HOLLY PLACE LOT 2
4071 BARBEQUE CHURCH RD
SANFORD, NC

GENERAL NOTES

1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AND REGULATIONS.
2. CONTRACTOR SHALL THOROUGHLY REVIEW ALL SHEETS IN PLAN SET AND VERIFY ALL DETAILS AND DIMENSIONS BEFORE BEGINNING CONSTRUCTION. ANY DISCREPANCIES SHALL BE REPORTED TO RENAISSANCE RESIDENTIAL DESIGN, INC. FOR JUSTIFICATION AND/OR CORRECTION BEFORE PROCEEDING WITH WORK. CONTRACTORS SHALL ASSUME RESPONSIBILITY FOR ERRORS THAT ARE NOT REPORTED PRIOR TO CONSTRUCTION.
3. ALL DIMENSIONS SHOULD BE READ OR CALCULATED AND NEVER SCALED.
4. CONTRACTOR SHALL ENSURE COMPATIBILITY OF THE BUILDING WITH ALL SITE REQUIREMENTS.

FRONT ELEVATION-C

SCALE: 1/4" = 1'-0"

PLANS DESIGNED TO THE
NORTH CAROLINA STATE
ENTIAL BUILDING CODE.

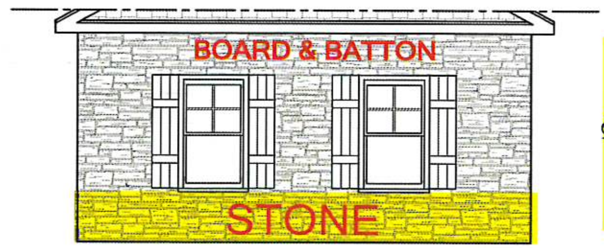
NOTICE TO CONTRACTOR
All construction must comply with current NC Building Codes and is subject to field inspection and verification.

APPROVED
Limited building only review
Permit holder responsible for full compliance with the code

09/18/2023

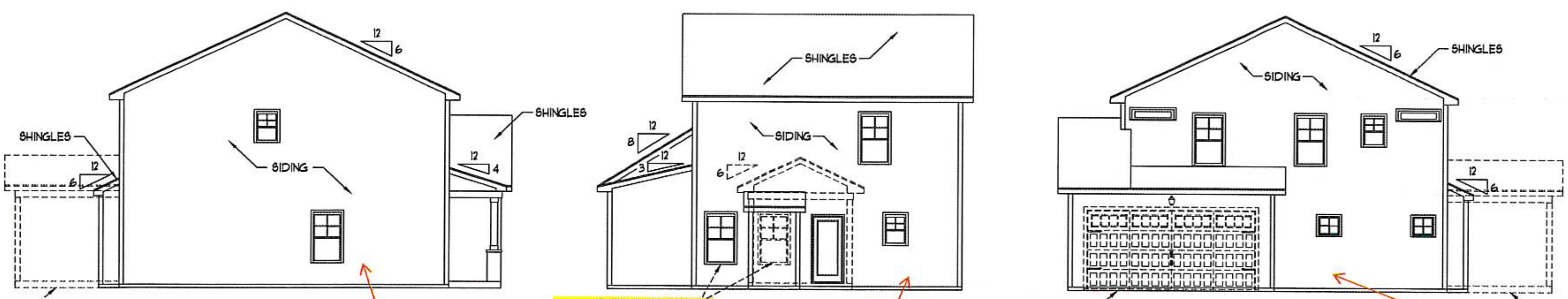
[illegible]

Harnett
COUNTY
NORTH CAROLINA



SIDE LOAD GARAGE

SCALE: 1/4" = 1'-0"



ERED
RCH

PARGE

LEFT ELEVATION
SCALE: 1/8" = 1'-0"

SHIFT WINDOW W/
FAMILY ROOM OPTION

REAR ELEVATION
SCALE: 1/8" = 1'-0"

PARGE

OPTIONAL
SIDE-LOAD GARAGE

RIGHT ELEVATION
SCALE: 1/8" = 1'-0"

PARGE

COVERED
PORCH

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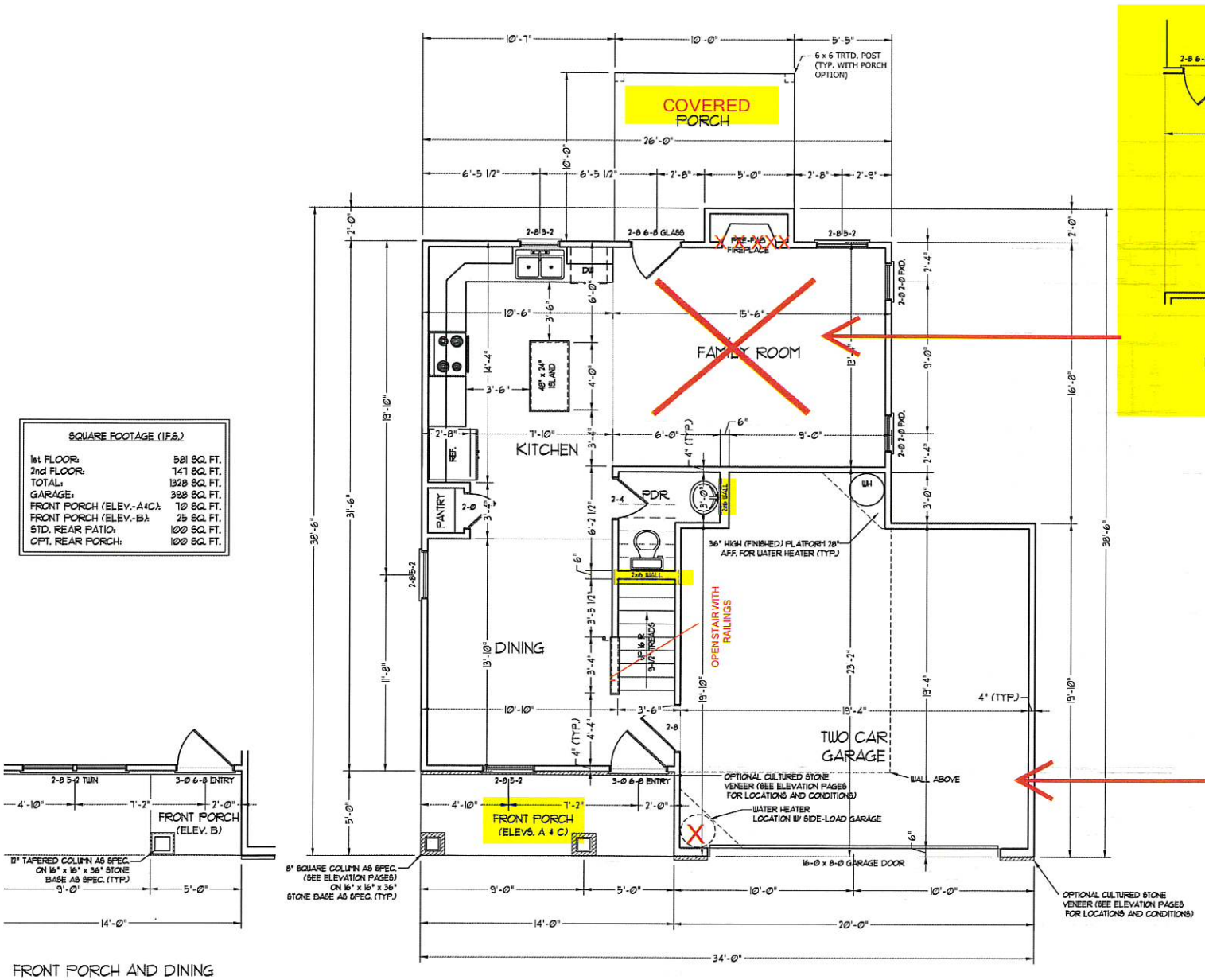
REVIEWED BY:

C - ELEVATIONS

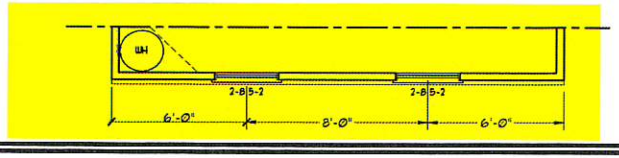
A-3

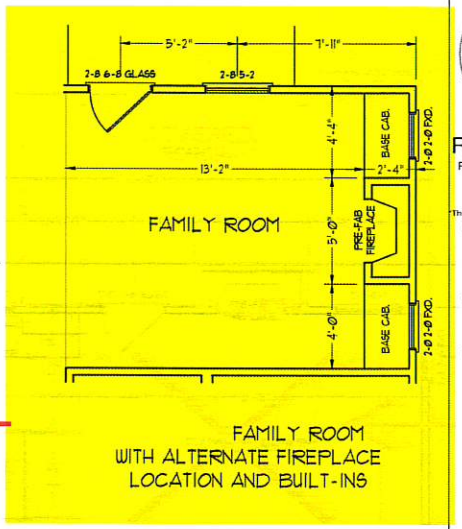
an-Weaver\Magnolia\Magnolia-II_4-23.dwg 4/23/2023 4:36:08 PM, AutoCAD PDF (General Documentation).pc3



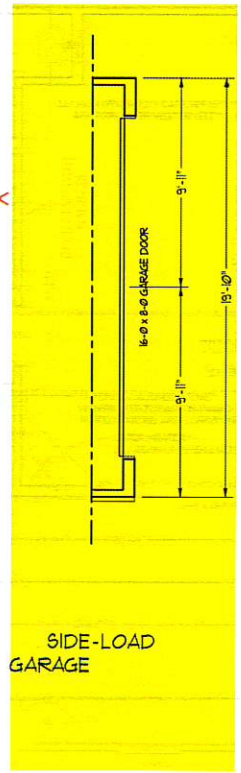
SQUARE FOOTAGE (I.F.S.)	
1st FLOOR:	581 SQ. FT.
2nd FLOOR:	747 SQ. FT.
TOTAL:	1328 SQ. FT.
GARAGE:	398 SQ. FT.
FRONT PORCH (ELEV.-A+C):	70 SQ. FT.
FRONT PORCH (ELEV.-B):	25 SQ. FT.
STD. REAR PATIO:	100 SQ. FT.
OPT. REAR PORCH:	100 SQ. FT.

FRONT PORCH AND DINING ROOM WINDOW ELEVATION B





FAMILY ROOM WITH ALTERNATE FIREPLACE LOCATION AND BUILT-INS



SIDE-LOAD GARAGE

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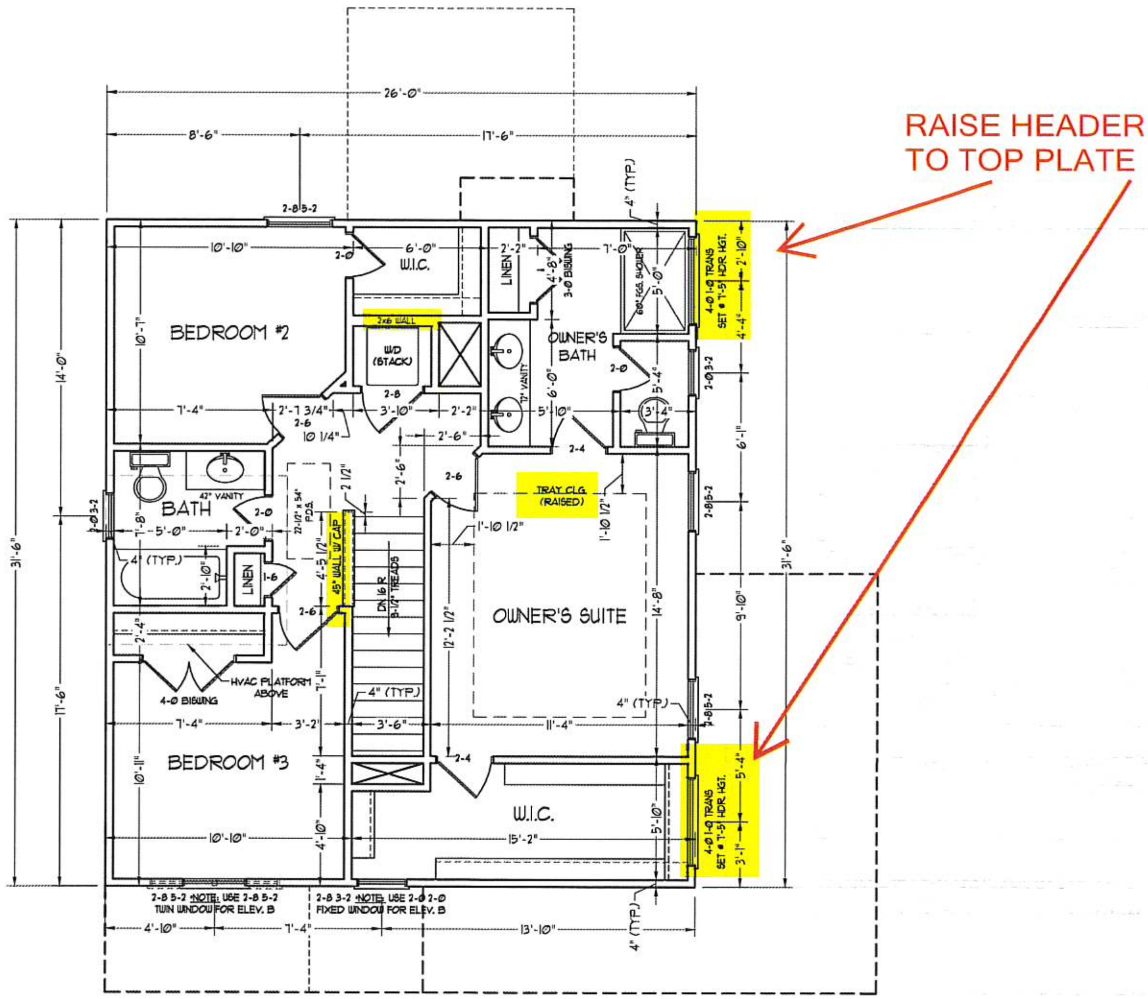
FIRST FLOOR PLAN

A-4



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REVIEWED BY:

SECOND FLOOR PLAN

A-5

CTRICAL LAYOUT NOTES:

BLOCK AND WIRE FOR ALL
ELING FANS PER PLAN.

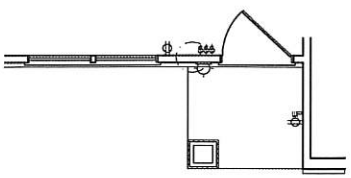
VANITY LIGHTS TO BE SET
80" A.F.F. (TYP.)

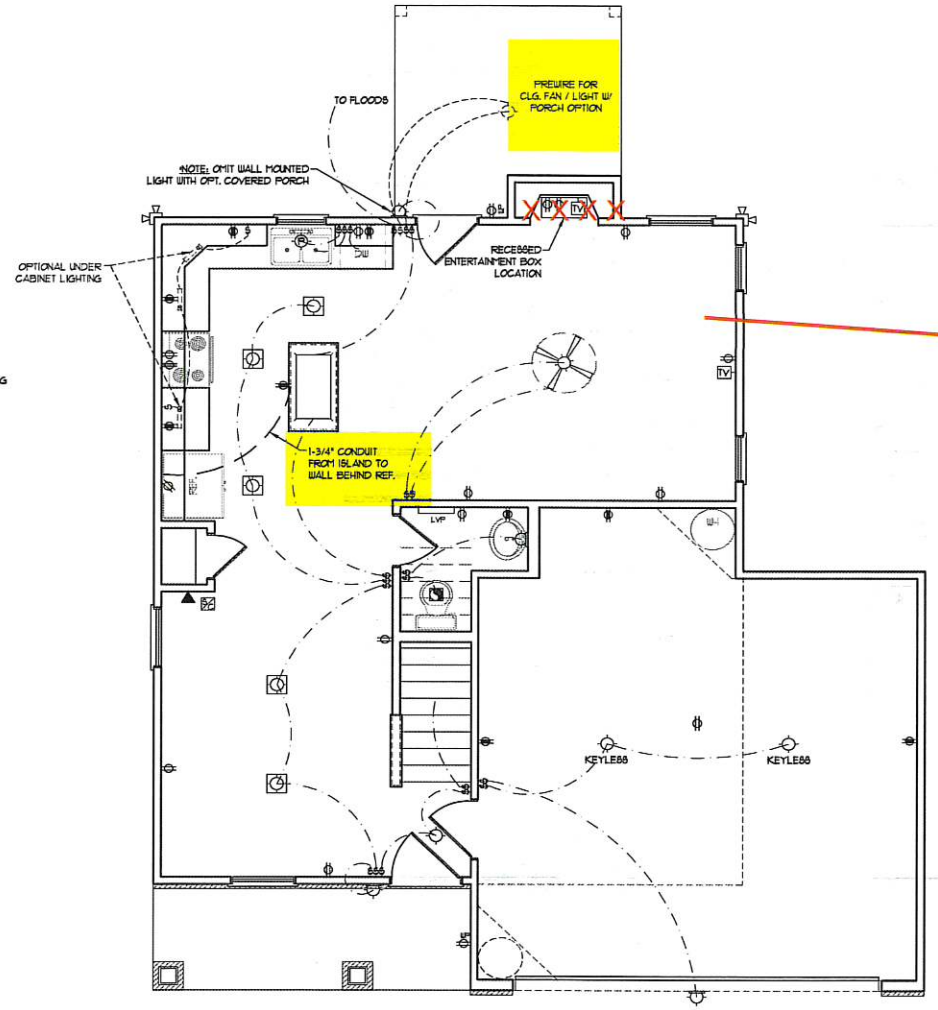
ADDITIONAL EXTERIOR OUTLETS
EQUIRED BY CODE TO BE
OCATED BY ELECTRICIAN.

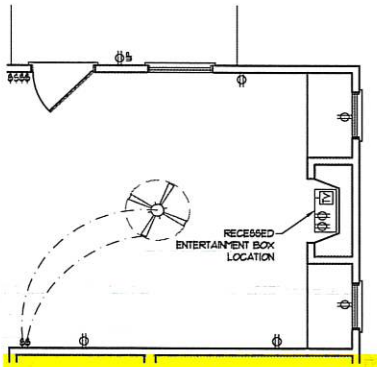
PLACE SWITCHES 8" (MIN.) FROM
OUGH OPENINGS.

RICAL LEGEND

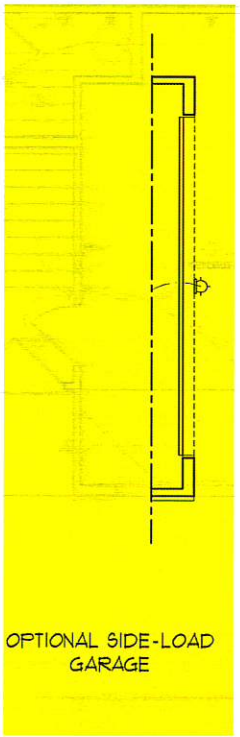
) V OUTLET	SWITCH
) V GFI OUTLET	DIMMER SWITCH
) V SWITCHED OUTLET	TELEPHONE
) V BASEBOARD OUTLET	DATA
PLEX	TELEPHONE AND DATA
OUNTER OR FLOOR MOUNTED	TV CONNECTION
OUNTER OR FLOOR MOUNTED 110V GFI	TV/ DATA
EATHERPROOF	CONDUIT FOR COMPONENT WIRING
0 V OUTLET	SPEAKER
) V DEDICATED CIRCUIT	110 V SMOKE/ CM DETECTOR
0 V DEDICATED CIRCUIT	110 V SMOKE DETECTOR
PECIAL PURPOSE (240 V, ETC.)	EXHAUST FAN
ALL MOUNT LIGHT	LOW VOLTAGE PANEL
EILING MOUNT LIGHT	ALARM PANEL
ENDANT LIGHT	
ECESSED CAN LIGHT	CEILING FAN
INI CAN LIGHT	
YEBALL LIGHT	CEILING FAN W/ LIGHT
LUORESCENT LIGHT	
NDERCABINET LIGHT	
LOOD LIGHT	



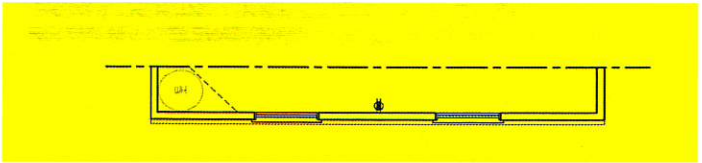




OPTIONAL FAMILY ROOM WITH ALTERNATE FIREPLACE LOCATION AND BUILT-INS



OPTIONAL SIDE-LOAD GARAGE



FRONT PORCH AND DINING ROOM WINDOW ELEVATION B

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ENGINEERED BY:
REVIEWED BY:

FIRST FLOOR ELECTRICAL PLAN

E-1

CTRICAL LAYOUT NOTES:

BLOCK AND WIRE FOR ALL EILING FANS PER PLAN.

VANITY LIGHTS TO BE SET 80" A.F.F. (TYP.)

ADDITIONAL EXTERIOR OUTLETS EQUIRED BY CODE TO BE OCATED BY ELECTRICIAN.

PLACE SWITCHES 8" (MIN) FROM OUGH OPENINGS.

RICAL LEGEND

) V OUTLET	SWITCH
) V GFI OUTLET	DIMMER SWITCH
) V SWITCHED OUTLET	TELEPHONE
) V BASEBOARD OUTLET	DATA
-PLEX	TELEPHONE AND DATA
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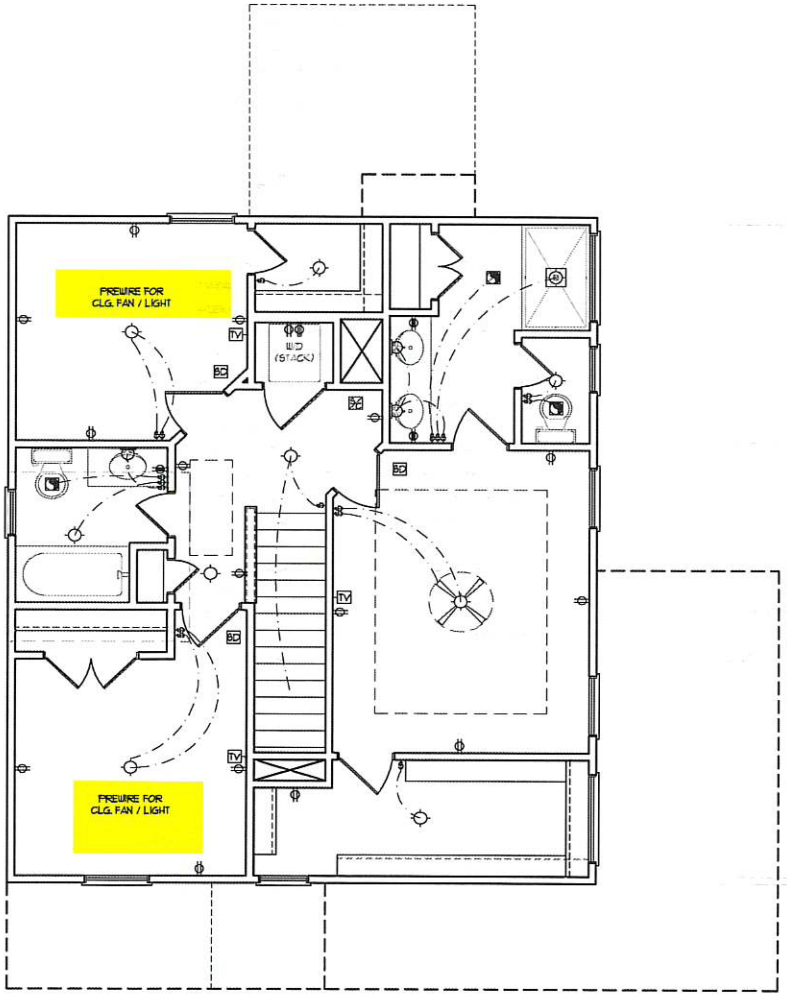
ECESSED CAN LIGHT	CEILING FAN
NI CAN LIGHT	
REBALL LIGHT	CEILING FAN W/ LIGHT
UORESCENT LIGHT	
NDERCABINET LIGHT	
OOD LIGHT	



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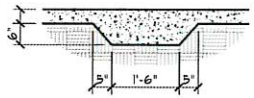
SECOND FLOOR
ELCTRICAL
PLAN

E-2

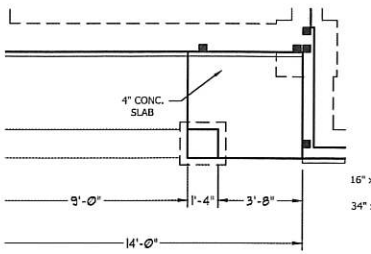
IGN WIND SPEED NOTES FOR LESS
4 30' MEAN ROOF HEIGHT:

DESIGN PER NORTH CAROLINA
CODE, 2018 EDITION.
WIND ZONES INSTALL 1/2" ANCHOR
.C. AND WITHIN 1'-0" FROM END OF
. ANCHOR BOLTS MUST EXTEND A
7" INTO CONCRETE OR 15" INTO
CATE BOLT WITHIN MIDDLE THIRD
)TH.
WIND ZONES INSTALL 1/2" ANCHOR
.C. AND WITHIN 1'-0" FROM END OF
. ANCHOR BOLTS MUST EXTEND A
7" INTO CONCRETE OR 15" INTO
CATE BOLT WITHIN MIDDLE THIRD
)TH.
EIGHT IS LESS THAN 30 FEET.
LLS DESIGNED FOR 120 OR 130 MPH

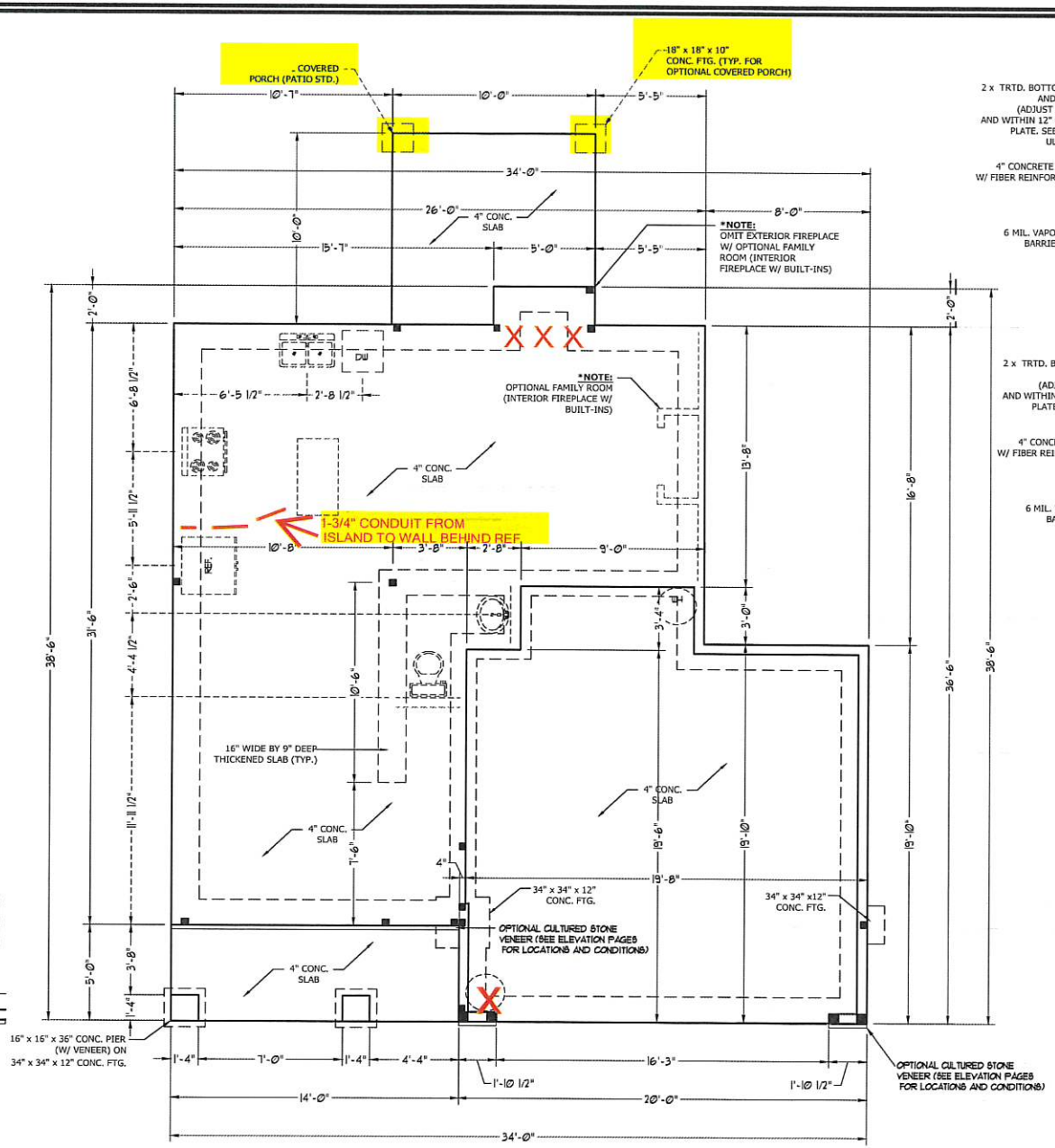
OSB SHEATHING ON ALL EXTERIOR
. STORIES IN ACCORDANCE WITH
.10.3 OF THE NCRC, 2018 EDITION.
IENCY COMPLIANCE AND
ALUES OF THE BUILDING TO BE IN
WITH CHAPTER 11 OF THE NCRC,

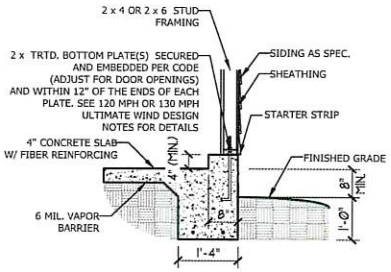


THICKENED SLAB DETAIL

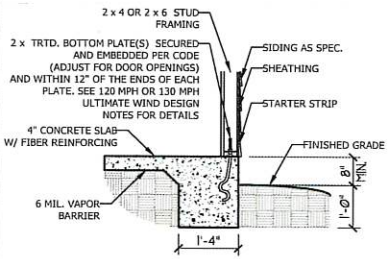


FRONT PORCH ELEVATION B

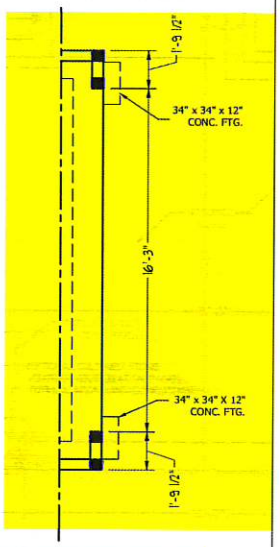




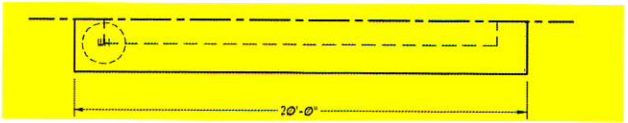
GARAGE CURB DETAIL



TYPICAL SLAB DETAIL



. SIDE-LOAD GARAGE



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MONO SLAB FOUNDATION PLAN

S-1

JRAL NOTES:

MING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2

D BEARING HEADERS TO BE (2) 2 x 6 (UNO).
. AN EXTRA JOIST UNDER WALLS PARALLEL TO FLOOR JOISTS
W AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING
&, END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD
EMENTS.
ES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR
TION. ALL SQUARES TO BE (2) STUDS (UNO.)
4 POSTS SHALL BE ANCHORED TO SLABS W/ SIMPSON ABU44 POST BASES
JAL) AND 6 X 6 POSTS W/ ABU66 POST BASES (OR EQUAL) (UNO). ALL 4 X 4
6 POSTS TO BE INSTALLED WITH 700 LB CAPACITY UPLIFT CONNECTORS
(UNO.)
ERGLASS, ALUMINUM, OR COLUMN ENG. BY OTHERS, SECURE TO SLAB W/
TAL ANGLES USING 2" CONC. SCREWS. FASTEN ANGLES TO COLUMNS W/
IROUGH BOLTS W/ NUTS AND WASHERS. LOCATE ANGLES ON OPPOSITE
F COLUMN. THROUGH BOLTS MUST BE INSTALLED PRIOR TO SETTING
.

WALL PANEL NOTES:

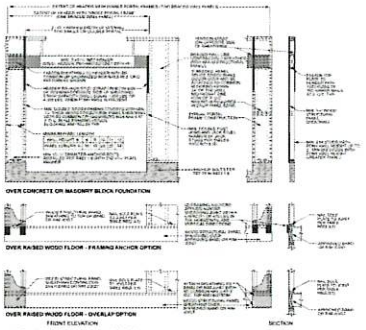
R WALLS: ALL EXTERIOR WALLS TO BE SHEALTHED WITH
R CS-SFB IN ACCORDANCE WITH SECTION R602.10.3
OTED OTHERWISE.

D LENGTH OF BRACING: REQUIRED BRACE WALL LENGTH
SIDE OF THE CIRCUMSCRIBED RECTANGLE ARE
ATED PER TABLE R602.10.3. METHODS CS-WSP AND CS-SFB
TE THIER ACTUAL LENGTH. METHOD GB CONTRIBUTES 0.5
L LENGTH. METHOD PF CONTRIBUTES 1.5 TIMES ITS
ENGTH.

: ALL INTERIOR SIDES OF EXTERIOR WALLS AND BOTH
INTERIOR WALLS TO HAVE 1/2" GYPSUM INSTALLED. WHEN
G METHOD GB GYPSUM TO BE FASTENED PER TABLE
METHOD GB TO BE FASTENED PER TABLE R602.10.1.

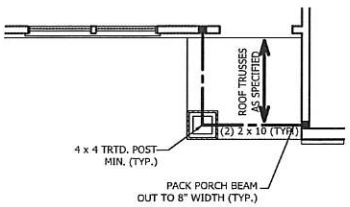
BS HOLD DOWN DEVICE FASTENED TO THE EDGE OF THE
LL PANEL NEAREST TO THE CORNER

S: PER TABLE R602.10.1

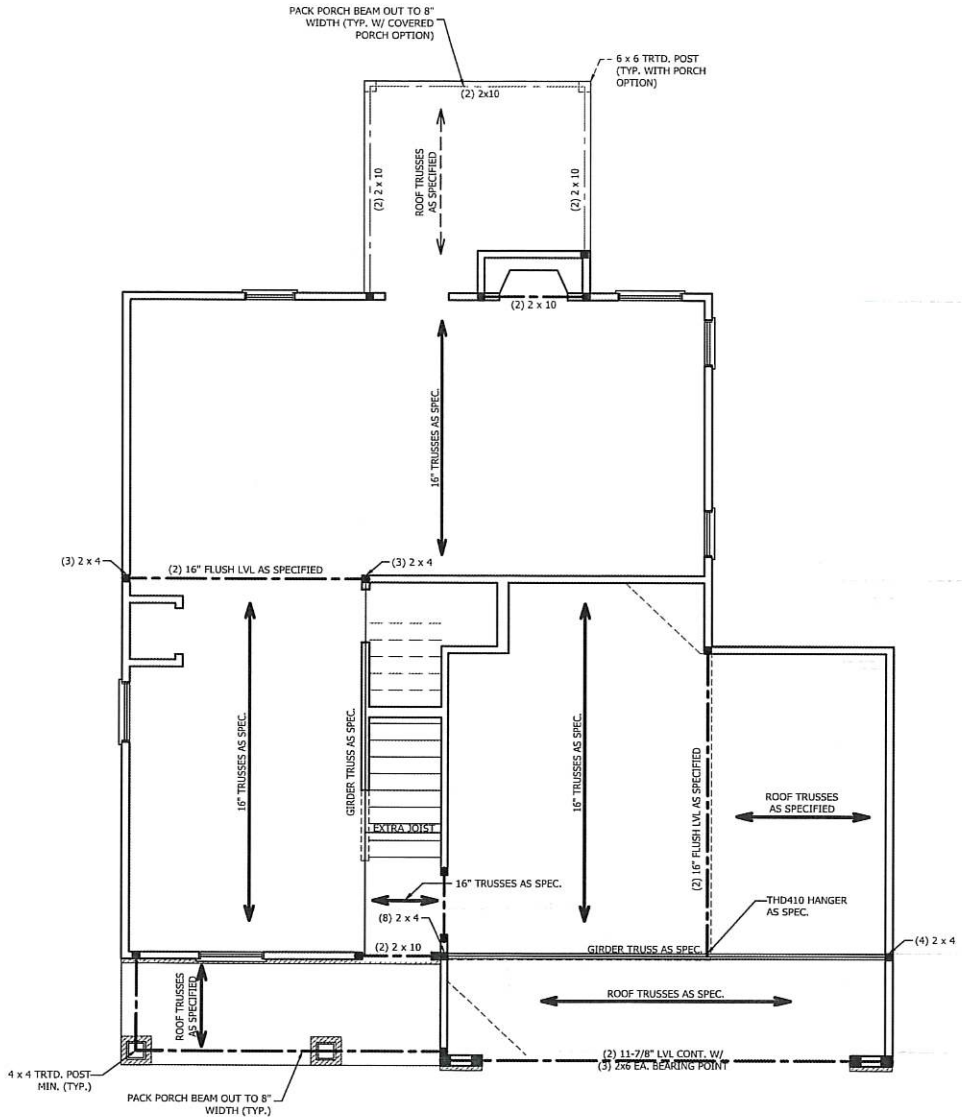


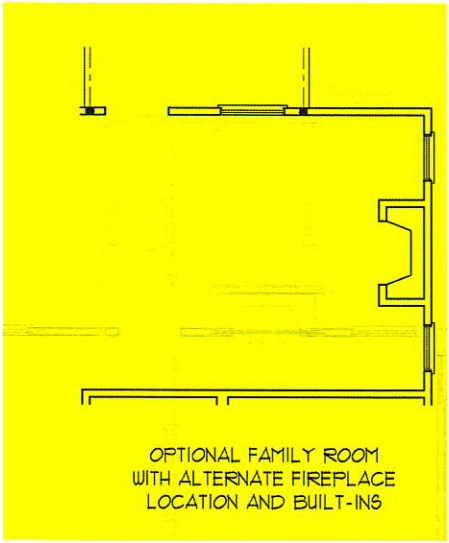
n. 1 foot = 305 mm 1 lb = 4.45 N.

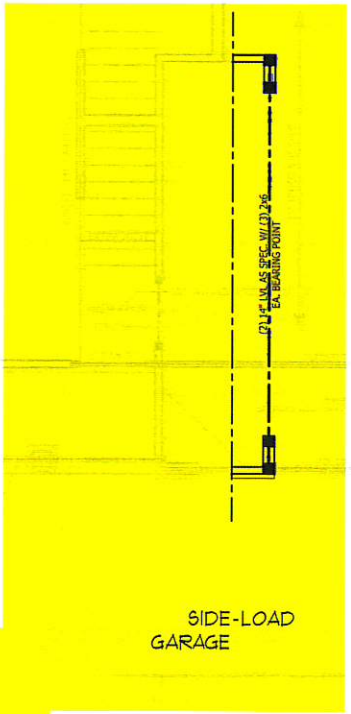
FIGURE R602.10.1
METHOD PF—PORTAL FRAME CONSTRUCTION



FRONT PORCH AND DINING
ROOM WINDOW ELEVATION B







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ENGINEERED BY:
REVIEWED BY:

SECOND FLOOR
FRAMING PLAN

S-2

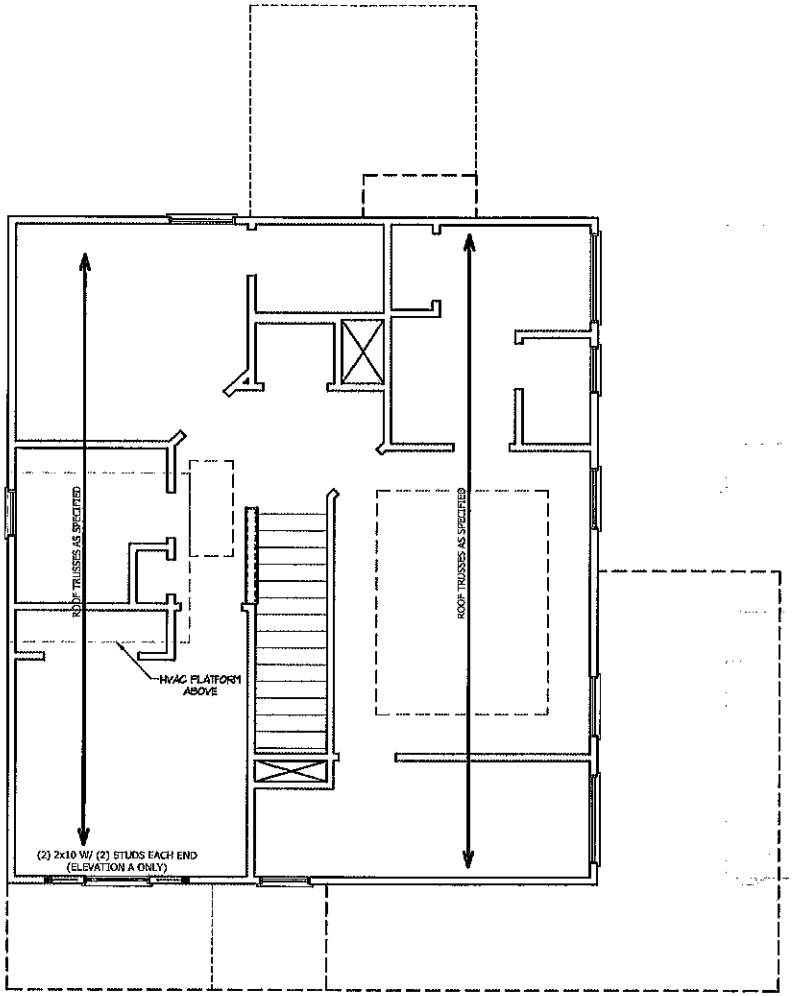

TABLE R602.7.5
MINIMUM NUMBER OF FULL HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS

HEADER SPAN (FEET)	MAXIMUM STUD SPACING (INCHES) (PER TABLE R602.3(5)	
	16	24
UP TO 3'	1	1
4'	2	1
8'	3	2
12'	5	3
16'	6	4

STRUCTURAL NOTES:

1. ALL FRAMING LUMBER TO BE SPF #2 (UNO). ALL TREATED LUMBER TO BE SYP #2 (UNO.)
2. ALL LOAD BEARING HEADERS TO BE (2) 2 x 6 (UNO).
3. WINDOW AND DOOR HEADERS TO BE SUPPORTED w/ (1) JACK STUD AND (1) KING STUD EA. END (UNO.). SEE TABLE R602.7.5 FOR ADDITIONAL KING STUD REQUIREMENTS.
4. SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. ALL SQUARES TO BE (2) STUDS (UNO.)

DSP - DOUBLE STUD POCKET
TSP - TRIPLE STUD POCKET

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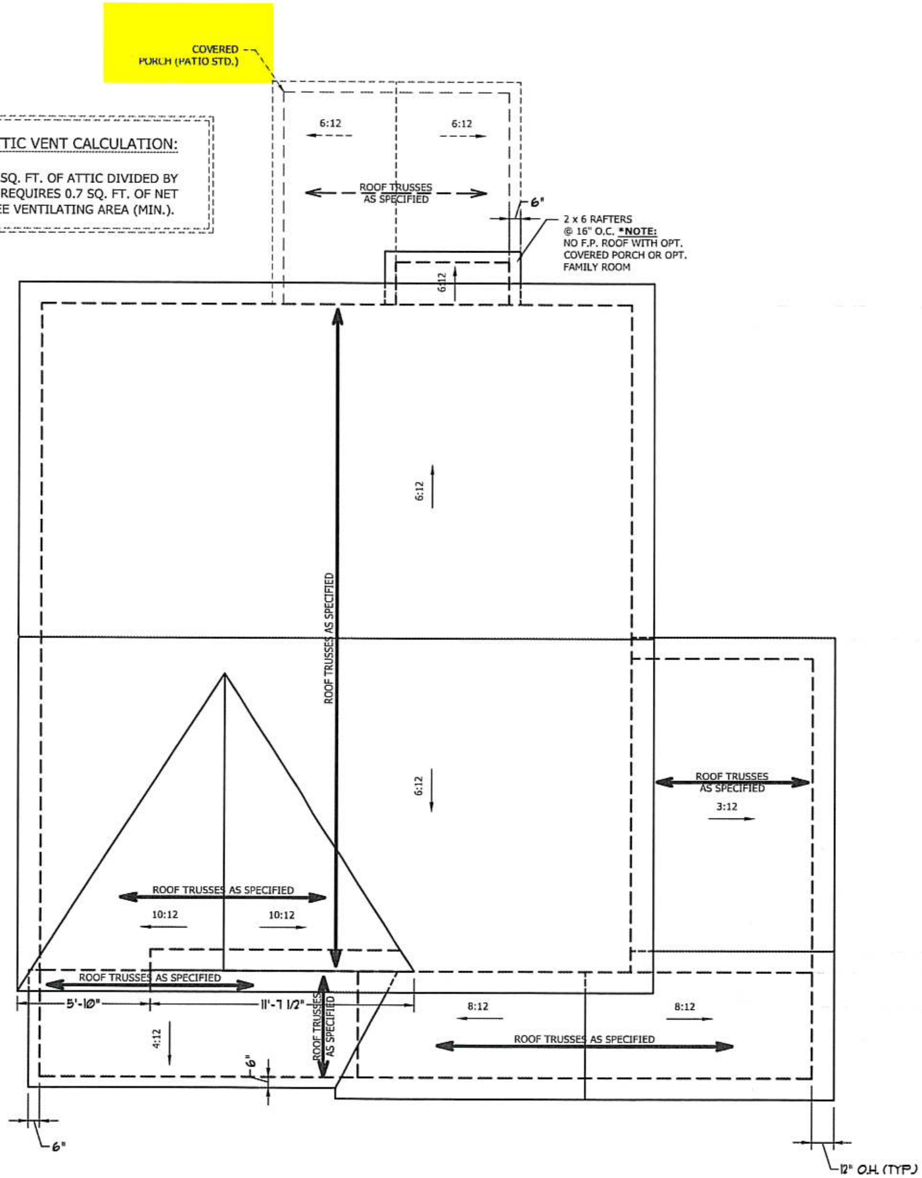
REVIEWED BY:

ATTIC FLOOR FRAMING PLAN

S-3

ATTIC VENT CALCULATION:

100 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 0.7 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).



ELEVATION C

ATTIC VENT CALCULATION:

1250 SQ. FT. OF ATTIC DIVIDED BY 150 REQUIRES 8.3 SQ. FT. OF NET FREE VENTILATING AREA (MIN.).

STRUCTURAL NOTES:

1. ALL FRAMING LUMBER TO BE #2 SPF (UNO).
2. HIP SPLICES ARE TO BE SPACED A MIN. OF 8'-0". FASTEN MEMBERS WITH THREE ROWS OF 12d NAILS @ 16" O.C. (TYP.)
3. STICK FRAME OVER-FRAMED ROOF SECTIONS W/ 2 x 8 RIDGES, 2 x 6 RAFTERS @ 16" O.C. AND FLAT 2 x 10 VALLEYS OR USE VALLEY TRUSSES.
4. FASTEN FLAT VALLEYS TO RAFTERS OR TRUSSES WITH SIMPSON H2.5A HURRICANE TIES @ 32" O.C. MAX. PASS HURRICANE TIES THROUGH NOTCH IN ROOF SHEATHING. EACH RAFTER IS TO BE FASTENED TO THE FLAT VALLEY WITH A MIN. OF (6) 12d TOE NAILS.
5. REFER TO SECTION R802.11 OF THE 2018 NCRC FOR REQUIRED UPLIFT RESISTANCE AT RAFTERS AND TRUSSES.

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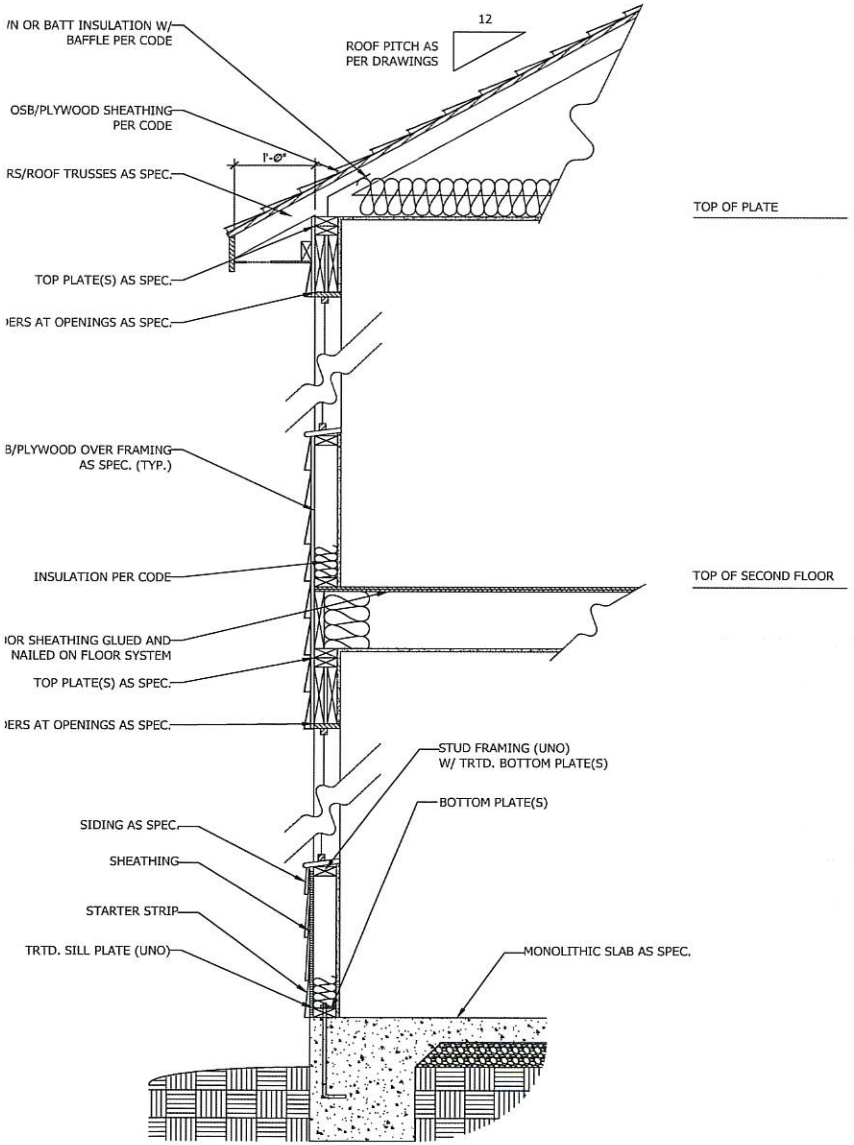
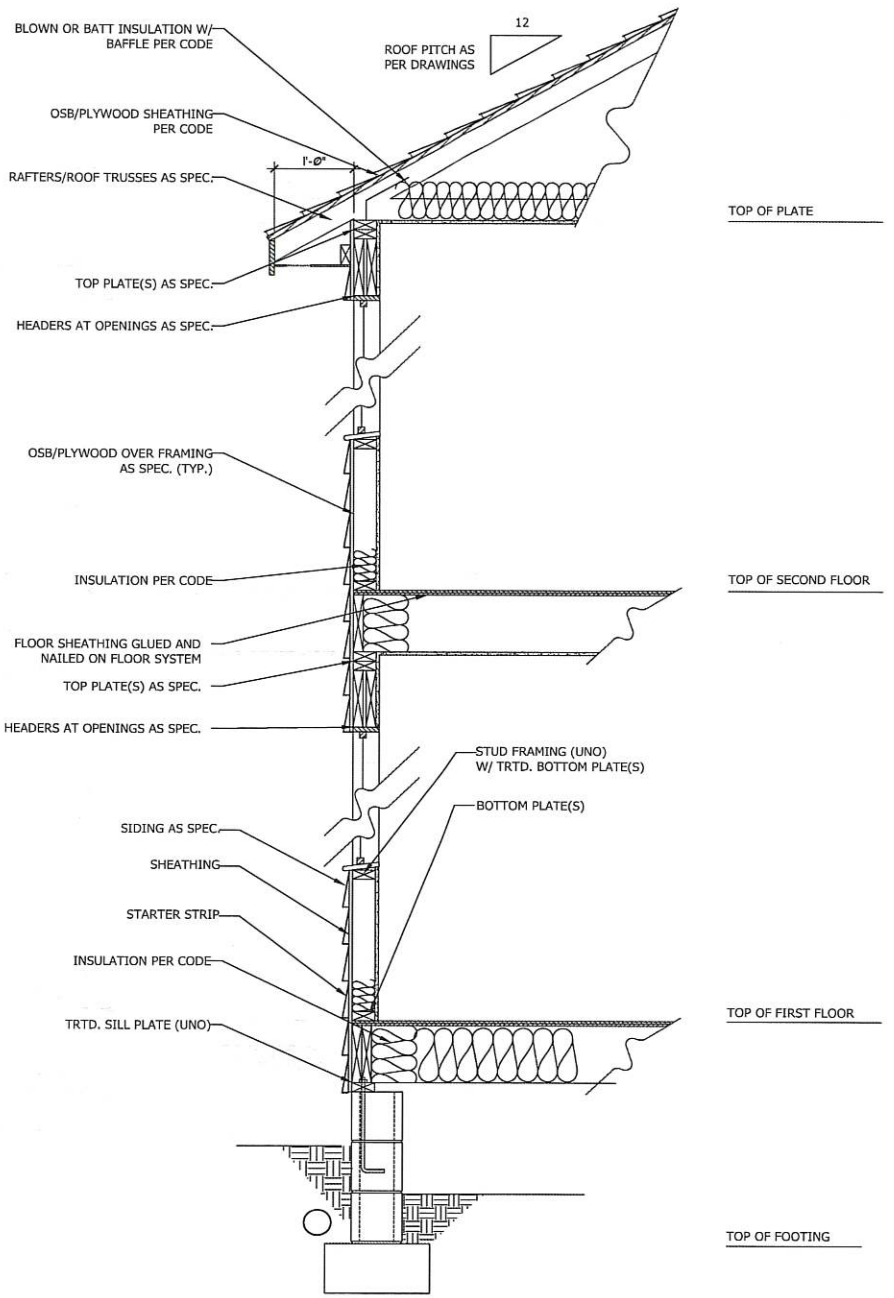
ROOF PLAN
ELEVATION - C
S-4



WALL SECTION W/ SLAB
W/ STD. SIDING SHOWN (NTS)



WALL SECTION W/ CRAWL SPACE
W/ STD. SIDING SHOWN (NTS)

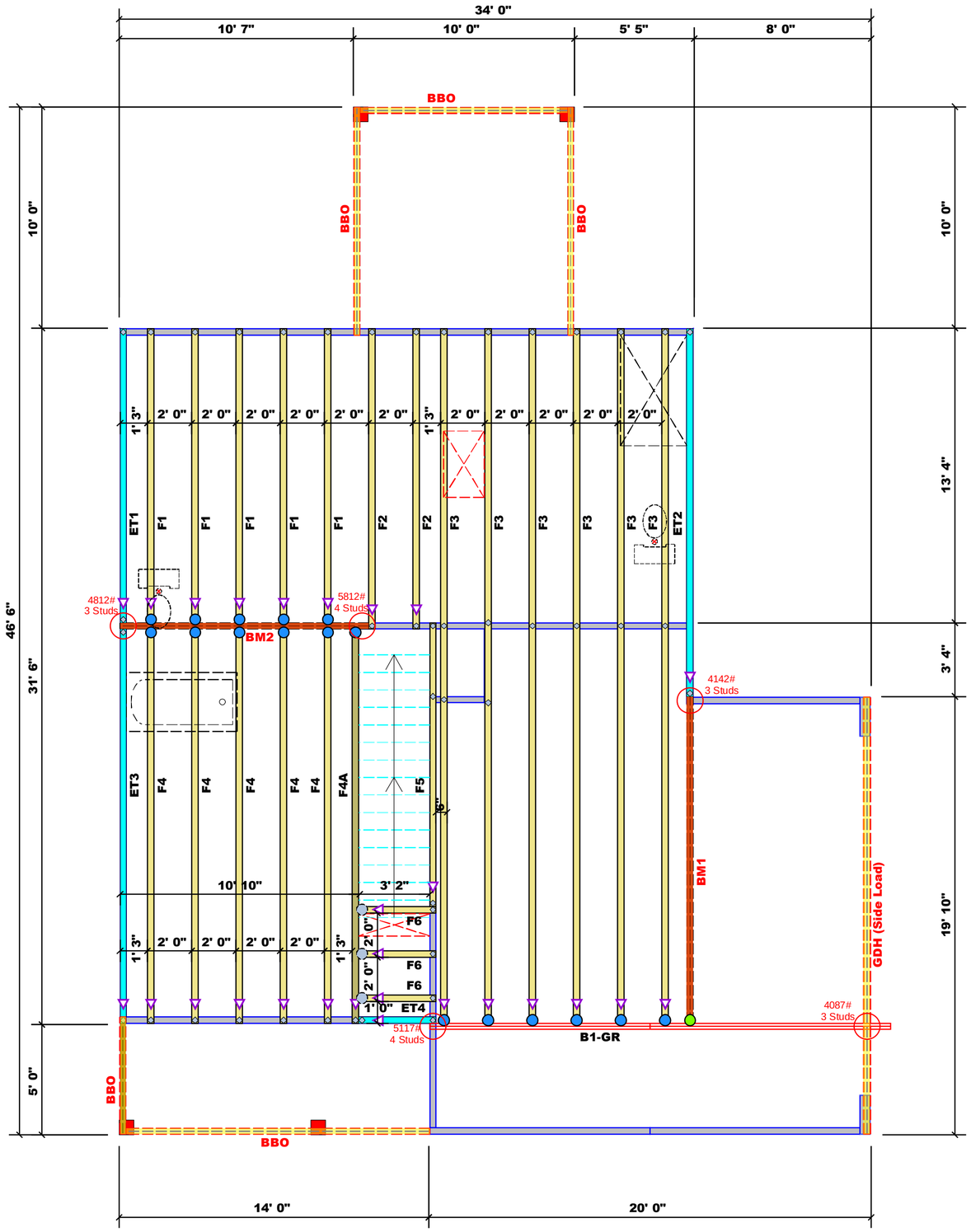
RENAISSANCE
RESIDENTIAL DESIGN, INC.
RALEIGH, NC 27612
(919) 649-4128
WWW.RRDCAROLINA.COM
The art of transforming your vision into reality.

WEAVER HOMES
CAROLINA COLLECTION
MAGNOLIA-II

DATE: FEBRUARY 19, 2021
REV.:
SCALE: 1/4" = 1'-0"
DRAWN BY: WG
ENGINEERED BY:
REVIEWED BY:

TYPICAL WALL SECTIONS

D-1



1 Truss Placement Plan
Scale: 1/4"=1'

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes
1. Plumbing drop locations shown are NOT exact. 2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses. 3. Adjust spacing as needed not to exceed 24"oc.

Dimension Notes
1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise 2. All interior wall dimensions are to face of stud unless noted otherwise 3. All exterior wall to truss dimensions are to face of stud unless noted otherwise

Hatch Legend
Second Floor Walls
Box Storage
Tray Ceiling
Flush Beam
Drop Beam

Connector Information					Nail Information	
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
●	HUS410	USP	17	NA	16d/3-1/2"	16d/3-1/2"
●	MSH422	USP	3	Varies	10d/3"	10d/3"
●	THD410	USP	1	NA	16d/3-1/2"	10d/3"

Products					
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM2	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
GDH (Side Load)	20' 0"	1-3/4"x 14" LVL Kerto-S	2	2	FF

▲= Denotes Left End of Truss
(Reference Engineered Truss Drawing)

COMTECH
ROOF & FLOOR TRUSSES & BEAMS
Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature Jonathan Landry

Jonathan Landry

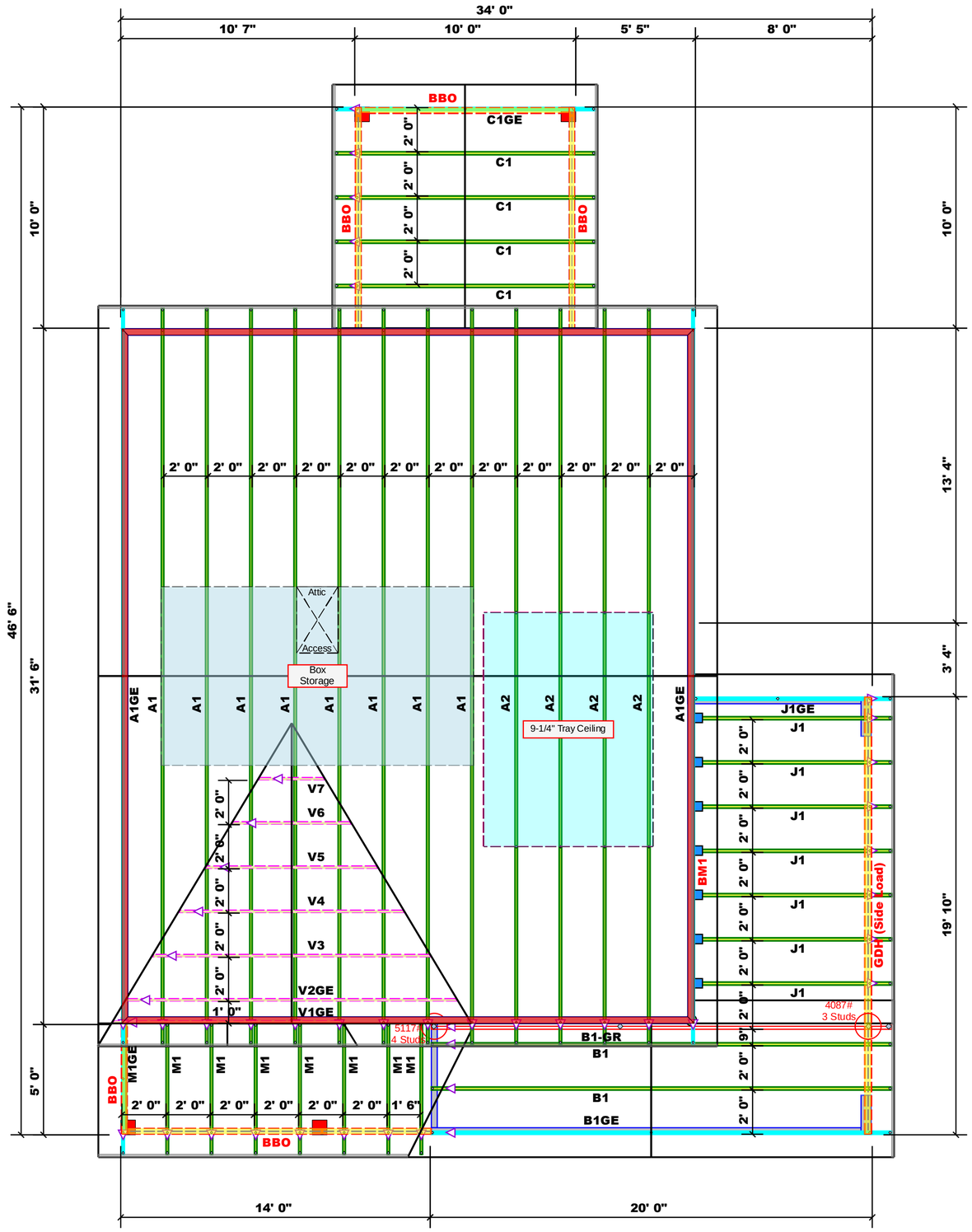
LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

BUILDER	Weaver Development
JOB NAME	Lot 2 Holly Place
PLAN	Magnolia II "C" / 2GRS, CP
SEAL DATE	N/A
QUOTE #	
JOB #	J0223-0920
CITY / CO.	Sanford / Harnett
ADDRESS	4071 Barbeque Church Road
MODEL	Floor
DATE REV.	04/26/23
DRAWN BY	Lenny Norris
SALES REP.	Jonathan Landry



All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

All Walls Shown Are Considered Load Bearing

Roof Area	= 1637.75 sq.ft.
Ridge Line	= 59.66 ft.
Hip Line	= 1.32 ft.
Horiz. OH	= 108.58 ft.
Raked OH	= 144.92 ft.
Decking	= 56 sheets

Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of stud unless noted otherwise
3. All exterior wall to truss dimensions are to face of stud unless noted otherwise

Hatch Legend

- Second Floor Walls
- Box Storage
- Flush Beam
- Drop Beam

Connector Information					Nail Information	
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	7	NA	16d/3-1/2"	16d/3-1/2"

Products

PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	15' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM2	12' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
GDH (Side Load)	20' 0"	1-3/4"x 14" LVL Kerto-S	2	2	FF

1 Truss Placement Plan
Scale: 1/4"=1'

▲= Denotes Left End of Truss (Reference Engineered Truss Drawing)

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Signature Jonathan Landry

Jonathan Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

BUILDER	Weaver Development
JOB NAME	Lot 2 Holly Place
PLAN	Magnolia II "C" / 2GRS, CP
SEAL DATE	N/A
QUOTE #	
JOB #	J0223-0919
CITY / CO.	Sanford / Harnett
ADDRESS	4071 Barbeque Church Road
MODEL	Roof
DATE REV.	05/23/23
DRAWN BY	Jonathan Landry
SALES REP.	Lenny Norris

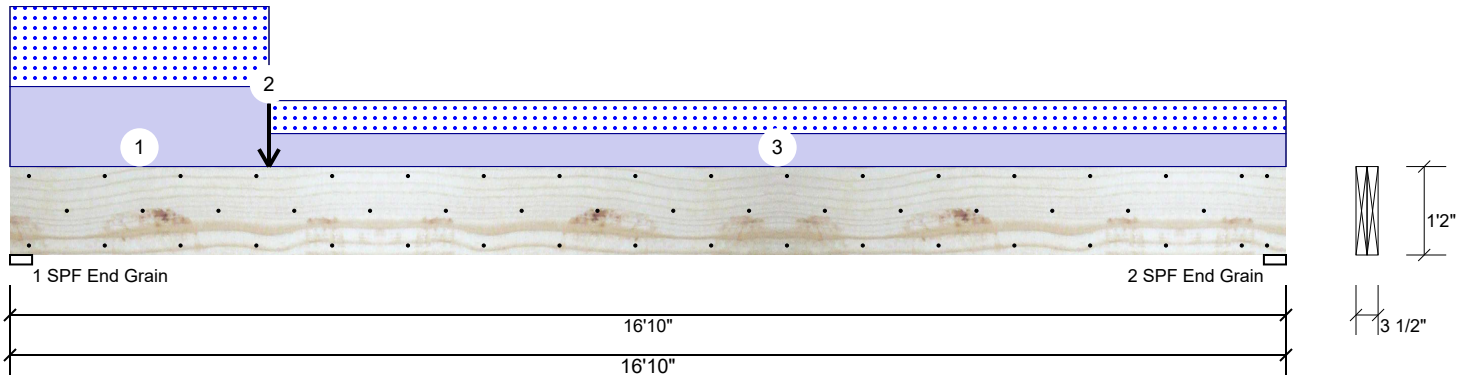
THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Client:	Weaver Development	Date:	5/23/2023
Project:	Magnolia II	Input by:	Jonathan Landry
Address:	4071 Barbeque Church Road Sanford, NC 27332	Job Name:	Lot 2 Holly Place
		Project #:	J0223-0920

GDH (Side Load) Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal - II	Ceiling:	Gypsum 1/2"
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	2855	2764	0	0
2	Vertical	0	1260	1169	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	55%	2855 / 2764	5619	L	D+S
2 - SPF End Grain	3.500"	Vert	24%	1260 / 1169	2429	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	15402 ft-lb	3'8 3/8"	31049 ft-lb	0.496 (50%)	D+S	L
Unbraced	15402 ft-lb	3'8 3/8"	15412 ft-lb	0.999 (100%)	D+S	L
Shear	5000 lb	1'5 1/2"	12021 lb	0.416 (42%)	D+S	L
LL Defl inch	0.228 (L/861)	7'8 1/16"	0.409 (L/480)	0.558 (56%)	S	L
TL Defl inch	0.468 (L/420)	7'8 5/16"	0.546 (L/360)	0.858 (86%)	D+S	L

Design Notes

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
3 Refer to last page of calculations for fasteners required for specified loads.
4 Girders are designed to be supported on the bottom edge only.
5 Top loads must be supported equally by all plies.
6 Top must be laterally braced at a maximum of 6'7 7/16" o.c.
7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Part. Uniform	0-0-0 to 3-5-0		Top	211 PLF	0 PLF	211 PLF	0 PLF	0 PLF	B1
2	Point	3-5-0		Top	2044 lb	0 lb	2044 lb	0 lb	0 lb	B1-GR
	Bearing Length	0-3-8								
3	Part. Uniform	3-5-0 to 16-10-0		Top	87 PLF	0 PLF	87 PLF	0 PLF	0 PLF	J1
	Self Weight				11 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

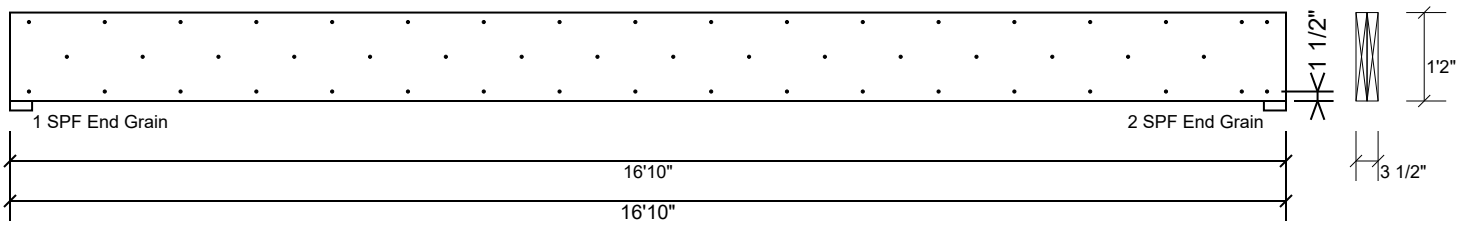
Metsä Wood
301 Merritt 7 Building, 2nd Floor
Norwalk, CT 06851
(800) 622-5850
www.metsawood.com/us

Comtech
Reilly Road Industrial Park P.O. Box 40408, NC
USA
28309
910-864-8787

Client:	Weaver Development	Date:	5/23/2023
Project:	Magnolia II	Input by:	Jonathan Landry
Address:	4071 Barbeque Church Road Sanford, NC 27332	Job Name:	Lot 2 Holly Place
		Project #:	J0223-0920

GDH (Side Load) Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

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2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
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