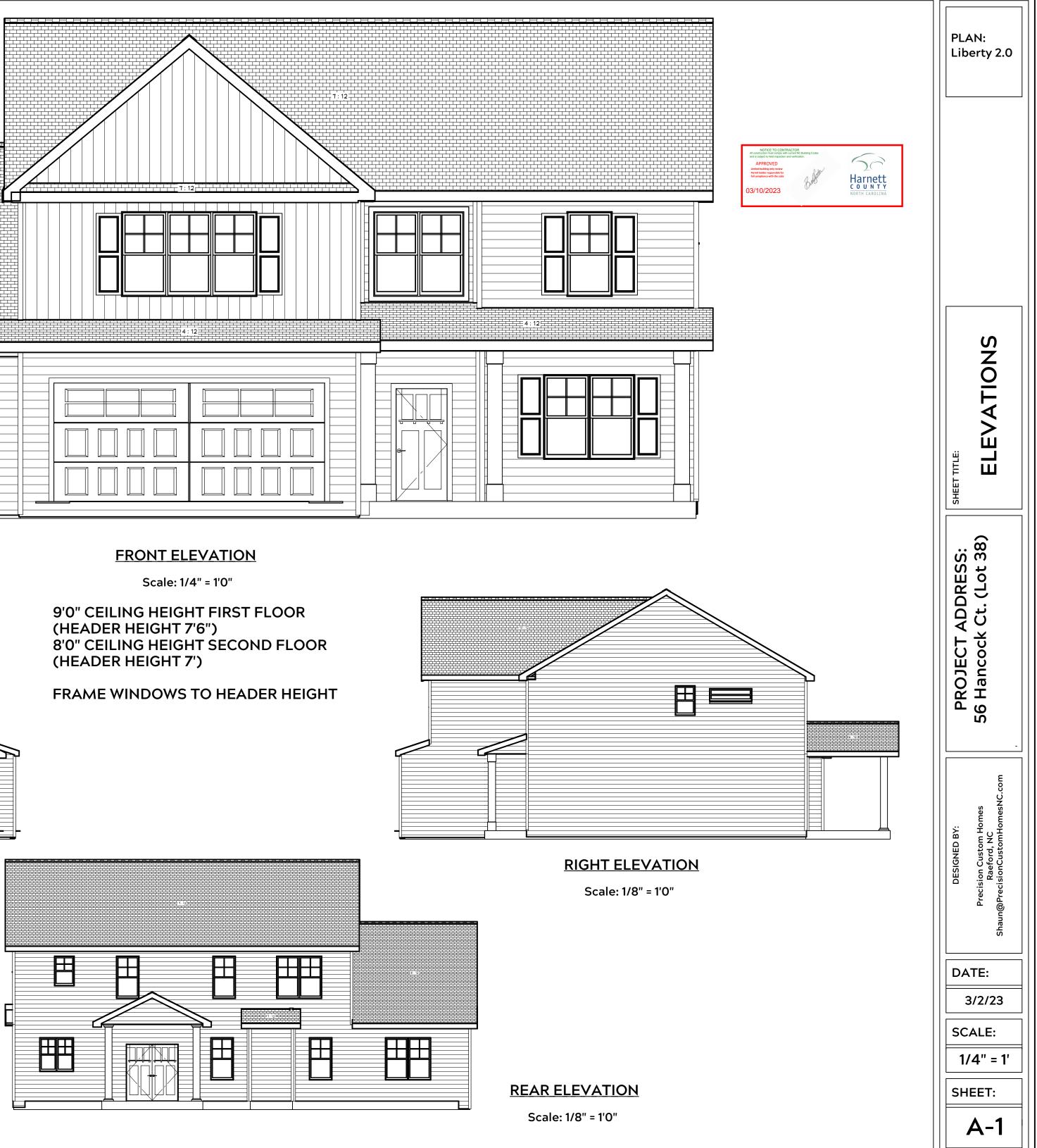
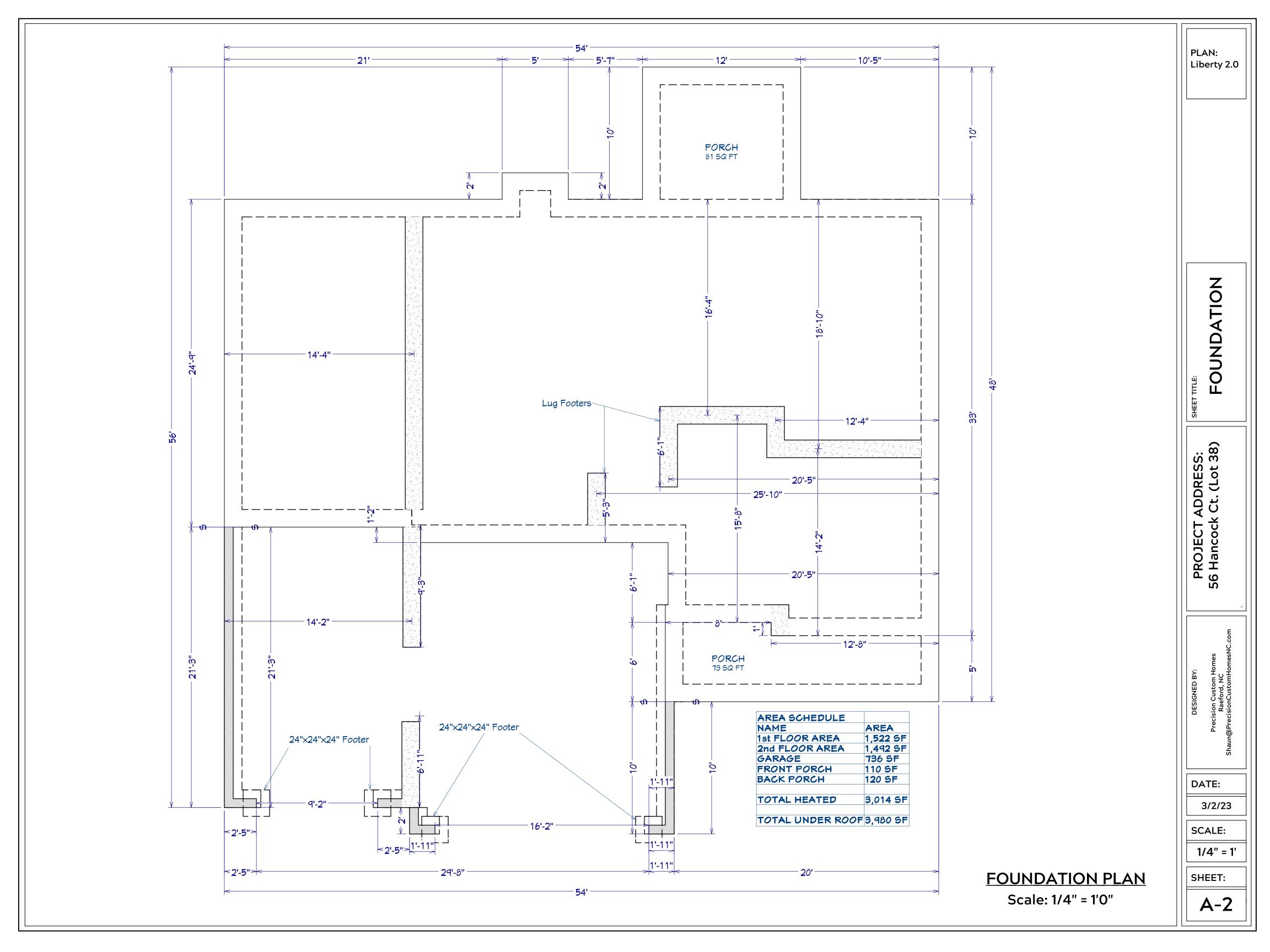
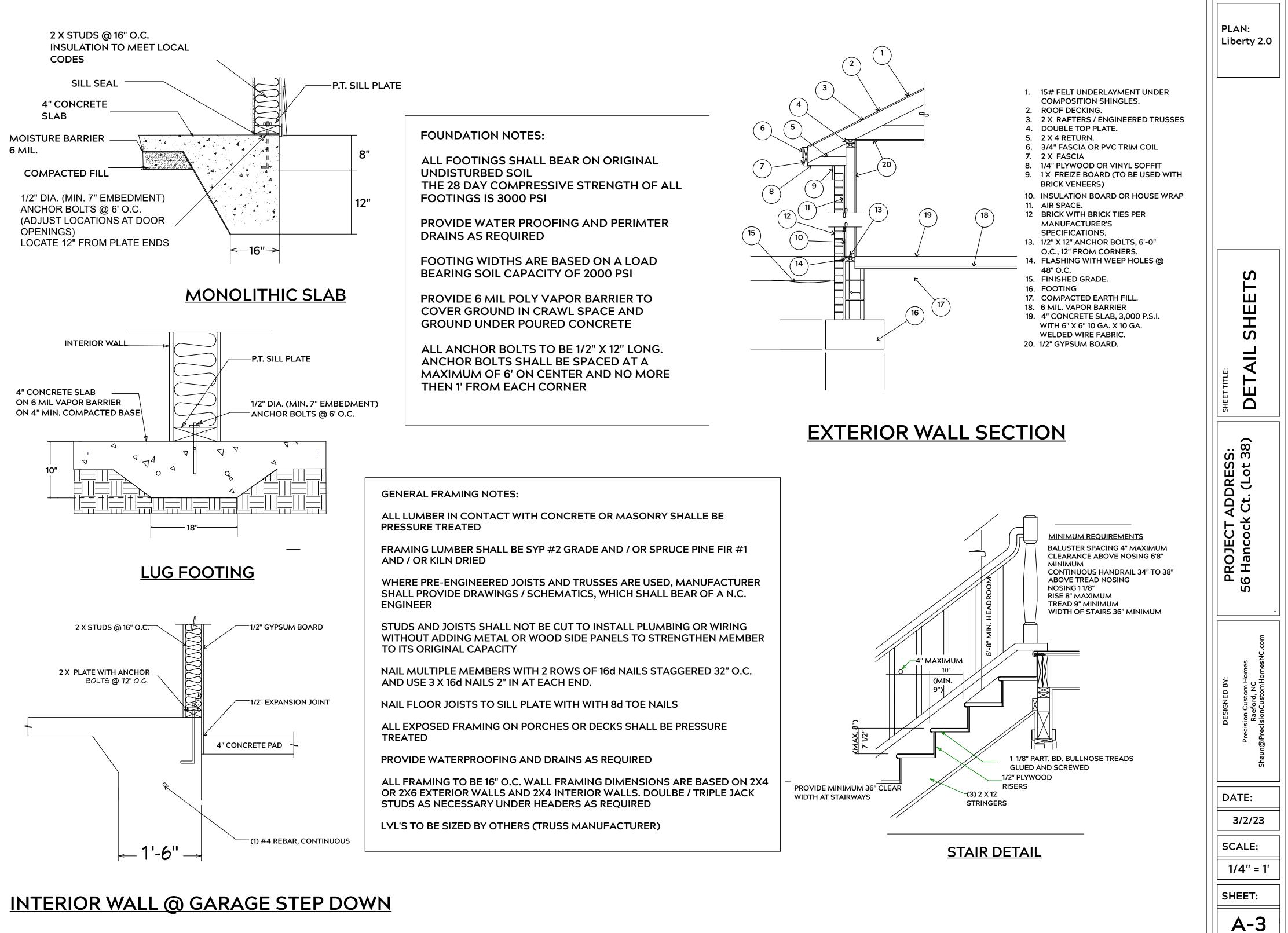


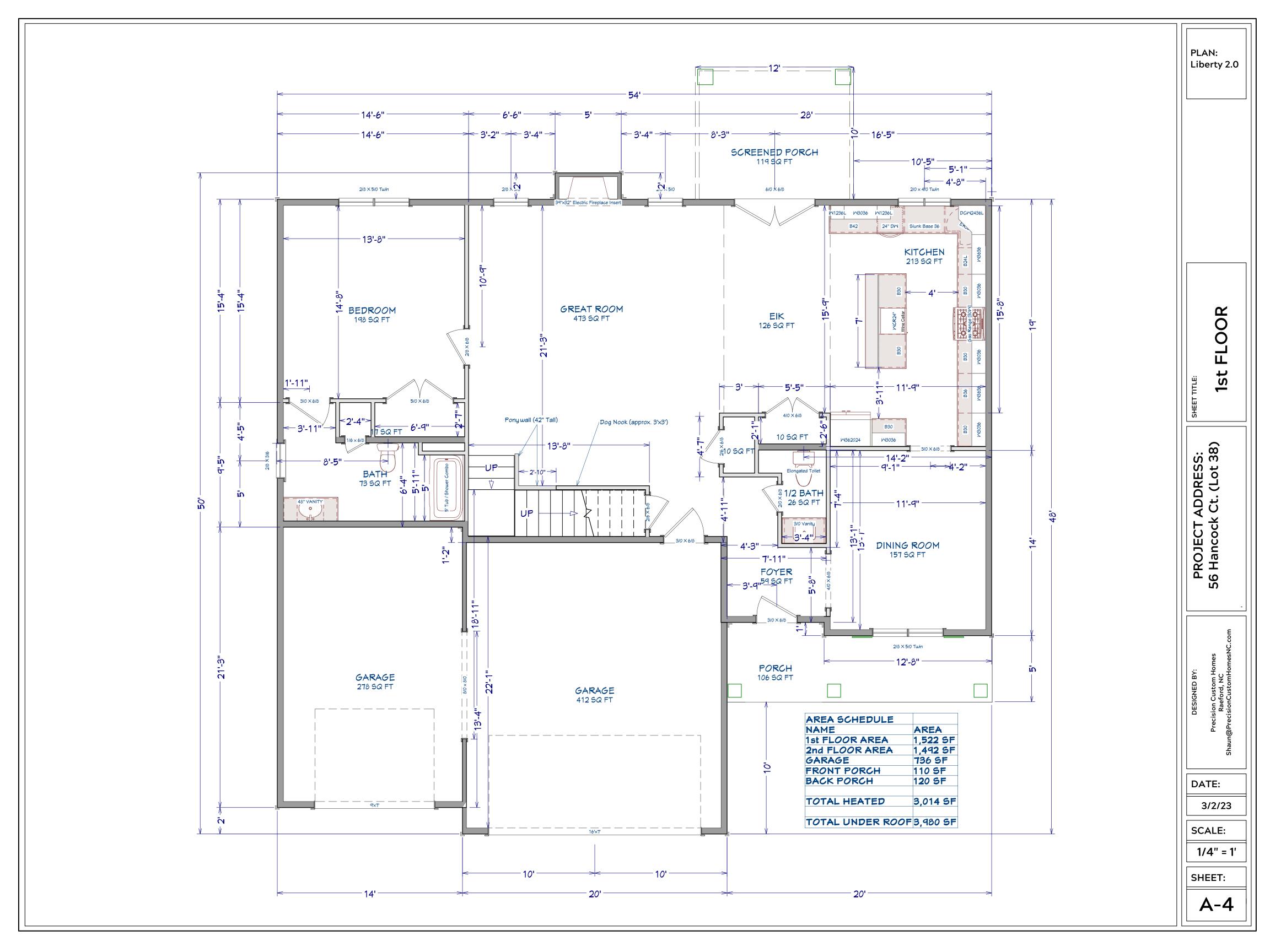
# LEFT ELEVATION

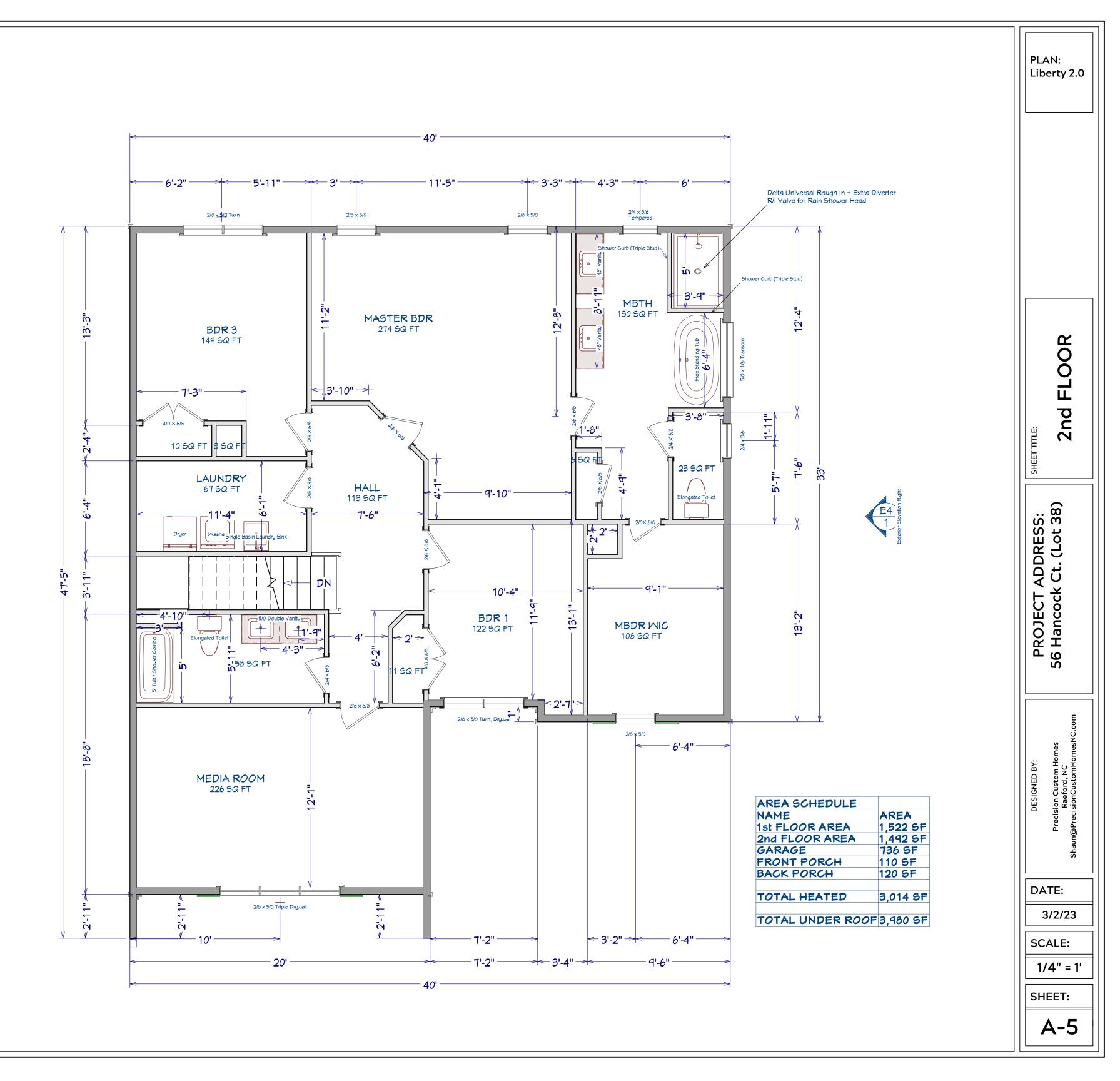
Scale: 1/8" = 1'0"

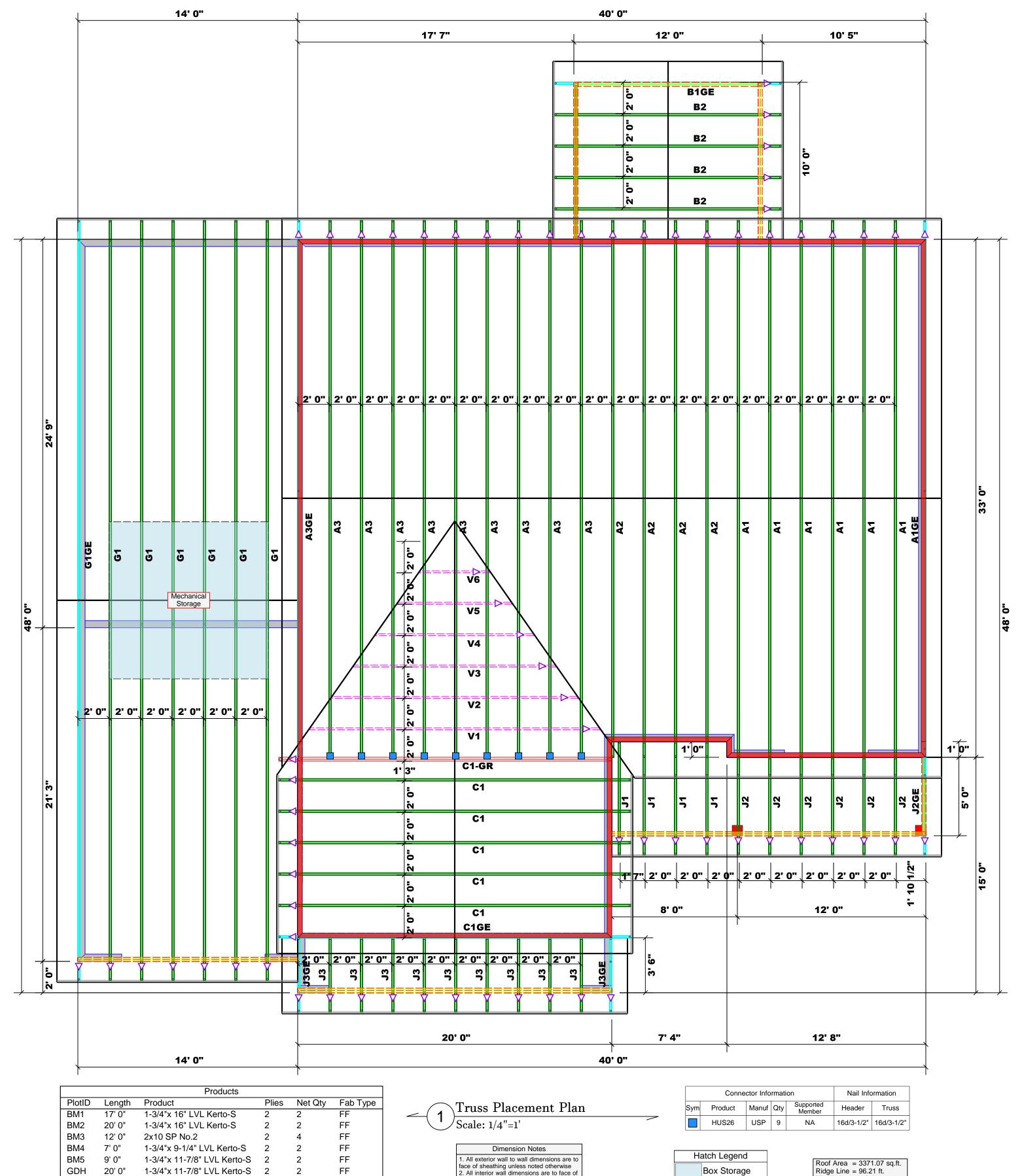












	-		-			
GDH2	14' 0"	2x12 SPF No.2	2	2	FF	
GDH	20' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF	
BM5	9' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF	
BM4	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF	
BM3	12' 0"	2x10 SP No.2	2	4	FF	
BM2	20' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF	
BM1	17' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF	

All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
 All interior wall dimensions are to face of frame wall unless noted otherwise
 All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

Hatch Legend	
Box Storage	Roof Area = 3371.07 sq.f Ridge Line = 96.21 ft.
Drop Beam	Hip Line = 0 ft. Horiz. OH = $183.58$ ft. Raked OH = $213.39$ ft.
2nd Floor Walls	Decking = 116 sheets

A = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do NOT Erect Truss Backwards

(BASE	ART FOR JAC	& (b))	BUILDER	Precision Custom Homes and Renovations	COUNTY	Cameron / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
O) DIS FOR TADER	NUMBER OF JACK STUDS REQUIDED @ EA END OF HEADER/GIRDER 80 20 20 20 20 20 20 20 20 20 20 20 20 20		JOB NAME	Lot 38 Liberty Meadow	ADDRESS	Lot 38 Liberty Meadow	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
END REAC (UP T (UP T) (2) PLY H	END REAC (UP Ti (UP Ti (UP Ti (3) PLY H	END REAC (UP T REQ'D STL (4) PLY H	PLAN	Liberty 2.0 w/ CP	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	<b>ROOF &amp; FLOOR</b>
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400168002102003	SEAL DATE	N/A	DATE REV.	03/01/23	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	<b>TRUSSES &amp; BEAMS</b> Reilly Road Industrial Park
6800 4 8500 5 10200 6	102004127505153006	13600 4 17000 5	QUOTE #		DRAWN BY	David Landry	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
11900 7 13600 8 15300 9			JOB #	J0922-4865	SALESMAN	Neil Baggett	Signature David Landry	Fax: (910) 864-4444

																	DA	ATE 03/01	/23 PAGE 1
Rea	ction	Sun	nma	ry of	Orde	r	R	EQ. C	τους	E DATE	E /	//			ORD	ER #			922-4865
				-					R DA			9/23/	/22			TE #			
							D	ELIV	ERY I	DATE	/				CUS	ТОМЕ	R ACCT	# 00	00007216
	-			& FLO			D	ATE	OF IN	VOICE	/				CUS	TOME	R PO #		
ူင၀	mTec	≿h∥ 1	TRUSSE	ES & BE	AMS		C	RDE	RED	ВΥ	S	haur	n Garderner		INVC	ICE #			
Reilly F	Road Indu	strial P	ark P.C	D. Box 40	0408		С	OUN	ΤY		H	larne	tt		TER	NS			
,	eville, N.C						S	UPEF	RINTE	NDANT	r s	haur	n Garderner		SAL	ES RE	Р	Ne	il Baggett
							J	OBSI	TE Pł	HONE #	: (9	910) 9	988-8172		SALI	ES AR	EA	Da	vid Landry
	recisio	n Cu	stom	Home	•		ME:Lot	3811	bertv	Meadow	v	,		10	<b>T #</b> 38	SUE	BDIV: Lib	ertv Me	wobe
s o	56 Bria			nome					borty		Liberty				B CATEG				
D						-				TAG	Liberty	y 2.0	W/ CF	10	B CATEG		_		
	aeford,			i	-		ound trip	UCIIC	JNS:										
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Ϊ Ρ L	ot 38 Li	bertv	Mead	wob	5	SPECIAL	INSTRU	CTION	NS:										
	ameror	-																	
ĽĽ	ameror	I, NC	2033	Ū													PLAN	SEAL D	
																		BY	
	DING DE	PART	MENT		HANG IN		EL HEIGI	HT	00-0	06-08	REQ.	LAY	OUTS	REQ. ENG	INEERING			DTL	03/01/23
Roof C	Jrder			END CU	T RETU		BLE STU	פחו	241	N. OC			DBSITE 1		JOBSITE	_	LAYOUT CUTTING	DTL DTL	03/01/23
									-			J	DBSITE 1		JOR211F		CUTTING		03/01/23
RO	OF TI	RUS	SES	•	)ADING FORMAT		CLL-TCDL-B		-	RESS INCR.		ROC	OF TRUSS S	SPACING:	24.0 IN. (	D.C. (T	YP.)		
		QTY	гIQ	СН	TYPE		0.0,10.0,0 BASE		0 IBER	1.15	DUAN	6							
PR	OFILE	PLY	TOP	вот	ID	-	O/A		BOT	LEFT	RHANG		REACTIC	ONS					
					COMM	ON :	33-00-00						Joint 2	Joint 8					
		6	7.00	0.00	A1	-	33-00-00	2 X 6	2 X 6	01-02-08	8 01-02	2-08	1525.2 lbs.	1525.2	lbs.				
													-90.9 lbs.	-90.9 lk	os.				
												Ī							
					GABL		33-00-00		244	01.00.0			Joint 2	Joint 20		nt 22	Joint		Joint 24
		1	7.00	0.00	A1GE	-  :	33-00-00	2 X 6	2 X 6	01-02-08	01-02 v	2-08	198.3 lbs.	172.7		09.1 lbs		4.5 lbs.	175.8 lbs.
	1												-61.9 lbs.	-4.6 lb	s	93.2 lbs	75	5.0 lbs.	-77.8 lbs.
					COMM		33-00-00						Joint 2	Joint 3	le:	nt 4	Joint	8	
/	$\wedge$	4	7.00	0.00	A2	-	33-00-00	2 X 6	2 X 6	01-02-08	8 01-02	2-08	1315.9 lbs.	150.9 l		37.0 lbs		25.2 lbs.	
		.											-17.7 lbs.	-112.7		26.1 lbs		).9 lbs.	
					COMM		33-00-00						Joint 2	Joint 8					
	$\mathbb{Z}$	9	7.00	0.00	A3	:	33-00-00	2 X 6	2 X 6	01-02-08	8		1529.4 lbs.	1461.3					
													-91.0 lbs.	-74.7 lk	os.				
					<u> </u>	_	00.00.00						laint O	1		-1.01			laist 00
		1	7.00	0.00	GABL A3GE		33-00-00 33-00-00	2 X 6	2 X 6	01-02-09	8		Joint 2	Joint 20		nt 21 20 5 lbc	Joint		Joint 23
		· '	1.00	0.00	7.001	-  `			2.00	02-00	~		197.6 lbs. -59.9 lbs.	117.2 l -2.3 lb		30.5 lbs 07.2 lbs		8.3 lbs. I.1 lbs.	177.0 lbs. -78.5 lbs.
													00.0 100.	-2.0 10	J1	51.2 IUS	51		70.0 103.
					GABL	.e   ·	12-00-00						Joint 2	Joint 8					
		1	6.00	0.00	B1GE		12-00-00	2 X 4	2 X 6	01-02-08	8 01-02	2-08	550.0 lbs.	550.01	bs.				
													-142.9 lbs.	-142.9	bs.				
_					COMM		12-00-00		0.4.5	04.00.5			Joint 2	Joint 4					
		4	6.00	0.00	B2	'	12-00-00	2 X 4	2 X 6	01-02-08	01-02 v	2-08	550.0 lbs.	550.01					
	1												-108.9 lbs.	-108.9	DS.				
					COM 48.4		20-00 00						loint 2	loint 4					
/	$\wedge$	5	10.00	0.00	COMM C1		20-00-00 20-00-00	2 X 6	2 X 6	01-02-08	8 01-02	2-08	Joint 2 1043.7 lbs.	Joint 4 1043.7	lbs				
	⊥_>	J J	. 0.00	0.00									-48.5 lbs.	-48.5 lt					
		1			COMM	ION 2	20-00-00						Joint 2	Joint 6					
		2 Ply	10.00	0.00	C1-G		20-00-00	2 X 6	2 X 8	01-02-08	8		6838.3 lbs.	6844.2	lbs.				
<i>a</i>													-436.9 lbs.	-424.1 l	bs.				
		Ī																	
	_				GABL		20-00-00		0.4.5	04.00.5			Joint 2	Joint 12					
	11111	1	10.00	0.00	C1G	-  2	20-00-00	2 X 6	2 X 6	01-02-08	o  01-02	2-08	859.7 lbs.	859.71					
													-169.5 lbs.	-169.5	DS.				
					COMM		46-00-00						Joint 2	Joint 12	lei	nt 18			
	∽,	6	6.00	0.00	G1		46-00-00	2 X 6	2 X 8	01-02-08	8 01-02	2-08	1250.3 lbs.	1365.2		147.0 lb	s.		
													-65.6 lbs.	-168.3		19.5 lbs			
									I	I	1				-				

																	D	ATE 03/0 <sup>2</sup>	1/23 PAGE 2
Re	action	Sun	nma	ry of	Orde	ər	Γ	REQ.	QUOT	E DATE	/	/			ORE	DER #		JO	922-4865
							Ī	ORDE	R DA	TE	09/	/23/22			QUC	DTE #			
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	. т.			& FLC				DATE	OF IN	IVOICE	/	/			CUS	томе	ER PO #		
	omTec	sn ji i	TRUSSE	S & BE	AMS		+	ORDE		BY	Sha	aun Gard	ernei	r	INV	OICE #	ŧ		
	y Road Indu							COUN				rnett			TER	MS			
Faye	etteville, N.C	. 283	09 (91	0) 864-	TRUS		Ļ	SUPE	RINTE	ENDANT	· Sha	aun Gard	erner	r	SAL	ES RE	P	Ne	eil Baggett
								JOBS	ITE PI	HONE #	(91	0) 988-87	172		SAL	ES AR	REA	Da	avid Landry
	Precisio	n Cu	stom	Home	s	JOB N	NAME: Lo	ot 38 L	iberty	Meadov	1			L	<b>)T #</b> 38	SU	BDIV: Lik	perty Me	adow
SOLD	256 Bria	r Hill	Rd.			MODE	EL:Roof			TAG	Liberty 2	2.0 w/ CF	<b>)</b>	JC	B CATE	GORY:			
	Raeford,				ŀ	DELIV	ERY INST	RUCTI	ONS:										
Ť	(910) 988					52 miles	s round tri	р											
	. ,																		
7	Precisio				s and	SPECI	AL INSTR	UCTIO	NS:										
	Lot 38 Li	-																	
то	Cameror	n, NC	2835	6														N SEAL I	
																		BY	
BUI	LDING DE	PART	MENT	OVER	HANG I	NFO H	IEEL HEI	GHT	00-0	06-08	REQ. L	AYOUTS		REQ. EN	GINEERING	G	QUOTE	DTL	03/01/23
Roo	of Order			END CU	JT RET	URN											LAYOUT		03/01/23
						G	BABLE ST	UDS	24	IN. OC		JOBSITE	1		JOBSITE	1	CUTTING	DTL	03/01/23
R	OOF T	RUS	SES	LC	DADING	;	TCLL-TCDL-	BCLL-BC	DL STF	RESS INCR.			221	SPACING	·24 0 IN				
				IN			20.0,10.0			1.15			555		IN. '	5.5. (	)		
P	ROFILE			СН	TYP		BASE O/A		<b>MBER</b>		RHANG		ACTI	ONS					
		PLY	TOP	BOT			46-00-00	_	BOT	LEFT	RIGH	T Joint 2	-	Joint 26		int 28	loir	nt 29	Joint 30
		1	6.00	0.00		-		-	2 X 6	01-02-08	3 01-02-0		2 2 lbs.			ani ∠o 219.1 lb		39.7 lbs.	165.1 lbs.
													2 lbs.	-3.2		-94.9 lbs		62.0 lbs.	-70.6 lbs.
					MONO 1			-				Joint 2	2	Joint 4					
-		4	4.00	0.00	J1	1	06-00-00	) 2 X 4	2 X 6	01-02-08	3 -00-01-	0.0.	0 lbs.	215.2					
					1							-132.	3 lbs.	-90.3	lbs.				
					MONO	חשבום	05-00-0					Joint 2	,	Joint 4					
		6	4.00	0.00		-		-	2 X 6	01-02-08	3 -00-01-		3 lbs.	174.4	lbs				
-		-											5 lbs.						
												1							
					MONO		05-00-0					Joint 2	2	Joint 5	Jo	int 6	Joir	nt 7	
		1	4.00	0.00	J20	GE	05-00-0	2 X 4	2 X 6	01-02-08	3		3 lbs.	3.5		52.4 lbs		22.4 lbs.	
												-89.3	3 lbs.	-3.4	bs	-34.0 lbs	s7	70.5 lbs.	
					MONO	חשבום	02.04.0					loint (	,	loint 4					
		9	4.00	0.00			03-04-0		2 X 6	01-02-08	3	Joint 2	2 8 lbs.	Joint 4 107.4	lbs				
6		-		2.00									8 lbs.	-14.2					
													-						
					MONO		03-06-0					Joint 2	2	Joint 7	Jo	int 8			
6		2	4.00	0.00	J30	GE	03-06-0	2 X 4	2 X 4	01-02-08	3		5 lbs.	52.0		125.2 lb			
												-91.	1 lbs.	-26.0	lbs	-37.7 lbs	s.		
					\/^! !		18 05 0	_				loint 4	1	loint F	1-	int 6	le:-	ot 8	loint 0
		1	10.00	0.00	VALI V		18-05-0 18-05-0		2 X 4			Joint 1	5 lbs.	Joint 5 182.2		int 6 560.1 lb		nt 8 10.9 lbs.	Joint 9 560.3 lbs.
4				2.00									lbs.	22.4		171.8 lb		55.3 lbs.	-171.9 lbs.
								1										-	
					VALI		15-07-1					Joint 1		Joint 5	Jo	int 6	Joir	nt 7	Joint 8
		1	10.00	0.00	V	2	15-07-1	1   2 X 4	2 X 4				3 lbs.	134.9		428.5 lb		10.8 lbs.	428.9 lbs.
									<u> </u>			-16.8	8 lbs.	10.7	lbs	142.4 lb	os. 5	56.2 lbs.	-142.5 lbs.
					1/11		12 40 0					laint d	1	laint F	1-	int C		ot 7	loint 9
		1	10.00	0.00	VALI V:		12-10-02 12-10-02		2 X 4			Joint 1	5 lbs.	Joint 5 84.7		int 6 329.9 lb		nt 7 42.2 lbs.	Joint 8 330.1 lbs.
4		.		0.00									1 lbs.	-8.6		123.9 lb		42.2 lbs. 54.2 lbs.	-124.1 lbs.
					VALI		10-00-08					Joint 1		Joint 3	Jo	int 4			
4		1	10.00	0.00	V	4	10-00-08	3   2 X 4	2 X 4				9 lbs.	198.9		346.9 lb			
	-								<u> </u>			-21.8	8 lbs.	-30.4	lbs.	10.9 lbs	5.		
							07.00	_											
		1	10.00	0.00	VALI V		07-02-1		2 X 4			Joint 1	6 lbs.	Joint 3 150.7		int 4 219.7 lb	s		
4		'	.0.00	0.00		-							6 IDS. 7 Ibs.	-28.7		219.7 lb 22.5 lbs			
							1		1	1				_0.1			-		

_			_		-								DA	TE 03/01/23	PAGE 3
Re	eaction Summary		REQ. Q	UOTE DAT	E	/ /				ORDER	#	J0922	-4865		
	$\wedge$				ORDER	DATE	0	9/23/22				QUOTE	#		
					DELIVE	RY DATE						CUSTO	MER ACCT	¥ 00000	07216
	ROOF &				DATE O	F INVOICE	Ξ.					CUSTO	MER PO #		
¶C	ComTech    TRUSSES &	& BEAN	٨S		ORDER	ED BY	S	haun Garde	rner			INVOICE	Ξ#		
Reil	ly Road Industrial Park P.O. B	Box 4040	08		COUNT	Y	Н	arnett				TERMS			
	etteville, N.C. 28309 (910)				SUPER	INTENDAN	IT S	haun Garde	rner			SALES	REP	Neil B	aggett
					JOBSIT	E PHONE	# (9	910) 988-817	72			SALES	AREA	David	Landry
	Precision Custom Ho	omes	JOE	B NAME:	ot 38 Lib	erty Meado	W				LOT	<b>T #</b> 38 <b>S</b>	SUBDIV: Libe	erty Meado	w
S O L D	256 Briar Hill Rd.		МО	DEL:Root	:	TAC	G: Libert	y 2.0 w/ CP			JOB	B CATEGOR	Y: _		
D T O	Raeford, NC 28376 (910) 988-8172			IVERY INS iles round t		NS:									
s	Precision Custom Ho	mes a	nd												
SHIP	Lot 38 Liberty Meadow		SPE	CIAL INST	RUCTIONS	S:									
ЧО	Cameron, NC 28356												PLAN	SEAL DAT	E:
														BY	DATE
BU	ILDING DEPARTMENT O	VERHA	NG INFO	HEEL HE	IGHT	00-06-08	REQ	LAYOUTS		REQ.	ENGI	NEERING	QUOTE	DTL	03/01/23
Roo	of Order EN	ND CUT	RETURN										LAYOUT	DTL	03/01/23

Roof Order			END CU	T RETURN												LAYOUT	DTL	03/01/23
					GABLE STU	DS	24 II	N. OC			JOBSITE	1		JOBSITE	1	CUTTING	DTL	03/01/23
ROOF T	RUS	SES		ADING FORMATION	тсц-тср-во 20.0,10.0,0			ESS INCR. 1.15		R	OOF TRU	ss s	SPACING	<b>3:</b> 24.0 IN. C	.C.	(TYP.)		
PROFILE	QTY	PIT	ГСН	TYPE	BASE	LUM	BER	OVE	RHA	NG	REA	OTIC						
	PLY	TOP	BOT	ID	O/A	TOP	BOT	LEFT	R	IGHT	KEA		SN2					
				VALLEY	04-05-05						Joint 1		Joint 3	Joir	t 4	Joint	17	Joint 18
	1	10.00	0.00	V6	04-05-05	2 X 4	2 X 4				85.8	bs.	85.8	lbs. 12	25.2	bs. 304	.2 lbs.	337.3 lbs.
											-12.9	bs.	-16.3	Blbs. 1	2.8	os31.	6 lbs.	-27.3 lbs.

# ITEMS

QTY	ITEM TYPE	SIZE	<b>LENGTH</b> FT-IN-16	PART NUMBER	NOTES	
9	Hangers, USP	HUS 26			SIMPSON (HUS26)	-



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0922-4865 Lot 38 Liberty Meadow

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I56917876 thru I56917898

My license renewal date for the state of North Carolina is December 31, 2023.

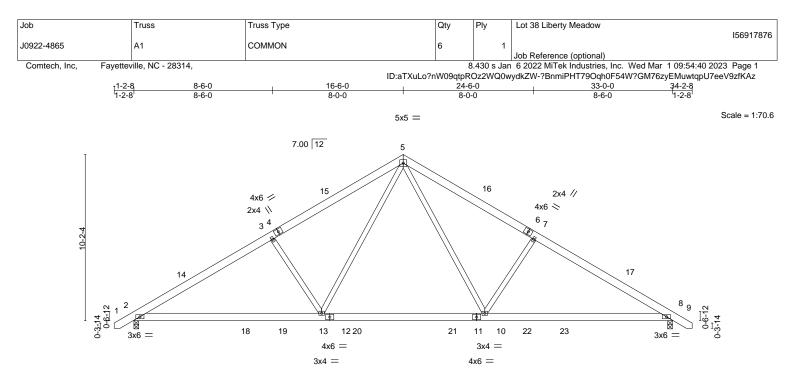
North Carolina COA: C-0844



March 1,2023

## Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



	<u> </u>		21-6-0 10-0-0	33-0-0 11-6-0	I	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.30 BC 0.53 WB 0.31 Matrix-S	DEFL.         in           Vert(LL)         -0.15         10           Vert(CT)         -0.26         4           Horz(CT)         0.05         10           Wind(LL)         0.05         10	8-10 >999 240 8 n/a n/a	<b>PLATES</b> MT20 Weight: 221 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=245(LC 11) Max Uplift 2=-91(LC 12), 8=-91(LC 13) Max Grav 2=1525(LC 19), 8=1525(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2295/423, 3-5=-2090/464, 5-7=-2091/464, 7-8=-2296/423

BOT CHORD 2-13=-222/2070, 10-13=-9/1347, 8-10=-233/1886

WEBS 3-13=-544/300, 5-13=-140/991, 5-10=-139/991, 7-10=-544/300

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-6 to 3-4-7, Interior(1) 3-4-7 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 34-0-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 2 and 91 lb uplift at joint 8.

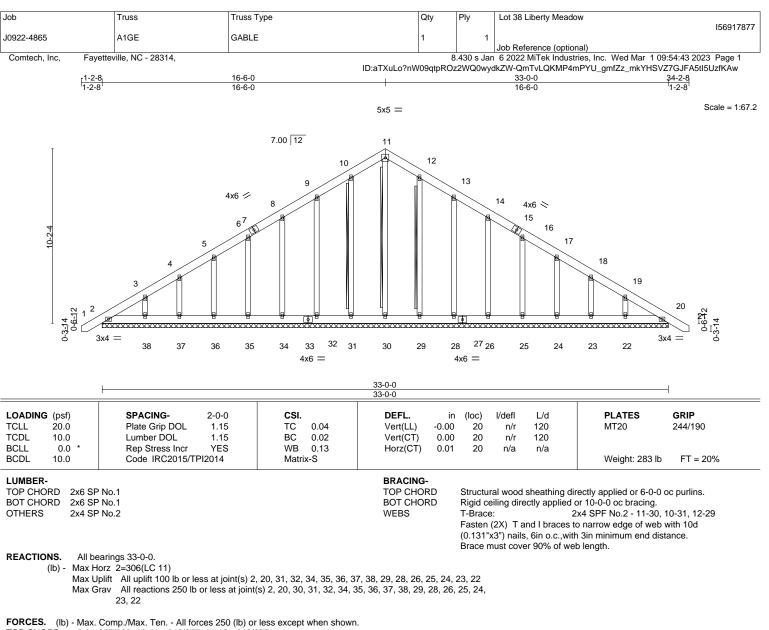


Structural wood sheathing directly applied or 4-11-2 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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TOP CHORD 2-3=-277/226, 10-11=-242/277, 11-12=-242/277

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 31, 32, 34, 35, 36, 37, 38, 29, 28, 26, 25, 24, 23, 22.

10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



ERGINEERING BY AMITEK Affiliate 818 Soundside Road

Edenton, NC 27932

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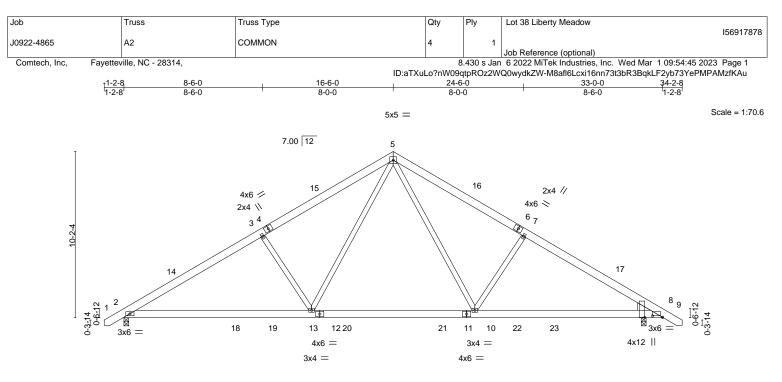


Plate Offsets (X,Y)	<u>  11-6-0</u> <u>  11-6-0</u> [8:0-7-6,0-1-1], [8:0-0-2,1-1-1]	1	<u>21-6-0</u> 10-0-0	2-0-0 <u>3</u> 3-0-0 0-6-0 <u>1-0-0</u>	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.65 BC 0.60 WB 0.31 Matrix-S	Vert(LL) -0.15 Vert(CT) -0.26 Horz(CT) 0.05	) MT20	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP WEDGE Right: 2x6 SP No.1	No.1		BRACING- TOP CHORD BOT CHORD	hing directly applied or 4-11-2 pplied or 10-0-0 oc bracing.	2 oc purlins.
Max U	e) 2=0-3-8, 8=0-3-8 orz 2=245(LC 11) plift 2=-91(LC 12), 8=-91(LC 13) rav 2=1525(LC 19), 8=1525(LC 20)				

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2295/423, 3-5=-2090/464, 5-7=-2091/464, 7-8=-2296/423

BOT CHORD 2-13=-222/2070, 10-13=-9/1347, 8-10=-233/1886

WEBS 3-13=-544/300, 5-13=-140/991, 5-10=-139/991, 7-10=-544/300

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-6 to 3-4-7, Interior(1) 3-4-7 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 34-0-6 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

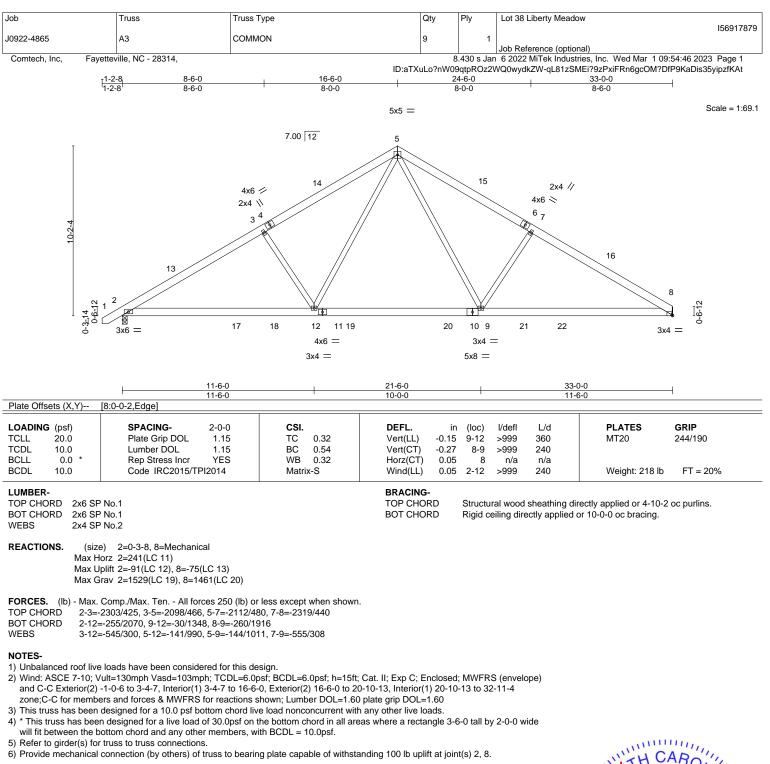
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



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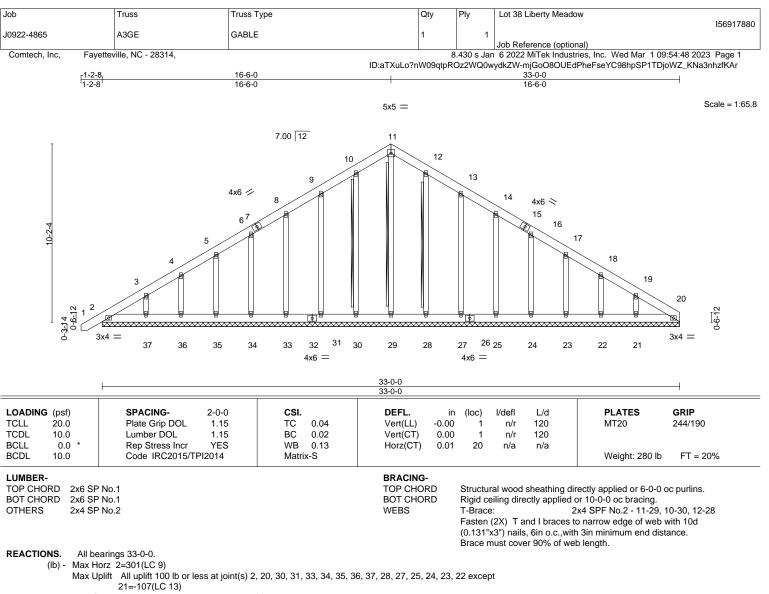
818 Soundside Road Edenton, NC 27932





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- Max Grav All reactions 250 lb or less at joint(s) 2, 20, 29, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22, 21
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-280/222, 10-11=-236/265, 11-12=-236/265

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

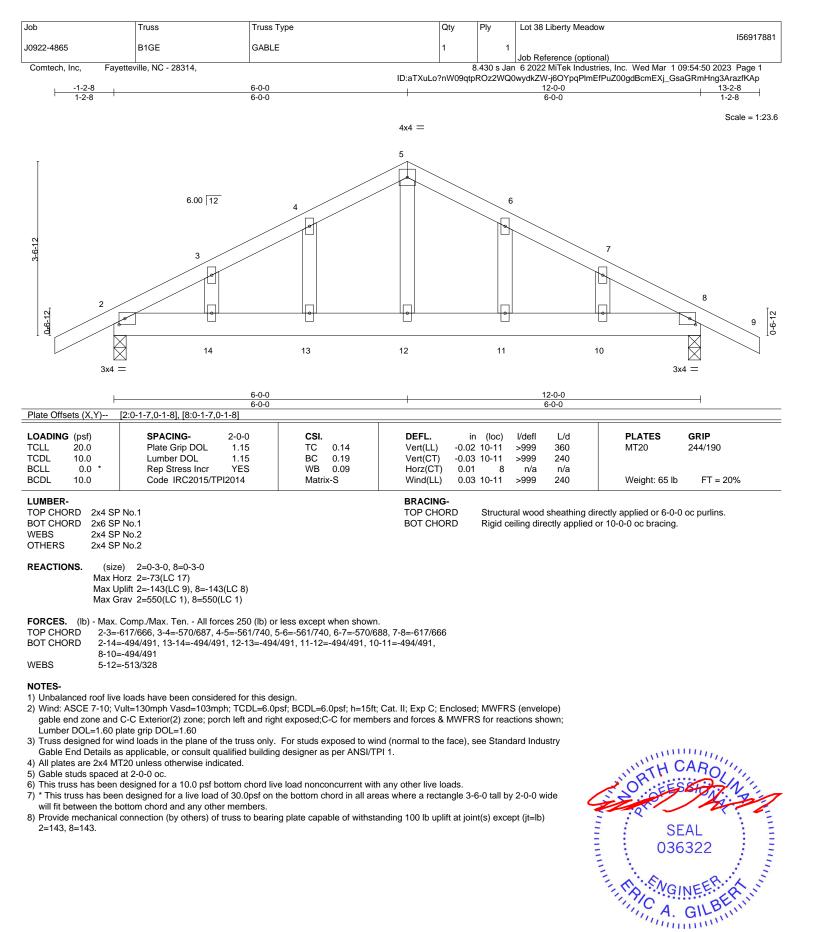
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 30, 31, 33, 34, 35, 36, 37, 28, 27, 25, 24, 23, 22 except (jt=lb) 21=107.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



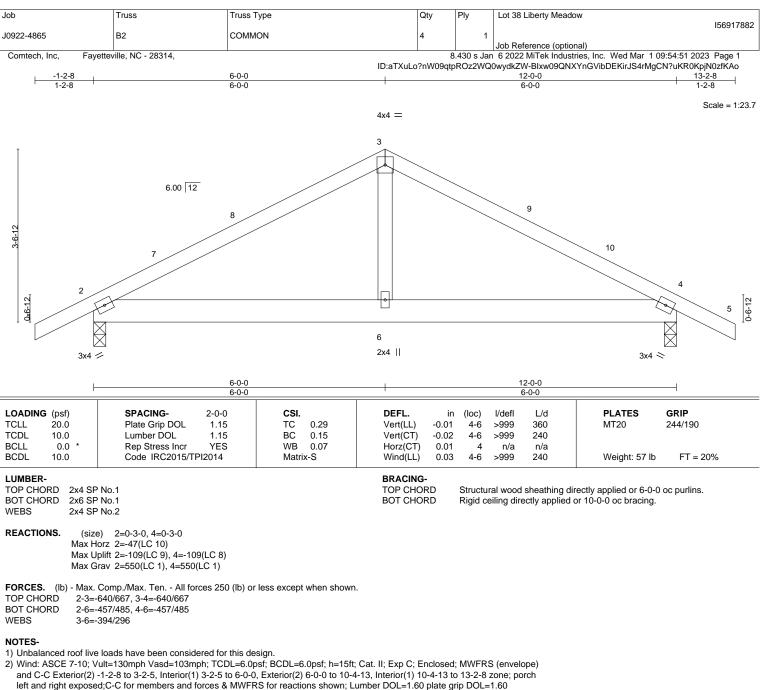


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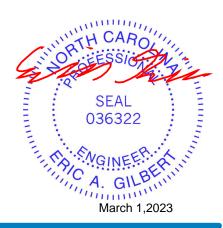




3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

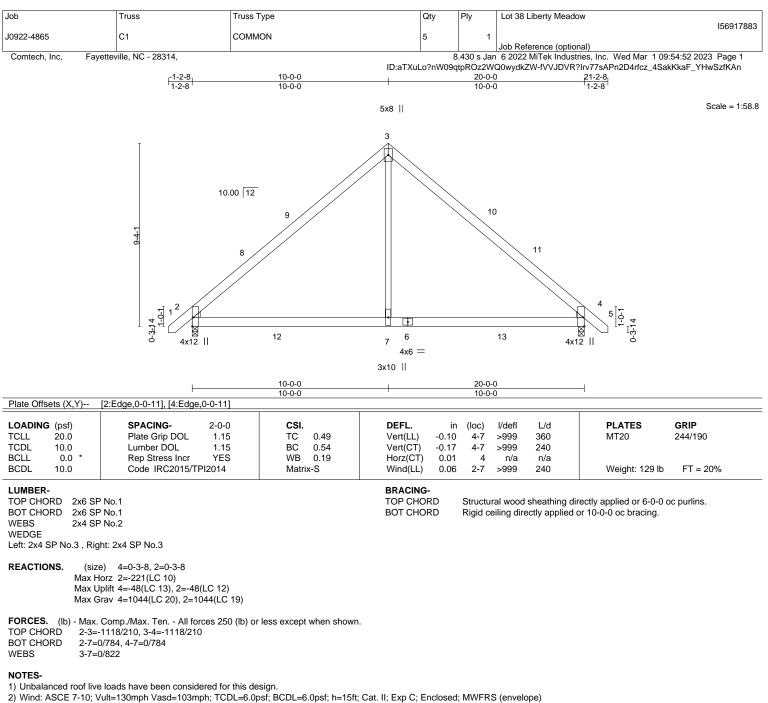
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=109. 4=109



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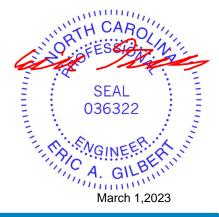


2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-8 to 3-4-4, Interior(1) 3-4-4 to 10-0-0, Exterior(2) 10-0-0 to 14-4-13, Interior(1) 14-4-13 to 21-0-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.



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818 Soundside Road Edenton, NC 27932

	Truss	Truss Type	Qty	Ply	Lot 38 Liberty Meadow		156917884
0922-4865	C1-GR	COMMON GIRDER	1	2	Job Reference (optional	)	
Comtech, Inc, Fay	retteville, NC - 28314,			.430 s Jan	6 2022 MiTek Industrie	s, Inc. Wed Mar 1	
	<mark>-1-2-8</mark> 1-2-8	5-0-0 10-0-0 5-0-0 5-0-0	15-0	-0	20-0-0 5-0-0		qqg20/000000002110 (j
	. 20			•			Scale = 1:58.
			5x8				
	F78	10.00 12 4x8 <del>//</del> 3			4x8 ≈ 5	6 1-0-1	
	5x8 :	$= \begin{array}{c} & & \\ 11 & 12 & 13 & 14 \\ & & \\ 4x12 & \\ 1 \end{array}$	9 8 15 6x8 =		7 <sup>17 18</sup> 5, (12		
			10x10 =				
	F	5-0-0         10-0-0           5-0-0         5-0-0	15-0 5-0-		<u>20-0-0</u> 5-0-0		
Plate Offsets (X,Y)	[2:0-0-0,0-0-9], [6:Edge,0-0-9]	, [9:0-5-0,0-6-4]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.	0-0 <b>CSI.</b> 15 TC 0.61 15 BC 0.45 NO WB 0.85	DEFL. ir Vert(LL) -0.07 Vert(CT) -0.13 Horz(CT) 0.03	9-10 × 9-10 ×	l/defl L/d >999 360 >999 240 n/a n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2015/TPI201		. ,		>999 240	Weight: 341	lb FT = 20%
WEBS 2x4 SF REACTIONS. (siz Max H Max L Max G	2 2400F 2.0E 2 No.2 e) 6=0-3-8, 2=0-3-8 lorz 2=217(LC 26) lplift 6=-424(LC 9), 2=-437(LC irav 6=6844(LC 2), 2=6838(LC	2)	BRACING- TOP CHORD BOT CHORD		I wood sheathing direc ling directly applied or		
TOP CHORD 2-3= BOT CHORD 2-10	-8399/550, 3-4=-5693/462, 4-5 =-410/6080, 9-10=-410/6081, 7		4/3461				
Top chords connect Bottom chords conn Webs connected as 2) All loads are consid ply connections hav 3) Unbalanced roof live 4) Wind: ASCE 7-10; \ Lumber DOL=1.60 p 5) This truss has bee will fit between the b 7) Provide mechanical 6=424, 2=437. 8) Hanger(s) or other of 2-0-12, 1360 lb dow down and 95 lb up a	e been provided to distribute of a loads have been considered /ult=130mph Vasd=103mph; T plate grip DOL=1.60 designed for a 10.0 psf botton in designed for a live load of 30 pottom chord and any other me connection (by others) of trust connection device(s) shall be p in and 95 lb up at 4-0-12, 1360 at 10-0-12, 1360 lb down and 95 lb up and 1360 lb down and 95 lb up	ggered at 0-9-0 oc. staggered at 0-7-0 oc. c. , except if noted as front (F) or back (I nly loads noted as (F) or (B), unless o for this design. CDL=6.0psf; BCDL=6.0psf; h=15ft; Ca n chord live load nonconcurrent with an 0.0psf on the bottom chord in all areas	therwise indicated. at. II; Exp C; Enclosed my other live loads. where a rectangle 3- ing 100 lb uplift at joir ted load(s) 1360 lb do 0 lb down and 95 lb u 1 95 lb up at 14-0-12,	); MWFRS 6-0 tall by 2 ht(s) excep own and 95 o at 8-0-12 and 1360	(envelope); 2-0-0 wide t (jt=lb) 5 lb up at 2, 1360 lb lb down and	NORTH SOFE SO3	EAL 6322
is the responsibility	dard alanced): Lumber Increase=1.	15, Plate Increase=1.15				1.10	GILBER

ent 818 Soundside Road Edenton, NC 27932

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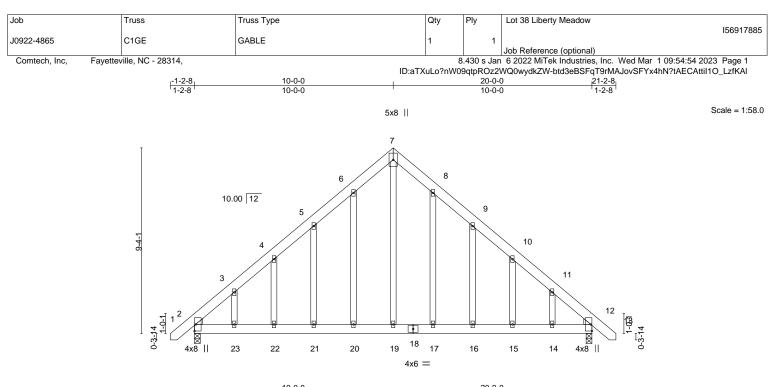
[	Job	Truss	Truss Type	Qty	Ply	Lot 38 Liberty Meadow
						156917884
	J0922-4865	C1-GR	COMMON GIRDER	1	2	
					-	Job Reference (optional)
	Comtech, Inc, Fayettev	rille, NC - 28314,		8	8.430 s Jan	6 2022 MiTek Industries, Inc. Wed Mar 1 09:54:56 2023 Page 2
			ID:aTXuLc	o?nW09qtp	ROz2WQ	0wydkZW-XGlp3tUVM4PYcUTA0tI00Vnd5hqqgzOA9cWU3DzfKAj

### LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 9=-1290(F) 11=-1290(F) 12=-1290(F) 13=-1290(F) 14=-1290(F) 15=-1290(F) 16=-1290(F) 17=-1290(F) 18=-1290(F)

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		<u>    10-0-0</u> 10-0-0	<u> </u>	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.20 BC 0.40 WB 0.40 Matrix-S	DEFL.         in         (loc)         //defl         L/d         PLATES           Vert(LL)         -0.09         15-16         >999         360         MT20           Vert(CT)         -0.14         15-16         >999         240           Horz(CT)         0.01         12         n/a         n/a           Wind(LL)         0.15         21-22         >999         240         Weight:	244/190

BRACING-TOP CHORD

BOT CHORD

```
LUMBER-
```

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.2
OTHERS	2x4 SP No.2
WEDGE	

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. (size) 12=0-3-8, 2=0-3-8 Max Horz 2=-276(LC 10) Max Uplift 12=-169(LC 13), 2=-169(LC 12) Max Grav 12=860(LC 1), 2=860(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-870/130, 3-4=-749/172, 4-5=-705/231, 5-6=-739/304, 6-7=-776/383, 7-8=-776/383, 8-9=-739/304, 9-10=-705/231, 10-11=-749/172, 11-12=-870/129

 BOT CHORD
 2-23=-39/567, 12-23=-39/567, 22-23=-39/567, 22-23=-39/567, 12-14=-39/567, 14-15=-39/567, 12-14=-39/567

WEBS 7-19=-309/660

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=169, 2=169.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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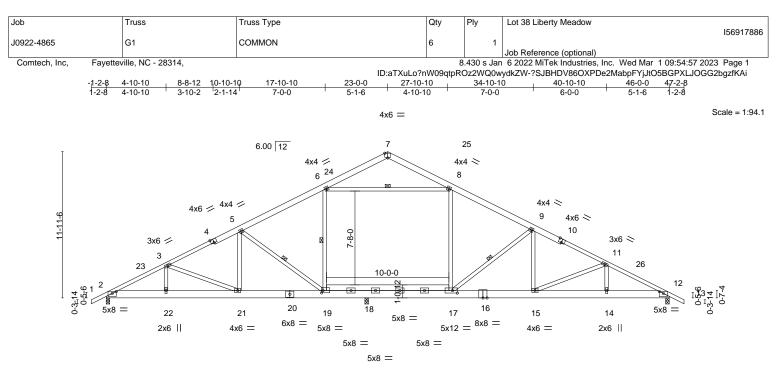


Plate Offsets (X,Y)-		7-0-0 3-3-0 ),Edge], [10:0-3-0,Edge], [	6-11-11 [12:0-4-0,0-1-15], [17:0	6-9-5 )-4-12,0-2-8], [19:0	<u>6-0-0</u> -1-8,0-2-4]	5-1-6				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.26 BC 0.44 WB 0.42 Matrix-S	Vert(LL) -0.2 Vert(CT) -0.4 Horz(CT) 0.0	in (loc) l/defl 24 15-17 >999 40 15-17 >739 03 12 n/a 16 15-17 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 376 lb	<b>GRIP</b> 244/190 FT = 20%			
LUMBER- TOP CHORD     2x6 SP No.1 *Except* 1-4,10-13: 2x4 SP No.1     BRACING- TOP CHORD       BOT CHORD     2x8 SP 2400F 2.0E *Except* 17-19: 2x6 SP No.1     BOT CHORD     Structural wood sheathing directly applied or 3-11-0 oc purlins. BOT CHORD       WEBS     2x4 SP No.2     WEBS     I Row at midpt     9-17, 6-19, 5-19, 6-8										
Ma Ma	size) 2=0-3-8, 12=0-3-8, 18=0-3-8 < Horz 2=156(LC 11) < Uplift 2=-66(LC 12), 12=-168(LC 13), 18= < Grav 2=1250(LC 1), 12=1365(LC 24), 18	( /								
TOP CHORD 2- 8-	ax. Comp./Max. Ten All forces 250 (lb) or 3=-2175/456, 3-5=-1759/445, 5-6=-1250/41 9=-1235/382, 9-11=-2082/468, 11-12=-236 22=-304/1880, 21-22=-304/1880, 19-21=-1	3, 6-7=-378/186, 7-8=-35 7/451	0/181,							
15	CHORD       2-22=-304/1880, 21-22=-304/1880, 19-21=-194/1515, 18-19=-52/1043, 17-18=-48/1031, 15-17=-245/1812, 14-15=-325/2045, 12-14=-325/2045         BS       9-17=-1058/260, 9-15=-26/593, 5-19=-875/234, 5-21=-23/446, 3-21=-393/120, 11-15=-281/104, 6-8=-846/319									

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-7 to 3-2-5, Interior(1) 3-2-5 to 23-0-0, Exterior(2) 23-0-0 to 27-4-13, Interior(1) 27-4-13 to 47-2-7 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

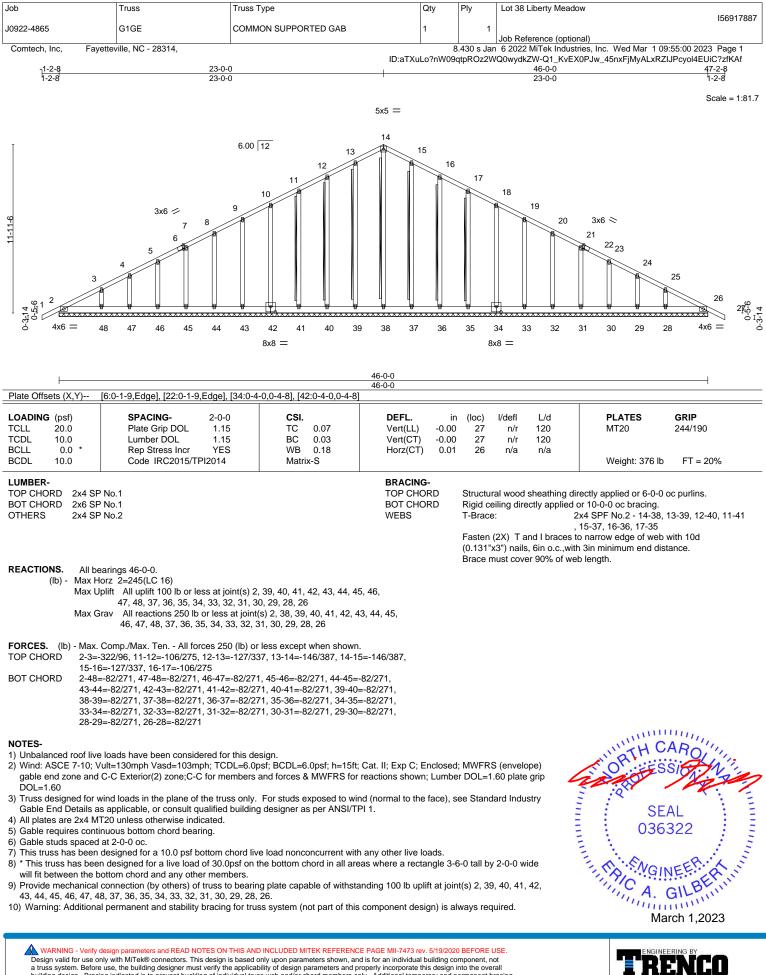
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 12=168, 18=120.



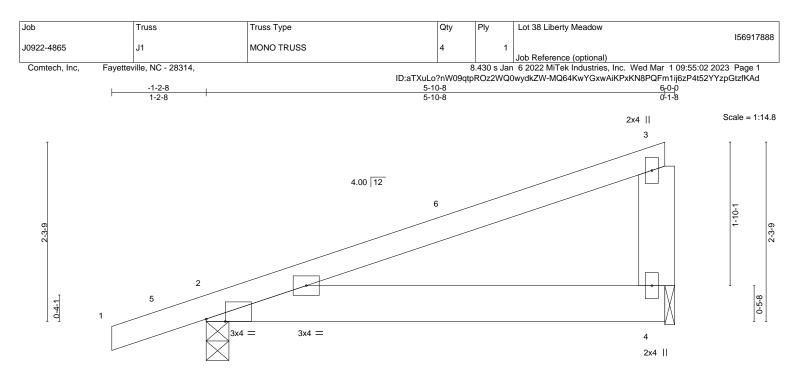
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818 Soundside Road Edenton, NC 27932



			6-0-0					
Plate Offsets (X,Y)	[2:0-2-15.Edge]		6-0-0				1	
	[2.0-2-13,Euge]							
_OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. i	n (loc)	l/defl	L/d	PLATES	GRIP
FCLL 20.0	Plate Grip DOL 1.15	TC 0.41	Vert(LL) -0.02	2-4	>999	360	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.12	Vert(CT) -0.03	2-4	>999	240		
CLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00	)	n/a	n/a		
SCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.03	3 2-4	>999	240	Weight: 29 lb	FT = 20%
UMBER-			BRACING-					

#### LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x6 SP No.1

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=83(LC 8) Max Uplift 2=-132(LC 8), 4=-90(LC 8)

Max Grav 2=316(LC 1), 4=215(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

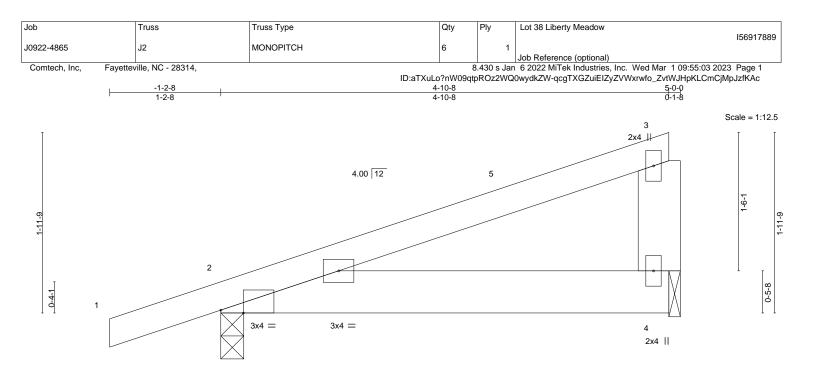
#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2 = 132



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	5-0-0							
Plate Offsets (X,Y)	[2:0-2-15,Edge]							
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL 1.15	TC 0.26	Vert(LL) -0	.01 2-4	>999 360	MT20	244/190	
TCDL 10.0	Lumber DOL 1.15	BC 0.08	Vert(CT) -0	.01 2-4	>999 240			
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0	.00	n/a n/a			
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0	.01 2-4	>999 240	Weight: 24 lb	FT = 20%	
LUMBER-			BRACING-			I		

TOP CHORD

BOT CHORD

REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x6 SP No.1

(size) 2=0-3-0, 4=0-1-8 Max Horz 2=72(LC 8)

Max Uplift 2=-119(LC 8), 4=-72(LC 8) Max Grav 2=277(LC 1), 4=174(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 4-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2 = 119



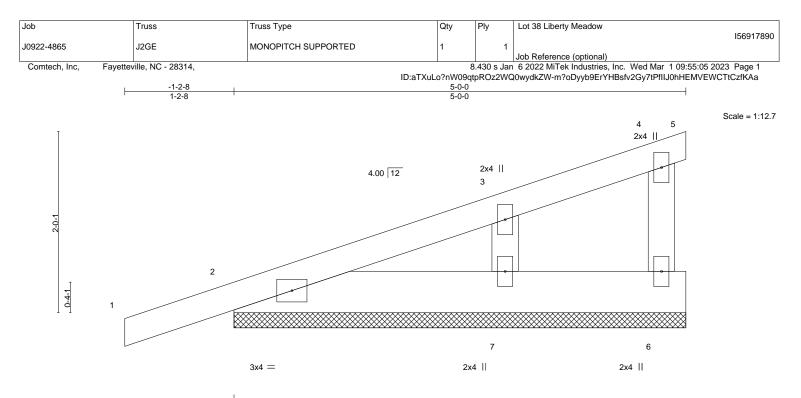
Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (I	oc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.07	Vert(LL) 0.00	1 n/r 120	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.02	Vert(CT) 0.00	1 n/r 120	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.03	Horz(CT) -0.00	5 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P			Weight: 24 lb FT = 20%

## P CHORD

2x6 SP No.1 BOT CHORD 2x4 SP No.2 WEBS OTHERS 2x4 SP No.2

TOP CHORE BOT CHORD

structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 5-0-0.

Max Horz 2=104(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 5, 6, 2, 7

Max Grav All reactions 250 lb or less at joint(s) 5, 6, 2, 7

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

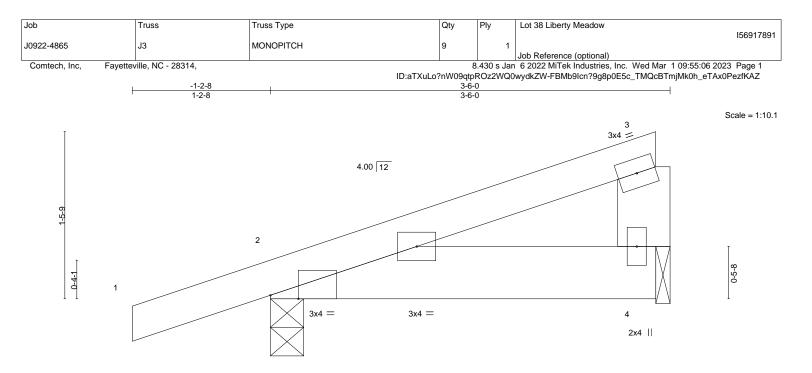
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6, 2, 7.

8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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		ł				<u>3-6-0</u> 3-6-0					
Plate Offsets (X,Y) [	2:0-2-15,Edge]					3-0-0					
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	-0.00	2-4	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.01	Horz(CT)	0.00		n/a	n/a		
BCDL 10.0	Code IRC2015/TF	PI2014	Matrix	κ-P	Wind(LL)	0.00	2	****	240	Weight: 17 lb	FT = 20%

#### LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1WEBS2x6 SP No.1

REACTIONS. (size) 2=0-3-8, 4=0-1-8 Max Horz 2=56(LC 8)

Max Uplift 2=-69(LC 8), 4=-14(LC 12) Max Grav 2=224(LC 1), 4=107(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

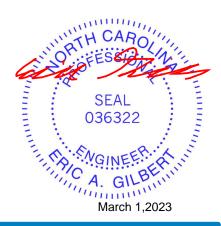
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

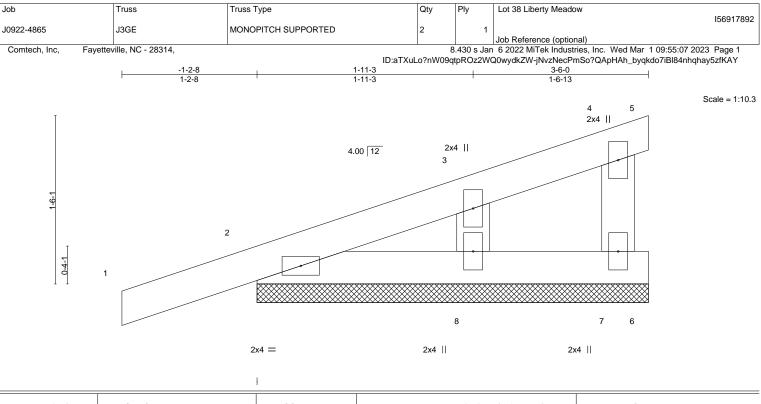


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BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



	· · ·	SPACING-	2-0-0	CSI.	0.07	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL TCDL	20.0 10.0	Plate Grip DOL Lumber DOL	1.15 1.15	TC BC	0.07 0.02	Vert(LL) Vert(CT)	0.00 0.00	4	n/r n/r	120 120	MT20	244/190
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code IRC2015/TF	YES PI2014	WB Matri	0.02 x-P	Horz(CT)	0.00		n/a	n/a	Weight: 15 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1WEBS2x4 SP No.2

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 3-6-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 7=3-6-0, 2=3-6-0, 8=3-6-0 Max Horz 2=79(LC 8) Max Uplift 7=-26(LC 8), 2=-91(LC 8), 8=-38(LC 12) Max Grav 7=52(LC 1), 2=164(LC 1), 8=125(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable End Details as applicable, of consult qualified building designer as
   Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

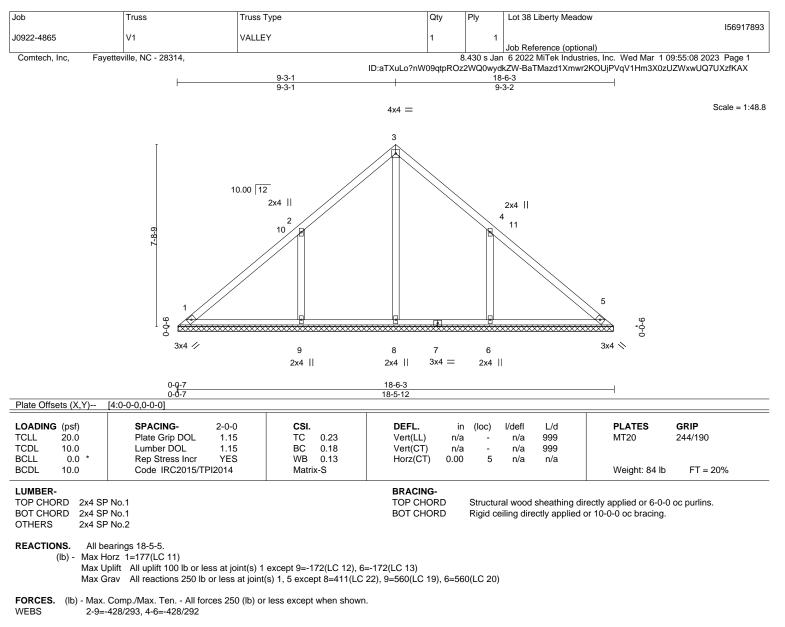
6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2, 8.



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NOTES-

Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 9-3-1, Exterior(2) 9-3-1 to 13-7-14, Interior(1) 13-7-14 to 18-1-6 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

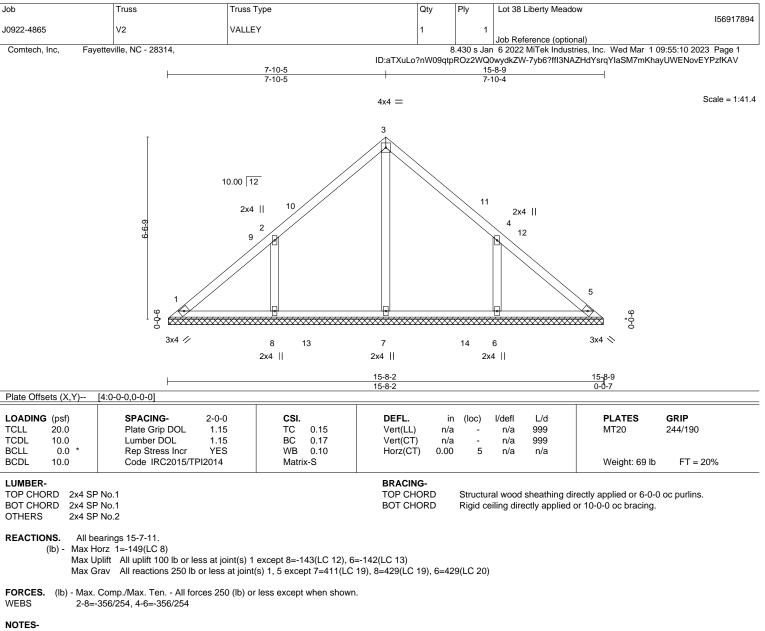
5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=172, 6=172.



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 7-10-5, Exterior(2) 7-10-5 to 12-3-1, Interior(1) 12-3-1 to 15-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

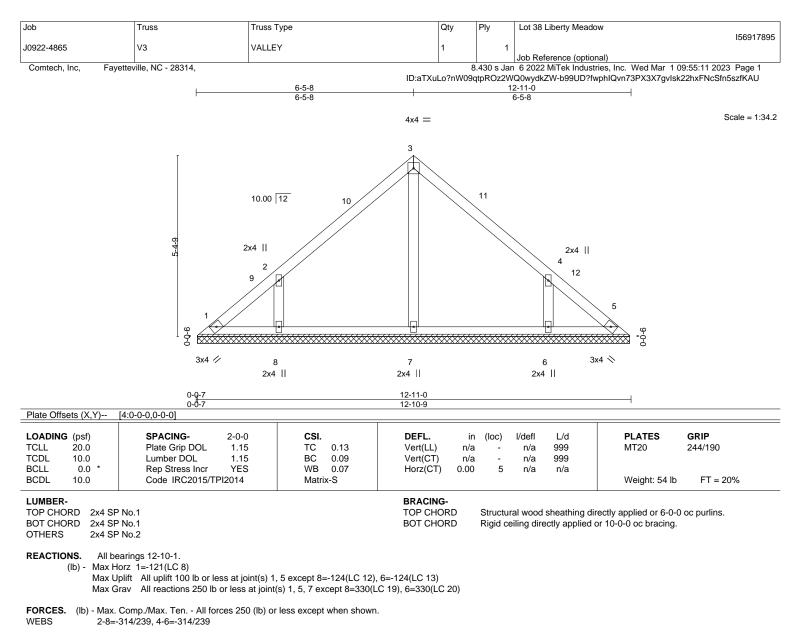
5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=143, 6=142.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-13 to 4-9-10, Interior(1) 4-9-10 to 6-5-8, Exterior(2) 6-5-8 to 10-10-5, Interior(1) 10-10-5 to 12-6-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

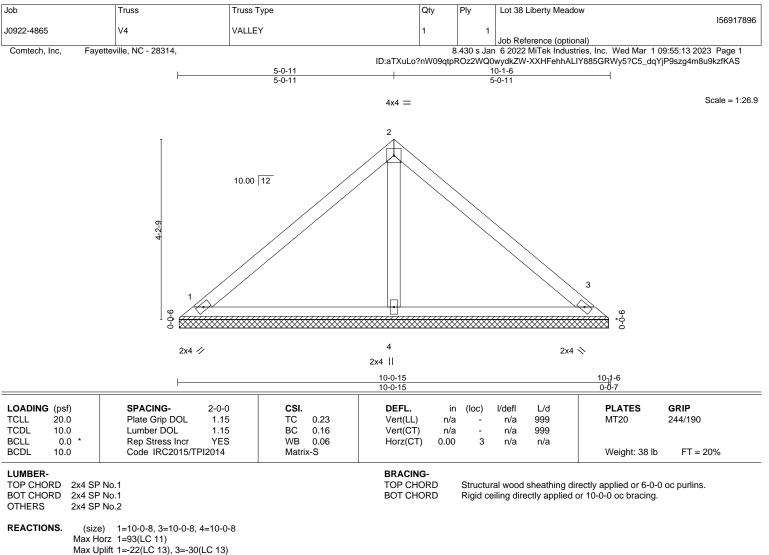
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=124, 6=124.

6) Non Standard bearing condition. Review required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Max Grav 1=199(LC 1), 3=199(LC 1), 4=347(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

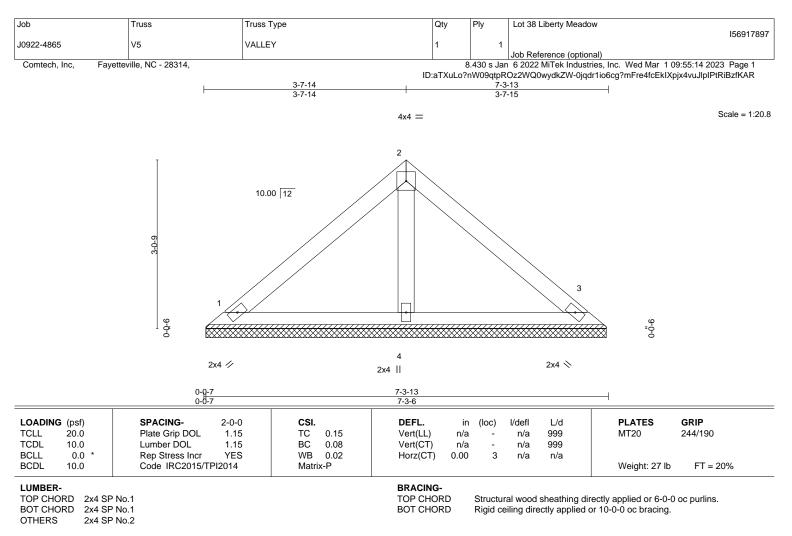
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





REACTIONS. (size) 1=7-2-14, 3=7-2-14, 4=7-2-14 Max Horz 1=-65(LC 8) Max Uplift 1=-23(LC 13), 3=-29(LC 13) Max Grav 1=151(LC 1), 3=151(LC 1), 4=220(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

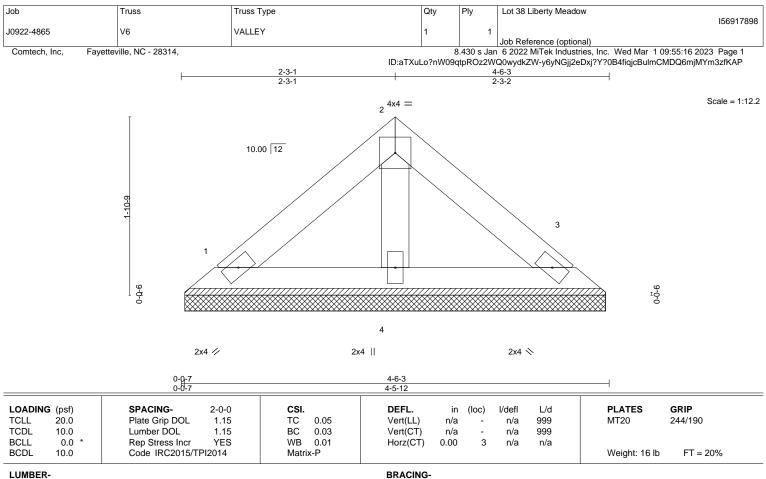
4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
 6) Non Standard bearing condition. Review required.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-6-3 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 1=4-5-5, 3=4-5-5, 4=4-5-5 (size) Max Horz 1=-37(LC 8) Max Uplift 1=-13(LC 13), 3=-16(LC 13) Max Grav 1=86(LC 1), 3=86(LC 1), 4=125(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

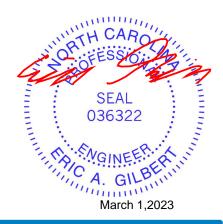
2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

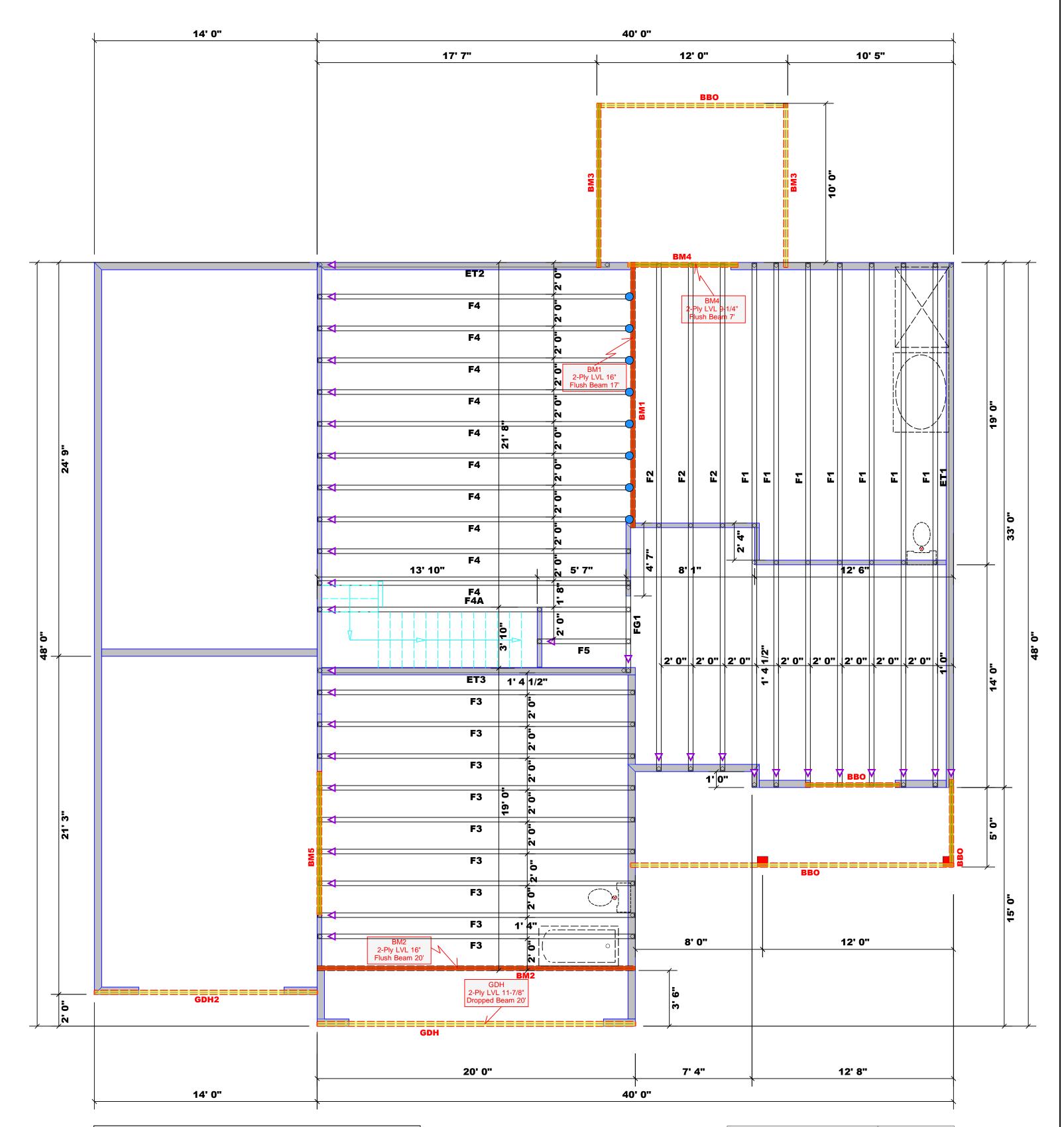
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



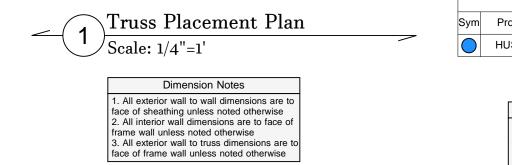
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







		Products			
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	17' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM2	20' 0"	1-3/4"x 16" LVL Kerto-S	2	2	FF
BM3	12' 0"	2x10 SP No.2	2	4	FF
BM4	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
BM5	9' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
GDH	20' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
GDH2	14' 0"	2x12 SPF No.2	2	2	FF

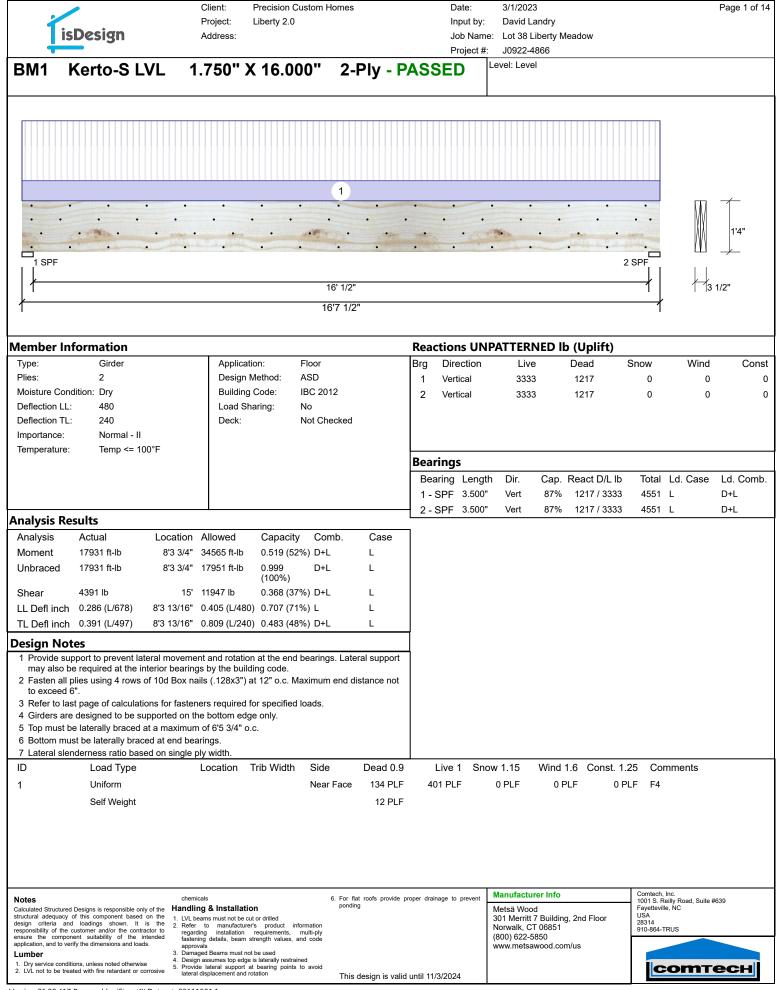


	Conne	Nail Info	ormation			
Sym	Product	Manuf Qty Supported Member			Header	Truss
$\bigcirc$	HUS410	USP	8	Varies	16d/3-1/2"	16d/3-1/2"

Plumbing Drop Notes
1. Plumbing drop locations shown are NOT exact.
2. Contractor to verify ALL plumbing drop
locations prior to setting Floor Trusses.
3. Adjust spacing as needed not to exceed 24"oc.

= Indicates Left End of Truss
 (Reference Engineered Truss Drawing)
 Do NOT Erect Truss Backwards

(BASED ON TAB	FOR JACK STUDS BLES R502.5(1) & (b)) DS REQUIRED @ EA END OF	BUILDER	Precision Custom Homes and Renovations	COUNTY	Cameron / Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer	
	ER/GIRDER	JOB NAME	Lot 38 Liberty Meadow	ADDRESS	Lot 38 Liberty Meadow	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package	соттесн
END REAC (UP TC (UP TC (UP TC (2) PLY HE (2) PLY HE (2) PLY HE (1) TC (1) TC	(UP T( REQ'D STU (3) PLY HH (3) PLY HH (UP T (UP T) (4) PLY H	PLAN	Liberty 2.0 w/ CP	MODEL	Floor	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	<b>ROOF &amp; FLOOR</b>
5100 3 765	00         2         6800         2           50         3         10200         3	SEAL DATE	N/A	DATE REV.	03/01/23	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787
		QUOTE #		DRAWN BY	David Landry	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	
11900 7 13600 8 15300 9		JOB #	J0922-4866	SALESMAN	Neil Baggett	Signature David Landry	Fax: (910) 864-4444



1	isDesign	Client: Project: Address:	Precision Custom Homes Liberty 2.0	Date: Input by: Job Name: Project #:	3/1/2023 David Landry Lot 38 Liberty Meadow J0922-4866	Page 2 of 7
BM1	Kerto-S L	.VL 1.750"	X 16.000" 2-Ply	- PASSED	evel: Level	
•	· · · ·	· · · · · · ·	· · · · ·	· · · ·		
1 SPF	F					
1			16' 1/2" 16'7 1/2"			<b>1</b> ] ]3 1/2"
I			167 1/2			I
lulti-Pl	y Analysis					
	l plies using 4 ro		(.128x3") at 12" o.c Maxi	mum end distance no	t to exceed 6".	
apacity ad		81.7 % 267.5 PLF				
eld Limit p		327.4 PLF				
eld Limit p eld Mode	per Fastener	81.9 lb. IV				
lge Distan n. End Dis		1 1/2" 3"				
ad Combi	ination	5 D+L				
ration Fa	ctor	1.00				
		chomissis	6 Far 8ct	provido propor drainana ta anguat	Manufacturer Info	Comtech, Inc.
Notes Calculated Strue	intured Decigns is responsible or	chemicals hly of the Handling & Installati		provide proper drainage to prevent	Metsä Wood	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC

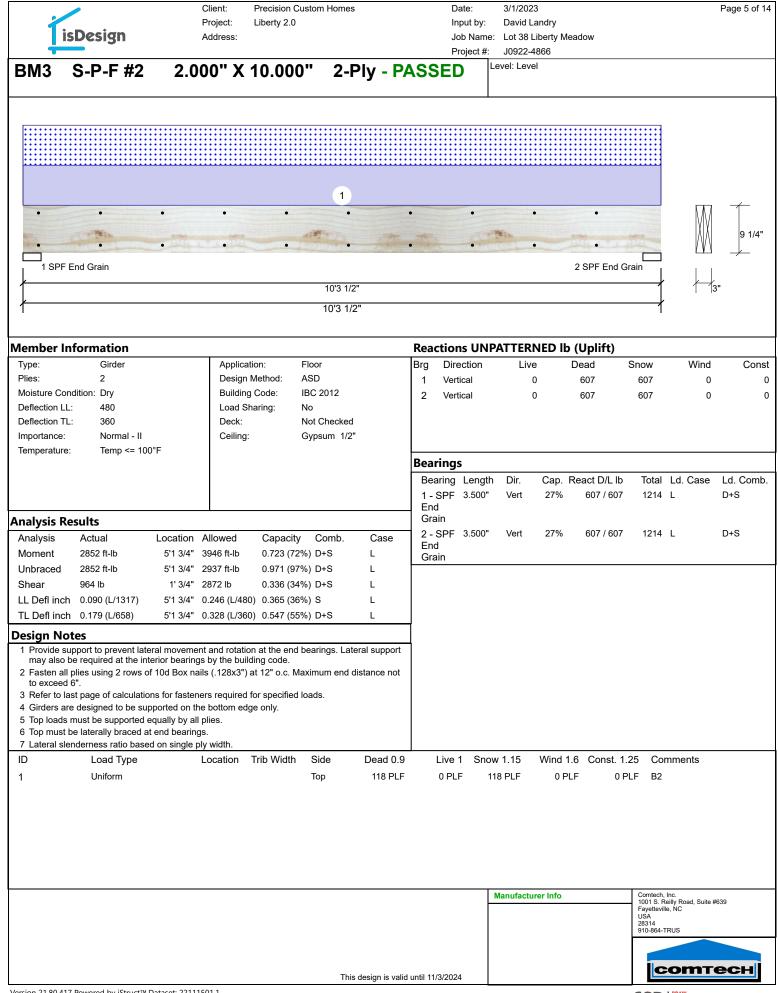
Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
	1. I VI beams must not be cut or drilled	ponding This design is valid until 11/3/2024	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	Fayetteville, NC USA 28314 910-864-TRUS

is	Design	Client: Project: Address:	Precision Custo Liberty 2.0	om Homes		l	Date: nput by: Job Name Project #:	3/1/2023 David La e: Lot 38 Li J0922-48	ndry berty Meado	w			Page 3 of 1
BM2 I	Kerto-S LVI	_ 1.750"	X 16.000	" 2-P	Ply - PA	ASSE	D	Level: Level					
		2						3					
1 SPF	- Man			1	17			140		•	2 SPF	$\mathbb{M}$	1'4"
				20' 20'								13	1/2"
				-							•		
lember In								PATTERN	•	,			
Type: Plies: Moisture Cond Deflection LL: Deflection TL:	480	Buildir	Method: AS g Code: IBC sharing: No			1 Ve	rection rtical rtical	Live 400 400	Dea 24( 24(	09	Snow 135 135	Wind 0 0	Con
Importance: Temperature:	Temp <= 100°F				-	1 - SPF	J Lengtl 3.500"	Vert		409 / 401	2811		Ld. Comb D+0.75(L+
nalysis Re	sults				[	2 - SPF	3.500"	Vert	54% 24	409 / 401	2811	L	D+0.75(L·
	13439 ft-lb 13439 ft-lb 2461 lb 0.059 (L/3960)	ocation Allowed 10' 34565 ft-lb 10' 13492 ft-lb 18'4 1/2" 11947 lb 10' 1/16" 0.489 (L/48	0.389 (39%) 0.996 (100%) 0.206 (21%) 0) 0.121 (12%)	D+L D+L 0.75(L+S)	Case L L L L								
		10' 1/16" 0.978 (L/24	0) 0.425 (42%)	D+0.75(L+S	) L								
may also b 2 Fasten all p to exceed 6 3 Refer to las 4 Girders are 5 Top loads r 6 Top must b 7 Bottom mu	oport to prevent lateral e required at the interio plies using 3 rows of 10	or bearings by the bu Dd Box nails (.128x3" for fasteners required rted on the bottom ed ally by all plies. maximum of 8'9 7/16 at end bearings.	lding code. at 12" o.c. Maxii for specified loa Ige only.	num end dist									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live	1 Snc	ow 1.15	Wind 1.6	Const. 1	.25 Co	mments	
1	Uniform			Гор	200 PLF	0 PI		0 PLF	0 PLF			Il Above, C10	GE
2	Tie-In Tie-In	0-0-0 to 20-0-0 0-0-0 to 20-0-0		<sup>F</sup> ar Face Near Face	15 PSF 27 PSF	40 PS 0 PS		0 PSF 27 PSF	0 PSF 0 PSF		PSF Flo PSF J3	or Load	
3	Self Weight	0-0-0 10 20-0-0	0-0-0 I	Near Face	27 PSF 12 PLF	0 P3	SF	27 PSF	0 956	01	-SF J3		
structural adequacy design criteria and responsibility of the ensure the compor	Designs is responsible only of th of this component based on th loadings shown. It is th ustomer and/or the contractor i ent suitability of the intende ify the dimensions and loads.	<ol> <li>LVL beams must not be</li> <li>Refer to manufacture</li> <li>regarding installation fastening details, beam approvals</li> <li>Damaged Beams must</li> </ol>	cut or drilled rer's product informa requirements, multi strength values, and c	ponding tion -ply	roofs provide pro	per drainage t	o prevent	Manufacture Metsä Wood 301 Merritt 7 Norwalk, CT (800) 622-58 www.metsaw	Building, 2nd 06851 50	Floor	Comtech, 1001 S. F Fayettevil USA 28314 910-864-	Reilly Road, Suite ≉ lle, NC	/639

Version 21.80.417 Powered by iStruct<sup>™</sup> Dataset: 22111501.1

		Client:	Precision Custom H	lomes		Date:	3/1/2023			Page 4 of 14
		Project:	Liberty 2.0			Input by:	David Landry			
isDe	sign	Address:				Job Name:	Lot 38 Liberty Meadow			
	9					Project #:	J0922-4866			
BM2 Ker	to-S LVL	1.750"	X 16.000"	2-Ply	- PASSE	D	evel: Level			
	· · · ·	· · ·	· · · ·	· · ·	· · ·	· · ·	· · · ·	· · · ·	+<1 1/2"	1'4"
1 SPF								2 SPF		
1				20'				,	T	3 1/2"
/				20'				,	•	
Multi-Ply Analy	cic									
Fasten all plies u		)d Box nails	(128v3") at 12"	o.c. Mavim	um and dis	ance no	t to exceed 6"			
Capacity	11.2 %		(.120,5) at 12		uni cha ais					
Load	27.5 F									
Yield Limit per Foot	245.6									
Yield Limit per Fastene										
Yield Mode	IV									
Edge Distance	1 1/2"									
Min. End Distance	3"									
Load Combination	D+L									
Duration Factor	1.00									

Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. <b>Lumber</b> 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	LVL beams must not be cut or drilled     Refer to manufacturer's product information     regarding installation requirements multi-nly	ponding This design is valid until 11/3/2024	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	Fayetteville, NC USA 28314 910-864-TRUS



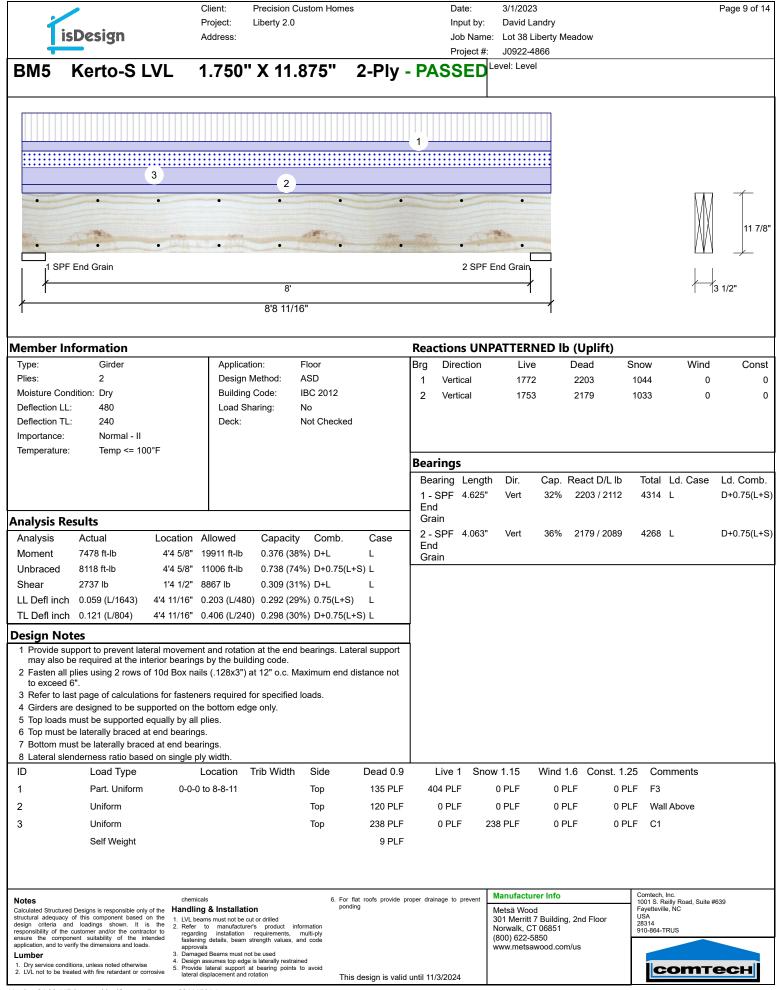
-			Precision Custon Liberty 2.0	n Homes	Date: Input b	3/1/2023 y: David Landry	Page 6 of 14
	isDesign	Address:	,			me: Lot 38 Liberty Meadow	
BM3	S-P-F #2	2.000" X	10.000"	2-Ply - P		Level: Level	
•	•	• •	•	•	• •	• •	₹ ₹ ₽ ₽ 1/4"
	• F End Grain	•••	•	•	· ·	• • • 2 SPF End G	
				10'3 1/2"			13"
1				10'3 1/2"			1
Multi-Ply	Analysis						
		s of 10d Box nails (.	128x3") at 12	"oc Maximun	n end distance	not to exceed 6"	
Capacity	plies using 2 row	0.0 %	120,3 7 41 12				
Load Yield Limit pe	er Foot	0.0 PLF 157.4 PLF					
Yield Limit pe		78.7 lb.					
Yield Mode Edge Distanc	e	IV 1 1/2"					
Min. End Dist	ance	3"					
Load Combin Duration Fact		1.00					
Duration raci		1.00					
						Manufacturer Info	Comtech, Inc.
							Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC
							USA 28314 910-864-TRUS
1						1	

This design is valid	d until 11/3/2024
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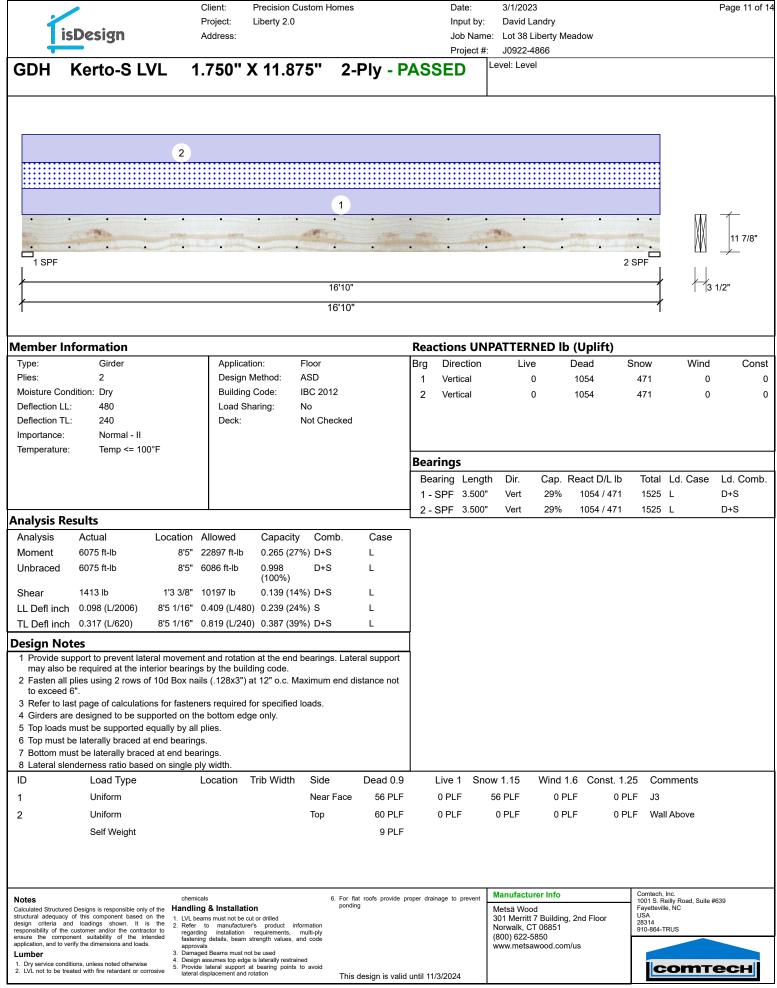
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Lis	Design	F		Precision C Liberty 2.0	ustom Home	S		Date: Input by Job Nan			adow			Page 7 of 1
	-			<u> </u>	<u></u>			Project #		4866				
BM4 I	Kerto-S L	VL 1	./50	Χ 9.2	50	2-Piy -	PAS	3ED						
						3	3			9				
	2													
	2			1						1				
		•		- 1 -		•	•		•				N A	1 1
													XXX	9 1
•	•	Sillin .	12-414	•	ALC: NO PARTY	•	•		- This					
	End Grain							2 SPF En	d Grain	]			ļ	
				6'					1				1	3 1/2"
1				6'7"						1				
ember Inf	formation						React	ions UN	NPATTER	NED Ib	(Uplift)			
уре:	Girder		Applicati		Floor			Direction			Dead	Snow	Wind	Cons
Plies: Aoisture Cond	2 lition: Dru		Design N Building		ASD IBC 2012			Vertical	214		1903	0	0	
Deflection LL:	480		Building Load Sh		No		2	Vertical	214	9	1903	0	0	
eflection TL:	360		Deck:	-	Not Checke	d								
nportance:	Normal - II													
emperature:	Temp <= 100°	'F					Beari	nac						
								ing Leng	th Dir.	Cap R	eact D/L lb	Total	Ld. Case	Ld. Com
								PF 3.500		•	1903 / 2149	4053		D+L
							End							
nalysis Re		Lesstian A	Married	Canacita	Camb	Casa	Grair	I PF 3.500	)" Vert	39%	1903 / 2149	4053	L	D+L
Analysis <i>N</i> oment	Actual 5774 ft-lb	Location A 3'3 1/2" 1	2542 ft-lb	Capacity 0.460 (46		Case L	End							
Jnbraced	5774 ft-lb		934 ft-lb	0.581 (58		L	Grair	1						
Shear	2750 lb	1' 3/4" 6	907 lb	0.398 (40	-	L								
L Defl inch	0.056 (L/1320)	3'3 1/2" 0	).153 (L/480)	) 0.364 (36	%) L	L								
L Defl inch	0.105 (L/700)	3'3 1/2" 0	0.204 (L/360)	) 0.514 (51	%) D+L	L								
esign Not	es													
	port to prevent later e required at the inte				bearings. La	teral support	]							
2 Fasten all p	lies using 2 rows of	•		•	aximum end	distance not								
to exceed 6 3 Refer to las	". t page of calculation	s for fastene	rs required f	or specified	loads.									
4 Girders are	designed to be supp	ported on the	bottom edg	•										
	nust be supported ec e laterally braced at													
	st be laterally braced	-												
	derness ratio based			<b>F</b> \ <b>A</b> /	0:4-	Decilor					0 Ormat 4	05 0-		
C	Load Type	L	ocation	Frib Width	Side	Dead 0.9			1.15		6 Const. 1		mments	
	Uniform				Тор Тор	102 PLF		PLF	0 PLF	0 PL		PLF F2		
2	Uniform				Тор Тор	349 PLF		PLF PLF	0 PLF 0 PLF	0 PL		PLF A1		
3	Uniform Solf Woight				Тор	120 PLF 7 PLF	0	┍┎┍	U PLF	0 PL		PLF Wa	ll Above	
	Self Weight					/ PLF								
						And and the second s			Manufactu	irer Info		Comtech	Inc.	
	Designs is responsible only of		y & Installatio		6. For pon	flat roofs provide p ding	roper draina	ge to prevent	Metsä Woo	od		1001 S. F Fayettevi	teilly Road, Suite #	639
sign criteria and	of this component based on loadings shown. It is ustomer and/or the contracto	the 2. Refer t	ms must not be cu to manufacturer	's product inf	ormation				Norwalk, C		2nd Floor	USA 28314 910-864-	TRUS	
sure the compone	ent suitability of the inten fy the dimensions and loads.		g installation g details, beam s s	requirements, trength values, a	multi-ply nd code				(800) 622-5		us	510-004		
umber		3. Damage	d Beams must not	be used is laterally restrain	hed									
	ons, unless noted otherwise		lateral support a										OMT	

isDesi	gn		Precision Custom F iberty 2.0	lomes	Date: Input by: Job Nam Project #	e: Lot 38 Liberty Meadow	Page 8 of 14
BM4 Kert	o-S LVL	1.750"	X 9.250"	2-Ply		Level: Level	
•	•	•	•	•	•	•	
•	•	•	•	•	•	• ~ 11/2"	9 1/2
1 SPF End Gra	in				2 SPF End		
			6' 6'7"				3 1/2"
						,	
Multi-Ply Analysis Fasten all plies usir		l Box nails (.1	28x3") at 12"	o.c Maximur	n end distance n	ot to exceed 6".	
Capacity Load	0.0 % 0.0 PLF						
Yield Limit per Foot Yield Limit per Fastener	163.7 Pl 81.9 lb.	LF					
Yield Mode Edge Distance	IV 1 1/2"						
Min. End Distance	3"						
Load Combination Duration Factor	1.00						
						Manufacturer Info	Comtech, Inc.
Notes Calculated Structured Designs is n	esponsible only of the Hand	micals	I	<ol><li>For flat roofs provide ponding</li></ol>	e proper drainage to prevent	Manufacturer Info Metsä Wood	Lorntech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA
structural adequacy of this comp design criteria and loadings responsibility of the customer and	shown. It is the 2 Red d/or the contractor to reg	beams must not be cut of fer to manufacturer's arding installation re	product information equirements, multi-ply			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	USA 28314 910-864-TRUS
ensure the component suitabili application, and to verify the dimen Lumber	sions and loads. app 3. Date: 2015 app 3. Date: 2015 app	tening details, beam stre provals maged Beams must not b	ength values, and code e used			(800) 622-5850 www.metsawood.com/us	
<ol> <li>Dry service conditions, unless r</li> <li>LVL not to be treated with fire</li> </ol>	noted otherwise 4. Des 5. Pro	sign assumes top edge is wide lateral support at eral displacement and rota	laterally restrained bearing points to avoid	This design is vo	lid until 11/3/2024		соттесн
				The design is Va		1	



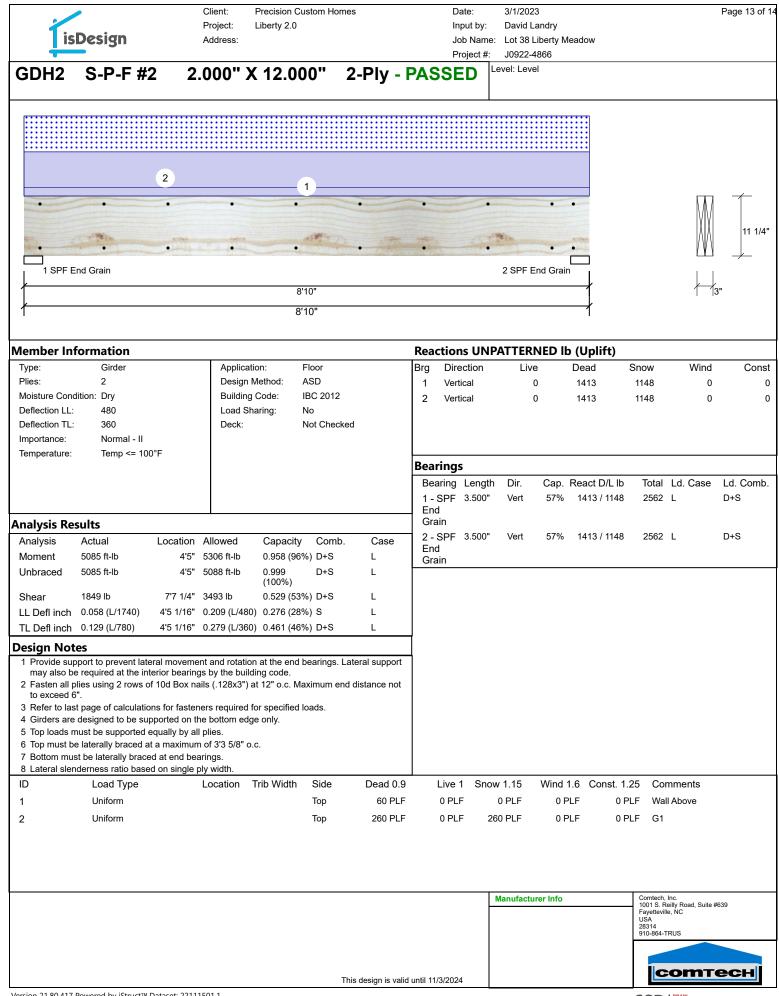
	Client: Precision Custom	Homes Date:	3/1/2023	Page 10 of 14
LioDopiero	Project: Liberty 2.0	Input by:	David Landry	
isDesign	Address:	Job Name Project #:	e: Lot 38 Liberty Meadow J0922-4866	
DM5 Karta SIVI	4 750" V 44 076	-	Level: Level	
BM5 Kerto-S LVL	1.750" X 11.875	5" 2-Ply - PASSED		
	• •	• • •	•	$\Lambda \Lambda \Lambda = 1$
			<1 1/2"	11 7/8"
	• •		•	
1 SPF End Grain		2 SPF	End Grain	
1	8'		1	3 1/2"
1	8'8 11/16"		1	
Multi-Ply Analysis				
Fasten all plies using 2 rows of 10d	Boy nails ( 128v3") at 12'	o c Maximum and distance n	at to exceed 6"	
Capacity 0.0 %		o.e Waximum end distance in		
Load 0.0 PLF				
Yield Limit per Foot163.7 PlYield Limit per Fastener81.9 lb.	LF			
Yield Mode IV				
Edge Distance 1 1/2" Min. End Distance 3"				
Load Combination				
Duration Factor 1.00				
Notes che	emicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Structured Designs is responsible only of the Hand structural adequacy of this component based on the	lling & Installation	ponding	Metsä Wood 301 Merritt 7 Building, 2nd Floor	Fayetteville, NC USA
design criteria and loadings shown. It is the 2. Ref responsibility of the customer and/or the contractor to reg	fer to manufacturer's product information arding installation requirements, multi-ply		Norwalk, CT 06851	28314 910-864-TRUS
application, and to verify the dimensions and loads. app	tening details, beam strength values, and code provals maged Beams must not be used		(800) 622-5850 www.metsawood.com/us	
1. Dry service conditions, unless noted otherwise     5. Pro	sign assumes top edge is laterally restrained wide lateral support at bearing points to avoid			сотесн
2. LVL not to be treated with fire retardant or corrosive late	ral displacement and rotation	This design is valid until 11/3/2024		



Version 21.80.417 Powered by iStruct<sup>™</sup> Dataset: 22111501.1

1	isDesign	Client: Project: Address:	Precision Custom Liberty 2.0	Homes	Date: Input b Job Na Project	me: Lot 38 Liberty Meadow #: J0922-4866	Page 12 o
GDH	Kerto-S LV	′L 1.750''	X 11.875"	2-Ply -	PASSED	Level: Level	
	• • •	• •	•••	· ·	• •	· · · ·	
1 SPF				16'10"			2 SPF
<u>/</u>				16'10"			
	<b>/ Analysis</b> plies using 2 rows	of 10d Box nails	: ( 128x3") at 12"	o.c. Maximi	um end distance	not to exceed 6"	
apacity oad		29.7 % 56.0 PLF				not to exceed 0.	
	er Foot er Fastener	188.3 PLF 94.1 lb.					
eld Mode dge Distand	ce	IV 1 1/2"					
in. End Dist bad Combin		3" D+S					
uration Fac		1.15					
Nets -		chemicals		6 For flat rack	vide proper drainage to preven	, Manufacturer Info	Comtech, Inc.
Notes Calculated Struct structural_adequa	tured Designs is responsible only of acy of this component based on	the Handling & Installa		6. For flat roots prov ponding	noc proper urainage to preven	Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC USA
design criteria responsibility of	and loadings shown. It is the customer and/or the contractor	the 2. Refer to manufac	e cut or drilled turer's product information n requirements, multi-ply			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	28314 910-864-TRUS
ensure the cor application, and to	mponent suitability of the inter to verify the dimensions and loads.		m strength values, and code			(800) 622-5850 www.metsawood.com/us	
1. Dry service co	onditions, unless noted otherwise e treated with fire retardant or corro	<ol> <li>Design assumes top e</li> <li>Provide lateral support</li> </ol>	dge is laterally restrained rt at bearing points to avoid				соттесн
∠. LVL HUL TO DE	a calcu with the relardant of corro	sive lateral displacement a	nd rotation	This design is	valid until 11/3/2024	i i	

This design is valid until 11/3/2024



is	Design	Client: Project: Address:	Precision Custom He Liberty 2.0	omes	Job	it by:	3/1/2023 David Landry Lot 38 Liberty Meadow J0922-4866	Ρ	age 14 of 14
GDH2	S-P-F #2	2 2.000"	X 12.000"	2-Ply			evel: Level		
	•	• •	•	•	•	•	•••	<11/2"	11 1/4"
1 SPF E	End Grain						2 SPF End Grain		<i>—</i> —
<del> </del>			8'10" 8'10"					<b>∤</b> → <b> </b> 3"	
Multi-Ply A	nalysis								
Capacity Load Yield Limit per F Yield Mode Edge Distance Min. End Distan Load Combinatio Duration Factor	astener ce	0.0 % 0.0 PLF 157.4 PLF 78.7 lb. IV 1 1/2" 3" 1.00							
						Ν	Nanufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC	)
								Fayetteville, NC USA 28314 910-864-TRUS	
				This design is	valid until 11/3/2024			comte	СН

_		-		-														DA	ΓΕ 03/0 <sup>-</sup>	1/23	PAGE	1
Re	action	Sum	nma	ry of (	Orde	er		REQ.	QUOT	E DA	TE	11	/			ORDER	R #		JO	922-	4866	
	$\wedge$							ORD	ER DA	ΓE		09/2	23/22			QUOTE	#					
1								DELI	VERY	DATE		11	/			CUSTO	MEF	R ACCT #	. 00	0000	)7216	
			ROOF	& FLOC	OR			DATE	OF IN	VOIC	E	11	1			CUSTO	MEF	R PO #				
∥C	omTeo	ch∥⊺	RUSS	ES & BEA	MS			ORD	ERED I	ЗY		Sha	un Garderner			INVOIC	E #					
Reilly	y Road Indu	ustrial Pa	ark P.C	D. Box 40	408			COU				Har	nett			TERMS	;					
	tteville, N.C							SUPE	RINTE		NT	Sha	un Garderner			SALES	REF	<b>)</b>	Ne	eil Ba	ggett	
,.			(	-,								1	0) 988-8172			SALES					andry	
												(31)	5) 300-0172									
s	Precisio	n Cus	stom	Homes		JOB	NAME:	.ot 38 L	_iberty					L	_OT #	\$ 38	SUB	DIV: Libe	rty Me	adow	1	
S O L D	256 Bria	r Hill F	٦d.			MOI	DEL:Floor	r		TA	G: Lib	erty 2	2.0 w/ CP	J	IOB (	CATEGO	RY:	_				
	Raeford	NC 2	28376	;			VERY INS		IONS:													
	(910) 98					52 mi	les round ti	rip														
S H I P	Precisio	n Cus	tom I	lomes	and	SPE	CIAL INST	RUCTIC	NS.													
	Lot 38 L	iberty	Mead	wob		01 23																
T O	Camero	n, NC	2835	6																	_	
		,																PLAN S				
				0//551	ANC .				00 (		-						<b>—</b>		B		DATE	
	LDING DE	PARTI	VIENT	OVERH END CUT			HEEL HE		00-0	06-08	R	<u></u>	AYOUTS	REQ. EI	NGINE				DTL DTL		03/01/2	
F100	r Order			END CUT	KET	URN	GABLE S	פחווד	241	N. OC		$\vdash$	JOBSITE 1			DBSITE		AYOUT	DTL		03/01/2	
							-		_						1 30				12.5		00,01/2	<u> </u>
F	LOOR	TRU	SSF		ADING		TCLL-TCD			RESS IN	CR.	F	LOOR TRUSS		NG: 24	4.0 IN. O.	.C. (1	TYP.)				
					ORM/		.0.0,10		_	1.00							`	,				
		QTY		PTH	BAS		O/A		TYPE		BEAR		REACTIO	NS								
PI	ROFILE	PLY		D	SPA	AN	SPAN	LEFT	RIGHT	SIZE	LOCA	TION										
								ı —	. <u> </u>		1											I
				04-00	20.44		32-11-00		<u></u>				Joint 29	Joint 30	ha	Joint 31	ha	Joint 32	lha	Joint		
		1	6	T1	32-11	1-00	32-11-00						23.4 lbs.	127.1	DS.	151.2	DS.	145.5	DS.	14	7.0 lbs.	
								I			1											1
<u></u>				04-00	40.04		40.04.00						Joint 17	Joint 18		Joint 19		Joint 20		Joint		
		1	E	T2	18-04	1-00	18-04-00	<u> </u>					32.7 lbs.	134.3	bs.	150.0 ll	bs.	145.8	bs.	146	6.9 lbs.	
								I			1											
				04-00	10.01		40.04.00	╓					Joint 18	Joint 19		Joint 20		Joint 21		Joint		
		1	E	T3	19-04	1-08	19-04-08						35.3 lbs.	136.7 l	DS.	149.5 ll	DS.	145.9	DS.	146	6.9 lbs.	
								ı —	. <u> </u>		1											I.
				04-00					凸				Joint 24	Joint 33		Joint 40						
		7		F1	32-11	-00	32-11-00						882.3 lbs. 204.1 lbs.	2190.2 1224.3		662.4 ll 22.2 lb						
													207.1 103.	1227.3	100.	22.2 IL						
		I		I		I			<u></u>	1	1		In int OO	1-1		1-1						I
~~~~		3		04-00 F2	31-11	1-00	31-11-00		<u></u>				Joint 22 794.9 lbs.	Joint 30 2078.8	lbs	Joint 36 719.4 II	hs					
		5			51-11		01-11-00						131.6 lbs.	1319.8		127.0						I
			01	04-00				<b>F</b>					Joint 14	Joint 24								
		9		F3	19-11	1-00	19-11-00		<b>_</b>				1075.4 lbs.	1075.4	lbs.							
		· 1		I		1			· 1				522.5 lbs.	522.5 l								!
			01-	04-00				<b></b>					Joint 16	Joint 25								
		10		F4	19-08	3-00	19-08-00	<b>—</b>	<b>_</b>				1067.9 lbs.	1061.7								
													512.1 lbs.	567.4 l	bs.							
																						,
	<u>minini d</u>			04-00			10						Joint 16	Joint 20		Joint 31						
		1	F	-4A	19-08	3-00	19-08-00	<u> </u>					275.1 lbs. 3.8 lbs.	1157.1 526.1 I		730.4 ll 198.6 ll						
													0.0 108.	J20.11	53.	190.01	53.					
		ј I				I			<u> </u>	1	1		In int 1	1-1								L
$\square$		1		04-00 F5	05-10	)-08	05-10-08						Joint 4 294.4 lbs.	Joint 8 288.3 I	bs							
		'			00-10	,	55 10-00						198.6 lbs.	193.7								

ROOF & FLOOR TRUSSES & BEAM Park P.O. Box 4040 309 (910) 864-TRU: ustom Homes I Rd.	S 8 5	REQ. QUOTE DATE ORDER DATE DELIVERY DATE DATE OF INVOICE ORDERED BY COUNTY SUPERINTENDANT JOBSITE PHONE #	/ / 09/23/22 / / / / Shaun Garderner Harnett Shaun Garderner		# MER ACCT # MER PO #		2-4866
TRUSSES & BEAM Park P.O. Box 4040 309 (910) 864-TRU: ustom Homes	S 8 S	DELIVERY DATE DATE OF INVOICE ORDERED BY COUNTY SUPERINTENDANT	/ / / / Shaun Garderner Harnett	CUSTOM CUSTOM INVOICE TERMS	MER ACCT # MER PO #	0000	007216
TRUSSES & BEAM Park P.O. Box 4040 309 (910) 864-TRU: ustom Homes	S 8 S	DATE OF INVOICE ORDERED BY COUNTY SUPERINTENDANT	/ / Shaun Garderner Harnett	CUSTON INVOICE TERMS	IER PO #	0000	007216
TRUSSES & BEAM Park P.O. Box 4040 309 (910) 864-TRU: ustom Homes	S 8 S	ORDERED BY COUNTY SUPERINTENDANT	Shaun Garderner Harnett	INVOICE TERMS	-		
Park P.O. Box 4040 309 (910) 864-TRU: ustom Homes	8 S	COUNTY SUPERINTENDANT	Harnett	TERMS	#		
309 (910) 864-TRU: ustom Homes	s	SUPERINTENDANT					
ustom Homes			Shaun Garderner				
	JOB NAME	JOBSITE PHONE #		JALES	(EP	Neil E	Baggett
	JOB NAME		(910) 988-8172	SALES A	REA	Davic	d Landry
		Lot 38 Liberty Meadow	!	LOT # 38 S	UBDIV: Libert	v Meado	ow
i ita.	MODEL: Flo		berty 2.0 w/ CP	JOB CATEGOR		,	
20276	-	ISTRUCTIONS:			<u>·· _</u>		
28376 72	52 miles round						
12							
ustom Homes ar	nd						
ty Meadow	SPECIAL INS	TRUCTIONS:					
•							
					PLAN SE		
							DATE 03/01/23
			EQ. LAYOUIS R	EQ. ENGINEERING			03/01/23
ENDCOT		STUDS 24 IN OC	JOBSITE 1	IOBSITE 1			03/01/23
DEPTH	BASE O/A	END TYPE INT BEAF					
01-01-00 1 FG1 C	05-06-00 05-06-0	₀	475.8 lbs. 4	455.3 lbs.			
ТҮРЕ	SIZE	<b>LENGTH</b> FT-IN-16	PART NUMBER		NOTES		
ers, USP	HUS 410			SIMPSON (HUS410)			
ms (Sized)   LVL, 1-3	8/4" x 9-1/4" (S)	07-00-00		BM4			
	/4" x 11-7/8" (S)	09-00-00		BM5			
ms (Sized)   I VI 1-3							
ms (Sized) LVL, 1-3	(4 X II //0 (0)						
			1	GDH			
	/4" x 11-7/8" (S)	20-00-00		GDH			
ms (Sized)   LVL, 1-3	/4" x 11-7/8" (S)	20-00-00					
ms (Sized)   LVL, 1-3				GDH BM1			
	rty Meadow IC 28356 RTMENT OVERHAN END CUT RUSSES LOAD INFO Y DEPTH ID 01-01-00 FG1 C	rty Meadow IC 28356 RTMENT OVERHANG INFO HEEL H END CUT RETURN GABLE RUSSES LOADING INFORMATION 40.0, Y DEPTH BASE O/A Y ID SPAN SPAN 1 01-01-00 FG1 05-06-00 05-06-0 M TYPE SIZE	SPECIAL INSTRUCTIONS:       Provide a structure       IC 28356       RTMENT       OVERHANG INFO       HEL HEIGHT       00-06-08       END CUT       RETURN       GABLE STUDS       24 IN. OC       RUSSES       LOADING INFORMATION       Y       DEPTH       BASE       Y       ID       SPAN       SPAN       LOADING       I       01-01-00       FG1       05-06-00       05-06-00       05-06-00       I       N TYPE       SIZE       LENGTH       FT-IN-16	SPECIAL INSTRUCTIONS:       SPECIAL INSTRUCTIONS:       SPECIAL INSTRUCTIONS:       SPECIAL INSTRUCTIONS:       SPECIAL INSTRUCTIONS:       SPECIAL INSTRUCTIONS:       OVERHANG INFO       END CUT RETURN       GABLE STUDS     24 IN. OC     JOBSITE     1       CUSSES     LOADING INFORMATION     TCLL-TCDL-BCLL-BCDL     STRESS INCR. 40.0,10.0,0.0,5.0     FLOOR TRUSS SF       Y     DEPTH     BASE     O/A     END TYPE     INT BEARING 475.8 lbs. 4 411.8 lbs. 3       1     OI-01-00     Joint 5     Joint 5       1     01-01-00     05-06-00     05-06-00     05-06-00     Joint 5     Joint 5       INT PE     SIZE     LENGTH FT-IN-16     PART NUMBER	SPECIAL INSTRUCTIONS:     SPECIAL INSTRUCTIONS:       RTMENT     OVERHANG INFO     HEEL HEIGHT     00-06-08     REQ. LAYOUTS     REQ. ENGINEERING       END CUT     RETURN     GABLE STUDS     24 IN. OC     JOBSITE     1     JOBSITE     1       RUSSES     LOADING INFORMATION     TCLL-TCDL-BCLL-BCDL     STRESS INCR. 40.0,10.0,0.0,5.0     1.00     FLOOR TRUSS SPACING: 24.0 IN. O.C       Y     DEPTH     BASE SPAN     O/A SPAN     END TYPE     INT BEARING LEFT     REACTIONS       1     01-01-00 FG1     05-06-00     05-06-00     Image: Construct of the stress of	SPECIAL INSTRUCTIONS:       PLAN SI       PLAN SIZE       <	SPECIAL INSTRUCTIONS:       PLAN SEAL DA       BY       COVERHANG INFO HEEL HEIGHT 00-06-08 REQ. LAYOUTS REQ. ENGINEERING QUOTE DTL       END CUT     RETURN     GABLE STUDS     24 IN. OC     JOBSITE     1     JOBSITE     1     LAYOUT     DTL       RUSSES     LOADING INFORMATION     TOLL-TCDL-BCIL-BCDL     STRESS INCR.     FLOOR TRUSS SPACING: 24.0 IN. O.C. (TYP.)       Y     DEPTH ID     BASE SPAN     O/A     END TYPE     INT BEARING SIZE     REACTIONS       1     01-01-00     05-06-00     05-06-00     05-06-00     Image: Size     Joint 5     Joint 8       411.8 lbs.     376.1 lbs.



Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0922-4866 Lot 38 Liberty Meadow

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I56917899 thru I56917908

My license renewal date for the state of North Carolina is December 31, 2023.

North Carolina COA: C-0844



March 1,2023

## Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job		Trus	ss			Trus	s Type					Qty	ł	Ply	Lot 3	8 Liberty	Meadow	w				1560	917899
J0922-4866		ET1				GAE	BLE					1		1	Joh R	eference	ontion	al)				150.	517055
Comtech, Inc,	Fayet	teville, I	NC - 28	314,							ID:a]	XuLo?r			n 6 202	22 MiTek	Industri	ies, Inc				2023 Pag fis35yipzfl	
0-1-8 H																						0-1- H	8
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						3x4 =	:	3x6 FP =						3x6	FP =								
1 2	3	4	5	6	7	8	9	10 11 12	13	14	15	16	17	1819	9 20	21	22	23	24	25	26	27 28	
																							28
56 55	54	53	52	51	50	49	48	47 46 45	44	43	42	41	40	39 3	38 37	36	35	34	33	32	31	30 29	
3x4 =							3x6 F							3x4								3x4	=
							3x4 =						3	x6 FP=	=								

32-11-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4-0 1-4

	3 (7,1)	[0.0-1-0,Luge], [20.0-1-0	,∟ugej, [30.0-	<sup>1-0</sup> ,⊏ugej, [+	5.0-1-0,∟uge							
LOADING (	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 4	40.0	Plate Grip DOL	1.00	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 1	10.0	Lumber DOL	1.00	BC	0.01	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.03	Horz(CT)	-0.00	38	n/a	n/a		
BCDL	5.0	Code IRC2015/T	PI2014	Matri	x-S						Weight: 147 lb	FT = 20%F, 11%E
LUMBER-						BRACING					·	
TOP CHORI	D 2x4 SF	PNo.1(flat)				TOP CHOP	RD	Structu	ral wood	sheathing d	irectly applied or 6-0-0 of	oc purlins,
BOT CHORI	D 2x4 SF	PNo.1(flat)						except	end verti	icals.		
WEBS	2x4 SF	P No.3(flat)				BOT CHOP	RD	Rigid c	eiling dire	ectly applied	or 6-0-0 oc bracing.	

REACTIONS. All bearings 32-11-0.

2x4 SP No.3(flat)

(lb) - Max Grav All reactions 250 lb or less at joint(s) 56, 29, 55, 54, 53, 52, 51, 50, 49, 48, 46, 45, 44, 43, 42, 41, 40, 38, 37, 36, 35, 34, 33, 32, 31, 30

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

OTHERS

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Job		Truss		Truss Typ	e			Qty	Ply	Lot 38 Libert	/ Meadow			1569179	000
J0922-4866		ET2		GABLE				1	1		(			1509173	900
Comtech, Inc,	Fayette	/ ville, NC - 28	314,				ID:aTX				<u>e (optional)</u> k Industries, Inc RkIAMJQVXnfL				
0- <u>1</u> -8													-,,		
														Scale = 1:	30.5
						3x4 =		3	x6 FP=					3x4	
1	2	3	4	5	6	7	8	9	10 11	12	13	14	15	16	
-4-00 00 00	•	•	•	•		- I		•		•		•	•	Ť	9
															1-4-0
														***	T
32	31	30	29	28	2726	25	24	23	22	21	20	19	18	17	
3x4 =					3x6 FP=	=	3x4 =							3x4	

1-4-0		3-0 <u>8-0-0</u> 9-4-0 1-0 1-4-0 1-4-0		2-0-0   13-4-0   14-8-0 1-4-0   1-4-0   1-4-0	<u>16-0-0</u> <u>17-4-0</u> <u>18-4-0</u> <u>1-4-0</u> <u>1-4-0</u> <u>1-0-0</u>
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrNOCode IRC2015/TPI2014	CSI. TC 0.07 BC 0.01 WB 0.03 Matrix-S	DEFL. i Vert(LL) n/ Vert(CT) n/ Horz(CT) 0.00	a - n/a 999	PLATES         GRIP           MT20         244/190           Weight: 84 lb         FT = 20%F, 11%E
BOT CHORD 2x4 SF	2 No.1 (flat) 2 No.1 (flat) 2 No.3 (flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 oc purlins, r 10-0-0 oc bracing.

REACTIONS. All bearings 18-4-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 32, 17, 31, 30, 29, 28, 27, 25, 24, 23, 22, 21, 20, 19, 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

OTHERS

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

2x4 SP No.3(flat)

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.





lob	Truss		Truss Type			Qty	Ply	Lot 38 Lib	erty Meadow	,		156917
10922-4866	ET3		GABLE			1	1	Job Refere	nce (optiona	l)		100917
Comtech, Inc, Faye	etteville, NC - 2831	4,			ID:aT)			6 2022 Mi	Tek Industrie	s, Inc. Wed I		48 2023 Page 1 9_KNa3nhzfKAr
0- <mark>1</mark> -8												
												Scale: 3/
					3x4	=	3x6 FP=					3x4
1 2	3 4	5	6	7 8	3 9	1(	) 11 1	2	13	14	15 f	16 17
-++ 												
34 33	32 31	30	29		26 25	24		23	22	21	20	19 18
3x4 =				3x6 FP =		3x4	=					3x4
<u>  1-0-0   2-4-0</u> 1-0-0   1-4-0		5-0-0		8-0 <u>9-0-0</u> 4-0 1-4-0	10-4-0	11-8-0 1-4-0	<u>13-0-0</u> 1-4-0	+ <u>14-4-0</u> 1-4-0	<u> </u>	17-0-0	18-4-0	19-4-8
Plate Offsets (X,Y)	9:0-1-8,Edge], [2	4.0-1-8 Edgel										

LOADING(psf)TCLL40.0TCDL10.0BCLL0.0BCDL5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.06 BC 0.01 WB 0.03 Matrix-S	DEFL. in Vert(LL) n/ Vert(CT) n/ Horz(CT) 0.0	a - a -	/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 89 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
BOT CHORD 2x4 SF	P No.1(flat) P No.1(flat)		BRACING- TOP CHORD	except en	d verticals.	rectly applied or 6-0-0	) oc purlins,

WEBS 2x4 SP No.3(flat) OTHERS 2x4 SP No.3(flat) BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS. All bearings 19-4-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 34, 18, 33, 32, 31, 30, 29, 27, 26, 25, 24, 23, 22, 21, 20, 19

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

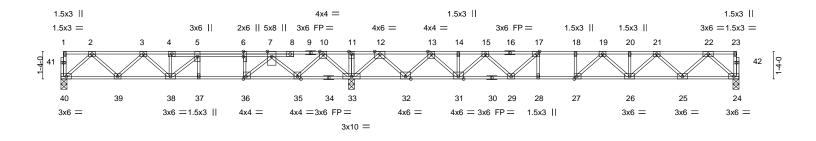
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.





Job	Truss	Truss Type	Qty	Ply	Lot 38 Liberty Meadow	
						156917902
J0922-4866	F1	Floor	7	1		
					Job Reference (optional)	
Comtech, Inc, Fay	etteville, NC - 28314,		8	3.430 s Jar	n 6 2022 MiTek Industries, Inc. Wed Mar 1	1 09:54:50 2023 Page 1
			ID:aTXuLo?nW09	qtpROz2V	VQ0wydkZW-o0QtpOLJoS9ECcj9e7r7skXc	JGilGJMHng3ArazfKAp
0-1-8						
<b>∦1-3-0</b>	2-1-4	L .			1-8-4	0-1-8
H		<u> </u>				Scale = 1:55.9



L	14-1-4					32-1			
Plate Offsets (X,	<u>14-1-4</u> Y) [6:0-3-0.0-0-0], [17:0-1-8,Edge], [27:0-1	-8 Edge] [36:0-1-8 Edge]				18-9	9-12		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.63	Vert(LL)	-0.20	27	>999	480	MT20	244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.82	Vert(CT)	-0.27		>820	360		
BCLL 0.0	Rep Stress Incr YES	WB 0.63	Horz(CT)	0.04	24	n/a	n/a		
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S						Weight: 180 lb	FT = 20%F, 11%E
LUMBER-			BRACING-						
	x4 SP No.1(flat)		TOP CHOR	D	Structu	ral wood	sheathing dire	ectly applied or 6-0-0 o	oc purlins,
BOT CHORD 2	x4 SP No.1(flat)				except	end verti	cals.	,	· · ·
WEBS 2	x4 SP No.3(flat)		BOT CHOR	D	Rigid c	eiling dire	ectly applied o	or 6-0-0 oc bracing.	
	(size) 40=0-3-8, 24=0-3-8, 33=0-3-8 Max Grav 40=662(LC 3), 24=882(LC 4), 33=2 Max. Comp./Max. Ten All forces 250 (lb) of 2-3=-1110/36, 3-4=-1659/212, 4-5=-1659/212 7-10=-392/1404, 10-11=0/2571, 11-12=0/257	r less except when shown 9, 5-6=-1602/621, 6-7=-16	602/621,						
	14-15=-1631/0, 15-17=-2564/0, 17-18=-2931 20-21=-2592/0, 21-22=-1586/0	, , ,	,						
BOT CHORD	39-40=0/704, 38-39=-106/1489, 37-38=-621/ 33-35=-1709/0, 32-33=-1291/0, 31-32=-200// 27-28=0/2931, 26-27=0/2856, 25-26=0/2193	935, 29-31=0/2220, 28-29	,						
WEBS	2-40=-935/0, 2-39=-59/564, 3-39=-528/98, 4 22-25=0/881, 21-25=-845/0, 21-26=0/542, 19 12-33=-1704/0, 12-32=0/1314, 10-33=-1399/ 6-36=-690/0, 13-32=-1288/0, 13-31=0/991,	9-26=-358/0, 19-27=-253/3 /0, 10-35=0/993, 7-35=-10	331, 033/0, 7-36=0/1214	·,					
,	por live loads have been considered for this d	esign.							1100

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

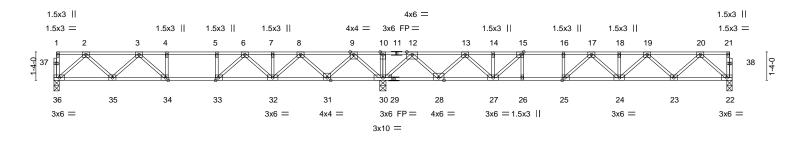
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.





Job	Truss	Truss Type	Qty	Ply	Lot 38 Liberty Meadow
					156917903
J0922-4866	F2	Floor	3	1	
					Job Reference (optional)
Comtech, Inc, Fayette	ville, NC - 28314,		8	3.430 s Jan	6 2022 MiTek Industries, Inc. Wed Mar 1 09:54:52 2023 Page 1
		ID:aTXul	.o?nW09q	tpROz2WC	Q0wydkZW-kPXeD3NZK3PyRvtYlYtbx9ctN4MlkEBaF_YHwSzfKAn
0-1-8					
H 1-3-0	2-2-12				1-9-12 0-1-8
					Scale = 1:54.



L	15-5-12		1		31-11-0		
	15-5-12				16-5-4		
Plate Offsets (X,	Y) [15:0-1-8,Edge], [25:0-1-8,Edge], [33:0-	1-8,Edgej, [34:0-1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	<b>CSI.</b> TC 0.85 BC 0.91 WB 0.54	Vert(LL) -0.18	(loc) l/def 24-25 >999 24-25 >776 22 n/a	9 480 6 360	<b>PLATES</b> MT20	<b>GRIP</b> 244/190
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S				Weight: 167 lb	FT = 20%F, 11%E
BOT CHORD 2	2x4 SP No.1(flat) 2x4 SP No.1(flat) 2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	except end ve	erticals.	ectly applied or 6-0-0 c r 6-0-0 oc bracing.	oc purlins,
REACTIONS.	(size) 36=0-3-0, 30=0-3-8, 22=0-3-0 Max Grav 36=719(LC 3), 30=2079(LC 1), 22=	795(LC 4)					
FORCES. (Ib) - TOP CHORD	Max. Comp./Max. Ten All forces 250 (lb) or 2-3=-1229/0, 3-4=-1941/0, 4-5=-1941/0, 5-6= 8-9=-241/659, 9-10=0/2218, 10-12=0/2218, 1 14-15=-1712/332, 15-16=-2291/0, 16-17=-22 19-20=-1395/0	-1941/0, 6-7=-1408/202, 7-  2-13=-523/817, 13-14=-17	12/332,				
BOT CHORD	35-36=0/769, 34-35=0/1668, 33-34=0/1941, 30-31=-1214/0, 28-30=-1129/0, 27-28=-547/24-25=0/2385, 23-24=0/1918, 22-23=0/852	,	,				
WEBS	2-36=-1022/0, 2-35=0/640, 3-35=-610/0, 3-3- 20-22=-1132/0, 20-23=0/755, 19-23=-728/0, 12-30=-1508/0, 8-31=-1027/0, 8-32=0/734, 6 12-28=0/1134, 13-28=-1085/0, 13-27=0/740,	19-24=-1/417, 17-25=-477/3 -32=-547/0, 6-33=0/628, 5-3	33, 33=-320/0,				
,	oor live loads have been considered for this de	esign.					

2) All plates are 3x4 MT20 unless otherwise indicated.

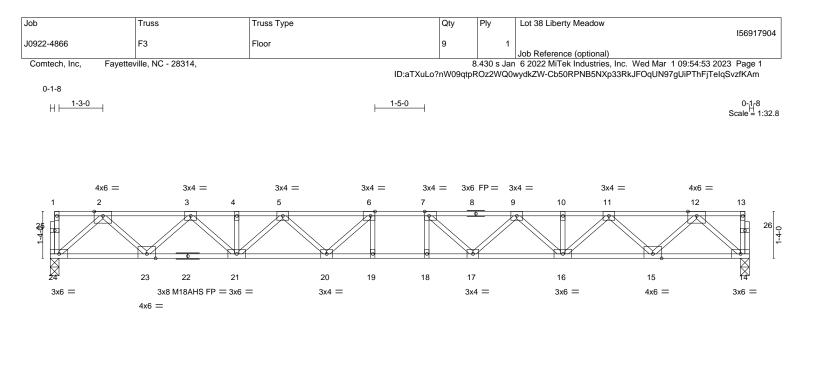
3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.







<b> </b>			<u>19-11-0</u> 19-11-0			
Plate Offsets (X,Y)	[6:0-1-8,Edge], [7:0-1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.56 BC 0.95 WB 0.55 Matrix-S	Vert(LL) -0.33	n (loc) l/defl L/d 3 18-19 >724 480 5 18-19 >526 360 8 14 n/a n/a	PLATES MT20 M18AHS Weight: 106 lb	<b>GRIP</b> 244/190 186/179 FT = 20%F, 11%E
LUMBER-       TOP CHORD     2x4 SP No.1(flat)       BOT CHORD     2x4 SP No.1(flat)       WEBS     2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o		oc purlins,
REACTIONS. (size Max G	e) 24=0-3-0, 14=0-3-0 rav 24=1075(LC 1), 14=1075(LC 1)					
TOP CHORD 2-3=-	Comp./Max. Ten All forces 250 (lb) or 2005/0, 3-4=-3408/0, 4-5=-3408/0, 5-6= 3408/0, 10-11=-3408/0, 11-12=-2005/	-4160/0, 6-7=-4391/0, 7-9=				

	3-103-00/0, 10-113-00/0, 11-122003/0
BOT CHORD	23-24=0/1172, 21-23=0/2810, 20-21=0/3918, 19-20=0/4391, 18-19=0/4391, 17-18=0/4391,
	16-17=0/3918, 15-16=0/2810, 14-15=0/1172
WEBS	2-24=-1557/0, 2-23=0/1160, 3-23=-1119/0, 3-21=0/814, 5-21=-693/0, 5-20=0/469,
	6-20575/87 12-141557/0 12-15-0/1160 11-151119/0 11-16-0/814 9-16693/0

<sup>1557/0, 12-15=0/1160, 11-15=-1119/0, 11-16=0/814, 9-16=-693/0,</sup> 575/87 9-17=0/469, 7-17=-575/87

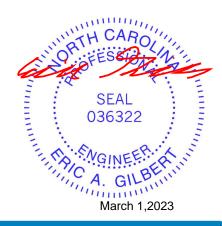
#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

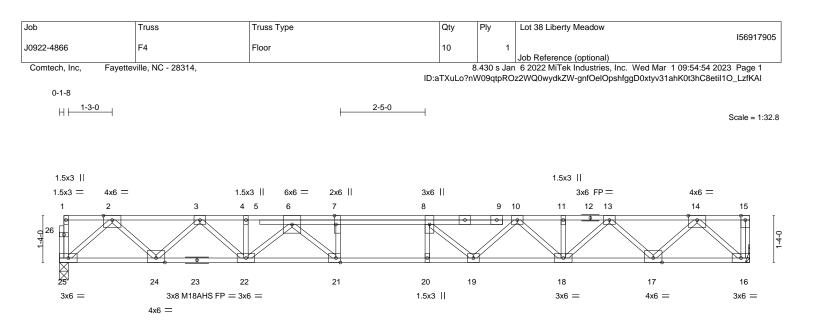
All plates are 1.5x3 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.
5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



			19-8-0 19-8-0						
Plate Offsets (X,Y	[7:0-3-0,Edge], [21:0-1-8,Edge]								
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	<b>CSI.</b> TC 0.39 BC 0.88 WB 0.54	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in -0.29 -0.40 0.08	(loc) 20 20 16	l/defl >806 >587 n/a	L/d 480 360 n/a	PLATES MT20 M18AHS	<b>GRIP</b> 244/190 186/179
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S						Weight: 112 lb	FT = 20%F, 11%E
LUMBER- TOP CHORD 2x	4 SP No.1(flat)		BRACING- TOP CHOR		Structu	ural wood	l choothing di	rectly applied or 6-0-0 c	
	4 SP No.1(flat)		TOP CHOR	U		end vert	0	rectly applied of 0-0-0 C	je pullins,
WEBS 2x	4 SP No.3(flat)		BOT CHOR	D	Rigid c	eiling dir	ectly applied of	or 10-0-0 oc bracing.	
REACTIONS. M	(size) 25=0-3-0, 16=Mechanical ax Grav 25=1062(LC 1), 16=1068(LC 1)								
( )	Max. Comp./Max. Ten All forces 250 (lb) o								
	2-3=-1976/0, 3-4=-3346/0, 4-6=-3350/0, 6-7=  0-11=-3354/0, 11-13=-3354/0, 13-14=-1976	, , ,	10=-4140/0,						
BOT CHORD	24-25=0/1156, 22-24=0/2765, 21-22=0/3951 17-18=0/2764, 16-17=0/1157 2-251537(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241097(0, 2-24-0/1140, 3-241007(0, 2-24-0/1140, 3-241007(0, 2-24-0/1140, 3-2410)))))	, 20-21=0/4437, 19-20=0/	,	23,					

WEBS 2-25=-1537/0, 2-24=0/1140, 3-24=-1097/0, 3-22=0/790, 6-22=-803/0, 6-21=0/970, 7-21=-557/0, 14-16=-1541/0, 14-17=0/1139, 13-17=-1096/0, 13-18=0/802, 10-18=-637/0, 10-19=0/581, 8-19=-621/0

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 3x4 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Refer to girder(s) for truss to truss connections.

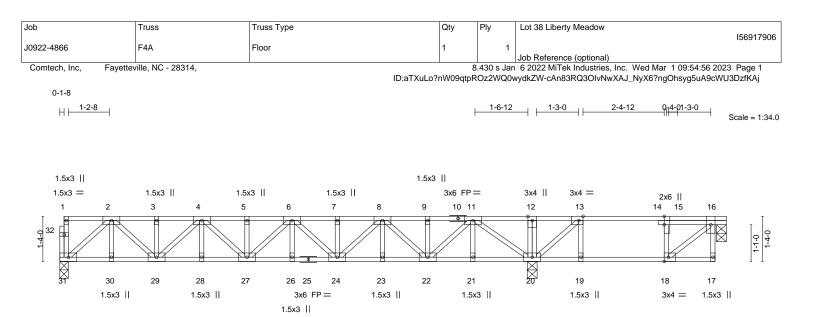
6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

7) CAUTION, Do not erect truss backwards.







1		13-11-4			1-6-0 1	I-2-6 <sup>'</sup> 1-2-6 <sup>'</sup> 1-	6-0 0-4-0
Plate Offsets (X,Y)	[13:0-1-8,Edge], [15:0-3-0,0-0-0], [18:0-	1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.40 BC 0.38 WB 0.37 Matrix-S	DEFL. i Vert(LL) -0.00 Vert(CT) -0.1 Horz(CT) 0.00	8 26 > 1 26 >	/defl L/d 999 480 999 360 n/a n/a	PLATES MT20 Weight: 115 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER-TOP CHORD2x4 SP No.1(flat)BOT CHORD2x4 SP No.1(flat)WEBS2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	except en Rigid ceili	d verticals.	rectly applied or 6-0-0 o or 10-0-0 oc bracing, 9.	•

REACTIONS. (size) 31=0-3-0, 16=0-3-8, 20=0-3-8 Max Grav 31=730(LC 8), 16=275(LC 4), 20=1157(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1331/0, 3-4=-1331/0, 4-5=-1969/0, 5-6=-1969/0, 6-7=-1961/0, 7-8=-1961/0, 8-9=-1306/0, 9-11=-1306/0, 11-12=0/365, 12-13=0/364, 13-15=-276/99, 15-16=-253/96

 BOT CHORD
 30-31=0/775, 29-30=0/775, 28-29=0/1732, 27-28=0/1732, 26-27=0/2047, 24-26=0/2047, 23-24=0/1717, 22-23=0/1717, 21-22=0/750, 20-21=0/750, 19-20=-96/253, 18-19=-96/253

 WEBS
 16-18=-125/330, 2-31=-1021/0, 2-29=0/751, 4-29=-541/0, 4-27=0/319, 8-24=0/345, 8-22=-569/0, 11-22=0/769, 11-20=-1229/0, 13-20=-575/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x6 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

13-11-4

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

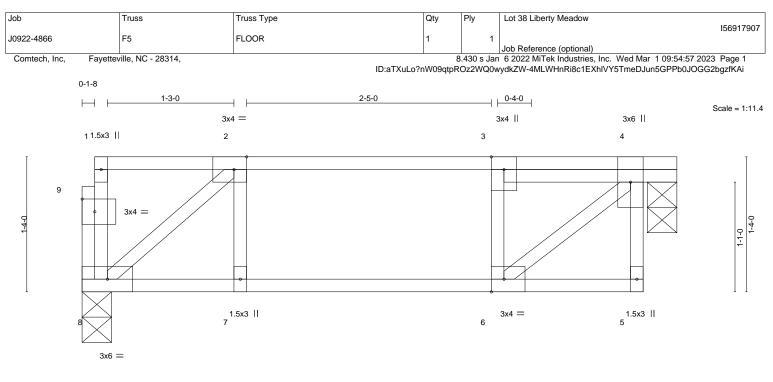
6) CAUTION, Do not erect truss backwards.



16-7-10 17-10-0 19-4-0 19-8<sub>1</sub>0

15-5-4





		5-6- 5-6-			<u> </u>	
Plate Offsets (X,Y)	[2:0-1-8,Edge], [6:0-1-8,Edge], [9:0-1-8	,0-1-8]				
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.23 BC 0.11 WB 0.18 Matrix-S	DEFL.         ir           Vert(LL)         -0.01           Vert(CT)         -0.01           Horz(CT)         -0.01	6 >999 480 6 >999 360	PLATES MT20 Weight: 32 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER-TOP CHORD2x4 SP No.1(flat)BOT CHORD2x4 SP No.1(flat)WEBS2x4 SP No.3(flat)			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di except end verticals. Rigid ceiling directly applied	<i>y</i> 11	8 oc purlins,

REACTIONS. (size) 8=0-3-8, 4=0-3-8 Max Grav 8=288(LC 1), 4=294(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-294/0, 3-4=-296/0

 BOT CHORD
 7-8=0/294, 6-7=0/294

 WEBS
 2-8=-382/0, 4-6=0/383

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

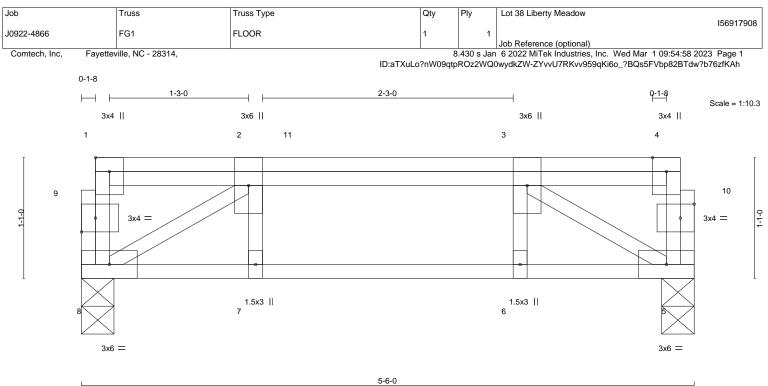
Strongbacks to be attached to walls at their outer ends or restrained by other means.

4) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

5) CAUTION, Do not erect truss backwards.







I			5-6-0	1
Plate Offsets (X,Y)	[1:Edge,0-1-8], [9:0-1-8,0-1-8], [10:0-1-	8,0-1-8]		
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL 40.0	Plate Grip DOL 1.00	TC 0.12	Vert(LL) -0.01 6 >999 480 MT20 244/19	90
TCDL 10.0	Lumber DOL 1.00	BC 0.17	Vert(CT) -0.01 6-7 >999 360	
BCLL 0.0	Rep Stress Incr NO	WB 0.19	Horz(CT) 0.01 5 n/a n/a	
BCDL 5.0	Code IRC2015/TPI2014	Matrix-S	Weight: 34 lb FT	= 20%F, 11%E
LUMBER-			BRACING-	
TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)			TOP CHORD Structural wood sheathing directly applied or 5-6-0 oc purl except end verticals.	ins,

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 8=0-3-8, 5=0-3-8 Max Grav 8=455(LC 1), 5=476(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-683/0

BOT CHORD 7-8=0/683, 6-7=0/683, 5-6=0/683

2x4 SP No.3(flat)

2-8=-802/0, 3-5=-801/0 WEBS

#### NOTES-

WEBS

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

# LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 5-8=-10. 1-4=-100 Concentrated Loads (lb)

