





▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)



| <br>\<br>\<br>\ |                |  | TI R  | ROC<br>RUS<br>eilly R<br>Fayet<br>Phon<br>Fax:  | DF &<br>SES<br>oad Ir<br>teville<br>e: (91(<br>: (910)  | <b>TC</b><br><b>&amp; FL</b> (<br><b>&amp; B</b><br>ndustr<br>, N.C. :<br>0) 864-<br>864-4   | <b>CH</b><br><b>DOF</b><br><b>EAN</b><br>ial Par<br>28309<br>-8787<br>444  | ₹<br>ΛS   |
|-----------------|----------------|--|---|---|---|--|--|---|
|                 | 10. 0.         | All Truss Reactions are Less<br>than 3,000 lbs. Unless Noted Otherwise.<br>Denotes Reaction Greater than 3,000 lbs.<br>Reaction / # of Studs   | Bearing I<br>deemed f<br>requirem<br>attached<br>requirem<br>size and<br>reactions<br>15000#. /<br>retained<br>reactions<br>Signatur  | eactions<br>to complete<br>ents. The<br>Tables (<br>ents) to<br>number (<br>greater<br>A register<br>to design<br>that exce<br>a register<br>to design<br>that exce<br>a greater<br>b design<br>that exce<br>a greater<br>b design<br>to design<br>that exce<br>a greater<br>b design<br>to design<br>that exce<br>a greater<br>b design<br>to | less thar<br>/ with the<br>contract<br>derived fi<br>determin<br>f wood s<br>than 3000<br>ed design<br>the supp<br>eds thosy<br>eds thosy<br>wath<br>onat<br>ART F(<br>on TABLI<br>ck STUDS | ) or equal<br>) prescript<br>or shall r<br>rom the p<br>e the mini<br>ituds requ<br>) # but not<br>n professi<br>oort syste<br>e specifie<br>port syste<br>off.<br>Man L<br>DR JAC<br>ES R502.5(1<br>REQUIRED  | to 3000#<br>ive Code<br>afer to th<br>rescriptivi<br>inum fou-<br>ired to s<br>greater t<br>ional sha<br>m for any<br>d in the a<br>onal shal<br>m for all<br>   | are<br>e e Code<br>indation<br>upport<br>than<br>II be<br>y<br>statched<br>II be<br>Y<br>JDS                            |
|                 | <u>.</u>       | All Walls Shown Are<br>Considered Load Bearing<br>Roof Area = $1637.8 \text{ sq.ft.}$<br>Ridge Line = $59.7 \text{ ft.}$<br>Hip Line = $1.32 \text{ ft.}$<br>Horiz. OH = $95 \text{ ft.}$<br>Raked OH = $134.32 \text{ ft.}$<br>Decking = $56 \text{ sheets}$  | NOLLOY 20 400<br>1700<br>3400<br>5100<br>6800<br>8500<br>10200<br>11900<br>13600<br>15300   | а абартана (2)<br>1 абартана (2)<br>1 абартана (2)<br>1 абартана (2)<br>3 абартана (2)<br>3 абартана (2)<br>3 абартана (2)<br>3 абартана (2)<br>5 бо<br>7 абартана (2)<br>8 абартана (2)  | 2550<br>5100<br>7650<br>10200<br>15300  | (GIRDER<br>WG 2012 S1012 S102 S1012 S102 S1012 S102 S10  | 340<br>680<br>1020   | 1 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0   |
|                 | 31.6"          | Dimension Notes         1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise         2. All interior wall dimensions are to face of stud unless noted otherwise         3. All exterior wall to truss dimensions are to face of stud unless noted otherwise         3. All exterior wall to truss dimensions are to face of stud unless noted otherwise         Box Storage         Tray Ceiling         Flush Beam         Drop Beam         Connector Information         Sym         Hus26       USP 7         NA       16d/3-1/2" | CITY / CO. Sanford / Harnett  | ADDRESS -   | MODEL Roof  | DATE REV. 03/01/23   | DRAWN BY Jonathan Landry   | SALES REP. Lenny Norris   |
| 880             | , <b>5. 0.</b> | $\begin{tabular}{ c c c c c c c c c c c c c c c c c c c$   | Weaver Development  | Lot 2 Holly Place   | Magnolia II "C" / 2GRF, CP  | N/A  |  | J0223-0919  |
|                 |                | ▲= Denotes Left End of Truss<br>(Reference Engineered Truss Drawing)   | THIS IS<br>THESE THE COMPORT<br>DESIGNATION<br>THIS IS<br>These the<br>comport<br>designe<br>see indi<br>identified<br>designe<br>permanna<br>for the of<br>support<br>and colo<br>designe<br>consult<br>truss de | A TRUSS<br>USSES AT TRUS  | FLACEN<br>PLACEN<br>e incorpt<br>coffication<br>ssign she<br>placemei<br>onsible fc<br>ng of the<br>placemei<br>onsible fc<br>ng of the<br>che respo<br>neral guia<br>and BCS<br>ckade or   | ENT DIA<br>ad as indi<br>orated into<br>a d as indi<br>orated into<br>a d as indi<br>orated into<br>a d s indi<br>orated into<br>a d as indi<br>a d as indi | # JLOOD<br>SRAM ON<br>b the buil<br>uilding dr<br>to the buil<br>uilding dr<br>to the buil<br>uilding dr<br>to the buil<br>ary and<br>floor syst<br>n of the buil<br>arding br<br>ided with<br>sbcindu | # <b>BOL</b><br>LLY.<br>iliding<br>design<br>iliding<br>iding<br>russ<br>, walls,<br>ding<br>racing,<br>the<br>strv.com |



RE: J0223-0919 Lot 2 Holly Place Trenco 818 Soundside Rd Edenton, NC 27932

# Site Information:

Customer: Weaver Development<br/>Lot/Block: 2Project Name: J0223-0919<br/>Model: Magnolia IIAddress:Subdivision: Holly Place<br/>State: NC

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 19 individual, dated Truss Design Drawings and 0 Additional Drawings.

| No. | Seal#     | Truss Name | Date      |
|-----|-----------|------------|-----------|
| 1   | 156812742 | A1         | 2/23/2023 |
| 2   | 156812743 | A1GE       | 2/23/2023 |
| 3   | 156812744 | A2         | 2/23/2023 |
| 4   | 156812745 | B1         | 2/23/2023 |
| 5   | 156812746 | B1-GR      | 2/23/2023 |
| 6   | 156812747 | B1GE       | 2/23/2023 |
| 7   | 156812748 | C1         | 2/23/2023 |
| 8   | 156812749 | C1GE       | 2/23/2023 |
| 9   | 156812750 | J1         | 2/23/2023 |
| 10  | 156812751 | J1GE       | 2/23/2023 |
| 11  | 156812752 | M1         | 2/23/2023 |
| 12  | 156812753 | M1GE       | 2/23/2023 |
| 13  | 156812754 | V1GE       | 2/23/2023 |
| 14  | 156812755 | V2GE       | 2/23/2023 |
| 15  | 156812756 | V3         | 2/23/2023 |
| 16  | 156812757 | V4         | 2/23/2023 |
| 17  | 156812758 | V5         | 2/23/2023 |
| 18  | 156812759 | V6         | 2/23/2023 |
| 19  | 156812760 | V7         | 2/23/2023 |

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2023

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



| L  | 10-8-8  | 1                              | 20-8-8                                  | 1                                  |                                 | 31-5-0              |                   |  |  |  |  |
|--|---|--------------------------------|---|------------------------------------|---------------------------------|---------------------|-------------------|--|--|--|--|
| I  | 10-8-8  |                                | 10-0-0                                  |                                    |                                 | 10-8-8              |                   |  |  |  |  |
| Plate Offsets (X,Y)  | late Offsets (X,Y) [7:0-3-0,Edge], [14:0-1-12,0-1-8], [16:0-1-12,0-1-8] |                                |   |                                    |                                 |                     |                   |  |  |  |  |
| LOADING (psf)<br>TCLL 20.0   | SPACING- 2-0-0<br>Plate Grip DOL 1.15                                   | <b>CSI.</b><br>TC 0.60         | DEFL.<br>Vert(LL) -0                    | in (loc)<br>.24 12-14              | l/defl L/c<br>>999 360          | PLATES<br>MT20      | S GRIP<br>244/190 |  |  |  |  |
| TCDL         10.0           BCLL         0.0         *           BCDL         10.0 | Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014          | BC 0.54<br>WB 0.40<br>Matrix-S | Vert(CT) -0<br>Horz(CT) 0<br>Wind(LL) 0 | 0.35 12-14<br>0.06 12<br>0.14 2-16 | >999 240<br>n/a n/a<br>>999 240 | )<br>a<br>) Weight: | 223 lb FT = 20%   |  |  |  |  |
| LUMBER-  |   |                                | BRACING-                                |                                    |                                 |                     |                   |  |  |  |  |

TOP CHORD

BOT CHORD

WEBS

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2

- REACTIONS. (size) 2=0-3-8, 12=0-3-8 Max Horz 2=107(LC 11) Max Uplift 2=-87(LC 12), 12=-87(LC 13) Max Grav 2=1364(LC 2), 12=1364(LC 2)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- 2-3=-2405/501, 3-5=-2109/439, 5-6=-1688/442, 6-7=0/269, 7-8=0/269, 8-9=-1688/442, TOP CHORD
- 9-11=-2109/439, 11-12=-2406/501
- BOT CHORD 2-16=-332/2114, 14-16=-171/1759, 12-14=-327/2079
- WEBS 3-16=-532/215, 5-16=-21/741, 9-14=-21/741, 11-14=-532/215, 6-8=-2012/395

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 15-8-8, Exterior(2) 15-8-8 to 20-1-5, Interior(1) 20-1-5 to 32-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 4-6-8 oc purlins.

6-8

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Row at midpt

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **MSIVTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 10-11=-111/275, 11-12=-111/275

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 31, 32, 33, 34, 35, 36, 37, 28, 27, 26, 25, 24, 23, 22.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 15-8-8, Exterior(2) 15-8-8 to 20-1-5, Interior(1) 20-1-5 to 32-1-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932

February 23,2023



| Plate Offs | late Offsets (X,Y) [9:0-5-0,0-6-4] |                       |          |              |          |        |     |                         |  |  |  |  |
|------------|------------------------------------|-----------------------|----------|--------------|----------|--------|-----|-------------------------|--|--|--|--|
|            | (psf)                              | <b>SPACING-</b> 2-0-0 | CSI.     | DEFL.        | in (loc) | l/defl | L/d | PLATES GRIP             |  |  |  |  |
| TCLL       | 20.0                               | Plate Grip DOL 1.15   | TC 0.70  | Vert(LL) -0. | 07 7-9   | >999   | 360 | MT20 244/190            |  |  |  |  |
| TCDL       | 10.0                               | Lumber DOL 1.15       | BC 0.60  | Vert(CT) -0. | 14 7-9   | >999   | 240 | M18AHS 186/179          |  |  |  |  |
| BCLL       | 0.0 *                              | Rep Stress Incr NO    | WB 0.71  | Horz(CT) 0.  | .03 5    | n/a    | n/a |                         |  |  |  |  |
| BCDL       | 10.0                               | Code IRC2015/TPI2014  | Matrix-S | Wind(LL) 0.  | 05 7-9   | >999   | 240 | Weight: 318 lb FT = 20% |  |  |  |  |

BRACING-

TOP CHORD

BOT CHORD

 LUMBER 

 TOP CHORD
 2x6 SP No.1

 BOT CHORD
 2x8 SP 2400F 2.0E

 WEBS
 2x4 SP No.2

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 5=0-3-8 Max Horz 1=172(LC 5) Max Uplift 1=-215(LC 8), 5=-321(LC 9) Max Grav 1=5117(LC 2), 5=4087(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-6421/326, 2-3=-5177/432, 3-4=-5171/432, 4-5=-6561/532

BOT CHORD 1-10=-270/4984, 9-10=-270/4984, 7-9=-354/5052, 5-7=-354/5052

WEBS 3-9=-384/5375, 4-9=-1448/244, 4-7=-141/1918, 2-9=-1352/546, 2-10=-457/2057

#### NOTES-

WEDGE

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-2-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

 4) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

5) All plates are MT20 plates unless otherwise indicated.

6) N/A



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

| Job                 | Truss               | Truss Type    | Qty      | Ply         | Lot 2 Holly Place                                      |           |
|---------------------|---------------------|---------------|----------|-------------|--|-----------|
|                     |                     |               |          |             |  | 156812746 |
| J0223-0919          | B1-GR               | COMMON GIRDER | 1        | 2           |  |           |
|                     |                     |               |          | <b>_</b>    | Job Reference (optional)                               |           |
| Comtech, Inc, Fayet | eville, NC - 28314, |               |          | 3.430 s Jar | 6 2022 MiTek Industries, Inc. Thu Feb 23 08:34:47 2023 | Page 2    |
|                     |                     | ID:IwPC       | H6hK8Jep | ott6SXqQC   | DJcyzm6C-nX4GZfiNo6d611DZ00Wj5mGUUskEQu5PUjrM4         | LziJBs    |

# NOTES-

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=215, 5=321.
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 419; bl down and 445 lb up at 11-9-0, 857 lb down at 0-7-12, 853 lb down at 2-7-12, 853 lb down at 4-7-12, 853 lb down at 6-7-12, and 853 lb down at 8-7-12, and 853 lb down at 10-7-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-6=-60, 1-5=-20 Concentrated Loads (lb)

Vert: 8=-4142(F) 11=-215(B) 12=-212(B) 13=-212(B) 14=-212(B) 15=-212(B) 16=-212(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





#### LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1OTHERS2x4 SP No.2

SLIDER Left 2x4 SP No.2 1-7-0, Right 2x4 SP No.2 1-7-0

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

### REACTIONS.

All bearings 19-11-0.
 (lb) - Max Horz 1=218(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 20, 21, 22, 18, 17, 16 except 23=-166(LC 12), 15=-147(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 13, 19, 20, 21, 22, 23, 18, 17, 16, 15

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 20, 21, 22, 18, 17, 16 except (jt=lb) 23=166, 15=147.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=115, 8=111

9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





| LUMBER-<br>TOP CHORD 2x6 SP | No.1  |                        | BRACING-<br>TOP CHORD |            | Structu      | ral wood            | sheathing d       | irectly applied or 6-0-0 | oc purlins,            |
|-----------------------------|---|------------------------|-----------------------|------------|--------------|---------------------|-------------------|--------------------------|------------------------|
| BCLL 0.0 *<br>BCDL 10.0     | Rep Stress Incr YES<br>Code IRC2015/TPI2014 | WB 0.00<br>Matrix-P    | Horz(CT)<br>Wind(LL)  | 0.00       | 2-0          | >333<br>n/a<br>**** | n/a<br>240        | Weight: 43 lb            | FT = 20%               |
| LOADING (psf)<br>TCLL 20.0  | SPACING- 2-0-0<br>Plate Grip DOL 1.15       | <b>CSI.</b><br>TC 0.36 | DEFL.<br>Vert(LL) -   | in<br>0.04 | (loc)<br>2-6 | l/defl<br>>999      | L/d<br>360<br>240 | PLATES<br>MT20           | <b>GRIP</b><br>244/190 |
| Plate Offsets (X,Y)         | [2:0-0-13,0-1-1]                            |                        |                       |            |              |                     |                   |                          |                        |

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

**REACTIONS.** (size) 6=Mechanical, 2=0-3-8

Max Horz 2=70(LC 12) Max Uplift 6=-43(LC 12), 2=-49(LC 8)

Max Grav 6=312(LC 1), 2=347(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-6-11 to 3-10-2, Interior(1) 3-10-2 to 8-0-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





ł

| LOADING<br>TCLL<br>TCDL<br>BCLL | (psf)<br>20.0<br>10.0<br>0.0 * | SPACING-<br>Plate Grip DOL<br>Lumber DOL<br>Rep Stress Incr | 2-0-0<br>1.15<br>1.15<br>YES | CSI.<br>TC<br>BC<br>WB | 0.05<br>0.03<br>0.04 | <b>DEFL.</b><br>Vert(LL)<br>Vert(CT)<br>Horz(CT) | in<br>0.00<br>0.00<br>0.00 | (loc)<br>5<br>5 | l/defl<br>n/r<br>n/r<br>n/a | L/d<br>120<br>120<br>n/a | PLATES<br>MT20          | <b>GRIP</b><br>244/190 |  |
|---------------------------------|--------------------------------|---|------------------------------|------------------------|----------------------|--|----------------------------|-----------------|-----------------------------|--------------------------|-------------------------|------------------------|--|
| BCDL                            | 10.0                           | Code IRC2015/TPI  | 2014                         | Matrix                 | (-P                  |  |                            |                 |                             |                          | Weight: 46 lb           | FT = 20%               |  |
| LUMBER-<br>TOP CHOI<br>BOT CHOI | RD 2x6 SP<br>RD 2x6 SP         | No.1<br>No.1  |                              |                        |                      | BRACING-<br>TOP CHOR                             | D                          | Structur        | ral wood<br>end verti       | sheathing d              | rectly applied or 6-0-0 | oc purlins,            |  |

BOT CHORD

| BOT CHORD | 2x6 SP No.1 |
|-----------|-------------|
| WEBS      | 2x4 SP No.2 |
| OTHERS    | 2x4 SP No.2 |

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 8-0-0.

Max Horz 2=100(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 8, 2, 9, 10 Max Grav All reactions 250 lb or less at joint(s) 8, 2, 9 except 10=293(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2, 9, 10.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





| LOADING | G (psf) | <b>SPACING-</b> 2-0-0 | CSI.     | DEFL.    | in    | (loc) | l/defl | L/d | PLATES        | GRIP     |
|---------|---------|-----------------------|----------|----------|-------|-------|--------|-----|---------------|----------|
| TCLL    | 20.0    | Plate Grip DOL 1.15   | TC 0.12  | Vert(LL) | -0.01 | 2-4   | >999   | 360 | MT20          | 244/190  |
| TCDL    | 10.0    | Lumber DOL 1.15       | BC 0.08  | Vert(CT) | -0.01 | 2-4   | >999   | 240 |               |          |
| BCLL    | 0.0 *   | Rep Stress Incr YES   | WB 0.00  | Horz(CT) | 0.00  |       | n/a    | n/a |               |          |
| BCDL    | 10.0    | Code IRC2015/TPI2014  | Matrix-P | Wind(LL) | 0.01  | 2-4   | >999   | 240 | Weight: 29 lb | FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

2x6 SP No.1

REACTIONS. 2=0-3-0, 4=0-1-8 (size) Max Horz 2=59(LC 12) Max Uplift 2=-89(LC 8), 4=-82(LC 8) Max Grav 2=235(LC 1), 4=182(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-9 to 3-9-4, Interior(1) 3-9-4 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932



| LOADING<br>TCLL<br>TCDL<br>BCLL<br>BCDL | (psf)<br>20.0<br>10.0<br>0.0 *<br>10.0 | SPACING-<br>Plate Grip DOL<br>Lumber DOL<br>Rep Stress Incr<br>Code IRC2015/TF | 2-0-0<br>1.15<br>1.15<br>YES<br>Pl2014 | CSI.<br>TC<br>BC<br>WB<br>Matrix | 0.06<br>0.06<br>0.01<br>-S | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT) | in<br>0.01<br>-0.01<br>-0.00 | (loc)<br>8<br>8<br>6 | l/defl<br>>999<br>>999<br>n/a | L/d<br>240<br>240<br>n/a | PLATES<br>MT20<br>Weight: 32 lb | <b>GRIP</b><br>244/190<br>FT = 20% |  |
|---|--|--|--|----------------------------------|----------------------------|---|------------------------------|----------------------|-------------------------------|--------------------------|---------------------------------|------------------------------------|--|
| LUMBER-                                 |  |  |  |                                  |                            | BRACING-                                  |                              |                      |                               |                          |                                 |                                    |  |

TOP CHORD

BOT CHORD

# LUMBER-

| TOP CHORD | 2x6 SP No.1 |
|-----------|-------------|
| BOT CHORD | 2x6 SP No.1 |
| WEBS      | 2x6 SP No.1 |
| OTHERS    | 2x4 SP No.2 |

REACTIONS. (size) 2=0-3-0, 6=0-1-8

Max Horz 2=85(LC 12) Max Uplift 2=-128(LC 8), 6=-119(LC 8) Max Grav 2=235(LC 1), 6=182(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=128, 6=119.



Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 10-4-13.

(lb) - Max Horz 1=-120(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-166(LC 12), 6=-165(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=275(LC 19), 6=275(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=166, 6=165.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **MSIVTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 16, 10 except (jt=lb) 14=114, 15=114, 12=112, 11=115.



818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=123, 6=123.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Max Uplift 1=-22(LC 13), 3=-31(LC 13) Max Grav 1=201(LC 1), 3=201(LC 1), 4=350(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





REACTIONS. (size) 1=7-8-10, 3=7-8-10, 4=7-8-10 Max Horz 1=70(LC 11) Max Uplift 1=-24(LC 13), 3=-31(LC 13) Max Grav 1=162(LC 1), 3=162(LC 1), 4=236(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

- and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. 1=5-3-13, 3=5-3-13, 4=5-3-13 (size) Max Horz 1=-46(LC 8) Max Uplift 1=-16(LC 13), 3=-20(LC 13) Max Grav 1=106(LC 1), 3=106(LC 1), 4=155(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



|  | 0-0-7   | 1-5-8  | 1-6-0   | l   |
|--|---|--|---|---|
| LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.15<br>Lumber DOL 1.15<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | <b>CSI.</b><br>TC 0.01<br>BC 0.01<br>WB 0.01<br>Matrix-P | DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         n/a         -         n/a         999           Vert(CT)         n/a         -         n/a         999           Horz(CT)         0.00         3         n/a         n/a | PLATES         GRIP           MT20         244/190           Weight: 10 lb         FT = 20% |

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

WEBS 2x4 SP No.2

REACTIONS. 1=2-11-0, 3=2-11-0, 4=2-11-0 (size) Max Horz 1=22(LC 9) Max Uplift 1=-8(LC 13), 3=-10(LC 13) Max Grav 1=51(LC 1), 3=51(LC 1), 4=74(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 2-11-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601









 $\bigcirc$ 

15' 0"

20' 0"

BM1

BM2

GDH

сотесн

**ROOF & FLOOR TRUSSES & BEAMS** 

**Reilly Road Industrial Park** 

Fayetteville, N.C. 28309

Phone: (910) 864-8787 Fax: (910) 864-4444

comply with the prescriptive Code tts. The contractor shall refer to the ables (derived from the prescriptive C its) to determine the minimum found umber of wood studs required to supp

reater than 3000# but not greater than istered design professional shall be sign the support system for any hat exceeds those specified in the atta A registered design professional shall be d to design the support system for all ons that exceed 15000#.

Jonathan Landry

Jonathan Landry

ñ ñ

REQ'D STUDS (3) PLY HEAL NEACTI (UP TO)

BB

2550 1

5100 2

7650 3

10200 4

12750 5

15300 6

2EQ'

E 1700 1 3400 2

5100 3

6800 4

8500 5

10200 6

11900 7

13600 8 15300 9

Дa

Sanford

СІТУ / СО.

ent

Weaver Developn

BUILDER

an S

3400 1 6800 2

10200 3

13600 4

17000 5

Jonathan Landry

DRAWN BY

03/01/23

DATE REV.

Floor

СЪ

26RF,

 $\overline{}$ 

Magnolia II "C"

PLAN

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

N/A

SEAL DATE

#

QUOTE

Holly Place

 $\sim$ 

Lot

JOB NAME

.

ADDRESS MODEL

Lenny Norris

SALES REP.

J0223-0920

#

JOB

END

LOAD CHART FOR JACK STUDS (BASED ON TABLES R502.5(1) & (b)) NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)







▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)



|   |   |  | Client:   | Weaver Developn  | nent                  | Da                      | ate:     | 3/1/2023                                  |                        | Page 2 of 6                       |
|---|---|--|---|--|-----------------------|-------------------------|----------|---|------------------------|-----------------------------------|
| Tic   | Design  |  | Project:  | Magnolia II  |                       | Inj                     | put by:  | Jonathan Landry                           |                        |                                   |
|   | Design  |  | Address.  |  |                       | JU                      | oiect #  | . LOL 2 HOILY FIACE                       |                        |                                   |
|   | Karta C   | 1 \ /1                                       | 4 7501  | V 40 000   |                       |                         | Ujeci #. | _evel: Level                              |                        |                                   |
| BINI  | Kerto-S   | LVL  | 1.750   | X 16.000   | 2-Piy                 | PASSEL                  | ן נ      |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
| •   | • •   | •  | •   | • •  | • •                   | •                       | •        | • •                                       | • • •                  | $\Pi \uparrow$                    |
|   |   |  |   |  |                       |                         |          |   |                        | <u>5</u>                          |
| •   | •   | •  | • •   | • •  | •                     | • •                     | •        | • •                                       | •                      | 1'4"                              |
| •   | • •   | •  | •   | • •  | • •                   | •                       | •        | • •                                       | • • • •                | ¥ []]                             |
| 1 SPF   |   |  |   |  |                       |                         |          |   | 2 SPF                  | Λ                                 |
| ,   |   |  |   |  | 4.44.69               |                         |          |   |                        |                                   |
|   |   |  |   |  | 14.10"                |                         |          |   |                        | 3 1/2"                            |
| 1   |   |  |   |  | 14'10"                |                         |          |   | 1                      |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
| Multi-Ply A                                   | nalysis   |  |   |  |                       |                         |          |   |                        |                                   |
| Fasten all pl                                 | ies usina 3   | rows of 1                                    | 0d Box nails                                    | (.128x3") at 12'   | ' o.c Maxim           | um end dista            | nce no   | ot to exceed 6".                          |                        |                                   |
| Capacity                                      |   | 27.6   | %   | (  |                       |                         |          |   |                        |                                   |
| Load  |   | 78.0   | PLF   |  |                       |                         |          |   |                        |                                   |
| Yield Limit per F<br>Yield Limit per F        | -oot<br>-astener  | 282.4<br>94 1                                | l PLF<br>lb                                     |  |                       |                         |          |   |                        |                                   |
| Yield Mode                                    |   | IV   |   |  |                       |                         |          |   |                        |                                   |
| Edge Distance                                 |   | 1 1/2  |   |  |                       |                         |          |   |                        |                                   |
| Min. End Distan<br>Load Combinati             | ice<br>ion  | 3"<br>D+S                                    |   |  |                       |                         |          |   |                        |                                   |
| Duration Factor                               | ·   | 1.15   |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
|   |   |  |   |  |                       |                         |          |   |                        |                                   |
| Notes   |   |  | chemicals                                       |  | 6. For flat roofs pro | vide proper drainage to | prevent  | Manufacturer Info                         | Comtech<br>Reilly Road | Industrial Park P.O. Box 40408, N |
| Calculated Structured<br>structural adequacy  | d Designs is responsible<br>of this component ba                  | le only of the Hased on the 1.               | Andling & Installa<br>LVL beams must not be     | tion<br>cut or drilled                                     | portaing              |                         |          | Metsä Wood<br>301 Merritt 7 Building, 2nd | d Floor 28309          |                                   |
| responsibility of the<br>ensure the composite | d loadings shown.<br>customer and/or the onent suitability of the | It is the 2.<br>contractor to<br>he intended | Refer to manufactu<br>regarding installation    | requirements, multi-ply                                    |                       |                         |          | Norwalk, CT 06851<br>(800) 622-5850       | 910-864-878            | 37                                |
| application, and to ve                        | rify the dimensions and   | d loads.                                     | approvals<br>Damaged Beams must                 | strength values, and code not be used                      |                       |                         |          | www.metsawood.com/us                      |                        |                                   |
| Dry service condit     VI not to be train     | tions, unless noted other   | erwise 5.                                    | Design assumes top ed<br>Provide lateral suppor | ge is laterally restrained<br>t at bearing points to avoid |                       |                         |          |   |                        | отесн                             |
| <ol> <li>LVL HOL TO DE TRE</li> </ol>         | aco war me retardant  | or corrosive                                 | lateral displacement an                         | d rotation   | This design is        | valid until 11/3/2024   | 1        |   |                        |                                   |
| Version 21.80/117                             | Powered by iStru  | setTM Datacati                               | 220610011                                       |  |                       |                         |          |   |                        |                                   |

| 2  |  | Clien<br>Proje   | nt: We<br>ect: Ma                      | eaver Develo<br>Ignolia II               | oment                |                 | D<br>In           | ate:<br>put by: | 3/1/2023<br>Jonathar        | n Landry            |            |                       |                    | Page 3 of 6         |
|--|--|--|--|--|----------------------|-----------------|-------------------|-----------------|-----------------------------|---------------------|------------|-----------------------|--------------------|---------------------|
| is   | Design   | Addr   | ress:                                  | 5  |                      |                 | Jo                | bb Name         | e: Lot 2 Ho                 | lly Place           |            |                       |                    |                     |
|  | Corto S I VI   | 1 75   |  | 16 000                                   | " <b>ว</b> เ         |                 |                   | roject #:       | J0223-09<br>Level: Level    | 920                 |            |                       |                    |                     |
|  |  | . 1.75   |  | 10.000                                   | <b>Z</b> -r          | - iy - P        | AJJEI             |                 |                             |                     |            |                       |                    |                     |
|  |  |  |  |  |                      |                 |                   |                 |                             |                     |            |                       |                    |                     |
|  |  |  |  |  |                      |                 |                   |                 |                             |                     |            |                       |                    |                     |
|  | 2  |  |  |  |                      |                 |                   |                 | 3.                          |                     |            |                       |                    |                     |
|  |  |  |  | 1  |                      |                 |                   |                 |                             |                     |            |                       |                    |                     |
| •  | •  | • •  |  | •  | •                    | •               | •                 | •               | • •                         |                     |            |                       | M                  | $\overline{1}$      |
| •  | •  | •  | •                                      |  | •                    | •               | •                 | • •             |                             |                     |            |                       | XIX                | 1'4"                |
|  | a riter  | inger .  |  |  | 111/11               | all.            |                   |                 | in the second               |                     |            |                       | Ŵ                  |                     |
| 1 SPF  |  |  |  |  |                      |                 |                   | :               | 2 SPF                       |                     |            |                       |                    |                     |
|  |  |  |  | 11'3"                                    |                      |                 |                   |                 | $\rightarrow$               |                     |            |                       |                    | 1/2"                |
| /  |  |  |  | 11'3"                                    |                      |                 |                   |                 | $\rightarrow$               |                     |            |                       |                    |                     |
|  |  |  |  |  |                      |                 |                   |                 |                             |                     |            |                       |                    |                     |
| Member In  | formation  |  | A 11 11                                |  |                      |                 | Reaction          | ns UN           | PATTERN                     | IED Ib (l           | Jplift)    |                       |                    |                     |
| Type:<br>Plies:  | Girder<br>2  |  | Application:<br>Design Met             | hod: ASI                                 | or<br>D              |                 | Brg Dire          | ection          | LIVE<br>3555                | De<br>12            | ead 3      | Snow<br>0             | VVind<br>0         | Const<br>0          |
| Moisture Cond  | dition: Dry  | 6  | Building Co                            | de: IBC                                  | /IRC 2015            |                 | 2 Vert            | tical           | 4305                        | 1:                  | 507        | 0                     | 0                  | 0                   |
| Deflection LL:   | 480  | l  | Load Sharir                            | ng: No                                   |                      |                 |                   |                 |                             |                     |            |                       |                    |                     |
| Deflection TL:   | 360  |  | Deck:                                  | Not                                      | Checked              |                 |                   |                 |                             |                     |            |                       |                    |                     |
| Temperature:   | Normai - II<br>Temp <= 100°F   |  | Celling:                               | Gyp                                      | sum 1/2"             |                 |                   |                 |                             |                     |            |                       |                    |                     |
| remperature.   | 1611p <= 100 1   |  |  |  |                      |                 | Bearing           | s               |                             |                     |            |                       |                    |                     |
|  |  |  |  |  |                      |                 | Bearing           | Lengt           | h Dir.                      | Cap. Re             | act D/L lb | Total                 | Ld. Case           | Ld. Comb.           |
|  |  |  |  |  |                      |                 | 1 - SPF           | 4.000"          | Vert                        | 81% 12              | 257 / 3555 | 4812                  | L                  | D+L                 |
| Analysis De  | a  |  |  |  |                      |                 | 2 - SPF           | 4.000"          | Vert                        | 98% 1               | 507 / 4305 | 5812                  | L                  | D+L                 |
| Analysis Ke  | Actual Lo  | ocation Allov  | wed (                                  | Capacity                                 | Comb                 | Case            | 1                 |                 |                             |                     |            |                       |                    |                     |
| Moment   | 12309 ft-lb  | 5'7 1/2" 3456  | 65 ft-lb 0                             | ).356 (36%)                              | D+L                  | L               |                   |                 |                             |                     |            |                       |                    |                     |
| Unbraced   | 12309 ft-lb  | 5'7 1/2" 1231  | 10 ft-lb 1                             | .000                                     | D+L                  | L               |                   |                 |                             |                     |            |                       |                    |                     |
| Shear  | 4527 lb  | 1'9" 1104  | (<br>1716 (                            | 100%)<br>) 370 (38%)                     | L+L                  |                 |                   |                 |                             |                     |            |                       |                    |                     |
| Snear  | 4527 ID<br>0.098 (I./1320)   | 18 1194<br>5'7 1/2" 0.268                                      | 8 (1 /480) (                           | ).379 (38%)                              | D+L                  | L               |                   |                 |                             |                     |            |                       |                    |                     |
| TL Defl inch   | 0.132 (L/975)  | 5'7 1/2" 0.358   | 8 (L/360) 0                            | ).369 (37%)                              | -<br>D+L             | L               |                   |                 |                             |                     |            |                       |                    |                     |
| Design Not   | tes  |  |  | . ,                                      |                      |                 | 1                 |                 |                             |                     |            |                       |                    |                     |
| 1 Provide su   | pport to prevent lateral   | movement and   | d rotation at                          | the end bea                              | rings. Later         | al support      | 4                 |                 |                             |                     |            |                       |                    |                     |
| may also b<br>2 Fasten all r   | e required at the interio  | or bearings by t<br>Id Box nails ( 1                           | the building                           | code.<br>2" o.c. Maxin                   | um end dis           | stance not      |                   |                 |                             |                     |            |                       |                    |                     |
| to exceed 6  | 5".  | d Dox rialis (. i  | 2010 / 41 1                            | 2 0.0. Maxin                             |                      |                 |                   |                 |                             |                     |            |                       |                    |                     |
| 3 Refer to las<br>4 Concentrat   | st page of calculations f<br>red load fastener specifi   | for fasteners re<br>ication is in add                          | equired for s                          | specified load                           | ls.<br>'s if a hange | er is           |                   |                 |                             |                     |            |                       |                    |                     |
| present.   |  |  |  | iger lactories                           | o ir a nangi         |                 |                   |                 |                             |                     |            |                       |                    |                     |
| 5 Girders are<br>6 Top must b  | e designed to be suppor<br>e laterally braced at a r   | rted on the bot<br>maximum of 9'9                              | tom edge o<br>9 3/16" o.c.             | nly.                                     |                      |                 |                   |                 |                             |                     |            |                       |                    |                     |
| 7 Lateral sler   | nderness ratio based or  | n single ply wid   | dth.                                   |  |                      |                 |                   |                 |                             |                     |            |                       |                    |                     |
| ID   | Load Type  | Loca   | ation Trik                             | Width S                                  | ide                  | Dead 0.9        | Live              | 1 Sno           | ow 1.15                     | Wind 1.6            | Const. 1.2 | 25 Cor                | nments             |                     |
| 1  | Uniform  |  |  | F  | ar Face              | 90 PLF          | 269 PL            | F               | 0 PLF                       | 0 PLF               | 0 PI       | LF F1                 |                    |                     |
| 2  | Uniform  |  |  | N  | ear Face             | 121 PLF         | 363 PL            | F               | 0 PLF                       | 0 PLF               | 0 PI       | LF F4                 |                    |                     |
| 3  | Point  | 11   | 1-0-0                                  | Ν  | ear Face             | 250 lb          | 750 I             | b               | 0 lb                        | 0 lb                | 0          | lb F4A                |                    |                     |
|  | Self Weight  |  |  |  |                      | 12 PLF          |                   |                 |                             |                     |            |                       |                    |                     |
|  |  |  |  |  |                      |                 |                   |                 |                             |                     |            |                       |                    |                     |
| Notes  |  | chemicals  |  |  | 6. For flat          | roofs provide p | roper drainage to | prevent         | Manufactur                  | er Info             |            | Comtech<br>Reilly Roa | ad Industrial Park | : P.O. Box 40408, N |
| Calculated Structured<br>structural adequacy                           | Designs is responsible only of the<br>of this component based on the                                 | <ul> <li>Handling &amp; Ir</li> <li>1. LVL beams mu</li> </ul> | nstallation<br>ust not be cut or o     | drilled                                  | ponding              | I               |                   |                 | Metsä Wood<br>301 Merritt 7 | l<br>' Building, 2n | d Floor    | USA<br>28309          |                    |                     |
| design criteria and<br>responsibility of the o<br>ensure the component | a loadings shown. It is the<br>customer and/or the contractor to<br>nent suitability of the intender | e 2. Refer to m<br>regarding in                                | nanufacturer's<br>nstallation requ     | product informat<br>urements, multi-     | on<br>ply<br>de      |                 |                   |                 | Norwalk, CT<br>(800) 622-59 | 06851               |            | 910-864-8             | 787                |                     |
| application, and to ver  | ify the dimensions and loads.  | approvals<br>3. Damaged Bea                                    | ams, beam streng                       | un values, and co<br>used                | uc                   |                 |                   |                 | www.metsav                  | vood.com/us         |            |                       |                    |                     |
| 1. Dry service condit<br>2. LVL not to be treat                        | ions, unless noted otherwise<br>ated with fire retardant or corrosive                                | 4. Design assume<br>5. Provide latera                          | es top edge is lat<br>al support at be | terally restrained<br>aring points to av | bid                  |                 |                   |                 |                             |                     |            |                       | от                 | есн                 |
|  |  | iateral displace   | ement and rotatio                      | N 1                                      | This d               | lesign is valid | until 11/3/2024   | 4               |                             |                     |            |                       |                    |                     |



Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6".

| Capacity                 | 98.6 %    |
|--------------------------|-----------|
| Load                     | 242.0 PLF |
| Yield Limit per Foot     | 245.6 PLF |
| Yield Limit per Fastener | 81.9 lb.  |
| Yield Mode               | IV        |
| Edge Distance            | 1 1/2"    |
| Min. End Distance        | 3"        |
| Load Combination         | D+L       |
| Duration Factor          | 1.00      |

# Concentrated Load

Fasten at concentrated side load at 11-0-0 with a minimum of (9) - 10d Box nails (.128x3") in the

pattern shown.

# Min/Max fastener distances for Concentrated Side Loads





Version 21.80.417 Powered by iStruct<sup>™</sup> Dataset: 22061001.1

|   |                                | Client:                         | Weaver Developm   | ent                   | Date <sup>.</sup>              | 3/1/2023               |                             | Page 6 of 6                 |
|---|--------------------------------|---------------------------------|---|-----------------------|--------------------------------|------------------------|-----------------------------|-----------------------------|
|   |                                | Project <sup>.</sup>            | Magnolia II   | ont                   | Input b                        | ov: Jonathan Landry    |                             | r ago o or o                |
| <b>isDesign</b>   | n                              | Address                         |   |                       | Job Na                         | ame: Lot 2 Holly Place |                             |                             |
|   |                                | , (44, 666).                    |   |                       | Project                        | t #                    |                             |                             |
|   | 0.1.7                          | 4                               | <u> </u>  |                       |                                |                        |                             |                             |
| GDH Kerto-  | SLVL                           | 1.750"                          | X 11.875"   | 2-Ply                 | PASSED                         | Level. Level           |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                | •                      |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
| • • •   | •                              | • •                             | • •   | • •                   | • •                            | • • •                  | 9                           | $\Pi \uparrow$              |
|   |                                |                                 |   |                       |                                |                        | ~                           | MM L                        |
|   | •                              |                                 |   |                       |                                |                        | <u> </u>                    | -   11 7/8"                 |
| 1 SPE End Grain   |                                |                                 |   |                       |                                |                        | 2 SPE End Grain             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   | 16'10"                |                                |                        |                             | 3 1/2"                      |
| /   |                                |                                 |   | 101101                |                                |                        |                             |                             |
|   |                                |                                 |   | 16'10"                |                                |                        | .                           |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
| Multi Dhy Analysia  |                                |                                 |   |                       |                                |                        |                             |                             |
| wulli-Piy Analysis  |                                |                                 |   |                       |                                |                        |                             |                             |
| Fasten all plies using  | 2 rows of 10                   | 0d Box nails                    | (.128x3") at 12"  | o.c Maxim             | um end distance                | not to exceed 6".      |                             |                             |
| Capacity  | 0.0 %                          | I                               |   |                       |                                |                        |                             |                             |
| Load  | 0.0 PI                         | LF                              |   |                       |                                |                        |                             |                             |
| Yield Limit per Foot  | 163.7                          | PLF                             |   |                       |                                |                        |                             |                             |
| Yield Limit per Fastener  | 81.91                          | D.                              |   |                       |                                |                        |                             |                             |
| Field Mode<br>Edge Distance   | 1 1/2"                         |                                 |   |                       |                                |                        |                             |                             |
| Min. End Distance   | 3"                             |                                 |   |                       |                                |                        |                             |                             |
| Load Combination  | Ũ                              |                                 |   |                       |                                |                        |                             |                             |
| Duration Factor   | 1.00                           |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                |                        |                             |                             |
|   |                                |                                 |   |                       |                                | 1                      |                             |                             |
| Notes   |                                | chemicals                       |   | 6. For flat roofs pro | vide proper drainage to prever | Manufacturer Info      | Comtech<br>Reilly Road Indu | ustrial Park P.O. Box 40408 |
| Calculated Structured Designs is respo<br>structural adequacy of this component                         | nsible only of the Ha          | andling & Installa              | tion  | ponding               |                                | Metsä Wood             | USA<br>28309                | ,                           |
| design criteria and loadings sho<br>responsibility of the customer and/or                               | the contractor to              | Refer to manufactu              | urer's product information                                  |                       |                                | Norwalk, CT 06851      | 910-864-8787                |                             |
| ensure the component suitability of<br>application, and to verify the dimensions                        | of the intended<br>s and loads | fastening details, bear         | n strength values, and code                                 |                       |                                | (800) 622-5850         |                             |                             |
| Lumber  | <u>.</u>                       | approvals<br>Damaged Beams must | not be used   |                       |                                | www.metsawood.com/u    |                             |                             |
| <ol> <li>Dry service conditions, unless noted</li> <li>LVL not to be treated with fire reter</li> </ol> | d otherwise 5.                 | Provide lateral support         | ige is laterally restrained<br>t at bearing points to avoid |                       |                                |                        |                             | тесн                        |
| be added with the feld  |                                | rateral displacement an         | u rotation  | This design is        | valid until 11/3/2024          |                        |                             |                             |



RE: J0223-0920 Lot 2 Holly Place Trenco 818 Soundside Rd Edenton, NC 27932

# Site Information:

Customer: Weaver Development<br/>Lot/Block: 2Project Name: J0223-0920<br/>Model: Magnolia IIAddress:Subdivision: Holly Place<br/>State: NC

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: N/A Roof Load: N/A psf Design Program: MiTek 20/20 8.4 Wind Speed: N/A mph Floor Load: 55.0 psf

This package includes 11 individual, dated Truss Design Drawings and 0 Additional Drawings.

| No. | Seal#     | Truss Name | Date      |
|-----|-----------|------------|-----------|
| 1   | 156812896 | ET1        | 2/23/2023 |
| 2   | 156812897 | ET2        | 2/23/2023 |
| 3   | 156812898 | ET3        | 2/23/2023 |
| 4   | 156812899 | ET4        | 2/23/2023 |
| 5   | 156812900 | F1         | 2/23/2023 |
| 6   | 156812901 | F2         | 2/23/2023 |
| 7   | 156812902 | F3         | 2/23/2023 |
| 8   | 156812903 | F4         | 2/23/2023 |
| 9   | 156812904 | F4A        | 2/23/2023 |
| 10  | 156812905 | F5         | 2/23/2023 |
| 11  | 156812906 | F6         | 2/23/2023 |

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2023

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



| Job    |               | Tru          | ISS       |          |           | Truss Ty  | /pe      |                  |         |        | Qty     | F       | Ply       | Lot 2 Ho | olly Place    |           |           |         |             |                   |       |
|--------|---------------|--------------|-----------|----------|-----------|-----------|----------|------------------|---------|--------|---------|---------|-----------|----------|---------------|-----------|-----------|---------|-------------|-------------------|-------|
|        |               |              |           |          |           |           |          |                  |         |        |         |         |           |          |               |           |           |         | I.          | 56812896          | ز     |
| J0223- | 0920          | ET           | 1         |          |           | GABLE     |          |                  |         |        | 1       |         | 1         |          |               |           |           |         |             |                   |       |
|        |               |              |           |          |           |           |          |                  |         |        |         |         | 100 1     | Job Ref  | erence (optio | nal)      | TI E I    |         | - 40 0000 - |                   |       |
| Com    | tech, Inc,    | ayetteville, | NC - 283  | 314,     |           |           |          |                  |         |        |         | 8.4     | 430 s Jar | 1 6 2022 | MITEK Indust  | ries, inc | . Inu Feb | 23 08:3 | 5:49 2023 F | age 1             |       |
|        |               |              |           |          |           |           |          |                  |         | D      | IWPOH6n | iK8Jept | tosxqQU   | JJCyzm6  | C-UYONAHWI    | AXXU9V    | VGLNXZIVK | dEms1   | 4y6_XIMSEF  | IZIJAU            |       |
|        |               |              |           |          |           |           |          |                  |         |        |         |         |           |          |               |           |           |         |             | 0 <sub>1</sub> 18 |       |
|        |               |              |           |          |           |           |          |                  |         |        |         |         |           |          |               |           |           |         | Scal        | le = 1:22.        | 1     |
|        |               |              |           |          |           |           |          |                  |         |        |         |         |           |          |               |           |           |         |             |                   |       |
|        |               |              |           |          |           |           |          |                  |         |        |         |         |           |          |               |           |           |         |             |                   |       |
|        | 3x4           |              |           |          |           |           |          |                  |         |        |         |         |           |          |               |           |           |         |             |                   |       |
|        | 1             | 2            |           | 3        |           | 4         |          | 5 <sup>3x4</sup> | =       | 6      |         | 7       |           | 8        |               | 9         |           | 10      | 11          |                   |       |
| I      |               |              |           |          |           |           |          | 1                |         |        |         |         |           |          |               |           |           |         | 0           | I I               |       |
| 1-4-0  |               |              |           |          |           |           |          |                  |         |        |         |         |           |          |               |           |           |         |             | 23                | 1-4-0 |
|        | H+            |              |           | Ц        |           | Ц         |          |                  |         | $\sum$ |         |         |           | Ц        |               |           |           | Ц       |             |                   |       |
|        |               | •            |           | •        |           | •         |          | •                |         |        |         | •       |           | •        |               | •         |           | •       |             | ₩                 |       |
| -      |               |              |           |          |           |           |          |                  |         |        |         |         | $\times$  |          |               |           |           |         |             | x -               |       |
|        | 22            | 21           |           | 20       |           | 19        |          | 18               |         | 17     |         | 16      |           | 15       |               | 14        |           | 13      | 12          | 2                 |       |
|        | 3x4           |              |           |          |           |           |          |                  |         | 3x4 =  |         |         |           |          |               |           |           |         | 3x-         | 4 =               |       |
|        |               |              |           |          |           |           |          |                  |         |        |         |         |           |          |               |           |           |         |             |                   |       |
|        |               |              |           |          |           |           |          |                  |         |        |         |         |           |          |               |           |           |         |             |                   |       |
|        |               |              |           |          |           |           |          |                  |         |        |         |         |           |          |               |           |           |         |             |                   |       |
|        |               |              |           |          |           |           |          |                  |         |        |         |         |           |          |               |           |           |         |             |                   |       |
|        | L 1-4-0       | I            | 2-8-0     | I        | 4-0-0     | 1         | 5-4-0    |                  | 6-8-0   | 1      | 8-0-0   |         | 9-4-0     | )        | 10-8-0        | I         | 12-0-0    |         | 13-3-8      |                   |       |
|        | 1-4-0         |              | 1-4-0     |          | 1-4-0     |           | 1-4-0    |                  | 1-4-0   |        | 1-4-0   |         | 1-4-0     | ) 1      | 1-4-0         |           | 1-4-0     |         | 1-3-8       | 1                 | _     |
| Plate  | Offsets (X,Y) | [1:Edge      | e,0-1-8], | [5:0-1-8 | ,Edge], [ | 17:0-1-8, | Edge], [ | 22:Edge          | ,0-1-8] |        |         |         |           |          |               |           |           |         |             |                   | _     |
|        |               |              | -         |          |           |           | -        |                  |         | -      | -       | -       |           | -        |               |           |           | -       |             | -                 | _     |

| LOADING         (ps           TCLL         40.           TCDL         10.           BCLL         0.           BCDL         5. | sf)<br>).0<br>).0<br>).0<br>5.0 | SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014 | CSI.<br>TC 0.06<br>BC 0.01<br>WB 0.03<br>Matrix-S | DEFL. ir<br>Vert(LL) n/z<br>Vert(CT) n/z<br>Horz(CT) 0.00 | (loc)<br>-<br>-<br>12 | l/defl<br>n/a<br>n/a<br>n/a | L/d<br>999<br>999<br>n/a | PLATES<br>MT20<br>Weight: 62 lb | <b>GRIP</b><br>244/190<br>FT = 20%F, 11%E |
|---|---------------------------------|---|---|---|-----------------------|-----------------------------|--------------------------|---------------------------------|---|
| LUMBER-<br>TOP CHORD<br>BOT CHORD   | 2x4 SP<br>2x4 SP                | No.1(flat)<br>No.1(flat)  |   | BRACING-<br>TOP CHORD                                     | Structu<br>except     | ral wood<br>end verti       | sheathing dii<br>cals.   | rectly applied or 6-0-0         | oc purlins,                               |

2x4 SP No.3(flat) BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.3(flat)

REACTIONS. All bearings 13-3-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 21, 20, 19, 18, 17, 16, 15, 14, 13

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

WEBS

OTHERS

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



| Job                   | Truss              | Truss Type | Qtv | Plv         | Lot 2 Holly Place  |          |
|-----------------------|--------------------|------------|-----|-------------|--|----------|
|                       |                    |            |     | ,           |  | 56812897 |
| J0223-0920            | ET2                | GABLE      | 1   | 1           |  |          |
|                       |                    |            |     |             | Job Reference (optional)                                 |          |
| Comtech, Inc, Fayette | rille, NC - 28314, |            |     | 3.430 s Jar | 6 2022 MiTek Industries, Inc. Thu Feb 23 08:35:50 2023 F | Page 1   |

8.430 s Jan 6 2022 MiTek Industries, Inc. Thu Feb 23 08:35:50 2023 Page 1 ID:IwPOH6hK8Jeptt6SXqQOJcyzm6C-MkMlodWKxF3LngrXEEU\_2YNPWGNJhZEh\_0bBnBziJAt

0-<u>1</u>-8

Scale = 1:27.8



| <u>1-4-0</u><br>1-4-0   | 2-8-0 4-0-0 5-4-0<br>1-4-0 1-4-0 1-4-0   | <u>6-8-0 8-0-0</u><br>1-4-0 1-4-0                 | 9-4-0 10-8   | -0 <u>12-0-0</u><br>-0 1-4-0                     | 13-4-0<br>1-4-0          | 14-8-0                          | 16-0-0 16-7-8<br>1-4-0 0-7-8             |
|---|--|---|--|--|--------------------------|---------------------------------|--|
| Plate Offsets (X,Y)   | [1:Edge,0-1-8], [6:0-1-8,Edge], [23:0  | -1-8,Edge], [30:Edge,0-1-8]                       |  |  |                          |                                 |  |
| LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDI         5.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.00<br>Lumber DOL 1.00<br>Rep Stress Incr YES<br>Code. IRC2015/TPI2014 | CSI.<br>TC 0.06<br>BC 0.01<br>WB 0.03<br>Matrix-S | DEFL. ii<br>Vert(LL) n/a<br>Vert(CT) n/a<br>Horz(CT) -0.00 | n (loc) l/defl<br>a - n/a<br>a - n/a<br>0 16 n/a | L/d<br>999<br>999<br>n/a | PLATES<br>MT20<br>Weight: 77 lb | <b>GRIP</b><br>244/190<br>FT = 20%F 11%F |
| LUMBER-<br>TOP CHORD 2x4 SF<br>BOT CHORD 2x4 SF   | 2 No.1(flat)   |   | BRACING-<br>TOP CHORD                                      | Structural wood                                  | sheathing dire           | ectly applied or 10-0           | -0 oc purlins,                           |

WEBS 2x4 SP No.3(flat)

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 16-7-8.

2x4 SP No.3(flat)

Max Uplift All uplift 100 lb or less at joint(s) 16 (lb) -

Max Grav All reactions 250 lb or less at joint(s) 30, 29, 28, 27, 26, 24, 23, 22, 21, 20, 19, 18, 17

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

OTHERS

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16.

7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

8) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



<sup>1)</sup> All plates are 1.5x3 MT20 unless otherwise indicated.

| Job                  | Truss              |   | Truss Type                              |   |   |          | Qty      | Ply       |      | Lot 2 Holly | Place          |                      |  |                |        |
|----------------------|--------------------|---|---|---|---|----------|----------|-----------|------|-------------|----------------|----------------------|--|----------------|--------|
| 10222 0020           | ГТЭ                |   |   |   |   |          | 1        |           | 4    |             |                |                      |  | 1568           | 12898  |
| JUZZ3-09Z0           | EIS                |   | GADLE                                   |   |   |          | 1        |           | 1    | Job Referer | nce (optional) | )                    |  |                |        |
| Comtech, Inc, Fayett | eville, NC - 28314 | ,                                       |   |   |   |          |          | 8.430 s   | Jan  | 6 2022 MiT  | ek Industries  | s, Inc. <sup>-</sup> | Thu Feb 23 08:35                       | :52 2023 Page  | e 1    |
|                      |                    |   |   |   |   | ID:lw    | POH6hk   | (8Jeptt6S | XqQ  | OJcyzm6C-   | I7TWCJYaTs     | J20z_v               | vMfWS7zSk133n                          | 9TkzRK4Is4ziJ  | lAr    |
| 0-1-8                |                    |   |   |   |   |          |          |           |      |             |                |                      |  | 0- <u>1</u> -8 | 8      |
|                      |                    |   |   |   |   |          |          |           |      |             |                |                      |  | Scale =        | 1:29.7 |
|                      |                    |   |   |   |   |          |          |           |      |             |                |                      |  |                |        |
|                      |                    |   |   |   |   |          |          |           |      |             |                |                      |  |                |        |
|                      |                    |   |   |   |   |          |          |           |      |             |                |                      |  |                |        |
|                      |                    |   |   |   | 3x4 ≡                                   |          | 3x6 FP   | =         |      |             |                |                      |  |                |        |
| 1 2                  | 3                  | 4                                       | 5                                       | 6                                       | 7                                       | 8        | 9 1      | 0         | 1    | 1           | 12             | 13                   | 14                                     | 15 16          |        |
|                      | •                  | 0                                       | •                                       | •                                       | - R                                     | •        | <u> </u> | •         |      | •           | •              | -                    | •                                      |                | ~      |
|                      |                    |   |   |   |   |          |          |           |      |             |                |                      |  |                | 34 C   |
|                      |                    |   |   |   |   |          |          |           |      |             |                |                      |  |                | -      |
|                      |                    |   |   | •                                       |   | <u>N</u> |          |           | ~~~~ | •           |                |                      |  |                | l      |
|                      | *****              | ~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~ | ~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~ | ~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~ | ~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~ | ~~~~~    |          |           | ~~~~ |             |                | XXXXX                | ×××××××××××××××××××××××××××××××××××××× | *********      |        |
| 32 31                | 30                 | 29                                      | 28 27                                   | 26                                      | 25                                      | 24       | 2        | 23        | 2    | 2           | 21             | 20                   | 19                                     | 18 17          |        |
| 3x4 =                |                    |   | 3x6 FP                                  | =                                       |   | 3x4 =    |          |           |      |             |                |                      |  | 3x4 =          | =      |
|                      |                    |   |   |   |   |          |          |           |      |             |                |                      |  |                |        |
|                      |                    |   |   |   |   |          |          |           |      |             |                |                      |  |                |        |
|                      |                    |   |   |   |   |          |          |           |      |             |                |                      |  |                |        |
|                      |                    |   |   |   |   |          |          |           |      |             |                |                      |  |                |        |
|                      |                    |   |   |   |   |          |          |           |      |             |                |                      |  |                |        |

| <u>1-4-0</u><br> -4-0<br>  −4-0                     | 2-8-0 4-0-0<br>1-4-0 1-4-0<br>[7:0-1-8.Edge], [24:0-1-8.E   | 5-4-0<br>1-4-0<br>Edgel      | 6-8-0<br>1-4-0         | 8-0-0<br>1-4-0       | 9-4-0 10-8-<br>1-4-0 1-4-0                | <u>0   1</u>             | 12-0-0<br>1-4-0       | 13-4-0<br>1-4-0                             | 14-8-0   16-0-0  <br>1-4-0 1-4-0 | <u>17-4-0</u> <u>17-10-</u> ρ<br><u>1-4-0</u> 0-6-0 |
|---|---|------------------------------|------------------------|----------------------|---|--------------------------|-----------------------|---|----------------------------------|---|
| LOADING (psf)<br>TCLL 40.0<br>TCDL 10.0<br>BCLL 0.0 | SPACING-<br>Plate Grip DOL<br>Lumber DOL<br>Rep Stress Incr | 2-0-0<br>1.00<br>1.00<br>YES | CSI.<br>TC<br>BC<br>WB | 0.06<br>0.01<br>0.03 | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT) | in<br>n/a<br>n/a<br>0.00 | (loc)<br>-<br>-<br>24 | l/defl L/d<br>n/a 999<br>n/a 999<br>n/a n/a | PLATES<br>MT20                   | <b>GRIP</b><br>244/190                              |
| BCDL 5.0  | Code IRC2015/TPI  | 12014                        | Matrix                 | x-S                  |   |                          |                       |   | Weight: 82 lb                    | FT = 20%F, 11%E                                     |
| LUMBER-<br>TOP CHORD 2x4 SF<br>BOT CHORD 2x4 SF     | ? No.1(flat)<br>? No.1(flat)                                |                              |                        |                      | BRACING<br>TOP CHOR                       | 2D 5                     | Structura<br>except e | al wood sheathir<br>and verticals.          | ng directly applied or 6-0-      | 0 oc purlins,                                       |

WEBS 2x4 SP No.3(flat) BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. OTHERS 2x4 SP No.3(flat)

REACTIONS.

All bearings 17-10-0.
 (lb) - Max Uplift All uplift 100 lb or less at joint(s) 17 Max Grav All reactions 250 lb or less at joint(s) 32, 31, 30, 29, 28, 26, 25, 24, 23, 22, 21, 20, 19, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17.

7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



#### LUMBER-

 TOP CHORD
 2x4 SP No.1(flat)

 BOT CHORD
 2x4 SP No.1(flat)

 WEBS
 2x4 SP No.3(flat)

 OTHERS
 2x4 SP No.3(flat)

 BRACING 

 TOP CHORD
 Structural wood sheathing directly applied or 3-6-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 3-6-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 8, 5, 7, 6

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

## LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 5-8=-10, 1-4=-100

Concentrated Loads (lb)

Vert: 2=-72 3=-76



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





| 13-3-8   |   |  |   |  |   |                        |  |  |  |  |  |  |
|--|---|--|---|--|---|------------------------|--|--|--|--|--|--|
| 1  |   |  | 13-3-8  |  |   | 1                      |  |  |  |  |  |  |
| Plate Offsets (X,Y) [1:Edge,0-1-8], [5:0-1-8,Edge], [13:0-1-8,Edge]                                      |   |  |   |  |   |                        |  |  |  |  |  |  |
| LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0 | SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYES | <b>CSI.</b><br>TC 0.34<br>BC 0.60<br>WB 0.31 | DEFL. ir<br>Vert(LL) -0.09<br>Vert(CT) -0.12<br>Horz(CT) 0.03 | n (loc) l/defl L/d<br>11-12 >999 480<br>11-12 >999 360<br>11-12 >999 360<br>9 n/a n/a        | <b>PLATES</b><br>MT20                         | <b>GRIP</b><br>244/190 |  |  |  |  |  |  |
| BCDL 5.0   | Code IRC2015/TPI2014  | Matrix-S                                     |   |  | Weight: 71 lb                                 | FT = 20%F, 11%E        |  |  |  |  |  |  |
| LUMBER-<br>TOP CHORD 2x4 SF<br>BOT CHORD 2x4 SF<br>WEBS 2x4 SF   | P No.1(flat)<br>P No.1(flat)<br>P No.3(flat)                    | I  | BRACING-<br>TOP CHORD<br>BOT CHORD                            | Structural wood sheathing dire<br>except end verticals.<br>Rigid ceiling directly applied or | ctly applied or 6-0-0<br>· 10-0-0 oc bracing. | oc purlins,            |  |  |  |  |  |  |

REACTIONS. (size) 15=Mechanical, 9=0-3-8 Max Grav 15=717(LC 1), 9=711(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1210/0, 3-4=-1904/0, 4-5=-1904/0, 5-6=-1819/0, 6-7=-1220/0

BOT CHORD 14-15=0/759, 13-14=0/1646, 12-13=0/1904, 11-12=0/1904, 10-11=0/1659, 9-10=0/754

2-15=-1010/0, 2-14=0/628, 3-14=-606/0, 3-13=0/505, 7-9=-1001/0, 7-10=0/648, WEBS

6-10=-611/0, 6-11=0/305, 5-11=-304/60

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





| <b> </b>   |   |   | 13-7-0<br>13-7-0   |   |  |   |
|--|---|---|--|---|--|---|
| Plate Offsets (X,Y)  | [5:0-1-8,Edge], [13:0-1-8,Edge]   |   |  |   |  |   |
| LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0          | SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014 | CSI.<br>TC 0.39<br>BC 0.65<br>WB 0.32<br>Matrix-S | <b>DEFL.</b> in<br>Vert(LL) -0.10<br>Vert(CT) -0.14<br>Horz(CT) 0.03 | i (loc) I/defl L/d<br>11-12 >999 480<br>11-12 >999 360<br>9 n/a n/a                   | <b>PLATES</b><br>MT20<br>Weight: 71 lb         | <b>GRIP</b><br>244/190<br>FT = 20%F, 11%E |
| LUMBER-           TOP CHORD         2x4 SP No.1(flat)           BOT CHORD         2x4 SP No.1(flat)           WEBS         2x4 SP No.3(flat) |   |   | BRACING-<br>TOP CHORD<br>BOT CHORD                                   | Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o | ectly applied or 6-0-0<br>r 10-0-0 oc bracing. | oc purlins,                               |
| REACTIONS. (siz<br>Max (   | e) 15=0-3-8, 9=0-3-8<br>Grav 15=727(LC 1), 9=727(LC 1)                              |   |  |   |  |   |

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1242/0, 3-4=-1987/0, 4-5=-1987/0, 5-6=-1882/0, 6-7=-1257/0

BOT CHORD 14-15=0/776, 13-14=0/1698, 12-13=0/1987, 11-12=0/1987, 10-11=0/1718, 9-10=0/770

WEBS 2-15=-1031/0, 2-14=0/649, 3-14=-634/0, 3-13=0/547, 7-9=-1022/0, 7-10=0/677,

6-10=-642/0, 6-11=0/302, 5-11=-320/48

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



| Job                    | Truss              | Truss Type | Qty     | Ply        | Lot 2 Holly Place                                    |                |
|------------------------|--------------------|------------|---------|------------|--|----------------|
|                        |                    |            |         |            |  | 156812902      |
| J0223-0920             | F3                 | Floor      | 6       | 1          |  |                |
|                        |                    |            |         |            | Job Reference (optional)                             |                |
| Comtech, Inc, Fayettev | ville, NC - 28314, |            | 8       | .430 s Jar | 6 2022 MiTek Industries, Inc. Thu Feb 23 08:35:57 20 | 23 Page 1      |
|                        |                    | ID:IwF     | OH6hK8J | eptt6SXq0  | QOJcyzm6C-f5HOG0cjHOxL6ltt9D6dq0AOb4ZRqcXjbcc        | 3XHziJAm       |
| 1-3-0                  | 2                  | -1-4       |         |            | 1-6-12   | 0-1-8          |
|                        |                    |            |         |            |  | Н              |
|                        |                    |            |         |            |  | Scale = 1:53.3 |
|                        |                    |            |         |            |  |                |
|                        |                    |            |         |            |  |                |



| Ļ        |                       | 17-11-12   |                               |               |  | 31-5-0  |                |                 |  |
|----------|-----------------------|--|-------------------------------|---------------|--|---|----------------|-----------------|--|
| 1        |                       | 17-11-12   |                               | l             |  | 13-5-4  |                |                 |  |
| Plate Of | ffsets (X,Y)          | [1:Edge,0-1-8], [17:0-1-8,Edge], [25:0-1           | -8,Edge], [31:0-1-8,Edge], [3 | 2:0-1-8,Edge] |  |   |                |                 |  |
|          | IC (nof)              |  | 681                           | DEEL          | in (loc)  /defl                                    | /4  |                |                 |  |
| LUADIN   | IG (pst)              | SPACING- 2-0-0                                     | CSI.                          | DEFL.         | In (IOC) I/defi                                    | _/d   | PLATES         | GRIP            |  |
| TCLL     | 40.0                  | Plate Grip DOL 1.00                                | TC 0.84                       | Vert(LL) -0.  | .22 32-33 >954 4                                   | 80  | MT20           | 244/190         |  |
| TCDL     | 10.0                  | Lumber DOL 1.00                                    | BC 0.87                       | Vert(CT) -0.  | .31 32-33 >698 3                                   | 60  |                |                 |  |
| BCLL     | 0.0                   | Rep Stress Incr YES                                | WB 0.57                       | Horz(CT) 0.   | .05 21 n/a i                                       | n/a   |                |                 |  |
| BCDL     | 5.0                   | Code IRC2015/TPI2014                               | Matrix-S                      |               |  |   | Weight: 164 lb | FT = 20%F. 11%E |  |
| 5055     |                       | 0000   |                               |               |  |   |                | 20,00,002       |  |
| LUMBE    | LUMBER- BRACING-      |  |                               |               |  |   |                |                 |  |
| TOP CH   | ORD 2x4 SF            | PNo.1(flat)  |                               | TOP CHORD     | Structural wood she                                | Structural wood sheathing directly applied or 5-8-3 oc purlins. |                |                 |  |
| BOT CH   | IORD 2x4 SF           | No.1 (flat)  |                               |               | except end verticals.                              |   |                |                 |  |
| WEBS     | 2x4 SF                | No 3(flat)   |                               | BOT CHORD     | Rigid ceiling directly applied or 6-0-0 oc bracing |   |                |                 |  |
| LDO      | 241 01                | No.o(nat)  |                               | DOT ONORD     | rugia coming anooniy                               |   | o oo brading.  |                 |  |
| REACT    | IONS (size            | e) 35=Mechanical 21=0-3-8 27=0-3-8                 | 3                             |               |  |   |                |                 |  |
| NEAU     | Mox C                 | 25 - 872(1 - 2) - 24 - 646(1 - 4) - 27 - 2         | ,<br>022/LC 1)                |               |  |   |                |                 |  |
|          | iviax G               | $5120^{-5}30^{-5}(10^{-5}), 21=040(10^{-4}), 21=2$ | 023(LC 1)                     |               |  |   |                |                 |  |
| FORCE    | C (III-) Marca        | O  | less successfully an allowing |               |  |   |                |                 |  |
| FURCE    | <b>3.</b> (ID) - Max. | Comp./wax. ren All forces 250 (lb) or              | less except when shown.       |               |  |   |                |                 |  |
| TOP CH   | iord 2-3=-            | ·1552/0, 3-4=-2532/0, 4-5=-2532/0, 5-6=            | -2790/0, 6-7=-2790/0, 7-8=-2  | 2790/0,       |  |   |                |                 |  |

8-9=-1850/0, 9-10=-1850/0, 10-12=-425/257, 12-13=0/1945, 13-14=0/1945, 14-15=-523/956, 15-16=-1501/333, 16-17=-1501/333, 17-18=-1536/105, 18-19=-1082/0 BOT CHORD  $34\text{-}35\text{=}0/935,\, 33\text{-}34\text{=}0/2147,\, 32\text{-}33\text{=}0/2772,\, 31\text{-}32\text{=}0/2790,\, 30\text{-}31\text{=}0/2337,$ 29-30=-16/1233, 27-29=-804/0, 26-27=-1227/0, 25-26=-670/1074, 24-25=-333/1501, 23-24=-333/1501, 22-23=0/1467, 21-22=0/675 WEBS 2-35=-1245/0, 2-34=0/858, 3-34=-828/0, 3-33=0/523, 12-27=-1589/0, 12-29=0/1200, 10-29=-1158/0, 10-30=0/874, 8-30=-700/0, 8-31=0/846, 5-33=-326/0, 5-32=-269/287, 7-31=-416/0, 19-21=-896/0, 19-22=0/566, 18-22=-536/30, 17-23=0/405, 17-24=-284/0, 14-27=-1267/0, 14-26=0/869, 15-26=-924/0, 15-25=0/901, 16-25=-370/0

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

All plates are 3x6 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





|   |  |  | 17-10-0                                   |   |  |  |  |   |
|---|--|--|---|---|--|--|--|---|
| Plate Offsets (X,Y)   | [17:0-1-8,Edge], [18:0-1-8,Edge]   |  |   |   |  |  |  |   |
| LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.00<br>Lumber DOL 1.00<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014                  | CSI.<br>TC 0.49<br>BC 0.72<br>WB 0.47<br>Matrix-S    | DEFL.<br>Vert(LL)<br>Vert(CT)<br>Horz(CT) | in (loc)<br>-0.21 17-18<br>-0.29 17-18<br>0.06 14 | l/defl<br>>996<br>>725<br>n/a            | L/d<br>480<br>360<br>n/a                 | <b>PLATES</b><br>MT20<br>Weight: 95 lb         | <b>GRIP</b><br>244/190<br>FT = 20%F, 11%E |
| LUMBER-       TOP CHORD     2x4 SP No.1(flat)       BOT CHORD     2x4 SP No.1(flat)       WEBS     2x4 SP No.3(flat)                |  |  | BRACING-<br>TOP CHORE<br>BOT CHORE        | 0 Structu<br>except<br>0 Rigid c                  | ral wood s<br>end vertic<br>eiling direc | sheathing dire<br>als.<br>ctly applied o | ectly applied or 6-0-0<br>r 10-0-0 oc bracing. | oc purlins,                               |
| REACTIONS. (size) 22=0-3-8, 14=Mechanical<br>Max Grav 22=961(LC 1), 14=967(LC 1)  |  |  |   |   |  |  |  |   |
| FORCES. (lb) - Max.<br>TOP CHORD 2-3=-<br>8-10:   | Comp./Max. Ten All forces 250 (lb) or<br>-1757/0, 3-4=-2926/0, 4-5=-2926/0, 5-6=<br>2926/0, 10-11=-2926/0, 11-12=-1757/0 | less except when shown.<br>-3487/0, 6-7=-3487/0, 7-8 | =-3487/0,                                 |   |  |  |  |   |

| BOT CHORD | 21-22=0/1042, 19-21=0/2442, 18-19=0/3275, 17-18=0/3487, 16-17=0/3275, 15-16=0/2442 |
|-----------|--|
|           | 14-15=0/1043   |
| WERS      | 2 22- 1285/0 2 21-0/004 2 21- 052/0 2 10-0/657 12 14- 1288/0 12 15-0/004           |

| VVEBS | 2-22=-1385/0, 2-21=0/994, 3-21=-953/0, 3-19=0/657, 12-14=-1388/0, 12-15=0/994,   |
|-------|--|
|       | 11-15=-953/0, 11-16=0/658, 8-16=-474/0, 8-17=-71/583, 5-19=-474/0, 5-18=-71/583, |
|       | 6-18=-290/0, 7-17=-290/0   |

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-TOP CHORD

BOT CHORD

WEBS

| UMBER-    |                   |
|-----------|-------------------|
| OP CHORD  | 2x4 SP No.1(flat) |
| BOT CHORD | 2x4 SP No.1(flat) |

2x4 SP No.3(flat) (size) 23=0-3-8, 15=Mechanical

REACTIONS. (size) 23=0-3-8, 15=Mechanical Max Grav 23=1158(LC 1), 15=1008(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-2158/0, 3-4=-3538/0, 4-5=-3538/0, 5-7=-3783/0, 7-8=-3783/0, 8-9=-3783/0, 9-11=-3102/0, 11-12=-3102/0, 12-13=-1847/0

 BOT CHORD
 22-23=0/1315, 20-22=0/2972, 19-20=0/3746, 18-19=0/3783, 17-18=0/3496, 16-17=0/2573, 10-1840/2000

|      | 15-16=0/1090   |
|------|--|
| WEBS | 2-23=-1708/0, 2-22=0/1143, 3-22=-1104/0, 3-20=0/752, 4-20=-279/0, 13-15=-1451/0, |
|      | 13-16=0/1053, 12-16=-1010/0, 12-17=0/718, 9-17=-535/0, 9-18=0/685, 5-20=-277/0,  |
|      | 5-19=-320/291, 8-18=-341/0   |

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Refer to girder(s) for truss to truss connections.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 159 lb down at 1-1-12, and 159 lb down at 3-1-12, and 159 lb down at 5-1-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 15-23=-10, 1-14=-100 Concentrated Loads (lb)

Vert: 4=-79(F) 25=-81(F) 26=-79(F)



A MiTek Affilia B18 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



|                                      | 1   |   | 9-4-8  |  |  | 9 <sub>1</sub> 6 <sub>1</sub> 0                | 12-10-0  | 1   |
|--------------------------------------|---|---|--|--|--|--|--|---|
|                                      |   |   | 9-4-8  |  |  | 0-1-8  | 3-4-0  | 1   |
| Plate 0                              | Offsets (X,Y)                                 | [1:Edge,0-1-8], [4:0-1-8,Edge], [5:0-1-8,   | Edge], [9:0-1-8,Edge], [10                               | ):0-1-8,Edge]  |  |  |  |   |
| LOAD<br>TCLL<br>TCDL<br>BCLL<br>BCDL | ING (psf)<br>40.0<br>10.0<br>0.0<br>5.0       | SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014 | <b>CSI.</b><br>TC 0.08<br>BC 0.05<br>WB 0.04<br>Matrix-S | <b>DEFL.</b> ir<br>Vert(LL) -0.00<br>Vert(CT) -0.00<br>Horz(CT) 0.00 | (loc) l/defl<br>13 >999<br>13 >999<br>12 n/a         | L/d<br>480<br>360<br>n/a                       | PLATES<br>MT20<br>Weight: 80 lb                  | <b>GRIP</b><br>244/190<br>FT = 20%F, 11%E |
| LUMB<br>TOP C<br>BOT C<br>WEBS       | ER-<br>CHORD 2x4 SF<br>CHORD 2x4 SF<br>2x4 SF | P No.1(flat)<br>P No.1(flat)<br>P No.3(flat)  |  | BRACING-<br>TOP CHORD<br>BOT CHORD                                   | Structural woo<br>except end ver<br>Rigid ceiling di | d sheathing dir<br>ticals.<br>rectly applied o | rectly applied or 6-0-0<br>or 10-0-0 oc bracing, | oc purlins,<br>Except:                    |

6-0-0 oc bracing: 16-17,15-16.

**REACTIONS.** All bearings 9-6-0 except (jt=length) 12=0-3-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 16, 17, 18, 21, 20, 19 except 15=301(LC 9), 15=290(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





|   |   |   | 3-6-0  |   |  |   |
|---|---|---|--|---|--|---|
| Plate Offsets (X,Y)   | - [1:Edge,0-1-8], [2:0-1-8,Edge], [3:0-1-8,   | Edge], [9:0-1-8,0-1-8]                            |  |   |  |   |
| LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0 | SPACING- 2-0-0<br>Plate Grip DOL 1.00<br>Lumber DOL 1.00<br>Rep Stress Incr YES<br>Code IRC2015/TPI2014 | CSI.<br>TC 0.08<br>BC 0.05<br>WB 0.04<br>Matrix-S | DEFL.         ir           Vert(LL)         -0.00           Vert(CT)         -0.00           Horz(CT)         0.00 | i (loc) l/defl L/d<br>7 >999 480<br>7 >999 360<br>5 n/a n/a                                 | <b>PLATES</b><br>MT20<br>Weight: 24 lb         | <b>GRIP</b><br>244/190<br>FT = 20%F, 11%E |
| LUMBER-         TOP CHORD       2x4 SP No.1(flat)         BOT CHORD       2x4 SP No.1(flat)         WEBS       2x4 SP No.3(flat)    |   |   | BRACING-<br>TOP CHORD<br>BOT CHORD   | Structural wood sheathing dire<br>except end verticals.<br>Rigid ceiling directly applied o | ectly applied or 3-6-0<br>r 10-0-0 oc bracing. | oc purlins,                               |

REACTIONS. (size) 8=Mechanical, 5=0-3-8 Max Grav 8=179(LC 1), 5=173(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



