-5=-39
	Concentrated Loads (lb)
	Vert: 9=43
13	) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Vert: 1-2=2 2-3=9 3-4=2 5-6=1 2-7=-12
	Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27
	Concentrated Loads (lb)
	Vert: 9=43
14	) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-11, 2-7=-12
	Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39
	Concentrated Loads (lb)
15	Vert: 9=43
15	ו שבמע + סוס אוויעראס איוחט (רטג. וחנפרוומו) אנח רמרמוופו: Lumber Increase=1.60, Plate Increase=1.60

3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25

Vert: 1-3=-20, 3-4=-20, 5-6=-40, 2-7=-40

## Continued on page 3

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 9=-350

Uniform Loads (plf)

![](_page_35_Picture_4.jpeg)

			-			
Job	Truss	Truss Type	Qty	Ply	Lot 3 Holly Place	
						157239364
J0223-0917	M2	Half Hip	3	1		
					Job Reference (optional)	
Comtech, Inc, Fayette	rille, NC - 28314,			8.430 s Ja	an 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:20 2023	Page 3

ID:1yUksKymplk2404ufZYCrxyoKUD-2\_6AHGZHcQwS9VgS?b?yx?jPeRplsyVxIryvkBza5jL

LOAD CASE(S) Standard Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=1, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-33, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-21, 2-7=-20 Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-6=-120, 2-7=-20 Concentrated Loads (lb) Vert: 9=-200 19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-8=-3, 8-10=13, 7-10=-3 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26 Concentrated Loads (lb) Vert: 9=-454 20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-86, 6-9=-116, 2-7=-20 Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0 Concentrated Loads (lb) Vert: 9=-375 21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-95, 6-9=-125, 2-7=-20 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9 Concentrated Loads (lb) Vert: 9=-375 22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-86, 6-9=-116, 2-7=-20 Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0 Concentrated Loads (lb) Vert: 9=-375 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-6=-40, 2-7=-20 Concentrated Loads (lb) Vert: 9=-400 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-40, 6-9=-80, 2-7=-20 Concentrated Loads (lb) Vert: 9=-400 25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 5-6=-100, 2-7=-20 Concentrated Loads (lb) Vert: 9=-350 26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-100, 6-9=-130, 2-7=-20 Concentrated Loads (lb) Vert: 9=-350

![](_page_36_Picture_3.jpeg)

![](_page_37_Figure_0.jpeg)

Vert: 1-3=-60, 3-4=-60, 5-9=-160, 6-9=-200, 2-7=-20

#### Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

March 17,2023

Job	Truss	Truss Type	Qty	Ply	Lot 3 Holly Place	
					15	7239365
J0223-0917	M2-GR	HALF HIP	1	2		
				<b>_</b>	Job Reference (optional)	
Comtech, Inc, Fayette	/ille, NC - 28314,			8.430 s Ja	an 6 2022 MiTek Industries, Inc. Fri Mar 17 07:27:21 2023 Pa	age 2

	ID:1yUksKymplk2404ufZYCrxyoKUD-WBgYUcZwNk2JnfFeZJWBUCFcIrA2bPl4XViTGeza5jK
LOAD CASE(S) Standard	
Concentrated Loads (lb)	
<ol> <li>2) Dead + 0.75 Roof Live (balanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.</li> </ol>	15
Uniform Loads (plf)	
Vert: 1-3=-50, 3-4=-50, 5-9=-220, 6-9=-250, 2-7=-20 Concentrated Loads (lb)	
Vert: 9=-350	
3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25	
Vert: 1-3=-20, 3-4=-20, 5-6=-160, 2-7=-40	
Concentrated Loads (Ib)	
4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60. Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=70, 2-3=58, 3-4=153, 5-6=-108, 2-8=52, 8-10=115, 7-10=52 Horz: 1-282, 2-370, 3-4165, 3-555	
Concentrated Loads (lb)	
Vert: 9=548	
5) Dead + 0.6 C-C wind (Pos. Internal) Case 2: Lumber increase=1.60, Plate increase=1.60 Uniform Loads (plf)	
Vert: 1-2=51, 2-3=58, 3-4=51, 5-6=-78, 2-8=52, 8-10=115, 7-10=52	
Horz: 1-2=-63, 2-3=-70, 3-4=-63, 3-5=-55 Concentrated Loads (Ib)	
Vert: 9=566	
6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (pir) Vert: 1-2=-1. 2-3=-45. 3-4=17. 5-6=-178. 2-8=-9. 8-10=2. 7-10=-9	
Horz: 1-2=-19, 2-3=25, 3-4=-37, 3-5=51	
Concentrated Loads (lb) Vert <sup>.</sup> 9=-420	
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Horz: 1-2=19, 2-3=25, 3-4=19, 3-5=51	
Concentrated Loads (lb)	
8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)	
Vert: 1-2=36, 2-3=21, 3-4=14, 5-6=-131, 2-8=10, 8-10=33, 7-10=10 Horz: 1-2=-48, 2-3=-33, 3-4=-26, 3-5=7	
Concentrated Loads (lb)	
Vert: 9=154 0) Dead + 0.6 MWERS Wind (Dec. Internal) Pight: Lumber Increase-1.60. Plate Increase-1.60.	
Uniform Loads (plf)	
Vert: 1-2=6, 2-3=12, 3-4=28, 5-6=-119, 2-7=-12	
Horz: 1-2=-18, 2-3=-24, 3-4=-40, 3-5=-27 Concentrated Loads (lb)	
Vert: 9=43	
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60	
Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-8=2, 8-10=25, 7-10=2	
Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=34	
Vert: 9=-339	
11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf) Vert: 1-2=-2 2-3=-9 3-4=-2 5-6=-141 2-7=-20	
Horz: 1-2=-18, 2-3=-11, 3-4=-18, 3-5=-0	
Concentrated Loads (lb)	
12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=	=1.60
Uniform Loads (plf)	
Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39	
Concentrated Loads (lb)	
Vert: 9=43	-1 60
Uniform Loads (plf)	=1.00
Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12	
Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb)	
Vert: 9=43	
14) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase	=1.60

Continued on page 3

![](_page_38_Picture_5.jpeg)

Job	Truss	Truss Type	Qty	Ply	Lot 3 Holly Place	
						157239365
J0223-0917	M2-GR	HALF HIP	1	2		
				<b></b>	Job Reference (optional)	
Comtech, Inc,	Fayetteville, NC - 28314,			8.430 s J	an 6 2022 MiTek Industries, Inc.	Fri Mar 17 07:27:21 2023 Page 3

ID:1yUksKymplk2404ufZYCrxyoKUD-WBgYUcZwNk2JnfFeZJWBUCFcIrA2bPl4XViTGeza5jK LOAD CASE(S) Standard Uniform Loads (plf) Vert: 1-2=14, 2-3=21, 3-4=14, 5-6=-131, 2-7=-12 Horz: 1-2=-26, 2-3=-33, 3-4=-26, 3-5=-39 Concentrated Loads (lb) Vert: 9=43 15) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=2, 2-3=9, 3-4=2, 5-6=-119, 2-7=-12 Horz: 1-2=-14, 2-3=-21, 3-4=-14, 3-5=-27 Concentrated Loads (lb) Vert: 9=43 16) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=6, 2-3=-1, 3-4=6, 5-6=-153, 2-7=-20 Horz: 1-2=-26, 2-3=-19, 3-4=-26, 3-5=-12 Concentrated Loads (lb) Vert: 9=-234 17) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-6, 2-3=-13, 3-4=-6, 5-6=-141, 2-7=-20 Horz: 1-2=-14, 2-3=-7, 3-4=-14, 3-5=-0 Concentrated Loads (lb) Vert: 9=-234 18) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-6=-240, 2-7=-20 Concentrated Loads (lb) Vert: 9=-200 19) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-8=-3, 8-10=13, 7-10=-3 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=26 Concentrated Loads (lb) Vert: 9=-454 20) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-37, 2-3=-42, 3-4=-37, 5-9=-206, 6-9=-236, 2-7=-20 Horz: 1-2=-13, 2-3=-8, 3-4=-13, 3-5=-0 Concentrated Loads (lb) Vert: 9=-375 21) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-31, 2-3=-36, 3-4=-31, 5-9=-215, 6-9=-245, 2-7=-20 Horz: 1-2=-19, 2-3=-14, 3-4=-19, 3-5=-9 Concentrated Loads (lb) Vert: 9=-375 22) Dead + 0.75 Roof Live (bal.) + 0.75 Attic Floor + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=-40, 2-3=-45, 3-4=-40, 5-9=-206, 6-9=-236, 2-7=-20 Horz: 1-2=-10, 2-3=-5, 3-4=-10, 3-5=-0 Concentrated Loads (lb) Vert: 9=-375 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-6=-160, 2-7=-20 Concentrated Loads (lb) Vert: 9=-400 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-160, 6-9=-200, 2-7=-20 Concentrated Loads (lb) Vert: 9=-400 25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-50, 3-4=-50, 5-6=-220, 2-7=-20 Concentrated Loads (lb) Vert: 9=-350 26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-20, 3-4=-20, 5-9=-220, 6-9=-250, 2-7=-20 Concentrated Loads (lb) Vert: 9=-350

![](_page_39_Picture_3.jpeg)

![](_page_40_Figure_0.jpeg)

**REACTIONS.** All bearings 12-10-9.

(lb) - Max Horz 1=231(LC 12)

1-2=-295/189

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10 except 11=-112(LC 12), 12=-107(LC 12), 13=-133(LC 12),

8=-126(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 7, 10, 9, 11, 12, 13, 8

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# TOP CHORD

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 7, 9, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10 except (jt=lb) 11=112, 12=107, 13=133, 8=126.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 7, 9, 8.

![](_page_40_Figure_18.jpeg)

818 Soundside Road Edenton, NC 27932

![](_page_41_Figure_0.jpeg)

<sup>(</sup>lb) - Max Horz 1=-140(LC 8)

- Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-135(LC 12), 6=-135(LC 13)
- Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=400(LC 19), 8=393(LC 19), 6=393(LC 20)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- WEBS 2-8=-338/247, 4-6=-338/247

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-5-0, Exterior(2) 7-5-0 to 11-9-13, Interior(1) 11-9-13 to 14-5-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=135, 6=135.

![](_page_41_Picture_13.jpeg)

![](_page_41_Picture_15.jpeg)

![](_page_42_Figure_0.jpeg)

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-2-10, Exterior(2) 6-2-10 to 10-7-7, Interior(1) 10-7-7 to 12-0-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.

![](_page_42_Picture_8.jpeg)

![](_page_42_Picture_10.jpeg)

![](_page_43_Figure_0.jpeg)

Max Grav 1=197(LC 1), 3=197(LC 1), 4=344(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_43_Picture_11.jpeg)

![](_page_43_Picture_13.jpeg)

![](_page_44_Figure_0.jpeg)

REACTIONS. (size) 1=7-7-10, 3=7-7-10, 4=7-7-10 Max Horz 1=68(LC 9) Max Uplift 1=-24(LC 13), 3=-30(LC 13) Max Grav 1=158(LC 1), 3=158(LC 1), 4=230(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_44_Picture_11.jpeg)

![](_page_44_Picture_13.jpeg)

![](_page_45_Figure_0.jpeg)

REACTIONS. (size) 1=5-2-13, 3=5-2-13, 4=5-2-13 Max Horz 1=-44(LC 8) Max Uplift 1=-15(LC 13), 3=-19(LC 13) Max Grav 1=102(LC 1), 3=102(LC 1), 4=149(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_45_Picture_11.jpeg)

![](_page_45_Picture_13.jpeg)

![](_page_46_Figure_0.jpeg)

BRACING-

TOP CHORD

BOT CHORD

LUMBER-TOP CHORD 2x4 SP No.1

BOT CHORD 2x4 SP No.1

REACTIONS. (size) 1=2-10-0, 3=2-10-0 Max Horz 1=-20(LC 8) Max Uplift 1=-4(LC 12), 3=-4(LC 13) Max Grav 1=81(LC 1), 3=81(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_46_Picture_13.jpeg)

Structural wood sheathing directly applied or 2-10-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

![](_page_47_Figure_0.jpeg)

![](_page_48_Figure_0.jpeg)

)								
THIS IS These to comport design See ind identified designed perman for the support and col designed consult truss de	BUILDER	Weaver Development Co. Inc.	СІТУ / СО.	Kenly / Johnston	LOA NUM NCLLOF (CL dr) 1700 3400 5100 6800 8500 10200 11900 13600 15300	deemed requiren attached requiren size and reactions 15000# retained reaction Tables Signatur	TI R Bearing	
A TRUSS russes ar nents to b at the spi ividual de ed on the er is respi ent braci overall st t structur umns is f er. For ge BCSI-B1 elivery pa	JOB NAME	Lot 3 Holly Place	ADDRESS	172 Colby Farm Drive	AD CHA (BASEL BBER OF JA BBER OF JA BBER OF JA BBER OF JA BBER OF JA 1 2 3 4 5 6 7 8 9	to compl nents. The Tables ( nents ) to number is s greater to design that exce A register to design that exce to design s that exce	ROC RUS eilly R Fayet Phon Fax:	-
S PLACEN e design be incorpo ecification esign she placemen placem	PLAN	Hickory II "A" / 2GRF, CP	MODEL	Floor	ART FC ON TABLE ON TABLE ICK STUDS HEADER 2550 5100 7650 10200 12750 15300	y with the e contract derived f determin of wood s than 3000 red design the supp reds those ed design the supp reds those and the supp reds the supp reds those and the supp reds the supp reds those and the supp reds those	OF 8 SES coad Ir teville e: (910)	
MENT DIA ed as ind prated int n of the b ets for ea nt drawin r tempor roof and The desig ng header nsibility of dance reg il-B3 prov online @	SEAL DATE	N/A	DATE REV.	03/17/23	DR JAG ES R502.5(: REQUIRED //INDER WD AGOVEN SCILLS A COVEN SCILLS A COVEN COVEN A COVENO	e prescrip tor shall r rom the p e the min studs req D# but no n profess port syste e specific n profess port syste 0#.	<b>Te</b> <b>&amp; FL</b> <b>&amp; B</b> ndustr , N.C. 0) 864 864-4	
GRAM ON ividual bu o the buil uilding de ach truss g. The bu ary and floor syst n of the t s, beams, of the buil parding br vided with sbcindus	QUOTE #		DRAWN BY	David Landry	CK STL () 4 (b)) () () () () () () () () () () () () () (	tive Code refer to th orescriptivi imum fou uired to s t greater t ional shal em for any d in the a ional shal em for all modul	CH OOF EAN ial Par 28309 -8787 444	
ILY. ilding ding esigner. design ilding tem and russ , walls, ding acing, the stry.com	JOB #	J0223-0918	SALES REP.	Lenny Norris	/DS 0 OF 0	e e ve Code indation upport than II be v tttached I be	AS AS	

Dimension Notes All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes Plumbing drop locations shown are NOT exact.
 Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
 Adjust spacing as needed not to exceed 24"oc.

	Conne	ctor Info	rmati	ion	Nail Info	ormation
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
$\bigcirc$	HUS410	USP	22	NA	16d/3-1/2"	16d/3-1/2"
$\bigcirc$	THDH412	USP	1	NA	16d /3-1/2"	16d /3-1/2"

		Products		
PlotID	Length	Product	Plies	Net Qty
BM1	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM2	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM3	12' 0"	2x10 SPF No.2	2	2
GDH	20' 0"	1-3/4"x 14" LVL Kerto-S	2	2
	$\bigcirc$	Truss Placement Plan		

(1) Scale: 1/4"=1'

# ۍ ا 32.

45' 6"

ò

lio

![](_page_49_Figure_0.jpeg)

I					T F	ROC RUS Reilly F Fayet Phon Fax	OF & SES coad Ir teville e: (910)	<b>TC</b> <b>&amp; FL</b> <b>&amp; B</b> dustr , N.C. 0) 864 864-4	CF OOF EAN ial Par 28309 -8787 444	AS k
		- <b>`</b> -		_	Bearing deemed requirer attached requirer size and	reactions to compl nents. The d Tables ( nents ) to d number	less that y with the contract derived f determin of wood s	n or equa e prescrip tor shall r rom the p the the min studs req	I to 3000# tive Code efer to th prescriptiv imum fou uired to s	# are e ve Code indation upport
					reaction 15000#. retained reaction Tables. retained reaction	A register A register I to design that exce A register I to design as that exce	than 3000 red desig n the sup eds thos red design n the sup ceed 1500	)# but not n profess port syste e specifie n profess port syste 10#.	t greater f ional sha em for any id in the a ional shal em for all	than II be y attached II be
	.0	0			Signatu	ire	Davi Davi	d Lo id Lai	<i>indr</i> ndry	.y
						AD CHI (BASEI WBER OF JI	ON TABLE	OR JAC ES R502.5(1 REQUIREL	CK STU ) & (b)) @ EA END	DS OF
		4	-		VCLLY 24 (0, 4, 7) 1700 3400 5100 6800 8500 10200 11900 13600 15300	1 2 2 3 4 5 5 1002 FOR 5 6 7 017 HEADER 6 7 9 117 HEADER 9 9	الله بعن المراجع المراجع المراجع المراجع ا مراجع المراجع الم مراجع المراجع ملي مراجع المراجع م مراجع المراجع المراجمع ا مراجع المراجع المراجع المراجع المراجع المراجع	BIROLR         BIROLR           BIROLR         BIROLR           BIROLR         BIROLR           BIROLR         BIROLR           D         1           C         2           D         3           D         4           D         5           D         6	34C 68C 1022 1366	00         00         00         00         00           1         2         3         3         4         00         5         00
		_	45' 6"	Dimension Notes         1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise         2. All interior wall dimensions are to face of frame wall unless noted otherwise         3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise         All of frame wall unless noted otherwise         Considered Load Bearing	:nly / Johnston	'2 Colby Farm Drive	oor	3/17/23	yvid Landry	nny Norris
				<ol> <li>Plumbing Drop Notes</li> <li>Plumbing drop locations shown are NOT exact.</li> <li>Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.</li> <li>Adjust spacing as needed not to exceed 24"oc.</li> </ol>	.0.	S 17	Ē		BY D	REP. Le
				Connector Information Nail Information	<u>ТТУ / 0</u>	NDDRES	VODEL	ATE RE	RAWN	SALES R
	"C 17C	7 17		Sym     Product     Manuf     Qty     Supported Member     Header     Truss       HUS410     USP     22     NA     16d/3-1/2"     16d/3-1/2"       THDH412     USP     1     NA     16d /3-1/2"     16d /3-1/2"			<			
6213# 4 Studs	<b>}</b>			$\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$	Weaver Development Co. Inc.	Lot 3 Holly Place	Hickory II "A" / 2GRF, CP	N/A		J0223-0918
1					BUILDER	JOB NAME	PLAN	SEAL DATE	QUOTE #	JOB #
				= Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Frect Truss Backwards	THIS IS These compo design See ind identifi design perman for the suppor and co design consul truss d	S A TRUS trusses a nents to I at the sp dividual d ed on the er is resp noverall si rt structur lumns is er. For ge t BCSI-B1 belivery pa	S PLACEN re design be incorpo ecification esign she placemen onsible fo ng of the ructure. 1 e includir the respo neral guia and BCS uckage or	IENT DIA ed as indi prated inti- n of the b ets for ea nt drawing or tempor- roof and The desig ng header nsibility c dance reg si-B3 prov- online @	GRAM ON vidual bu o the buil uilding de ach truss g. The bu ary and floor syst n of the tu s, beams of the buil arding br ided with sbcindus	ILY. iliding esigner. design ilding tem and russ , walls, ding racing, the stry.com

![](_page_50_Figure_0.jpeg)

CSD DESIG

	Client: Weaver Developm	nent Date:	3/17/2023	Page 2 of 8
isDesign	Address:	Job Nam	ne: Lot 3 Holly Place	
		Project #	: J0223-0918	
BM1 Kerto-S I	_VL 1.750" X 16.000"	2-Ply - PASSED		
• • •	• • • •	• • • •	••]	$\Pi$ $\uparrow$
				M ala"
	• • • •		<u> </u>	
1 SPF				
/	11'11"			3 1/2"
1	11'11"			
Multi-Ply Analysis				
Fasten all plies using 3 rc	ows of 10d Box nails (.128x3") at 12	o.c Maximum end distance r	ot to exceed 6".	
Load	0.0 PLF			
Yield Limit per Foot Yield Limit per Fastener	245.6 PLF 81.9 lb.			
Yield Mode	IV			
Edge Distance Min, End Distance	1 1/2" 3"			
Load Combination	-			
Duration Factor	1.00			
				_
Notes	chemicals Handling & Installation	6. For flat roofs provide proper drainage to prevent ponding	Manufacturer Info	Comtech, Inc. Lot 35 Briarwood Bluff Broadway, NC
structural adequacy of this component base design criteria and loadings shown. It	d on the 1. LVL beams must not be cut or drilled is the 2. Refer to manufacturer's product information		301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
ensure the component suitability of the application, and to verify the dimensions and lo	intended ads. approvals		(800) 622-5850 www.metsawood.com/us	
Lumber 1. Dry service conditions, unless noted otherw 2. LVL not to be treated with fire retardant or	3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation			соттесн
Version 21.80.417 Powered by iStruct	™ Dataset: 22111501.1	i nis design is valid until 11/3/2024	1	

![](_page_52_Figure_0.jpeg)

_	Client	Weaver Davelopm	ont	Det	2/17/2022	Dage 4 of 9
	Dient.	weaver Developin	ent	Dau	z =	Fage 4 01 o
TisDesign	Project:			Inpu		
Ispesign	Address:			JOD		
				Proj	ect #: J0223-0918	
BM2 Kerto-S LVL	1.750"	X 16.000"	2-Ply -	PASSED	Level: Level	
			-			
	•	• •	•••	•	••••	• . M
		• •	•		• • •	🗧 🍿 💷
						$\sum M$
• • • •	•	• •	• •	•	• • • •	
1 SPF End Grain					2 SPF End	ι Grain Λ΄
1		1	4'7 1/2"			3 1/2"
· · · · · · · · · · · · · · · · · · ·		4	417 4 /0"			<b>/</b>
		I	471/2			I
Multi Ply Apolycic						
Nulti-Fly Analysis						
Fasten all plies using 3 rows o	f 10d Box nails	s (.128x3") at 12"	o.c Maximu	um end distan	ce not to exceed 6".	
Capacity 79	9.8 %					
Load 19	96.0 PLF					
Yield Limit per Foot 24	45.6 PLF					
Yield Limit per Fastener 81	1.9 lb.					
Yield Mode IV	4/0"					
Edge Distance	1/Z <sup>*</sup>					
Load Combination	+1					
Duration Factor 1	00					
N /	abay-11-		C For Art - 1	ida nana- tata t	Manufacturer Info	Comtech, Inc.
Notes Calculated Structured Designs is responsible only of the	cnemicals Handling & Install:	ation	<ul> <li>er flat roofs prov ponding</li> </ul>	nue proper drainage to pr	Metsä Wood	Lot 35 Briarwood Bluff Broadway, NC
structural adequacy of this component based on the design criteria and loadings shown it is the	1. LVL beams must not b	e cut or drilled			301 Merritt 7 Building, 2nd Floor	USA 28314
responsibility of the customer and/or the contractor to ensure the component suitability of the interded	<ol> <li>Reter to manufac regarding installation</li> </ol>	n requirements, multi-ply			Norwalk, CT 06851 (800) 622-5850	910-864-TRUS
application, and to verify the dimensions and loads.	fastening details, bea approvals	m strength values, and code			www.metsawood.com/us	
Lumber	<ol> <li>Damaged Beams mus</li> <li>Design assumes top e</li> </ol>	t not be used dge is laterally restrained				
2. LVL not to be treated with fire retardant or corrosive	<ol> <li>Provide lateral support lateral displacement at</li> </ol>	rt at bearing points to avoid nd rotation	This decign is	valid until 11/2/2024		соттесн
			This design IS	rana anar 11/3/2024	1	

![](_page_54_Figure_0.jpeg)

	is	Design	1	Clien Proje Addre	t: W ct: ess:	/eaver Develop	ment		Date: Input by: Job Name:	3/17/2023 David Landry Lot 3 Holly Pla	ace			Page 6 of 8
									Project #:	J0223-0918				
BN	13	S-P-F	#2	2.000	' X 1	0.000"	2-Ply	- PASS	SED	evel: Level				
													-	
[	,	•	•	•	•	٠	•	٠	•	٠	•	• •	112	
		•	•	•	•	•	•	•	•	•	•	• • -	<u> </u>	9 1/4"
	J 1 SPF E	End Grain									2 SPF	End Grain		
							12'						3"	
<del> </del>  -							12'					1		
Mult	I-PIY P	Analysis												
Faste	n all pl	lies using	2 rows c	of 10d Box ı	hails (.1	28x3") at 12	" o.c Maxii	mum end	distance not	to exceed	6".			
Capaci	ty		0	.0 %										
Load			0	.0 PLF										
Yield Li	imit per F	Foot	1	57.4 PLF										
Yield Li	imit per F	astener	7	8.7 lb.										
Yield M	lode		IN	/										
Edge D	listance		1	1/2"										
Min. Er	nd Distar	nce	3											

Manufacturer Info	Comtech, Inc. Lot 35 Briarwood Bluff Broadway, NC USA 28314 910-864-TRUS
	соттесн

Load Combination Duration Factor

1.00

	•	C	lient:	Weaver Dev	elopment/		Date	e:	3/17/20	23				Page 7 of 8
Lie	Decign	P	Project:				Inpu	it by:	David L	andry				
	Design	A	ddress:				JOD Proi	Name	: Lot 3 Ho	DIIY Place				
	Karta C I	V/I 4	750"	× 4 4 00	<u>יי</u> מנ				_evel: Leve	el				
GDU I	Nerto-5 L	.VL 1.	150 /	X 14.00	JU 2	-Piy - P	ASSED							
							·····			••••				
							3							
		2												
					1									
•	•		•	•	• • •		· ·	-	•		•	•	M	1
The state	· ····································	Contraction of the second			·	· OT	· ·	and the	- TALLA			•	e W	1'2"
1 SPF End	· ·		•			•					2 SPF E	nd Grain	<u>а</u> ш	$\rightarrow$
1					20'								1 13	5 1/2"
1					20'								1	
Member In	formation						Reactions	UNF	PATTER	NED Ib	(Uplift)			
Туре:	Girder		Applicat	ion:	Floor		Brg Direct	tion	Live	e [	Dead	Snow	Wind	Const
Plies:	2		Design I	Method:	ASD		1 Vertica	al	(	D	1804	196	0	0
Moisture Cond	dition: Dry		Building	Code:	IBC/IRC 201	5	2 Vertica	al	(	D	1750	141	0	0
Deflection LL:	480 360		Load Sr Deck	iaring:	NO Not Checker	4								
Importance:	Normal - II		Ceiling:		Gypsum 1/2									
Temperature:	Temp <= 10	00°F												
							Bearings							
							Bearing L	ength	Dir.	Cap. R	eact D/L I	b Tota	al Ld. Case	Ld. Comb.
							1 - SPF 3 End	.500"	Vert	19%	1804 / 19	6 200	10 L	D+S
Analysis Re	sults						Grain							
Analysis	Actual	Location A	llowed	Capacity	Comb.	Case	2-SPF 3	.500"	Vert	18%	1750 / 14	1 189	11 L	D+S
Moment	8592 ft-lb	9'11 11/16" 2	4299 ft-lb	0.354 (35	%) D	Uniform	Grain							
Unbraced	9503 ft-lb	9'11 1/2" 9	509 ft-lb	0.999	D+S	L								
Shear	1553 lb	1'5 1/2" 9	408 lb	0.165 (17	%) D	Uniform								
LL Defl inch	0.041 (L/5726)	9'11 1/16" 0	.489 (L/480	) 0.084 (8%	5) S	L								
TL Defl inch	0.430 (L/546)	9'11 7/8" 0	.651 (L/360	) 0.660 (66	%) D+S	L								
Design Not	es													
1 Provide sup	port to prevent la	teral movement	and rotatio	n at the end	bearings. La	teral support	1							
2 Fasten all p	lies using 3 rows	of 10d Box nails	by the build s (.128x3") a	at 12" o.c. Ma	aximum end	distance not								
to exceed 6	)". t nore of colouisti	ana far fastanar		ion on o sifio d	laada									
4 Girders are	designed to be su	upported on the	bottom edg	le only.	ioaus.									
5 Top loads n	nust be supported	equally by all p	lies.											
7 Lateral sler	e laterally braced	at a maximum o ed on single ply	width.	0.C.										
ID	Load Type	L	ocation	Trib Width	Side	Dead 0.9	Live 1	Snov	w 1.15	Wind 1.	6 Const.	1.25 C	comments	
1	Uniform				Тор	60 PLF	0 PLF		0 PLF	0 PL	F (	PLF V	Vall	
2	Uniform				Тор	90 PLF	0 PLF		0 PLF	0 PL	F (	) PLF E	1GE	
3	Tie-In	0-0-0 to	16-10-0	1-0-0	Тор	20 PSF	0 PSF	2	20 PSF	0 PS	FC	PSF F	loof Load	
	Self Weight					11 PLF								
									Manufactu	ror info		Comto	ch Inc	
Notes Calculated Structured	Designs is responsible on	chemicals ly of the Handling	s & Installation	on	6. For pone	flat roofs provide p ding	roper drainage to pre	event	Metsä Woo	d		Lot 35 Broad	Briarwood Bluff way, NC	
structural adequacy of design criteria and	of this component based loadings shown. It	on the 1. LVL bean is the 2. Refer t	ns must not be cu o manufacture	ut or drilled r's product info	ormation				301 Merritt Norwalk, C	7 Building, 1 T 06851	2nd Floor	USA 28314	A-TRUS	
ensure the compon application, and to ver	ent suitability of the i ify the dimensions and load	ntended fastening	details, beam s	requirements, strength values, a	multi-ply nd code				(800) 622-5	5850	IS	310-80		
Lumber	ons, unless noted otherwis	3. Damageo 4. Design a	d Beams must no ssumes top edge	t be used is laterally restrain	ed									
2. LVL not to be trea	ted with fire retardant or c	orrosive 5. Provide lateral dis	ateral support a splacement and r	at bearing points otation	to avoid Thi	s design is valid	until 11/3/2024					Ľ	COMT	есн

Í	isDesign	Client: Project: Address:	Weaver Development	Date: Input by: Job Name Project #:	3/17/2023 David Landry : Lot 3 Holly Place J0223-0918	Page 8 of
GDH	Kerto-S LVL	1.750"	X 14.000" 2-Ply	- PASSED	_evel: Level	
· .	· · · · ·	· · · · · · · · · · · · · · · · · · ·	· · · · · · ·	· · · ·	· · · · ·	· · · · · · · · · · · · · · · · · · ·
1 SPF	End Grain				2 SPF End	Grain
[			20'			3 1/2"
1			20'			1
lulti-Plv	Analysis					
isten all	plies using 3 rows or	f 10d Box nails (	(.128x3") at 12" o.c Maxim	um end distance no	ot to exceed 6".	
pacity ad	0.0	0 % 0 PLF				
ld Limit pe	er Foot 24	15.6 PLF				
ld Limit pe Id Mode	er Fastener 81 IV	.9 lb.				
ge Distand	xe 1	1/2"				
n. End Dis	tance 3"					
uration Fac	tor 1.0	00				
				•		
Notes	Balana I. Balana I.	chemicals	6. For flat roofs pro	ovide proper drainage to prevent	Manufacturer Info	Comtech, Inc. Lot 35 Briarwood Bluff Broadway, NC
aculated Struct	urea Designs is responsible only of the	nanunng & Installâti	VII		IVIEISA VVOOD	Didauway, NO

Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc. Lot 35 Briarwood Bluff
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. <b>Lumber</b> 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	Handling & Installation 1. UL beams must not be cut or drilled 2. Refer to manufacturer's product information requirements, multi-phy fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation	ponding This design is valid until 11/3/2024	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	Broadway, NC USA 28314 910-864-TRUS

![](_page_58_Picture_0.jpeg)

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0223-0918 Lot 3 Holly Place

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I57239373 thru I57239380

My license renewal date for the state of North Carolina is December 31, 2023.

North Carolina COA: C-0844

![](_page_58_Picture_7.jpeg)

March 17,2023

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

![](_page_59_Figure_0.jpeg)

REACTIONS. All bearings 8-7-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

- 2) Gable requires continuous bottom chord bearing.
- 3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

#### LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

#### Vert: 9-16=-10, 1-8=-100 Concentrated Loads (lb)

Vert: 4=-71 7=-77 19=-71 20=-71

![](_page_59_Picture_15.jpeg)

![](_page_59_Picture_17.jpeg)

Job	Truss	Truss Type	Qty	Ply	Lot 3 Holly Place	
		- · - · -				157239374
J0223-0918	ET2	GABLE	1	1		
Camtach Inc. Fouette	ville NC 29214			0.420 a.1	Job Reference (optional)	Fri Mar 17 07:06:47 2022 Dage 1
Contech, Inc, Fayette	wille, NC - 20314,			0.430 S J		FILMAL 17 07.20.47 2023 Page I
		ID.	yukskympi	KZ404UIZ I	CIXYOROD-DTEOLX39QVG200W	F I gZKBATPUOCUE I P4JD 3g3cza5js
0 <sub>11</sub> 8						0 <sub>11</sub> 8
						Scale = 1:18.0
		2×4 —				
1 2	3	4 3 4 5	6		7 8	y 9 10
				0		
		LA L	-			
20						21
q 🗣						
<u>4</u>						
				•		
19 18	17	16 15	14		13 1	12 11

1	1-4-0	' 1-4-0	1-4-0	1	1-4-0		1-4-0			-4-0		1-4-0	1	1-4-0	' 0-5-0 '
Plate Offs	ets (X,Y)	[4:0-1-8,Edge], [15:0-1-8	,Edge]												
LOADING TCLL TCDL BCLL	i (psf) 40.0 10.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.00 1.00 YES	CSI. TC BC WB	0.06 0.01 0.03		DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 11	l/defl n/a n/a n/a	L/d 999 999 n/a		PLATES MT20	<b>GRIP</b> 244/190	
BCDL	5.0	Code IRC2015/T	PI2014	Matri	x-S								Weight: 54 lb	FT =	20%F, 11%E
LUMBER-	RD 2x4 S	P No.1(flat)				·	BRACING- TOP CHOR	:D	Structu	ral wood	sheathin	g directly	applied or 6-0-	0 oc purlins	,

6-8-0

8-0-0

9-4-0

3x4 =

5-4-0

BOT CHORD	2x4 SP No.1(flat)		except end verticals.
WEBS	2x4 SP No.3(flat)	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3(flat)		

REACTIONS. All bearings 11-1-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 19, 11, 18, 17, 16, 15, 14, 13, 12

4-0-0

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

3x4 =

1-4-0

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

2-8-0

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

![](_page_60_Picture_14.jpeg)

3x6 =

11-1-0

10-8-0

![](_page_60_Picture_16.jpeg)

![](_page_61_Figure_0.jpeg)

#### LUMBER-

 TOP CHORD
 2x4 SP No.1(flat)

 BOT CHORD
 2x4 SP No.1(flat)

 WEBS
 2x4 SP No.3(flat)

 OTHERS
 2x4 SP No.3(flat)

 
 BRACING 

 TOP CHORD
 Structural wood sheathing directly applied or 3-5-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 3-5-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

![](_page_61_Picture_15.jpeg)

![](_page_61_Picture_17.jpeg)

![](_page_62_Figure_0.jpeg)

L	14-7-12			26-5-0							
I	14-7-12		I	11-9-4							
Plate Offsets (X,Y)	[4:0-1-8,Edge], [5:0-1-8,Edge], [13:0-1-8	3,Edge], [19:0-1-8,Edge]									
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.55 BC 0.76 WB 0.44 Matrix-S	DEFL. Vert(LL) -0. Vert(CT) -0. Horz(CT) 0.	in (loc) l/defl 10 26-27 >999 13 26-27 >999 03 16 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 136 lb	<b>GRIP</b> 244/190 FT = 20%F 11%F				
BCDE 3:0	Code 11(02013/11 12014	Matrix-5				Weight. 150 lb	1 1 = 20781, 1178E				
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural woo except end ver Rigid ceiling di	d sheathing dire ticals. rectly applied or	ectly applied or 6-0-0 or 6-0-0 or 6-0-0 oc bracing.	oc purlins,				
Max G	Grav 29=727(LC 10), 16=1063(LC 4), 21	=1671(LC 1)									
FORCES. (lb) - Max. TOP CHORD 15-16 8-9=( 13-14	Comp./Max. Ten All forces 250 (lb) or 5=-537/0, 2-3=-1257/0, 3-4=-1883/0, 4-5 0/1401, 9-10=0/1401, 10-11=-468/367, 1 4=-884/0	less except when shown =-2002/0, 5-6=-1658/0, 6 1-12=-1158/0, 12-13=-11	n. 5-8=-756/225, 158/0,								
BOT CHORD 28-29 22-24 17-18	9=0/771, 27-28=0/1717, 26-27=0/2002, 2 4=-34/1340, 21-22=-444/147, 20-21=-63 8=0/1158, 16-17=0/586	25-26=0/2002, 24-25=0/2 7/19, 19-20=-175/900, 18	2002, 3-19=0/1158,								
WEBS 2-29= 13-17 10-20	=-1023/0, 2-28=0/677, 3-28=-640/0, 8-21 7=-372/90, 10-21=-1064/0, 8-22=0/923, D=0/687, 11-20=-690/0, 11-19=0/582, 12	1=-1289/0, 14-16=-780/0, 6-22=-882/0, 6-24=0/527 2-19=-278/0	, 14-17=0/415, 7, 5-24=-633/0,								
NOTES- 1) Unbalanced floor liv. 2) All plates are 3x4 M 3) Plates checked for a 4) Refer to girder(s) for 5) Recommend 2x6 str Strongbacks to be a 6) CAUTION, Do not e LOAD CASE(S) Stan.	e loads have been considered for this de T20 unless otherwise indicated. a plus or minus 1 degree rotation about ir r truss to truss connections. rongbacks, on edge, spaced at 10-0-0 o uttached to walls at their outer ends or re rect truss backwards.	esign. ts center. oc and fastened to each tr strained by other means.	russ with 3-10d (0.131'	X 3") nails.	6	UNTH C	AROUNT SOLUTION				
1) Dead + Floor Live (b Uniform Loads (plf) Vert: 16-29: Concentrated Loads Vert: 15=-50	dard balanced): Lumber Increase=1.00, Plate =-10, 1-15=-100 \$ (lb) 00	Increase=1.00				SE 036	AL 322 VEERENTING				
						(minin	mini				

March 17,2023

![](_page_62_Picture_4.jpeg)

![](_page_63_Figure_0.jpeg)

		10-	-10-0			
		10-	-10-0			1
Plate Offsets (X,Y)	[4:0-1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.36 BC 0.46 WB 0.21 Matrix-S	DEFL.         in           Vert(LL)         -0.07           Vert(CT)         -0.09           Horz(CT)         0.01	(loc) I/defi L/d 10 >999 480 10 >999 360 7 n/a n/a	<b>PLATES</b> MT20 Weight: 57 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,
REACTIONS. (siz Max (	ze) 12=0-3-8, 7=0-3-8 Grav 12=576(LC 1), 7=576(LC 1)					

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-923/0, 3-4=-1242/0, 4-5=-925/0

BOT CHORD 11-12=0/606, 10-11=0/1242, 9-10=0/1242, 8-9=0/1242, 7-8=0/603

WEBS 2-12=-805/0, 2-11=0/441, 3-11=-469/0, 5-7=-800/0, 5-8=0/449, 4-8=-475/0

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

![](_page_63_Picture_11.jpeg)

![](_page_63_Picture_13.jpeg)

![](_page_64_Figure_0.jpeg)

Ι			11-11-0			I
Plate Offsets (X,Y)	[5:0-1-8,Edge], [11:0-1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrNOCode IRC2015/TPI2014	CSI. TC 0.60 BC 0.70 WB 0.27 Matrix-S	DEFL.         in           Vert(LL)         -0.13           Vert(CT)         -0.16           Horz(CT)         0.02	(loc) I/defl L/d 11-12 >999 480 11-12 >894 360 8 n/a n/a	PLATES MT20 Weight: 62 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing direct except end verticals. Rigid ceiling directly applied or	ctly applied or 6-0-0 10-0-0 oc bracing.	oc purlins,

REACTIONS.	(size)	13=0-3-8, 8=0-3-8
	Max Grav	13=635(LC 1), 8=1142(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 7-8=-548/0, 2-3=-1060/0, 3-4=-1495/0, 4-5=-1495/0, 5-6=-1059/0

BOT CHORD 12-13=0/676, 11-12=0/1395, 10-11=0/1495, 9-10=0/1495, 8-9=0/659

WEBS 2-13=-898/0, 2-12=0/534, 3-12=-466/0, 3-11=-19/356, 6-8=-877/0, 6-9=0/557, 5-9=-616/0

## NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

## LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 8-13=-10, 1-7=-100

Concentrated Loads (lb) Vert: 7=-500

![](_page_64_Picture_19.jpeg)

![](_page_64_Picture_21.jpeg)

![](_page_65_Figure_0.jpeg)

L			14-6-0			
	[5:0.4.9.Edge] [44:0.4.9.Edge]		14-6-0			
Plate Olisets (X, Y)	[5:0-1-8,Edge], [14:0-1-8,Edge]	1				
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.62 BC 0.78 WB 0.35 Matrix-S	DEFL.         ir           Vert(LL)         -0.17           Vert(CT)         -0.22           Horz(CT)         0.03	n (loc) l/defl L/d 12-13 >999 480 12-13 >790 360 10 n/a n/a	<b>PLATES</b> MT20 Weight: 76 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied or	ectly applied or 6-0-0 r 10-0-0 oc bracing.	oc purlins,

#### REACTIONS. (size) 16=0-3-8, 10=Mechanical

Max Grav 16=778(LC 1), 10=784(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1348/0, 3-4=-2244/0, 4-5=-2244/0, 5-6=-2152/0, 6-7=-2152/0, 7-8=-1359/0 BOT CHORD

15-16=0/834, 14-15=0/1856, 13-14=0/2244, 12-13=0/2244, 11-12=0/1857, 10-11=0/835 WEBS

2-16=-1109/0, 2-15=0/714, 3-15=-707/0, 3-14=0/697, 4-14=-339/0, 8-10=-1111/0,

8-11=0/729, 7-11=-693/0, 7-12=0/401, 5-12=-438/123

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

![](_page_65_Picture_17.jpeg)

![](_page_65_Picture_19.jpeg)

![](_page_66_Figure_0.jpeg)

I			11-7-8			I
Plate Offsets (X,Y)	[1:Edge,0-1-8], [5:0-1-8,Edge], [11:0-1-	8,Edge]				
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.44 BC 0.56 WB 0.25 Matrix-S	<b>DEFL.</b> ir Vert(LL) -0.10 Vert(CT) -0.13 Horz(CT) 0.02	n (loc) l/defl L/d ) 11-12 >999 480 3 11-12 >999 360 2 8 n/a n/a	<b>PLATES</b> MT20 Weight: 61 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 3 BOT CHORD 2x4 3 WEBS 2x4 3	SP No.1(flat) SP No.1(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 r 10-0-0 oc bracing.	oc purlins,

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 13=Mechanical, 8=0-3-8 Max Grav 13=626(LC 1), 8=619(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1025/0, 3-4=-1427/0, 4-5=-1427/0, 5-6=-1022/0

BOT CHORD 12-13=0/658, 11-12=0/1345, 10-11=0/1427, 9-10=0/1427, 8-9=0/644

WEBS 2-13=-876/0, 2-12=0/510, 3-12=-446/0, 3-11=-41/323, 6-8=-854/0, 6-9=0/526,

NOTES-

=

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

5-9=-570/0

![](_page_66_Picture_16.jpeg)

![](_page_66_Picture_18.jpeg)

![](_page_67_Figure_0.jpeg)