

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0722-3740 Precision/Lot 25 Liberty Meadows/Harnet

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I56905190 thru I56905220

My license renewal date for the state of North Carolina is December 31, 2023.

North Carolina COA: C-0844



March 1,2023

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



⊢	9-6-0	<u>18-6-8</u> 9-0-8	27-5-2 8-10-10	36-3-11 8-10-10	45-2	<u>2-5 46-0₁0</u>)-10 0-9-11	<u>51-0-0</u>
Plate Offsets (X,Y)	[7:0-2-8,Edge], [46:0-2-8,0-2-	-12], [47:0-3-0,0-1-4]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2- Plate Grip DOL 1 Lumber DOL 1 Rep Stress Incr Y Code IRC2015/TPI20	0-0 CSI. 1.15 TC 0.59 1.15 BC 0.33 YES WB 0.67 14 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.15 48 -0.29 1-48 0.26 31 0.16 48	l/defl L/d >999 360 >824 240 n/a n/a >999 240	PLATES MT20 Weight: 537	GRIP 244/190 lb FT = 20%
LUMBER- TOP CHORD 2x6 SF 26-29: BOT CHORD 2x6 SF WEBS 2x4 SF 4-46: 2 OTHERS 2x4 SF SLIDER Left 2x	P No.1 *Except* 2x4 SP No.1 P No.1 P No.2 *Except* 2x8 SP No.1 P No.2 4 SP No.2 4-7-4		BRACING- TOP CHOF BOT CHOF WEBS JOINTS	RD Structur except 2-0-0 oc RD Rigid ce 6-0-0 oc T-Brace Fasten (0.131"x Brace m 1 Brace	al wood sheathing dir c purlins (6-0-0 max.): iling directly applied of c bracing: 30-31,28-30 c: 2 (2X) T and I braces tr (3") nails, 6in o.c.,with nust cover 90% of wel at Jt(s): 49, 50, 51, 5	rectly applied or 5-8- 7-18. or 10-0-0 oc bracing 0. 1x4 SPF No.2 - 18-33 13-43, 11-46, 9-51, o narrow edge of we n 3in minimum end c b length. 2	12 oc purlins, , Except: 9, 17-40, 16-41, 14-42 19-38, 21-37 b with 10d listance.
REACTIONS. All but (lb) - Max H Max U Max C	earings 27-5-8 except (jt=leng lorz 1=-376(LC 10) Jplift All uplift 100 lb or less a 12), 46=-249(LC 9), 45=- 13), 33=-173(LC 13), 32 Grav All reactions 250 lb or le except 1=838(LC 23), 46 1), 31=1689(LC 1), 31=1	th) 1=0-5-8, 31=0-3-8, 31=0-3-8 at joint(s) 40, 41, 42, 43, 38 exce -239(LC 3), 37=-122(LC 13), 35= :=-1258(LC 1), 31=-1061(LC 9) ses at joint(s) 39, 40, 41, 42, 43, 6=1176(LC 1), 33=508(LC 1), 32 689(LC 1)	8, 31=0-3-8. ept 1=-158(LC =-112(LC 13), 34=-100 38, 37, 35, 34 2=947(LC 9), 31=1689)(LC			
FORCES. (lb) - Max. TOP CHORD 1-3= 8-9= 13-1 19-2 BOT CHORD 1-48 42-4 37-3	Comp./Max. Ten All forces -1423/398, 3-4=-1199/391, 4-0 -275/260, 9-10=-275/261, 10- 4=-275/261, 14-16=-275/261, 1=-257/204, 24-25=-332/180, =-404/1224, 47-48=-197/879, 3=-155/334, 41-42=-155/334, 8=-155/334, 35-37=-155/334,	250 (lb) or less except when sh 6=-507/243, 6-7=-225/295, 7-8= 11=-275/261, 11-12=-275/261, 1 16-17=-275/261, 17-18=-275/26 25-26=-537/387, 26-27=-720/72 46-47=-172/795, 45-46=-155/33 40-41=-155/334, 39-40=-155/33 34-35=-155/334, 3-34=-155/33	own. -275/261, 12-13=-275/261, 50, 18-19=-318/287, 27, 27-28=-746/732 34, 43-45=-155/334, 34, 38-39=-155/334, 34, 32-33=-155/334,			UNITH ORTH	CAROUN,
31-3 WEBS 3-48 51-5 6-53	2=-155/334, 30-31=-681/777, 47/279, 48-49=-232/939, 4- 2=-996/290, 50-51=-974/296, 72/521, 24-33=-379/322, 25	28-30=-681/777 49=-973/296, 49-53=-1183/348, 46-50=-838/271, 12-45=-41/250 5-32=-680/901, 26-31=-1358/890	52-53=-854/302,), 11-46=-833/336,)			S 03	EAL 6322
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; \ gable end zone and Lumber DOL=1.60 3) Truss designed for \ Gable End Details a Communication	e loads have been considered /ult=130mph Vasd=103mph; ⁻ I C-C Exterior(2) zone; cantiler plate grip DOL=1.60 wind loads in the plane of the as applicable, or consult qualifi rainage to prevent water pond	I for this design. TCDL=6.0psf; BCDL=6.0psf; h= ver right exposed ;C-C for memb truss only. For studs exposed to ied building designer as per ANS ling.	15ft; Cat. II; Exp C; Er bers and forces & MW o wind (normal to the f SI/TPI 1.	iclosed; MWFRS FRS for reaction ace), see Standa	S (envelope) is shown; ard Industry	A A A A A A A A A A A A A A A A A A A	INEER GILBERT
WARNING - Verify Design valid for use of a truss system. Befor building design. Brac is always required for fabrication, storage, of Safety Information	design parameters and READ NOTES only with MITek® connectors. This des e use, the building designer must verif ing indicated is to prevent buckling of stability and to prevent collapse with lelivery, erection and bracing of trusse available from Truss Plate Institute, 26	ON THIS AND INCLUDED MITEK REFER- ign is based only upon parameters show by the applicability of design parameters a individual truss web and/or chord membe possible personal injury and property dara s and truss systems, see ANSI 670 Crain Highway, Suite 203 Waldorf, M	RENCE PAGE MII-7473 rev rn, and is for an individual bu and properly incorporate this ers only. Additional tempore mage. For general guidance //PPI1 Quality Criteria, DSE ID 20601	5/19/2020 BEFORE L iliding component, no s design into the over ary and permanent bir regarding the 1-89 and BCSI Build	JSE. ot ratil racing ling Component	818 Sound: Edenton, N	NEERING BY EENCOO A MITek Affiliate side Road C 27932

Job	Truss	Truss Type	Qty	Ply	Precision/Lot 25 Liberty Meadows/Harnet	
					156	6905190
J0722-3740	A01-GE	GABLE	1	1		
					Job Reference (optional)	
Comtech, Inc, Fa	yetteville, NC - 28314,			3.430 s Jar	6 2022 MiTek Industries, Inc. Tue Feb 28 15:24:20 2023 Pa	age 2
		ID:52	2SvgMJAa	HxrWTaEx	<pre>drbuZyFiSD- gVhEo8aDA0j2WkE7?EnBneacDVOlb7yIzfhVAz</pre>	aZiv

NOTES-

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 40, 41, 42, 43, 38 except (jt=lb) 1=158, 46=249, 45=239, 37=122, 35=112, 34=100, 33=173, 32=1258, 31=1061.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

- 12) This truss has large uplift reaction(s) from gravity load case(s). Proper connection is required to secure truss against upward movement at the bearings. Building designer must provide for uplift reactions indicated.
- 13) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	9-6-0	18-6-8	:	27-5-2	36-3-11		45-2-5	5 46-0 ₁ 0 8	51-0-0
	9-6-0	9-0-8	8	3-10-10	8-10-10	I	8-10-10	0 0-9-11	5-0-0
Plate Offsets (X,Y)	[5:0-4-12,0-2-12], [8:0-6-0	,0-3-12], [11:0-2-12,0-2	2-8], [19:0-4-0,0)-3-13]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 C 1.15 T 1.15 B YES W 2014 M	SI. C 0.97 C 0.54 B 0.56 atrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(L)	in (loc) -0.19 17-19 -0.35 19-20 0.23 14 0.11 19-20	l/defl L >999 34 >999 24 n/a r	_/d 60 40 n/a 40	PLATES MT20 Weight: 455 lb	GRIP 244/190 FT = 20%
	0000 11(02010/11			Wind(EE)	0.11 13 20	2000 Z	10	Weight: 400 lb	11 = 2070
LUMBER- TOP CHORD 2x6 SP 11-13: BOT CHORD 2x6 SP WEBS 2x6 SP 3-20,5- SLIDER Left 2x	P No.1 *Except* 2x4 SP No.1 P No.1 P No.1 *Except* -20,10-16,11-16,11-14: 2x4 4 SP No.2 4-7-4	\$P No.2		BRACING TOP CHC BOT CHC WEBS	5- RD Structu 2-0-0 c RD Rigid c 6-0-0 c 1 Row	aral wood she oc purlins (5-6 eiling directly oc bracing: 12 at midpt	eathing directl 6-6 max.): 5-8 • applied or 16 -14. 5-19	ly applied, except 3. 0-0-0 oc bracing, 1, 6-19, 6-17, 11-16	Except:
REACTIONS. (size Max H Max U Max G	e) 1=0-5-8, 14=0-3-8 lorz 1=-295(LC 10) lplift 1=-42(LC 12), 14=-10 irav 1=1804(LC 1), 14=23-	6(LC 13) 45(LC 2)							
FORCES. (lb) - Max. TOP CHORD 1-3=- 10-11 10-12 BOT CHORD 1-20= 12-14 WEBS 3-20= 8-17=	Max Grav 1=1804(LC 1), 14=2345(LC 2) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-3=-3970/778, 3-5=-3710/795, 5-6=-1744/559, 6-8=-1812/548, 8-10=-2282/601, 10-11=-2382/449, 11-12=-1044/999 BOT CHORD 1-20=-458/3241, 19-20=-111/1986, 17-19=-125/1871, 16-17=-38/1543, 14-16=0/718, 12-14=-933/1071 WEBS 3-20=-231/268, 5-20=-297/2453, 5-19=-310/141, 6-19=-436/212, 6-17=-427/232, 8-17=-120/762, 8-16=-126/580, 10-16=-521/326, 11-16=-746/1366, 11-14=-2470/861								
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V and C-C Exterior(2) 32-4-0 to 37-4-8, Int shown; Lumber DOI 3) Provide adequate di 4) This truss has been 5) * This truss has been % This truss has been 6) Bearing at joint(s) 1 capacity of bearing 2) Provide mechanical	a loads have been conside (ult=130mph Vasd=103mp 0-2-2 to 5-2-10, Interior(1) erior(1) 37-4-8 to 52-2-8 zc =1.60 plate grip DOL=1.61 rainage to prevent water por designed for a 10.0 psf bo n designed for a 10.0 psf bo n designed for a 11 we load co bottom chord and any other considers parallel to grain surface.	red for this design. h; TCDL=6.0psf; BCDI 5-2-10 to 13-8-0, Exte one; cantilever right ex onding. ttom chord live load no of 30.0psf on the botton members, with BCDL value using ANSI/TPI	==6.0psf; h=15f rior(2) 13-8-0 to posed ;C-C for i poconcurrent with n chord in all ar = 10.0psf. 1 angle to grain	t; Cat. II; Exp C; E o 18-8-9, Interior(' members and for th any other live l reas where a rect of formula. Buildin tanding 100 lb up	inclosed; MWFR) 18-8-9 to 32-4 xes & MWFRS fo pads. angle 3-6-0 tall b g designer shoul	S (envelope) -0, Exterior(2) or reactions y 2-0-0 wide d verify	(Anno	SE 036	AROLULI AL 322

ig p 14=106.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	6-11-12 13-8-0	23-0-0	32-4-0		38-9-2	45-2-5 45 ₁ 10-4 51-0-0
Plate Offsets (X Y)	[2:0-6-3 Edge] [6:0-6-0 0-3-12] [9:0-6-0	9-4-0	<u>9-4-0</u> 3] [19:0-4-0 0-4-8]		6-5-2	6-5-2 0-7-15 5-1-12
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.40 BC 0.49 WB 0.51 Matrix-S	DEFL. in Vert(LL) -0.14 Vert(CT) -0.23 Horz(CT) 0.07 Wind(LL) 0.04	(loc) 19-21 19-21 15 15 19	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES GRIP MT20 244/190 Weight: 442 lb FT = 20%
LUMBER- TOP CHORD 2x6 SI 12-14: BOT CHORD 2x6 SI WEBS 2x4 SI 6-19,9 SLIDER Left 2x	P No.1 *Except* . 2x4 SP No.1 P No.1 P No.2 *Except* -19: 2x6 SP No.1 k4 SP No.2 4-5-13		BRACING- TOP CHORD BOT CHORD WEBS	Structu 2-0-0 o Rigid ca 6-0-0 o 1 Row a	ral wood sheathing d c purlins (5-2-0 max. eiling directly applied c bracing: 13-15. at midpt	Lirectly applied or 4-4-8 oc purlins, except): 6-9. I or 10-0-0 oc bracing, Except: 4-21, 7-19, 11-18
REACTIONS. (siz Max H Max U Max C	te) 2=0-3-8, 15=0-3-8 Horz 2=-296(LC 10) Jplift 2=-58(LC 12), 15=-107(LC 13) Grav 2=2148(LC 2), 15=2458(LC 2)					
FORCES. (lb) - Max. TOP CHORD 2-4= 11-1 11-1 BOT CHORD 2-22 15-1 15-1 WEBS 4-22 9-19 12-1	. Comp./Max. Ten All forces 250 (lb) or 2817/519, 4-6=-2345/593, 6-7=-2056/58 2=-2417/388, 12-13=-1018/1031 =-216/2111, 21-22=-216/2111, 19-21=-5 6=0/724, 13-15=-965/1046 =0/345, 4-21=-499/245, 6-21=-76/734, 6 =-191/694, 9-18=-40/670, 11-18=-441/18 5=-2565/820	less except when shown 32, 7-9=-2057/593, 9-11=- 6/1752, 18-19=-30/1670, -19=-176/605, 7-19=-628, 34, 11-16=-204/263, 12-16	-2294/552, 16-18=-138/1807, /303, 6=-698/1407,			
 NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-10; ' and C-C Exterior(2) 32-4-0 to 37-4-8, In shown; Lumber DO 3) Provide adequate d 4) All plates are 4x6 M 5) This truss has been 6) * This truss has been will fit between the I 7) Provide mechanical 15=107. 8) Graphical purlin rep 	e loads have been considered for this de Vult=130mph Vasd=103mph; TCDL=6.0 1 -1-1 to 3-11-8, Interior(1) 3-11-8 to 13 terior(1) 37-4-8 to 52-2-8 zone; cantileve L=1.60 plate grip DOL=1.60 Irainage to prevent water ponding. IT20 unless otherwise indicated. In designed for a 10.0 psf bottom chord live an designed for a live load of 30.0psf on t bottom chord and any other members, wil I connection (by others) of truss to bearin presentation does not depict the size or th	sign. sf; BCDL=6.0psf; h=15ft; -8-0, Exterior(2) 13-8-0 to r right exposed ;C-C for m e load nonconcurrent with he bottom chord in all are ith BCDL = 10.0psf. Ig plate capable of withsta he orientation of the purlin	Cat. II; Exp C; Enclosed 18-8-9, Interior(1) 18-8- nembers and forces & MV n any other live loads. nas where a rectangle 3-6 anding 100 lb uplift at join along the top and/or bot	l; MWFR 9 to 32-4 WFRS fo 6-0 tall by nt(s) 2 ex	S (envelope) -0, Exterior(2) r reactions y 2-0-0 wide cept (jt=lb) rd.	SEAL 036322

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



March 1,2023



Job	Truss	Truss Type	Qty	Ply	Precision/Lot 25 Liberty Meadows/Harnet	
						156905193
J0722-3740	A04	PIGGYBACK BASE	1	2		
				~	Job Reference (optional)	
Comtech, Inc, Fa	ayetteville, NC - 28314,			3.430 s Jar	6 2022 MiTek Industries, Inc. Tue Feb 28 15:24:25 2023	B Page 2
		ID:528	SygMJAaHx	rWTaExIrb	uZyFiSD-KelaHWCj2je?8HcCvYqyuqLVZE5UQwhiREMS/	AOzgZjq

NOTES-

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1525 lb down and 217 lb up at 27-6-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-6=-60, 6-9=-60, 9-12=-60, 12-14=-60, 2-13=-20

Concentrated Loads (lb)

Vert: 27=-1381(B)

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Edenton, NC 27932



besign valid to less only with with the contractors. This besign is based only upon parameters and properly incorporate this design into the overall a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Precision/Lot 25 Liberty Meadows/Harnet	
						156905195
J0722-3740	A06	PIGGYBACK BASE	1	2		
				–	Job Reference (optional)	
Comtech, Inc,	Fayetteville, NC - 28314,			3.430 s Jai	1 6 2022 MiTek Industries, Inc. Tue Feb 28 15:24:28 202	23 Page 2

ID:52SygMJAaHxrWTaExIrbuZyFiSD-ID_jvXFcKe0a?ILnbgNfWTzzZREgdBH87Cb6nizgZjn

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-6=-125, 6-9=-125, 9-10=-125, 2-11=-42 Concentrated Loads (lb)

Vert: 23=-2176(F)

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A MiTek Affi 818 Soundside Road Edenton, NC 27932



12) Attic room checked for L/360 deflection.

mmm March 1,2023

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818 Soundside Road Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	Precision/Lot 25 Liberty Meadows/Harnet	
10722 2740	4.00	ATTIC	2	-	156905	5198
50722-3740	A09	Arrie	3	2	Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,			3.430 s Jar	6 2022 MiTek Industries, Inc. Tue Feb 28 15:24:33 2023 Page 2	2

s Jan 6 2022 MiTek Industries, Inc. Tue Feb 28 15:24:33 Page ID:52SygMJAaHxrWTaExIrbuZyFiSD-5AncyFJk9Aet6WDkNEzqDWgoGSuZIO4tHUItSwzgZji

NOTES-

12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14 except (jt=lb) 13=178.
13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
14) Attic room checked for L/360 deflection.

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Edenton, NC 27932



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Job	Truss	Truss Type	Qty	Ply	Precision/Lot 25 Liberty Meadows/Harnet	
					156	905201
J0722-3740	A11	FLAT	1	2		
				-	Job Reference (optional)	
Comtech, Inc, Fayettev	ille, NC - 28314,		8	.430 s Jar	6 2022 MiTek Industries, Inc. Tue Feb 28 15:24:38 2023 Pag	ge 2

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Feb 28 15:24:38 2023 Page 2 ID:52SygMJAaHxrWTaExIrbuZyFiSD-S8aV0yMt_iH9CH6iAnZ?waNnZTfPzr9cQm0e77zgZjd

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 4=-1056 9=-1048 10=-1048

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WEBS 4-9=-29/960, 5-9=-277/262, 3-9=-277/270

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-2 to 4-7-1, Interior(1) 4-7-1 to 9-6-0, Exterior(2) 9-6-0 to 13-10-13, Interior(1) 13-10-13 to 20-1-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



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7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

8) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard



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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 11-11-8, Exterior(2) 11-11-8 to 16-4-5, Interior(1) 16-4-5 to 24-10-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 2-3, 7-8, 3-7; Wall dead load (5.0psf) on member(s).8-11, 2-13

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

7) Attic room checked for L/360 deflection.



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REACTIONS. (size) 1=0-3-8, 9=0-3-8 Max Horz 1=299(LC 9)

Max Grav 1=1545(LC 21), 9=1545(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2140/0, 2-3=-1187/149, 7-8=-1186/149, 8-9=-2140/0

BOT CHORD 1-12=0/1265, 10-12=0/1265, 9-10=0/1265

WEBS 8-10=0/1035, 2-12=0/1035, 3-7=-1451/171

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 11-11-8, Exterior(2) 11-11-8 to 16-4-5, Interior(1) 16-4-5 to 23-9-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 2-3, 7-8, 3-7; Wall dead load (5.0psf) on member(s).8-10, 2-12

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 10-12

7) Attic room checked for L/360 deflection.



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[-			BI		
Job	Truss	Truss Type	Qty	РІУ	Precision/Lot 25 Liberty N	leadows/Harnet I56905207
J0722-3740	C4	ATTIC GIRDER	1	2	Job Reference (ontional)	
Comtech, Inc, Fay	vetteville, NC - 28314,		۲ <u>۵</u>	3.430 s Jai	n 6 2022 MiTek Industries,	Inc. Tue Feb 28 15:24:45 2023 Page 1
	L	5-8-4 8-11-6 1	ID:52SygMJAaH 1-11-8 14-11-10 18-2	-12	23-11-0	AYM824IB6I2ArPI216_k61LCWtD2g2JW
	I	5-8-4 3-3-2	3-0-2 ' 3-0-2 ' 3-3	-2	5-8-4	
			6x8 =			Scale = 1:79.2
		12.00 12				
	T		5 4			
		8x8 1/	8x8 5	>		
		$4x6 = \frac{4}{4}$		k6 =		
		3	7			
		4x12				
	⁸	2		$\langle \rangle$	4x12 8	
	13-5				\backslash	
			φ- Ζ -4			
		4-10	12-1-0			
	1		12 1 0		9	
	q					
		↓ 13 14 15 16	17 11 18	Ļ	I	
	588 -	- 12	7x14 M18AHS =	10	5x8 —	
	<u> </u>	3x10 5-8-4 11-11-8	18-2-12	3x10	23-11-0	
Plate Offsets (X,Y)	[1:0-4-13,0-2-8], [2:0-9-4,0-1-4],	<u>5-8-4</u> <u>6-3-4</u> [5:0-4-0,Edge], [8:0-9-4,0-1-4], [<u>6-3-4</u> 9:0-4-13,0-2-8], [10:0-7-8	,0-1-8], [1	<u>5-8-4</u> 12:0-7-8,0-1-8]	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.59	Vert(LL) -0.24	10-12	>999 360	MT20 244/190
BCLL 0.0 *	Rep Stress Incr NC	WB 0.32	Horz(CT) -0.46	10-12 9	n/a n/a	W18ARS 186/179
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.15	10-12	>999 240	Weight: 547 lb FT = 20%
LUMBER-	PNo 1 *Except*		BRACING-	Structur	ral wood sheathing directly	applied or 6-0-0 oc purling
1-4,6-9	9: 2x10 SP 2400F 2.0E		BOT CHORD	Rigid ce	eiling directly applied or 10	-0-0 oc bracing.
WEBS 2x6 SF	9 2400F 2.0E 9 No.1					
REACTIONS. (size	e) 1=0-3-8, 9=0-3-8					
Max H	lorz 1=-296(LC 24)					
Max U Max G	Jpliπ 1=-373(LC 9), 9=-45(LC 8) Grav 1=6343(LC 2), 9=5175(LC 2)				
FORCES. (lb) - Max.	Comp./Max. Ten All forces 25) (lb) or less except when showr).			
TOP CHORD 1-2=-	-6911/193, 2-3=-3009/118, 3-5=-	141/1313, 5-7=-53/1082, 7-8=-3	240/207,			
BOT CHORD 1-12	=-23/3940, 10-12=-23/3977, 9-10	=-23/3945				
WEBS 8-10=	=-6/4586, 2-12=-279/5259, 3-7=-	5610/268				
NOTES-	practed together with 10d (0 131	'v?") naile as follows:				
Top chords connect	ed as follows: 2x10 - 2 rows stag	gered at 0-9-0 oc.				
Webs connected as	follows: 2x6 - 2 rows staggered	taggered at 0-7-0 oc. at 0-9-0 oc.				
 All loads are consider ply connections have 	ered equally applied to all plies, e been provided to distribute only	except if noted as front (F) or bac / loads noted as (F) or (B), unles	ck (B) face in the LOAD C so otherwise indicated.	ASE(S) s	section. Ply to	ANNULLE.
3) Unbalanced roof live	e loads have been considered for	this design.				WITH CARO
Lumber DOL=1.60 p	plate grip DOL=1.60	DL=6.0psi; BCDL=6.0psi; n=15i	; Cat. II; Exp C; Enclosed	; WWFR3	s (envelope);	OR EESTING MUL
5) All plates are MT206) This truss has been	plates unless otherwise indicated designed for a 10.0 psf bottom of	1. hord live load nonconcurrent wit	h anv other live loads.		- Ca	the last
7) * This truss has bee	en designed for a live load of 30.0	psf on the bottom chord in all ar	eas where a rectangle 3-	6-0 tall by	/ 2-0-0 wide	
8) Ceiling dead load (1	0.0 psf) on member(s). 2-3, 7-8,	3-7; Wall dead load (5.0psf) on	member(s).8-10, 2-12		E	SEAL
 9) Bottom chord live lost 10) Provide mechanica 	ad (40.0 psf) and additional botto al connection (by others) of truss	m chord dead load (10.0 psf) ap to bearing plate capable of with	pplied only to room. 10-12 standing 100 lb uplift at jo	: int(s) 9 e:	xcept (jt=lb)	030322
1=373. 11) Hanger(s) or other	connection device(s) shall be pr	ovided sufficient to support conc	entrated load(s) 1311 lb (hown and	190 lb up at	ALEN ALLE
2-0-12, 1311 lb do	wn and 190 lb up at 4-0-12, 513	Ib down and 115 lb up at 6-0-12	2, 1431 lb down and 220	lb up at 7	7-11-0, 1431	NGINEE A
ט down and 220 b design/selection of	o up at 12-0-0, and 1148 lb dowr f such connection device(s) is the	at 15-9-0, and 1431 lb down a responsibility of others.	na 220 lb up at 21-8-0 or	bottom c	cnora. The	A. GILBHIN
12) Attic room checked	d for L/360 deflection.					March 1 2023
COARGASE(S)geStan	dard					
	design parameters and READ NOTES ON		CE PAGE MII-7473 rev. 5/19/202	0 BEFORE I	USE.	ENGINEERING BY

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818 Soundside Road Edenton, NC 27932

RE

Job	Truss	Truss Type	Qty	Ply	Precision/Lot 25 Liberty Meadows/Harnet	
						156905207
J0722-3740	C4	ATTIC GIRDER	1	2		
				-	Job Reference (optional)	
Comtech, Inc, Fayettev	ille, NC - 28314,			3.430 s Jar	6 2022 MiTek Industries, Inc. Tue Feb 28 15:24:45 2023	Page 2

8.430 s Jan 6 2022 MiTek Industries, Inc. Tue Feb 28 15:24:45 2023 Page 2 ID:52SygMJAaHxrWTaExIrbuZyFiSD-IUW8ULSGKs9AYM824IBei2ArPIz16_ke1LCWtDzgZjW

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-3=-80, 3-5=-60, 5-7=-60, 7-8=-80, 8-9=-60, 1-12=-20, 10-12=-40, 9-10=-20, 3-7=-20 Drag: 8-10=-10, 2-12=-10

Concentrated Loads (lb)

Vert: 13=-1219(B) 14=-1219(B) 15=-411(B) 16=-1144(B) 17=-1144(B) 18=-756(B) 19=-1144(B)

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Plate OI	sets (X, Y)	[2:0-5-8,Edge], [4:0-5-8,E	zagej									
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.25	Vert(LL)	0.05	4-6	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.32	Vert(CT)	-0.04	4-6	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.21	Horz(CT)	0.01	4	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-S						Weight: 114 lb	FT = 20%
LUMBER	۶.			•		BRACING						

TOP CHORD

BOT CHORD

LUMBER-

2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.2 OTHERS 2x4 SP No.2 WEDGE

Left: 2x4 SP No.2 , Right: 2x4 SP No.2

REACTIONS.

(size) 2=0-3-0. 4=0-3-0 Max Horz 2=171(LC 11) Max Uplift 2=-146(LC 12), 4=-146(LC 13) Max Grav 2=662(LC 1), 4=662(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-699/643, 3-4=-699/643
- BOT CHORD 2-6=-370/462, 4-6=-370/462
- WEBS 3-6=-462/359

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=146, 4=146.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



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3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=182, 6=178.



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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.05 WB 0.15 Matrix-S	DEFL. in Vert(LL) 0.00 Vert(CT) 0.00 Horz(CT) 0.00	(loc) 10 11 10	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 106 lb	GRIP 244/190 FT = 20%
LUMBER-			BRACING-					

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. All bearings 17-4-8.

(lb) -Max Horz 2=227(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 18, 13, 10 except 17=-119(LC 12), 19=-154(LC 12), 15=-109(LC 13), 12=-161(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 16, 17, 18, 19, 15, 13, 10 except 12=261(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 18, 13, 10 except (jt=lb) 17=119, 19=154, 15=109, 12=161.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 11=170, 8=170.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.31 BC 0.32 WB 0.28 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 1 n/r 120 Vert(CT) -0.00 1 n/r 120 Horz(CT) 0.00 7 n/a n/a Weight: 66 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF	2 No.1		BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

 BOT CHORD
 2x4 SP No.1

 BOT CHORD
 2x4 SP No.1

 WEBS
 2x4 SP No.2

 OTHERS
 2x4 SP No.2

REACTIONS. All bearings 13-3-12.

(lb) - Max Horz 2=176(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 7, 2 except 9=-188(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 2 except 7=275(LC 20), 8=672(LC 19), 9=573(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 4-8=-304/62, 3-9=-481/339

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 9-4-0, Exterior(2) 9-4-0 to 13-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2 except (jt=lb) 9=188.

6) Non Standard bearing condition. Review required.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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A MITek Aft 818 Soundside Road

Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-74/3 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Precision/Lot 25 Liberty Meadows/Harnet	
						156905215
J0722-3740	PB4	PIGGYBACK	4	1		
					Job Reference (optional)	
Comtech, Inc,	Fayetteville, NC - 28314,			3.430 s Jar	1 6 2022 MiTek Industries, Inc. Tue Feb 28 15:24:54 2023	Page 2

ID:52SygMJAaHxrWTaExIrbuZyFiSD-_DYYNQZvDdlu7kKn68rlZy2TLwy7j?1z6FuUiCzgZjN

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-148, 4-7=-147, 2-6=-49

Concentrated Loads (lb)

Vert: 4=-2600(F=-1300, B=-1300)

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TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 11-7-13.

(lb) -Max Horz 1=-164(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10, 9 except 11=-168(LC 12), 8=-170(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 7, 11, 8 except 10=271(LC 19), 9=266(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10, 9 except (it=lb) 11=168. 8=170.



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¹⁾ Unbalanced roof live loads have been considered for this design.



								000		
LOADING	i (psf)	SPACING- 2-0-0	CSI.	DEFL.	in ((loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.20	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.14	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.05	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S						Weight: 38 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

TOP CHORD

REACTIONS. 1=9-2-2, 3=9-2-2, 4=9-2-2 (size) Max Horz 1=-102(LC 8)

Max Uplift 1=-25(LC 13), 3=-25(LC 13) Max Grav 1=194(LC 1), 3=194(LC 1), 4=296(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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LOADING (pst) SPACING- 2-0-0 CSI. DEFL. TCLL 20.0 Plate Grip DOL 1.15 TC 0.11 Vert(LL) TCDL 10.0 Lumber DOL 1.15 BC 0.05 Vert(CT) BCLL 0.0 * Rep Stress Incr YES WB 0.01 Horz(CT) BCDL 10.0 Code IRC2015/TPI2014 Matrix-P Matrix-P Matrix-P	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 23 lb	GRIP 244/190 FT = 20%
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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. 1=5-10-2, 3=5-10-2, 4=5-10-2 (size) Max Horz 1=62(LC 9) Max Uplift 1=-23(LC 13), 3=-23(LC 13) Max Grav 1=127(LC 1), 3=127(LC 1), 4=163(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 5-10-14 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.

6) Non Standard bearing condition. Review required.



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REACTIONS. (size) 1=6-8-5, 3=6-8-5, 4=6-8-5 Max Horz 1=-46(LC 8) Max Uplift 1=-20(LC 12), 3=-24(LC 13) Max Grav 1=124(LC 1), 3=125(LC 1), 4=209(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.6) Non Standard bearing condition. Review required.



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		C	Client: Project:			Date: Input by:	2/28/2023 Neal Baggett				Page 1 of 10
is	Design	A	ddress:			Job Nam Broject #	e: 25 LIBERTY M	EADOWS			
BM1	Kerto-S L	VL 1	.750" X	9.250"	2-Ply -	PASSED	Level: Level				
•	•	•	1	•	•					\mathbb{M}	
	(1980) -	- Pin •	4.174		2. M					Ŵ	9 1/4
1 SPF	End Grain		5144 1		2	SPF End Grain					
			5'11"							1 1	3 1/2"
			511			I					
Member In	formation					Reactions UN	PATTERNED	b (Uplift)			
Type:	Girder		Application:	Floor		Brg Direction	Live	Dead	Snow	Wind	Const
Moisture Con	dition: Dry		Building Code	e: IBC/IRC	2015	2 Vertical	0	1835	1813	0	0
Deflection LL	: 480		Load Sharing	: No							
Importance:	: 360 Normal - II		Deck:	Not Chec	cked						
Temperature:	Temp <= 100	۴F									
						Bearings					
						Bearing Lengt	h Dir. Cap. Vert 41%	React D/L lb	Total	Ld. Case	Ld. Comb.
						End	Vent 4170	10557 1015	5040	L	D+3
Analysis Re	esults				2	Grain 2 - SPF 3 000"	' Vert 41%	1835 / 1813	3648	I	D+S
Analysis Moment	Actual 4734 ft_lb	Location A	Allowed Ca	ipacity Comi	o. Case	End		1000 / 1010		-	2 0
Unbraced	4734 ft-lb	2'11 1/2" 1	1027 ft-lb 0.4	29 (43%) D+S	L	Grain					
Shear	2395 lb	4'10 3/4" 7	943 lb 0.3	802 (30%) D+S	L						
LL Defl inch	0.037 (L/1820)	2'11 1/2" 0	.139 (L/480) 0.2	264 (26%) S	L						
TL Defl inch	0.074 (L/904)	2'11 1/2" 0	.185 (L/360) 0.3	98 (40%) D+S	L	4					
Design No	tes				1 - 4 1	4					
may also b 2 Fasten all to exceed	pport to prevent late be required at the inte plies using 2 rows of 6".	rai movement erior bearings 10d Box nails	by the building c (.128x3") at 12"	ode. o.c. Maximum e	end distance not						
3 Refer to la	st page of calculation	ns for fastener	s required for sp	ecified loads.							
5 Top loads	e designed to be sup must be supported e	ported on the qually by all p	lies.	у.							
6 Top must b	e laterally braced at	end bearings	nas								
8 Lateral sle	nderness ratio based	d on single ply	width.								
ID	Load Type	L	ocation Trib \	Nidth Side	Dead 0.9	Live 1 Sno	ow 1.15 Wind	1.6 Const. 1	1.25 Com	iments	
1	Uniform			Тор	613 PLF	0 PLF	613 PLF 0	PLF 0	PLF A02		
	Self Weight				7 PLF						
Notes		chemical	s	A	For flat roofs provide	proper drainage to prevent	Manufacturer Info		Comtech, In	IC.	320
Calculated Structured structural adequacy	d Designs is responsible only of of this component based or	of the Handling	& Installation	ed U	ponding	, , <u></u> to protoin	Metsä Wood 301 Merritt 7 Buildin	ng 2nd Floor	Fayetteville, USA	NC NC	2029
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2. LVL not to be treated with fire retardant or corrosive lateral displacement and rotation This design is valid until 11/3/2024	Lumber	ions unless noted otherwise	3. Damaged 4. Design as	d Beams must no ssumes top edge	t be used is laterally restrain	ed				www.met	sawoou.con	11/43			
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			Client:			Date:	2/28/2023	Page 4 of 10
1	isDesign		Address			Job Na		
- 4	150 051511		/ (00000			Project	#:	
GDH	Korto-S	I VI	1 750"	X 11 875"	2-Plv	PASSED	Level: Level	
ODII	Nerto-O		1.750	X 11.075	Z-1 1y -	AGGED		
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		·						
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1 SPF	End Grain						2 SPF E	nd Grain
1					16'7"			1 13 1/2"
1					16'7"			ł
/lulti_Dl	v Analysis							
	y Anarysis Ludiae weiner 2 r			(10021) + 121	a a Massima			
asten al	i piles using 2 r	OWS OF I	Ud Box halls	s (.128x3°) at 12°	o.c Maxim	um end distance	not to exceed 6".	
apacity		0.0 % 0.0 PI	F					
ield Limit p	er Foot	163.7	PLF					
ield Limit p	er Fastener	81.9 I	b.					
ield Mode		IV						
dge Distan		1 1/2"						
oad Combi	nation	3						
uration Fac	ctor	1.00						
Notes Calculated Struct	ctured Designs is responsible	e only of the Ha	chemicals Indling & Install	ation	6. For flat roofs prov ponding	vide proper drainage to prevent	Manufacturer Info Metsä Wood	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
lesign criteria esponsibility of	and loadings shown.	It is the 2.	LVL beams must not b Refer to manufac	e cut or drilled turer's product information			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	28314 910-864-TRUS
nsure the co application, and	omponent suitability of the to verify the dimensions and	e intended loads.	fastening details, bea approvals	im strength values, and code			(800) 622-5850 www.metsawood.com/us	
	conditions unloss poted atter	3. 4.	Damaged Beams mus Design assumes top e	t not be used dge is laterally restrained				
2. LVL not to b	e treated with fire retardant of	or corrosive 5.	Provide lateral support lateral displacement a	ort at bearing points to avoid nd rotation	This design is	valid until 11/3/2024		соттесн
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			Client:				Date	e:	2/28/20	23				Page 5 of 10
Tis	Design	l	Project: Address:				Inpu Job	ut by: Name	Neal Ba e: 25 LIBE	ggett RTY MEA	DOWS			
		-					Proj	ject #:						
BM4	Kerto-S L	VL ŕ	1.750"	X 9.2	50" 2	-Ply -	PASSE	D	Level: Leve	9				
	1	2												
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								<u></u>						
	¥	. ↓		•	•	3	•		•				N A	1 1
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	· ·		es ili	•	1. A.		-	and the	- ALIA				///`	5 1/-
	End Grain						2 SP	PF End	l Grain]				
				6'7"					,	ĺ			1	3 1/2"
1				6'7"					,	ſ				
Member In	formation						Reactions	UNI	PATTERI	NED Ib	(Uplift)			
Type:	Girder		Applicati	on: I	Floor		Brg Direc	tion	Live	e [Dead	Snow	Wind	Const
Plies: Moisture Con	2 dition: Drv		Design N Building	lethod: /	ASD BC/IRC 2015		1 Vertic	al	()	2148	2124	0	0
Deflection LL:	480		Load Sha	aring:	No		2 Vertic	al	()	1663	1640	0	0
Deflection TL:	360		Deck:	J I	Not Checked									
Importance:	Normal - II													
Temperature:	Temp <= 100	Э°F												
							Bearings							
							Bearing L	_ength 3.000"	n Dir. Vert	Cap. R 48%	eact D/L lb 2148 / 2124	o Total 4273	Ld. Case L	Ld. Comb. D+S
Analysis Re	sults						End Grain							
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2-SPF 3	3.000"	Vert	37%	1663 / 1640	3303	L	D+S
Moment	6600 ft-lb	1'11 1/2"	14423 ft-lb	0.458 (46%	6) D+S	L	End							
Unbraced	6600 ft-lb	1'11 1/2"	10370 ft-lb	0.636 (64%	%) D+S	L	Grain							
Shear	4271 lb	1' 1/4"	7943 lb	0.538 (54%	%) D+S	L								
LL Defl inch	0.058 (L/1296)	3' 3/4" (0.155 (L/480)	0.370 (37%	%) S	L								
TL Defl inch	0.116 (L/644)	3' 13/16" (0.207 (L/360)	0.559 (56%	%) D+S	L								
Design Not	tes						1							
1 Provide su	pport to prevent late	eral movemen	t and rotatior	at the end b	pearings. Late	eral support	1							
2 Fasten all	e required at the int	erior bearings f 10d Box nail	s by the build (128x3") a	ing code. t 12" o.c. Ma	wimum end d	istance not								
to exceed 6	6".		13 (. 120x0) a	C 12 0.0. Wit		istance not								
3 Refer to las	st page of calculatio	ns for fastene	ers required for	or specified l	oads.									
5 Top loads r	nust be supported e	equally by all r	olies.	e only.										
6 Top must b	e laterally braced a	t end bearings	3.											
7 Bottom mu	st be laterally brace	ed at end bear	ings.											
8 Lateral slei	I ood Type	d on single pl	y width.	Trib Width	Sido	Dood 0 0	Livo 1	Sno	W 1 15	Wind 1	6 Const	1.25 Con	monto	
1	Point	L	1_3_0		Ton	672 lb		3110	672 lb		h Const.		ments	
1	Rearing Length	1	0-3-8		юр	07210	010		07210	01	D			
2	Point	1	1-11-8		Top	1671 lb	0 lb		1671 lb	01	b	0 lb A06		
-	Bearing Length	ı	0-3-8		4	107 110	010			01	-	3.2 A00		
3	Part. Uniform	. 2-11-8	3 to 6-7-0		Тор	392 PLF	0 PLF	3	92 PLF	0 PL	F 0	PLF A07		
Ū	Self Weight				·	7 PLF								
Notes		chemica	als		6. For fla	at roofs provide p	roper drainage to pr	revent	Manufactu	rer Info		Comtech, I	nc.	639
Calculated Structured	Designs is responsible only	of the Handling	g & Installatio	n	pondir	ig , p	. <u> </u>	F	Metsä Woo	d Z Duilleli	0md El	Fayetteville	, NC	
design criteria and responsibility of the	d loadings shown. It is customer and/or the contract	the 1. LVL bea	to manufacturer	or drilled s product info	rmation				301 Merritt Norwalk, C	7 Building, T 06851	2nd Floor	28314 910-864-TF	RUS	
ensure the compor application, and to ver	nent suitability of the interint the dimensions and loads	ended fastenin approva	g details, beam sl Is	requirements, n rength values, an	d code				(800) 622-5 www.metsa	850 wood.com/	us			
Lumber	ions unless noted otherwise	 Damage Design 	ed Beams must not assumes top edge	be used s laterally restraine	ed					2.5011/				
2. LVL not to be treat	ated with fire retardant or cor	rosive 5. Provide lateral d	lateral support at lisplacement and ro	bearing points to tation	o avoid This	desian is valid	until 11/3/2024					C	omt	есн

		Client:	Date:	2/28/2023	Page 6 of 10
1	isDesign	Address:	Job Nan	ne: 25 LIBERTY MEADOWS	
BM4	Kerto-S LV	L 1.750" X 9.250	" 2-Ply - PASSED	t: Level: Level	
•	•	• •	• •	•	\mathbf{M} 1
	•			•	9 1/4
	PF End Grain		2 SPF Er		
		6'7"			3 1/2"
1		6'7"			
Multi-Ply	y Analysis				
Fasten all	l plies using 2 rows o	f 10d Box nails (.128x3") at 12	" o.c Maximum end distance r	not to exceed 6".	
Load Vield Limit p	0. 0. er Foot 16	0 % 0 PLF			
Yield Limit po	er Fastener 81	I.9 lb.			
Edge Distan	ice 1	1/2"			
Min. End Dis Load Combi	stance 3" nation				
Duration Fac	ctor 1.	00			
				Manufacturor Info	Comtech Inc.
Notes Calculated Struc	ctured Designs is responsible only of the	chemicals Handling & Installation	6. For flat roofs provide proper drainage to prevent ponding	Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC
structural adequi design criteria responsibility of	and loadings shown. It is the the customer and/or the contractor to	 LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-plu 	,	301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	28314 910-864-TRUS
ensure the co application, and Lumber	omponent suitability of the intended to verify the dimensions and loads.	fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used		ເວບບ) ຽ22-ວຽວປ www.metsawood.com/us	
1. Dry service o 2. LVL not to be	conditions, unless noted otherwise treated with fire retardant or corrosive	 Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation 	This design is valid until 11/3/2024		соттесн
L					



	Client:			Date:	2/28/2023	Page 8 of 10
isDesign	Address	:		Job Nam	e: 25 LIBERTY MEADOWS	
BM2 Kerto-S	SLVL 1.75	0" X 9.250"	2-Ply -		Level: Level	
			,			
• •	٠	٠	٠	٠		\mathbf{M} 1
	•	•	•	•	•	9 1/4
1 SPF End Grain				2 SPF End		
		6'7"				3 1/2"
ŕ		6'7"				
Multi-Ply Analysis						
Fasten all plies using 2	rows of 10d Box nai	ls (.128x3") at 12" (o.c Maximun	n end distance n	ot to exceed 6".	
Load Yield Limit per Foot	0.0 % 0.0 PLF 163 7 PLF					
Yield Limit per Fastener	81.9 lb.					
Edge Distance	1 1/2"					
Min. End Distance Load Combination	3"					
Duration Factor	1.00					
Notes	chemicals		6. For flat roofs provide	proper drainage to prevent	Manufacturer Info	Comtech, Inc.
Calculated Structured Designs is responsib structural adequacy of this component b	le only of the Handling & Insta ased on the 1. LVL beams must not	llation t be cut or drilled	ponding	. , <u>9</u> - 15 protont	Metsä Wood 301 Merritt 7 Building. 2nd Floor	Fayetteville, NC USA 29314
design criteria and loadings shown. responsibility of the customer and/or the ensure the component suitability of t	It is the 2. Refer to manufa contractor to he intended fastening details, be	acturer's product information tion requirements, multi-ply eam strength values, and code			Norwalk, CT 06851 (800) 622-5850	20314 910-864-TRUS
application, and to verify the dimensions and Lumber 1. Dry service conditions, unlose noted ath	a loads. approvals 3. Damaged Beams m 4. Design assumes top	ust not be used edge is laterally restrained			www.metsawood.com/us	
2. LVL not to be treated with fire retardant	t or corrosive 5. Provide lateral sup lateral displacement	port at bearing points to avoid and rotation	This design is val	id until 11/3/2024		соттесн

		CI	ient [.]					Date:		2/28/202	3				Page 9 of 10
		Pr	oject:					Input I	by:	Neal Ba	gett				. ugo o oi io
is	Design	Ad	ddress:					Job N	ame:	25 LIBEI	RTY ME	ADOWS			
								Projec	ct #:						
BM3	Kerto-S L	.VL 1	.750"	X 9.25	50" 2	-Ply -	PAS	SEC)	evel: Level					
							2								
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	nd Grain					25	SPF End	Grain						1	
			5'1	1"		20			\downarrow					<u>}</u>	3 1/2"
<u> </u>			<u>۲۱</u>	1"					\downarrow					I	,
I			51						1						
Manaharata	formette :						D	4 1		****		11-120			
Type	Girder		Applicatio	on. E	loor		Bro	Directic			מו עםי	Dead	Snow	Wind	Const
Plies:	2		Design M	lethod: A	SD		1	Vertical	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	0		1770	1748	0	001130
Moisture Cond	dition: Dry		Building (Code: II	BC/IRC 2015		2	Vertical		0		3001	2979	0	0
Deflection LL:	480		Load Sha	aring: N	lo										
Deflection TL:	360		Deck:	Ν	lot Checked										
Importance:	Normal - II	205													
lemperature:	100 Temp <= 100	J°F					Bear	inas							
							Bea	ring le	nath	Dir	Can	React D/L lb	Total I	d Case	I.d. Comb
							1-3	SPF 3.0	00"	Vert	40%	1770 / 1748	3518 L	-	D+S
							Enc								
Analysis Re	sults						Gra	IN SDE 30	00"	Vert	68%	3001 / 2979	5980 1		D+S
Analysis	Actual	Location Al	lowed	Capacity	Comb.	Case	Enc	51 0.0	00	Ven	0070	000172070	0000 1	-	0.0
Moment	5104 ft-lb	3'3 3/16" 14	423 π-ID	0.354 (35%) D+S	L	Gra	in							
Shear	5104 IL-ID 4741 Ib	3 3 3/ 10 11 1/10 3/1" 70	027 IL-ID	0.403 (40%) D+S										
	4741 ID 0.040 (I./1650)	3'1 1/16" 0	139 (I /480)	0.397 (00%	5) D+3										
TL Defl inch	0.081 (L/821)	3'1" 0.	185 (L/360)	0.439 (44%	5) D+S	L									
Design Not	205				-,	_	1								
1 Provide sup	port to prevent late	eral movement a	and rotation	at the end b	earings. Late	ral support	1								
may also b	e required at the int	erior bearings b	by the buildi	ng code.											
to exceed 6	olies using 2 rows o 5".	T TUO BOX NAIIS	(.128x3°) at	12" 0.C. Ma	ximum ena ai	stance not									
3 Refer to las	t page of calculatio	ns for fasteners	s required fo	or specified lo	oads.										
4 Girders are	designed to be supported	ported on the l	pottom edge	e only.											
6 Top must b	e laterally braced a	t end bearings.	65.												
7 Bottom mu	st be laterally brace	ed at end bearin	gs.												
8 Lateral sler	I ood Turco	d on single ply	width.	rih \N/idth	Sido	Dead 0.0	1	ive 1	Snow	1 15	Mind 4	6 Const 1	25 Com	monto	
1	Part Uniform		$- 4_{-1}$		Ton	535 DI F			wono נכש	5 PI F	י אווע ו				
י ס	Point	U-U-U I	5_1_0		Top	25/2 ILF			33 24	543 lh	۹U	∟r UI lb			
۷	Rearing Long	1	0-3-8		ioh	2040 10		U U	23	UI UFU	U	U.	UID AU4		
	Self Weight	1	0-3-8												
Notes		chemicals	& Installation	n	6. For fla pondin	it roofs provide p g	roper drair	age to preve	ent N	Anufactur	er Info		Comtech, Ind 1001 S. Reill Favetteville	o. y Road, Suite i NC	#639
structural adequacy of design criteria and	of this component based of loadings shown. It is	on the 1. LVL beams	s must not be cut	or drilled	mation				3	01 Merritt 7	Building	, 2nd Floor	USA 28314		
responsibility of the o ensure the compon	ent suitability of the int	ended fastening	installation i details, beam str	equirements, m equirements, m	ulti-ply d code				N (8	orwalk, CT 800) 622-58	06851 350		910-864-TRI	JS	
application, and to ver Lumber	ity the dimensions and loads	approvals 3. Damaged	Beams must not I	be used					Ŵ	/ww.metsav	vood.con	n/us			
 Dry service conditi LVL not to be treat 	ons, unless noted otherwise ted with fire retardant or cor	4. Design as 5. Provide la lateral disr	sumes top edge is iteral support at placement and rot	bearing points to ation	avoid			2/2024					C	эmт	есн
					I NIS (uesiyn is valld	unui T1/	JIZUZ4	1						

2		Client: Project:	Date: Input by:	2/28/2023 Neal Baggett	Page 10 of 1
1	isDesign	Address:	Job Name	e: 25 LIBERTY MEADOWS	
BM3	Kerto-S LVL	1.750" X 9.250"	2-Ply - PASSED	Level: Level	
•	•	• •	• • •		
•	•	• •		<u> </u>	9 1/-
1 SF	PF End Grain		2 SPF End Grain	-+	
		5'11" 5'11"			3 1/2"
M	. Analysis				
-asten all	l plies using 2 rows of	10d Box nails (.128x3") at 12" c	o.c Maximum end distance n	ot to exceed 6".	
;apacity .oad ⁄ield Limit p	0.0 0.0 ver Foot 163	% PLF .7 PLF			
ield Limit p ield Mode	ver Fastener 81.9) lb.			
dge Distan	ice 1 1/	2"			
oad Combi	ination				
Juration Fac	ctor 1.00	J			
				Manufacturer Info	Comtech, Inc.
NOTES Calculated Structural adecu	ctured Designs is responsible only of the lucy of this component based on the	Handling & Installation	ponding	Metsä Wood	 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA
design criteria responsibility of	the customer and/or the contractor to	LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply		Norwalk, CT 06851 (800) 622-5850	28314 910-864-TRUS
application, and	to verify the dimensions and loads.	tastening details, beam strength values, and code approvals 3. Damaged Beams must not be used		www.metsawood.com/us	
 Dry service of 2. LVL not to be 	conditions, unless noted otherwise be treated with fire retardant or corrosive	 Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation 	This design is valid until 11/2/2024		соттесн