

RE: J0223-0759 Lot 51 Liberty Meadows Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J0223-0759 Lot/Block: Address: City:

Model: Subdivision: State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 20 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	155636908	A1	12/8/2022
2	155636909	A2	12/8/2022
3	155636910	A2-P	12/8/2022
4	155636911	A3	12/8/2022
5	155636912	A4	12/8/2022
6	155636913	A5	12/8/2022
7	155636914	A6	12/8/2022
8	155636915	A7	12/8/2022
9	155636916	A8	12/8/2022
10	155636917	B1	12/8/2022
11	155636918	B2	12/8/2022
12	155636919	C01	12/8/2022
13	155636920	M1	12/8/2022
14	155636921	M2	12/8/2022
15	155636922	M4	12/8/2022
16	155636923	M5	12/8/2022
17	155636924	P1	12/8/2022
18	155636925	P2	12/8/2022
19	155636926	V1	12/8/2022
20	155636927	V2	12/8/2022

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2023

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



22, 2

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 9-10=-97/284, 10-11=-113/327, 11-12=-113/328, 12-13=-97/285

#### NOTES-

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-10 to 38-3, Exterior(2) 3-8-3 to 16-0-0, Corner(3) 16-0-0 to 20-4-13, Exterior(2) 20-4-13 to 32-8-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 30, 31, 32, 33, 34, 35, 36, 28, 27, 26, 25, 24, 23, 22, 2.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



	10-9-3 10-9-3		21-2-13 10-5-11	+	32-0-0 10-9-3
LOADING   (psf)     TCLL   20.0     TCDL   10.0     BCLL   0.0     BCDL   10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.26 BC 0.49 WB 0.20 Matrix-S	DEFL.   in   (loc)     Vert(LL)   -0.21   10-12     Vert(CT)   -0.29   10-12     Horz(CT)   0.05   8     Wind(LL)   0.04   2-12	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES   GRIP     MT20   244/190     Weight: 205 lb   FT = 20%

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-5-8, 8=0-5-8 Max Horz 2=-109(LC 10) Max Uplift 2=-89(LC 12), 8=-89(LC 13) Max Grav 2=1318(LC 1), 8=1318(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-2137/485, 4-5=-1939/499, 5-6=-1939/499, 6-8=-2137/485

BOT CHORD 2-12=-313/1866, 10-12=-98/1255, 8-10=-315/1823

WEBS 5-10=-123/785, 6-10=-433/279, 5-12=-123/785, 4-12=-433/279

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13, Interior(1) 20-4-13 to 32-8-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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TOP CHORDStructural wood stBOT CHORDRigid ceiling direct

Structural wood sheathing directly applied or 5-1-2 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



	10-9-3 10-9-3	12-0-0 1-2-13	20-0-0 8-0-0	21-2-13 1-2-13	32-0-0 10-9-3	
LOADING   (psf)     TCLL   20.0     TCDL   10.0     BCLL   0.0     BCDL   10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.26 BC 0.49 WB 0.21 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) //defl -0.11 10-14 >999 -0.23 10-14 >999 0.05 8 n/a 0.05 10-14 >999	L/d <b>PLATES</b> 360 MT20 240 n/a 240 Weight: 224 lb	<b>GRIP</b> 244/190 • FT = 20%

BOT CHORD

LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2 *Except
	11-13: 2x6 SP No.1

REACTIONS. (size) 2=0-5-8, 8=0-5-8 Max Horz 2=109(LC 11) Max Grav 2=1418(LC 1), 8=1418(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-2346/270, 4-5=-2120/282, 5-6=-2120/282, 6-8=-2346/270

BOT CHORD 2-14=-125/2025, 10-14=0/1366, 8-10=-127/1998

WEBS 5-10=-9/880, 6-10=-421/290, 5-14=-9/880, 4-14=-421/290

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13, Interior(1) 20-4-13 to 32-8-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) 200.0lb AC unit load placed on the bottom chord, 16-0-0 from left end, supported at two points, 5-0-0 apart.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 4-10-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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	<u>10-9-3</u> 10-9-3	ł	21-2-13 10-5-11	ł	<u>32-0-0</u> 10-9-3
LOADING   (psf)     TCLL   20.0     TCDL   10.0     BCLL   0.0     BCDL   10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.26 BC 0.49 WB 0.21 Matrix-S	DEFL.   in   (lc     Vert(LL)   -0.21   9-     Vert(CT)   -0.29   9-     Horz(CT)   0.05   9-     Wind(LL)   0.04   2-	bc) l/defl L/d 11 >999 360 11 >999 240 8 n/a n/a 11 >999 240	PLATES   GRIP     MT20   244/190     Weight: 203 lb   FT = 20%

BRACING-TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

 WEBS
 2x4 SP No.2

 REACTIONS.
 (size)
 8=0-5

NS. (size) 8=0-5-8, 2=0-5-8 Max Horz 2=109(LC 11) Max Uplift 8=-75(LC 13), 2=-89(LC 12) Max Grav 8=1261(LC 1), 2=1319(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-2139/486, 4-5=-1940/499, 5-6=-1943/516, 6-8=-2141/505

BOT CHORD 2-11=-323/1865, 9-11=-104/1254, 8-9=-324/1829

WEBS 5-9=-126/789, 6-9=-437/283, 5-11=-123/785, 4-11=-433/279

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13, Interior(1) 20-4-13 to 31-9-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 5-0-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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![](_page_4_Picture_19.jpeg)

![](_page_5_Figure_0.jpeg)

L	10-9-3		21-2-13	31-6-8	
I	10-9-3	1	10-5-11	10-3-11	I
Plate Offsets (X,Y)	[8:0-5-8,Edge]				
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	<b>CSI.</b> TC 0.35	<b>DEFL.</b> in (loc) I/de Vert(LL) -0.21 9-11 >99	efl L/d <b>PLATES</b> 99 360 MT20	<b>GRIP</b> 244/190

TCLL   20.0     TCDL   10.0     BCLL   0.0 *     BCDL   10.0	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	TC 0.35 BC 0.49 WB 0.20 Matrix-S	Vert(LL) -0.29 Vert(CT) -0.29 Horz(CT) 0.05 Wind(LL) 0.04	9-11   >999   360     9-11   >999   240     5   8   n/a   n/a     4   2-11   >999   240	MT20 244/190 Weight: 202 lb FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x6 SF WEBS 2x4 SF WEDGE Right: 2x4 SP No.2	P No.1 P No.1 P No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathin Rigid ceiling directly appl	g directly applied or 5-1-8 oc purlins. ied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-5-8, 8=Mechanical

Max Horz 2=109(LC 9) Max Uplift 2=-89(LC 12), 8=-73(LC 13) Max Grav 2=1307(LC 1), 8=1248(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-2114/481, 4-5=-1915/494, 5-6=-1890/507, 6-8=-2081/495

BOT CHORD 2-11=-317/1845, 9-11=-99/1232, 8-9=-309/1760

WEBS 5-9=-118/745, 6-9=-401/273, 5-11=-123/787, 4-11=-434/279

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13, Interior(1) 20-4-13 to 31-5-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_5_Figure_17.jpeg)

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![](_page_5_Picture_19.jpeg)

![](_page_6_Figure_0.jpeg)

	10-4-12		20-6-0	24-0-0	29-6-8 31-6	<u>5-8</u>		
Plate Offsets (X,Y)	[7:0-3-0.Edge]. [8:0-1-13.0-2-12]. [12:0-	0-4.0-2-41	10-1-4	3-0-0	<u> </u>	-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.87 BC 0.91 WB 0.60 Matrix-S	DEFL. in Vert(LL) -0.21 Vert(CT) -0.36 Horz(CT) 0.24 Wind(LL) 0.10	(loc) l/defl L/d 14-16 >999 360 14-16 >999 240 9 n/a n/a 11-12 >999 240	PLATES MT20 Weight: 212 lb	<b>GRIP</b> 244/190 FT = 20%		
LUMBER-   TOP CHORD 2x6 SP No.1   BOT CHORD 2x6 SP No.1   WEBS 2x4 SP No.2   WEDGE BRACING-   Right: 2x4 SP No.2   REACTIONS. (size)   9=Mechanical, 2=0-5-8   Max Horz 2=109(LC 9)   Max Grav 9=1262(LC 1),   Max Grav 9=1262(LC 1),								
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.   TOP CHORD 2-3=-2131/468, 3-5=-1946/512, 5-6=-1816/491, 6-8=-2746/581, 8-9=-893/226   BOT CHORD 2-16=-308/1845, 14-16=-100/1222, 12-14=-97/915, 12-13=-394/1830, 11-12=-396/2456, 8-11=-396/2456   WEBS 3-16=-437/280, 5-16=-139/798, 5-13=-104/718, 13-14=-991/177, 6-13=-1206/308, 6-11=0/556								
NOTES- 1) Unbalanced roc 2) Wind: ASCE 7-	NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)							

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13, Interior(1) 20-4-13 to 31-5-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 2.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_6_Figure_8.jpeg)

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A MiTek Af 818 Soundside Road Edenton, NC 27932

![](_page_7_Figure_0.jpeg)

10-4-12		20-6-0		24-0-0	<u>29-6-8</u> 3 <sup>-</sup> 5-6-8 2	1-6-8
[7:0-3-0,Edge], [8:0-1-13,0-2-12], [12:0-	0-4,0-2-4]	10-1-4		3-0-0	<u> </u>	-0-0
SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.87 BC 0.91 WB 0.60 Matrix-S	<b>DEFL.</b> Vert(LL) -0.2 Vert(CT) -0.3 Horz(CT) 0.2 Wind(LL) 0.1	in (loc) 1 14-16 6 14-16 4 9 0 11-12	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 212 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- BRACING-   TOP CHORD 2x6 SP No.1   BOT CHORD 2x6 SP No.1   WEBS 2x4 SP No.2   WEDGE 6-0-0 oc bracing: 9-10.   Right: 2x4 SP No.2 WEBS   REACTIONS. (size)   9=Mechanical, 2=0-5-8   Max Horz 2=109(LC 9)   Max Grav 9=1262(LC 1), 2=1308(LC 12)   Max Grav 9=1262(LC 1), 2=1308(LC 1)						oc purlins. Except:
Comp./Max. Ten All forces 250 (lb) or -2131/468, 3-5=-1946/512, 5-6=-1816/45 =-308/1845, 14-16=-100/1222, 12-14=-9 =-396/2456 =-437/280, 5-16=-139/798, 5-13=-104/71 =0/556 e loads have been considered for this de	less except when shown. 1, 6-82746/581, 8-98 7/915, 12-13394/1830, 8, 13-14991/177, 6-13- sign.	93/226 11-12=-396/2456, 1206/308,				
	10-4-12 10-4-12 [7:0-3-0,Edge], [8:0-1-13,0-2-12], [12:0-( SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014 P No.1 P No.1 P No.1 P No.2 22 23 24 24 24 25 25 24 25 25 25 25 25 25 25 25 25 25	10-4-12   10-4-12   [7:0-3-0,Edge], [8:0-1-13,0-2-12], [12:0-0-4,0-2-4]   SPACING- 2-0-0 CSI.   Plate Grip DOL 1.15   Lumber DOL 1.15   Rep Stress Incr YES   WB 0.60   Code IRC2015/TPI2014 Matrix-S   P No.1 P No.1   P No.1 P No.1   P No.1 P No.2   220 9=Mechanical, 2=0-5-8   Horz 2=109(LC 9) Jplift 9=-66(LC 13), 2=-88(LC 12)   Grav 9=1262(LC 1), 2=1308(LC 1) .   . Comp./Max. Ten All forces 250 (lb) or less except when shown.   e-2131/468, 3-5=-1946/512, 5-6=-1816/491, 6-8=-2746/581, 8-9=-8   =-308/1845, 14-16=-100/1222, 12-14=-97/915, 12-13=-394/1830, =-396/2456   =-437/280, 5-16=-139/798, 5-13=-104/718, 13-14=-991/177, 6-13==0/556   re loads have been considered for this design.	ID4-12   ID4-14     10-4-12   10-1-4     [7:0-3-0,Edge], [8:0-1-13,0-2-12], [12:0-0-4,0-2-4]   DEFL.     SPACING-   2-0-0   CSI.   DEFL.     Plate Grip DOL   1.15   TC   0.87   Vert(LL)   -0.2     Lumber DOL   1.15   BC   0.91   Vert(CT)   -0.3     Rep Stress Incr   YES   WB   0.60   Horz(CT)   0.2     Code IRC2015/TPI2014   Matrix-S   Wind(LL)   0.1     P No.1   DP CHORD   BOT CHORD   BOT CHORD     P No.1   BOT CHORD   BOT CHORD   WEBS     2ce)   9=Mechanical, 2=0-5-8   WEBS   WEBS     Horz   2=109(LC 9)   WEBS   WEBS     Uplitf 9=-66(LC 13), 2=-88(LC 12)   Srav 9=1262(LC 1), 2=1308(LC 1)   WEBS    2131/468, 3-5=-1946/512, 5-6=-1816/491, 6-8=-2746/581, 8-9=-893/226   S=-308/1845, 14-16=-100/1222, 12-14=-97/915, 12-13=-394/1830, 11-12=-396/2456, =-396/2456     S=-308/1845, 14-16=-100/1222, 12-14=-97/915, 12-13=-394/1830, 11-12=-396/2456, =-396/2456   S=-308/12456     S=-437/280, 5-16=-139/798, 5-13=-104/718, 13-14=-991/177, 6	IO44-12   IO4-12   IO4-12     10-4-12   10-1-4     [7:0-3-0,Edge], [8:0-1-13,0-2-12], [12:0-0-4,0-2-4]     SPACING-   2-0-0   CSI.   DEFL.   in (loc)     Plate Grip DOL   1.15   TC   0.87   Vert(LL)   -0.21   14-16     Lumber DOL   1.15   BC   0.91   Vert(CT)   -0.36   14-16     Rep Stress Incr   YES   WB   0.60   Horz(CT)   0.24   9     Code IRC2015/TPI2014   Matrix-S   Wind(LL)   0.10   11-12     P No.1   PNo.2   TOP CHORD   Structu   BOT CHORD   Rigid c     P No.2   6-0-0 c   WEBS   1 Row   WEBS   1 Row     2e)   9=Mechanical, 2=0-5-8   -0-10   WEBS   1 Row     2e)   9=Mechanical, 2=0-5-8   -0-2   6-0-0 c   WEBS   1 Row     2e)   9=Mechanical, 2=0-5-8   -0-308/LC 13)   -2131/468, 3-5=-1946/512, 5-6=-1816/491, 6-8=-2746/581, 8-9=-893/226   -308/1445, 14-16=-100/1222, 12-14=-97/915, 12-13=-394/1830, 11-12=-396/2456, =-396/2456   -3	IO4-12   IO4-14   20-6-0   240-0     [7:0-3-0,Edge], [8:0-1-13,0-2-12], [12:0-0-4,0-2-4]   IO-1-4   3-6-0   IO-1-4   3-6-0     SPACING-   2-0-0   CSI.   DEFL.   in (loc)   V/defl   L/d     Plate Grip DOL   1.15   TC   0.87   Vert(LL)   -0.21   14-16   >999   360     Lumber DOL   1.15   BC   0.91   Vert(CT)   -0.36   14-16   >999   240     Rep Stress Incr   YES   WB   0.60   Horz(CT)   0.24   9   n/a   n/a     Code IRC2015/TPI2014   Matrix-S   Wind(LL)   0.10   11-12   >999   240     P No.1   P   Po.2   BRACING-   TOP CHORD   Structural wood sheathing di BOT CHORD   Rigid ceiling directly applied 6-0-0 oc bracing: 9-10.     Yets   29   9=Mechanical, 2=0-5-8    WEBS   1 Row at midpt   5     tray   2=109(LC 9)   Jplift 9=-66(LC 13), 2=-88(LC 12)	104-12 + 200-0 + 24-00 + 24-00 + 36-0 + 10-14 + 36-0

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13, Interior(1) 20-4-13 to 31-5-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 2.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_7_Figure_8.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_7_Picture_10.jpeg)

![](_page_8_Figure_0.jpeg)

I	10-9-3	I	21-2-13	31-6-8	I
	10-9-3		10-5-11	10-3-11	1
Plate Offsets (X,Y)	[8:0-5-8,Edge]				
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	<b>CSI.</b> TC 0.35	<b>DEFL.</b> in (loc) I/de Vert(LL) -0.21 9-11 >99	efl L/d <b>PLATES</b> 99 360 MT20	<b>GRIP</b> 244/190

TCLL   20.0     TCDL   10.0     BCLL   0.0 *     BCDL   10.0	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	TC 0.35 BC 0.49 WB 0.20 Matrix-S	Vert(LL) -0.21 Vert(CT) -0.29 Horz(CT) 0.05 Wind(LL) 0.04	9-11 >999 3 9-11 >999 2 6 8 n/a 1 2-2-11 >999 2	240 MT20 240 n/a 240 Weight: 2	244/190 202 lb FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x6 SF WEBS 2x4 SF WEDGE Right: 2x4 SP No.2	P No.1 P No.1 P No.2		BRACING- TOP CHORD BOT CHORD	Structural wood she Rigid ceiling directly	eathing directly applied or y applied or 10-0-0 oc brad	5-1-8 oc purlins. cing.

REACTIONS. (size) 2=0-5-8, 8=Mechanical

Max Horz 2=109(LC 9) Max Uplift 2=-89(LC 12), 8=-73(LC 13) Max Grav 2=1307(LC 1), 8=1248(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-2114/481, 4-5=-1915/494, 5-6=-1890/507, 6-8=-2081/495

BOT CHORD 2-11=-317/1845, 9-11=-99/1232, 8-9=-309/1760

WEBS 5-9=-118/745, 6-9=-401/273, 5-11=-123/787, 4-11=-434/279

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-10 to 3-8-3, Interior(1) 3-8-3 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13, Interior(1) 20-4-13 to 31-5-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_8_Figure_17.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_8_Picture_19.jpeg)

![](_page_9_Figure_0.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A MiTek Affilia 818 Soundside Road

Edenton, NC 27932

December 8,2022

![](_page_10_Figure_0.jpeg)

![](_page_10_Picture_1.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_10_Picture_3.jpeg)

![](_page_11_Figure_0.jpeg)

TRENCO AMITEK ATTILIATE 818 Soundside Road

Edenton, NC 27932

#### Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Job	Truss	Truss Type	Qty	Ply	Lot 51 Liberty Meadows	
					15563	36918
J0223-0759	B2	Common Girder	1	2		
				<b>_</b>	Job Reference (optional)	
Comtech, Inc, Fay	etteville, NC - 28314,			8.430 s Ja	n 6 2022 MiTek Industries, Inc. Thu Dec 8 07:17:51 2022 Page	e 2
		ID:3	B2lliU9aT	R6OtFvgl	EVAlyq8tk-kvNE?gS2?v5Xx4MIBGa372vWtn7eHq8ZeOsgckyB5g	g_

## LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 7=-1242(F) 9=-1228(F) 10=-1242(F) 11=-1242(F) 12=-1242(F) 14=-1242(F) 16=-1242(F) 17=-1242(F) 18=-1228(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_12_Picture_4.jpeg)

![](_page_13_Figure_0.jpeg)

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-8-12 to 3-8-1, Exterior(2) 3-8-1 to 6-0-0, Corner(3) 6-0-0 to 10-4-13, Exterior(2) 10-4-13 to 12-8-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 14, 11, 10.

10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

![](_page_13_Picture_12.jpeg)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See MSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

![](_page_14_Figure_0.jpeg)

Plate Offsets (X,Y)	[4:Edge,0-2-0]		
LOADING   (psf)     TCLL   20.0     TCDL   10.0     BCLL   0.0   *     BCDL   10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.39 BC 0.20 WB 0.00 Matrix-P	DEFL.   in   (loc)   l/defl   L/d     Vert(LL)   -0.03   2-4   >999   360     Vert(CT)   -0.05   2-4   >999   240     Horz(CT)   -0.00   4   n/a   n/a     Wind(LL)   0.00   2   ****   240   Weight: 20 lb   FT = 20%
LUMBER-			BRACING-

## LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.2 OTHERS 2x4 SP No.2

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=90(LC 8) Max Uplift 2=-94(LC 8), 4=-66(LC 12) Max Grav 2=256(LC 1), 4=180(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 4-9-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_14_Picture_17.jpeg)

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![](_page_14_Picture_19.jpeg)

![](_page_15_Figure_0.jpeg)

Plate Offsets (X,Y)	[4:Edge,0-2-0]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.28 BC 0.20 WB 0.00	DEFL.   in   (loc)   l/defl   L/d   PLATES   GRIP     Vert(LL)   -0.03   2-4   >999   360   MT20   244/190     Vert(CT)   -0.05   2-4   >999   240   MT20   244/190     Horz(CT)   0.00   4   n/a   n/a   Misisht 40 lb   ET   20%
IUMBER-	Code IRC2015/1PI2014	Matrix-P	Wind(LL)   0.00   2   240   Weight: 19 lb   F I = 20%     BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 WEBS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 4=0-1-8

Max Horz 2=63(LC 8) Max Uplift 2=-50(LC 8), 4=-30(LC 12)

Max Grav 2=256(LC 1), 4=180(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 4-9-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_15_Picture_16.jpeg)

Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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![](_page_15_Picture_18.jpeg)

![](_page_16_Figure_0.jpeg)

	5-0-0						
LOADING   (psf)     TCLL   20.0     TCDL   10.0     BCLL   0.0 *     BCDL   10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.17 BC 0.40 WB 0.00 Matrix-P	<b>DEFL.</b> Vert(LL) -( Vert(CT) -( Horz(CT) ( Wind(LL) (	in (loc) 0.02 2-5 0.05 2-5 0.00 0.04 2-5	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 22 lb	<b>GRIP</b> 244/190 FT = 20%

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1 BOT CHORD

WEBS 2x4 SP No.2

REACTIONS. 2=0-5-8, 4=Mechanical (size) Max Horz 2=54(LC 8) Max Uplift 2=-66(LC 8), 4=-60(LC 12)

Max Grav 2=350(LC 1), 4=627(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 3-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the responsibility of others.

## LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
- Vert: 1-3=-60, 2-4=-20 Concentrated Loads (lb) Vert: 8=-600

![](_page_16_Picture_20.jpeg)

Structural wood sheathing directly applied or 4-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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![](_page_16_Picture_22.jpeg)

![](_page_17_Figure_0.jpeg)

	5-0-0					
LOADING   (psf)     TCLL   20.0     TCDL   10.0     BCLL   0.0     BCDL   10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.21 BC 0.10 WB 0.00 Matrix-P	DEFL.   in   (loc)   l/defl   L/d     Vert(LL)   -0.01   2-5   >999   360   MT20   244/190     Vert(CT)   -0.02   2-5   >999   240   MT20   244/190     Horz(CT)   0.00   n/a   n/a   Wind(LL)   0.01   2-5   >999   240			

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.1 2x6 SP No.1 BOT CHORD

WEBS 2x4 SP No.2

REACTIONS. 2=0-5-8, 4=Mechanical (size) Max Horz 2=76(LC 8) Max Uplift 2=-102(LC 8), 4=-27(LC 12) Max Grav 2=255(LC 1), 4=123(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 3-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2 = 102
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_17_Picture_16.jpeg)

Structural wood sheathing directly applied or 4-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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![](_page_18_Figure_0.jpeg)

	5-0-0 5-0-0		<u> </u>					1
LOADING   (psf)     TCLL   20.0     TCDL   10.0     BCLL   0.0     BCDL   10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	<b>CSI.</b> TC 0.27 BC 0.21 WB 0.05 Matrix-S	DEFL. Vert(LL) -0 Vert(CT) -0 Horz(CT) 0 Wind(LL) 0	in (loc) 0.01 2-6 0.03 2-6 0.01 4 0.02 2-6	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 39 lb	<b>GRIP</b> 244/190 FT = 20%
I UMBER-			BRACING-					

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 WEBS OTHERS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=-23(LC 17) Max Uplift 2=-69(LC 8), 4=-69(LC 9) Max Grav 2=450(LC 1), 4=450(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-649/462, 3-4=-649/462 TOP CHORD

BOT CHORD 2-6=-327/557, 4-6=-327/557

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-10-8 to 3-6-5, Exterior(2) 3-6-5 to 5-0-0, Corner(3) 5-0-0 to 9-4-13, Exterior(2) 9-4-13 to 10-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

![](_page_18_Figure_17.jpeg)

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

![](_page_18_Picture_19.jpeg)

![](_page_19_Figure_0.jpeg)

	<u>5-0-0</u> 5-0-0		+		
LOADING   (psf)     TCLL   20.0     TCDL   10.0     BCLL   0.0     BCDL   10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	<b>CSI.</b> TC 0.24 BC 0.21 WB 0.05 Matrix-S	DEFL.   in     Vert(LL)   -0.01     Vert(CT)   -0.03     Horz(CT)   0.01     Wind(LL)   0.01	(loc) l/defl L/d 2-6 >999 360 2-6 >999 240 4 n/a n/a 2-6 >999 240	PLATES   GRIP     MT20   244/190     Weight: 36 lb   FT = 20%

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=-23(LC 17) Max Uplift 2=-69(LC 8), 4=-69(LC 9) Max Grav 2=450(LC 1), 4=450(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-649/241, 3-4=-649/241

BOT CHORD 2-6=-152/557, 4-6=-152/557

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-8 to 3-6-5, Interior(1) 3-6-5 to 5-0-0, Exterior(2) 5-0-0 to 9-4-13, Interior(1) 9-4-13 to 10-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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![](_page_19_Picture_17.jpeg)

SEAL 036322 December 8,2022

![](_page_20_Figure_0.jpeg)

#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 8-0-2.

(lb) - Max Horz 1=80(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-113(LC 12), 6=-113(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7, 8, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=113, 6=113.

![](_page_20_Picture_16.jpeg)

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![](_page_20_Picture_18.jpeg)

![](_page_21_Figure_0.jpeg)

REACTIONS. (size) 1=6-1-12, 3=6-1-12, 4=6-1-12 Max Horz 1=48(LC 9) Max Uplift 1=-19(LC 12), 3=-23(LC 13) Max Grav 1=120(LC 1), 3=120(LC 1), 4=188(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

- and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

![](_page_21_Picture_11.jpeg)

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![](_page_21_Picture_13.jpeg)

![](_page_22_Figure_0.jpeg)