MORGAN





PLAN ID 042920.0601

QUALITY | INTEGRITY | VALUE

110 VILLAGE TRAIL SUITE 215 WOODSTOCK, GA. 30188

DRAWING INDEX COVER SHEET A0.0 FRONT ELEVATIONS A1.1 A2.1 SIDE & REAR ELEVATIONS SLAB FOUNDATIONS A3.1 FIRST FLOOR PLANS A5.1 A5.2 SECOND FLOOR PLAN A6.1 ROOF PLANS A7.2-A7.3 **ELECTRICAL PLANS**

AREA TABULATION			
FIRST FLOOR	1024		
SECOND FLOOR	1376		
TOTAL	2400		
GARAGE	416		
FRONT PORCH (COVERED)	144		
REAR PATIO (COVERED)	120		

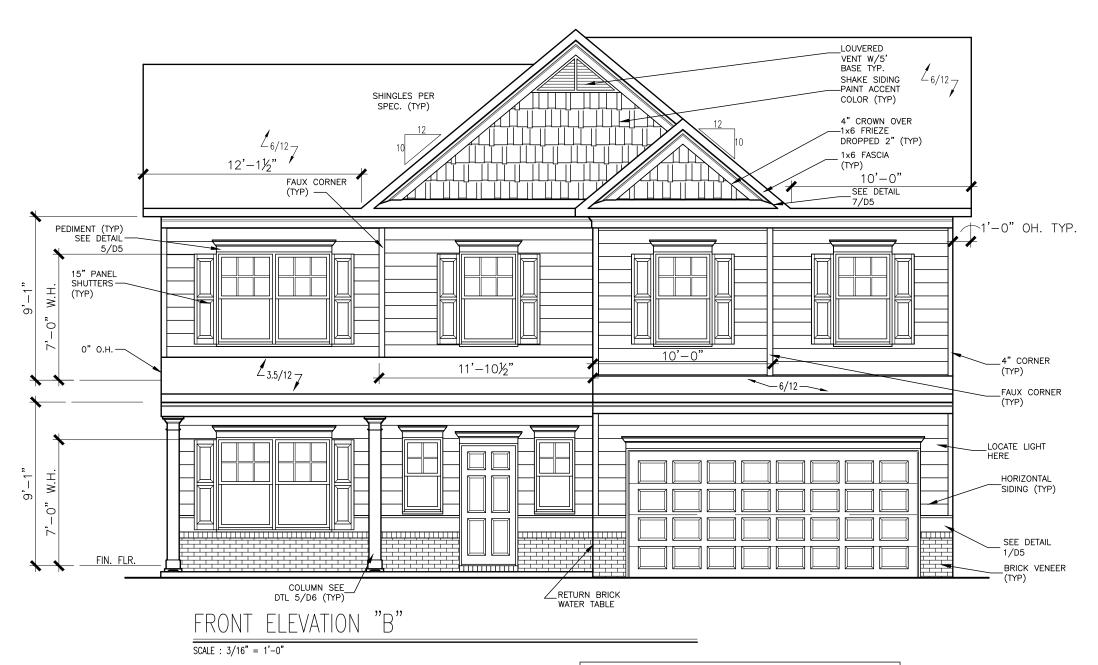
GOVERNMENTAL CODES & STANDARDS

HOME TO BE BUILT TO CONFORM TO ALL APPLICABLE LOCAL CODES, PRACTICES AND STANDARDS

BUILDING CODE ANALYSIS / DESIGN CRITERIA

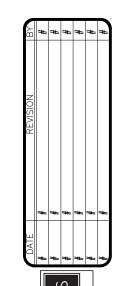
HOME TO BE BUILT TO MEET OR EXCEED ALL LOCAL CODES AND DESIGN CRITERIA

PLAN REVISIONS				
DATE	BY	REVISION PAGE #		
4/29/2020	AW	PROTOTYPE WALK CHANGES - SEE REVISION SHEET	ALL	
11/5/2020	MM	Removed overhang at front patio	A1.1-A1.9, A6.1-A6.3	
3/1/2021	AW	Relocated plumbing drain to chase between stairs and kitchen cabinets	A5.1	
6/1/2021	MM	Changed hall bath vanity from 42" vanities to 36" vanities	A5.2, A7.3	



ALL NON-MASONRY RETURNS TO BE HORIZONTAL SIDING

SEE SHEET D3 OF SDH TYPICAL DETAILS FOR SOFFIT DETAILS PER SOFFIT MATERIAL



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ELEVATIONS FRONT ELEVATION MORGAN

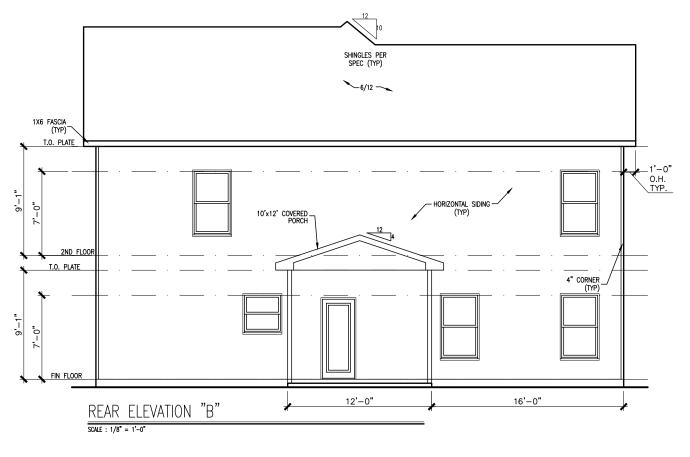
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SHINGLES PER SPEC (TYP) <u>_10/12</u> 1X6 FASCIA - HORIZONTAL SIDING -(TYP) Z_{4/12}7 LEFT ELEVATION "B" SCALE: 1/8" = 1'-0"

DUNCANS CROSSING LOT 27

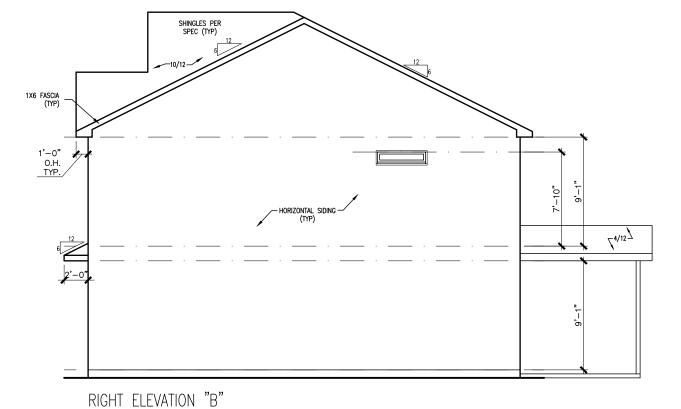


SMITH DOUGLAS HOMES & REAR MORGAN ELEVATIONS

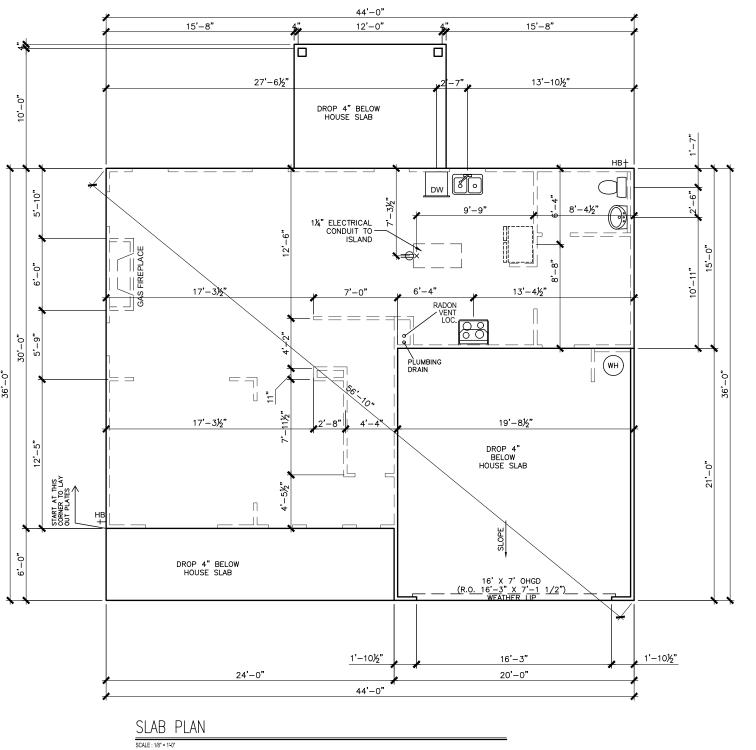
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SIDES

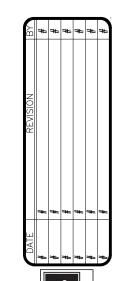




SCALE : 1/8" = 1'-0"



REFER TO DETAIL 3/D1 FOR BRICK LEDGE DETAIL WHEN BRICK VENEER IS CHOSEN

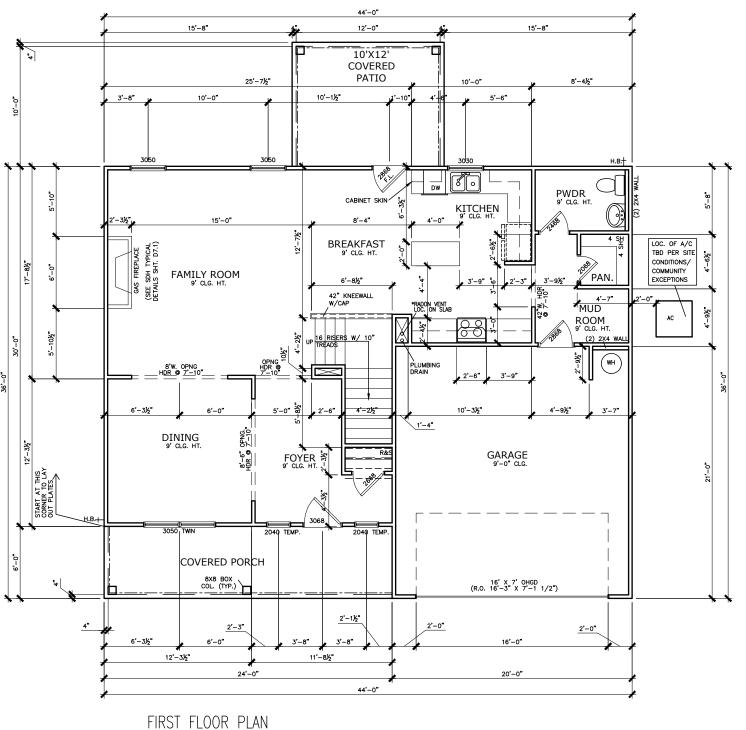


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FOUNDATION PLAN SLAB PLAN MORGAN

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SCALE : 1/8" = 1'-0"

*RADON VENT PROVIDED PER LOCAL CODE

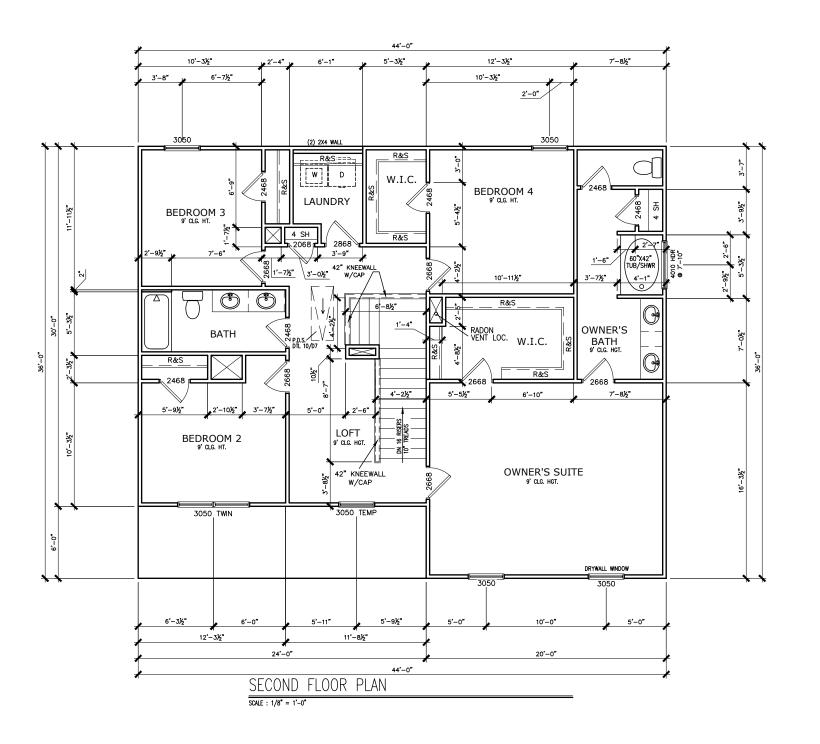
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FLOOR PLAN FIRST FLOOR MORGAN

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PLAN
FLOOR
SMITH DOUGLAS HOMES
##
CAN

FLOOR PLAN SECOND FLOO MORGAN

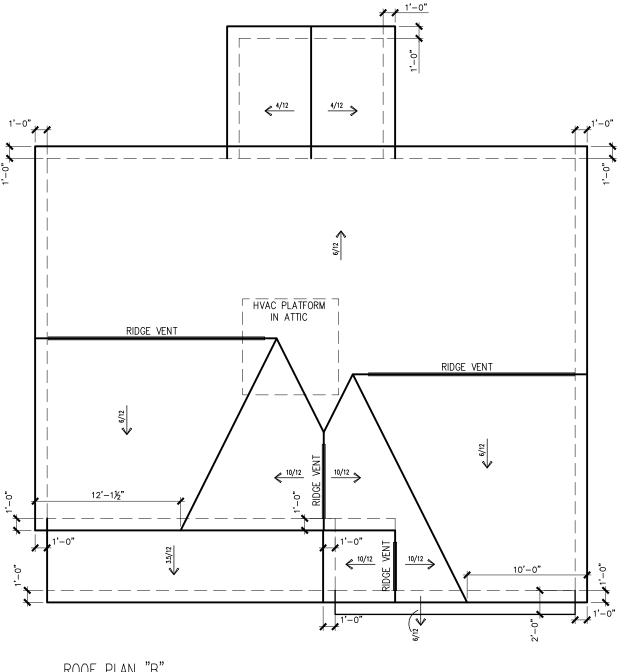
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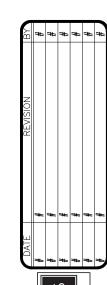
*RADON VENT PROVIDED PER LOCAL CODE

REFER TO MANUFACTURER'S SPECS. FOR DRAIN LOCATIONS ON DETAIL SHEETS D12, D12.1, D12.2 & D12.3



ROOF PLAN "B"

SCALE: 1/8" = 1'-0"



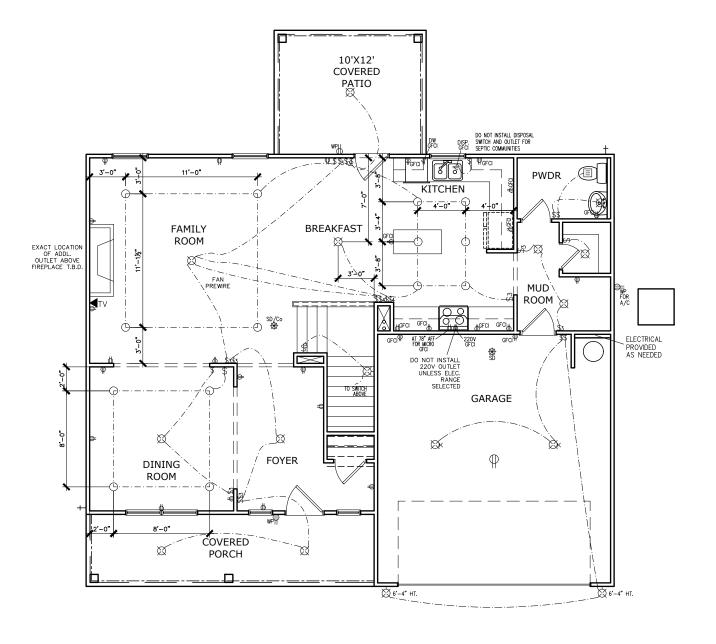


ROOF PLAN ROOF LAYOUT MORGAN

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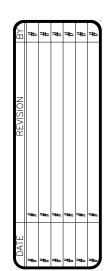


FIRST FLOOR ELECTRICAL PLAN

CALE : 1/8" = 1'-0"

ELE	ectrical l	EGE	ND	
\$	SWITCH	TV ▼	TV	
\$3	3 WAY SWITCH	φ	120V RECEPTACLE	
\$4	4 WAY SWITCH	•	120V SWITCHED RECEPTACLE	
Ø	CEILING FIXTURE	Φ	220V RECEPTACLE	
- \(\big _{K}	KEYLESS	P _{GFCI}	GFCI OUTLET	
M	WALL MOUNT FIXTURE	PAFCI	ARCH FAULT CIRCUIT	
0	CEILING FIXTURE	T _{GL}	GAS LINE	
•	FLEX CONDUIT	T _{wL}	WATER LINE	
СН	CHIMES	¥	HOSE BIBB	
PH	TELEPHONE	8	FLOOD LIGHT	
SD/Co	SMOKE DETECTOR & CARBON MONOXIDE		1x4 LUMINOUS FIXTURE	
SO	SECURITY OUTLET			
	GARAGE DOOR OPENER		CEILING FAN	
≣	EXHAUST FAN		ELECTRICAL WIRING	
0	FAN/LIGHT		CEILING FIXTURE	
ELEC ⁻	FRICAL PLANS TO FOLLOW	ALL LOCAL	CODES	
APPRO	X. FIXTURE HGTS (MEASUR	ED FROM B	OTTOM OF FIXTURE)	
BREA	KFAST/DINING ROOM	63" ABO	VE FINISHED FLOOR	
KITCHEN PENDANT LIGHTS		33" ABO	VE COUNTER TOP	
TWO	STORY FOYER FIXTURE	96" ABOVE FINISHED FLOOR		
CELLIN	NG FAN	96" ABO	VE FINISHED FLOOR	

NOTE: FINAL PLACEMENT OF PHONE/CABLE T.B.D. ON SITE BY THE BUILDER



SMITH DOUGLAS HOME

ELECTRICAL PLAN FIRST FLOOR MORGAN

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BEDROOM 3 LNDRY W.I.C. BEDROOM 4 W.I.C. OWNER'S BATH SIJON OWNER'S SUITE SIJON PH

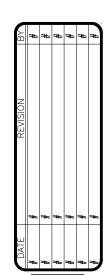
SECOND FLOOR ELECTRICAL PLAN

SCALE : 1/8" = 1'-0"

DUNCANS CROSSING LOT 27

ELE	ELECTRICAL LEGEND				
\$	SWITCH	TV.	TV		
\$3	3 WAY SWITCH	Ф	120V RECEPTACLE		
\$4	4 WAY SWITCH		120V SWITCHED RECEPTACLE		
Ø	CEILING FIXTURE	0	220V RECEPTACLE		
- ∳ _K	KEYLESS	P _{GFCI}	GFCI OUTLET		
+XX	WALL MOUNT FIXTURE	PAFCI	ARCH FAULT CIRCUIT INTERRUPTER		
0	CEILING FIXTURE	† _{GL}	GAS LINE		
•	FLEX CONDUIT	† _{wL}	WATER LINE		
СН	CHIMES	¥	HOSE BIBB		
PH	TELEPHONE	8	FLOOD LIGHT		
SD/Co	SMOKE DETECTOR & CARBON MONOXIDE		1x4 LUMINOUS FIXTURE		
SO	SECURITY OUTLET		050000 500		
	GARAGE DOOR OPENER	X	CEILING FAN		
	EXHAUST FAN		ELECTRICAL WIRING		
<u> </u>	FAN/LIGHT		CEILING FIXTURE		
ELECT	ELECTRICAL PLANS TO FOLLOW ALL LOCAL CODES				
APPRO:	APPROX. FIXTURE HGTS (MEASURED FROM BOTTOM OF FIXTURE)				
BREAK	KFAST/DINING ROOM	63" ABO	VE FINISHED FLOOR		
KITCH	KITCHEN PENDANT LIGHTS		VE COUNTER TOP		
TWO S	STORY FOYER FIXTURE	96" ABOVE FINISHED FLOOR			
CEILIN	IG FAN	96" ABOVE FINISHED FLOOR			

NOTE: FINAL PLACEMENT OF PHONE/CABLE T.B.D. ON SITE BY THE BUILDER



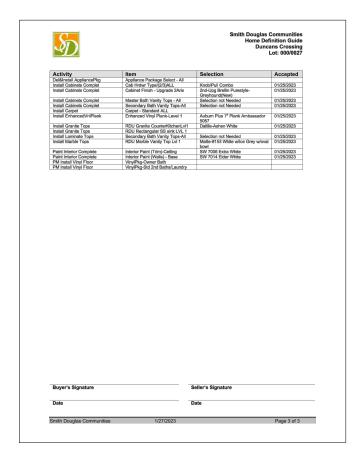
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SECOND FLOOR
MORGAN

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			D	Definition Guide Juncans Crossing Lot: 000/0027
Option Name	Option Description	Price	Qty	Ext Price
FIPkg 5EA-EVP1, StdCpt (f/Pkg1)	Flooring Package 5EA - Enhanced Vinvl Plank 1, Standard Carpet (from		1	
FirPkg Opt-Powder EVP1	Package 1) Flooring Package - Option Powder Room Enhanced Vinyl Plank 1	l H	-1	H
Paint Interior Ceiling White	Room Enhanced Vinyl Plank 1	l H	_	H
Dining Room Ceiling Fixture	Dining Room Lights - Low Profile Flush	l H		H
Lights	Mount LED Lights. Family/Great Room Lights - Low Profile		1	
Family/Great Room Ceiling	Family/Great Room Lights - Low Profile	1 [1	
Fixture Lights Kitchen Ceiling Fixture Lights	Flush Mount LED Lights per plan. Kitchen Lights - Low Profile Flush	l H	-1	H
ILO Std	Mount LED Lights per Plan ILO		- 1	
Addtl Standard Interior Electric	Standard Light. Standard Interior Duplex Outlet. This is	1 H		H
Outlet	not an exterior weather proof outlet or a		- 1	
	GFCI protected outlet. A quantity of 1	1		
	gives you one additional outlet.	1 P		μ
		1		
		1		
Buyer's Signature		Seller's Signature		
		Date		
Date				
	·			







DETAILS LOT DEFINITION MORGAN

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DESIGN SPECIFICATIONS:

Construction Type: Commerical ☐ Residential ☒

Applicable Building Codes:

- 2018 North Carolina Residential Building Code
- ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

Design Loads:

1.	Roof

l.l Live	20 PSF
12 Dead	10 PSF
1.3 Snow	15 PSF
1.3.1 Importance Factor	1.0
2. Floor Live Loads	
2.1 Typ. Dwelling	40 PSF
2.2 Sleeping Areas	30 PSF
2.3 Balconies (exterior) and Decks	40 PSF
2.4 Garage Parking	50 PSF
3. Floor Dead Loads	
3.1 Conventional 2x	10 PSF
3.2 -Joist	15 PSF
3.3 Floor Truss	15 PSF
4. Ultimate Wind Speed (3 sec. gust)	130 MPH
4.1 Exposure	В
4.2 Importance Factor	1.0

4.3 Wind Base Shear 4.3.1 Vx = 4.3.2 Vy =

5. Component and Čladding (in PSF)

MEAN ROOF HT.	UP TO 30'	3Ø'1"-35'	35'1"-40'	40'1"-45'
ZONE 1	16.7,-18.0	17.5,-18.9	18.2,-19.6	18.7,-20.2
ZONE 2	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 3	16.7,-21.0	17.5,-22.1	18.2,-22.9	18.7,-23.5
ZONE 4	18.2,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
ZONE 5	18.2,-24.0	19.2,-25.2	19.9,-26.1	20.4,-26.9

6. Seismic

6.1 Site Class	D
62 Design Category	С
6.3 Importance Factor	10
64 Seismic Use Group	1

6.5 Spectral Response Acceleration

6.5.1 Sms = %g 6.5.2 Sml = %g

6.6 Seismic Base Shear

6.6.1 Vx =

6.6.2 Vy =

6.7 Basic Structural System (check one)

☑ Bearing Wall☑ Building Frame

☐ Moment Frame

☐ Dual w/ Special Moment Frame

☐ Dual w/ Intermediate R/C or Special Steel

☐ Inverted Pendulum

6.8 Arch/Mech Components Anchored?_____No

...... 2000psf



STRUCTURAL PLANS PREPARED FOR:

MORGAN

PROJECT ADDRESS: TBD OWNER:

Smith Douglas Homes - Raleigh 2520 Reliance Ave Apex, NC 27539

ARCHITECT/DESIGNER:

Smith Douglas Homes 110 Village Trail, Suite 215 Woodstock, GA 30188

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineer of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory 4 Testing, P.C. before construction begins.

PLAN ABBREVIATIONS:

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
ΕW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
OC	ON CENTER	TYP	TYPICAL
PSF	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	wwF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory 4 Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by SMITH DOUGLAS HOMES. Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

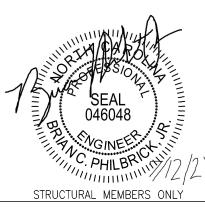
SHEET LIST:

Sheet No.	Description
CS1	Cover Sheet, Specifications, Revisions
C52	Specifications Continued
S1.Øm	Monolithic Slab Foundation
S1.Øs	Stem Wall Foundation
S1.0c	Crawl Space Foundation
S1.0b	Basement Foundation
S2.Ø	Basement Framing Plan
S 3.Ø	First Floor Framing Plan
S4.Ø	Second Floor Framing Plan
S5.Ø	Roof Framing Plan
S6.Ø	Basement Bracing Plan
57.Ø	First Floor Bracing Plan
58.Ø	Second Floor Bracing Plan

REVISION LIST:

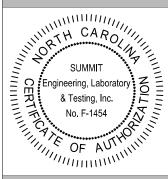
Revision No.	Date	Project No.	Description
1	11/1/19	305R	Revised notes
2	3/4/20	328	Revised to add 2x6 wall note with elevations B,E,H & brick
3	12/8/20	328R	Revised BEH per new roof truss layout
4	12/10/20	328R2	Fixed graphical errors
5	12/15/20	328R3	Revised footings for the optional decks
6	1/8/21	3832.T0548	Add elevations C,F,I to First Floor Framing over garage & added elevations A,D,G,&C,F,I in the OPT. side load garage
7	7/1/21	T0548	Added LIB Bracing Options

Duncans Lot 27





3070 Hammond Business Place Suite 171, RALEIGH, NC 27603 OFFICE: 919.380.9991 FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM



Coversheet client Smith Douglas Homes - Raleig 2520 Reliance Ave Apex, NC 21539

CURRENT DRAWING

ROJECT Morgan LH

DATE: Ø1/1/2Ø21

SCALE: 1/8"=1'-0"

PROJECT *: 3832.TØ548

DRAWN BY: DGT

CHECKED BY: BCP

ORIGINAL DRAWING

 DATE
 PROJECT ●

 IØ/29/2ØI9
 3832.TØ548

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEE1

CS1

GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.
- The structure is only stable in its completed form. The
 contractor shall provide all required temporary bracing
 during construction to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- 5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- This structure and all construction shall conform to all applicable sections of the international residential code.
- 8. This structure and all construction shall conform to all applicable sections of the 2018 North Carolina Residential Code (NCRC) and any local codes or restrictions

FOUNDATIONS:

- Foundations shall be constructed in accordance with chapter 4
 of the 2018 NC Residential Building Code (Special
 consideration shall be given to Chapter 45 in wind zones
 above 130mph)
- Footing sizes based on a presumptive soil bearing capacity of 2000 PSF. Contractor is solely responsible for verifying the suitability of the site soil conditions at the time of construction
- Maximum depth of unbalanced fill against masonry walls to be as specified in section R404.1 of the 2018 NCRC
- The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
 The bottom of all footings shall extend below the frost line
- for the region in which the structure is to be constructed.

 However, the bottom of all footings shall be a minimum of 12" below grade.
- Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
- Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- 8. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.
- Each crawl space pier shall bear in the middle third of its respective footing and each girder shall bearing in the middle third of the piers. Pilasters to be bonded to perimeter foundation wall
- IO. Crawl spaced to be graded level and clear of all debris
 II. Provide foundation waterproofing and drain with positive slope to outlet as required by site conditions
- 12. Energy efficiency compliance and insulation of the structure to be in accordance with chapter 11 of the 2018 NCRC

CONCRETE:

- Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:
- 3.1. Footings: 5%
- 3.2. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER
- Concrete slabs on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab Construction"
- 6. The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.
- 7. Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
- Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- Reinforcing steel may not extend through a control joint.
 Reinforcing steel may extend through a saw cut joint.
- 10. All welded wire fabric (WWF.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The WWF. shall be securely supported during the concrete pour. Fibermesh may be used in lieu of WWF.

CONCRETE REINFORCEMENT:

- Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.1% by volume (1.5 pounds per cubic yard)
- Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry standard.
- Steel Reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"
- Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice.
- Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement.
 The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

WOOD FRAMING:

- I. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Spruce-Pine-Fir (SPF) #2.
- LYL or PSL engineered wood shall have the following minimum design values:
- 2.1. E = 1,900,000 psi
- 2.2. Fb = 2600 psi
- .3. Fv = 285 psi
- 2.4. Fc = 700 psi
- Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- 4. Nails shall be common wire nails unless otherwise noted.
- Lag screws shall conform to ANSI/ASME standard BI82.1-1981.
 Lead holes for lag screws shall be in accordance with NDS specifications.
- All beams shall have full bearing on supporting framing members unless otherwise noted.
- . Exterior and load bearing stud walls are to be 2x4 SPF*2 @16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- . Individual studs forming a column shall be attached with one 10d nail @6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be fully blocked at all floor levels to ensure proper load transfer.
- 9. Multi-ply beams shall have each ply attached wth (3) 10d nails @ 24" OC
- IO. Flitch beams and four and five ply beams shall be bolted together with (2) rows of 1/2" dia. through bolts staggered @24" O.C. w/ 2" edge distance and (2) bolts located at 6" from each end, unless noted otherwise.

1000 TRUSSES:

- The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- P. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design Specification for Metal Plate Connected Wood Trusses."
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

WOOD STRUCTURAL PANELS:

- Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards.
- All structurally required wood sheathing shall bear the mark of the APA.
- . Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- 4. Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- b. Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T4G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

STRUCTURAL FIBERBOARD PANELS:

- Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards.
- Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

EXTERIOR WOOD FRAMED DECKS:

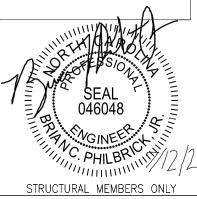
Duncans

Lot 27

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.
 STRUCTURAL STEEL:

Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and of the manual of Steel Construction "Load Resistance Factor Design" latest editions.

- . All steel shall have a minimum yield stress (Fy) of 36 ksi unless otherwise noted.
- B. Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS DI.I. Electrodes for shopt and field welding shall be class ETOXX. All welding shall be performed by a certified welder per the above standards.





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Coversheet CLIENT Smith Douglas Homes - Raleigh 2520 Reliance Ave Apex, NC 21539

CURRENT DRAWING

Yorgan LH

DATE: Ø7/1/2Ø21

SCALE: 1/8"=1'-@"

PROJECT *: 3832.TØ548

DRAWN BY: DGT

CHECKED BY: BCP

ORIGINAL DRAWING

DATE PROJECT * 10/29/2019 3832.T0548

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

CS2

FOUNDATION NOTES:

- FOUNDATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL
- APENDRENS
 APENDR
- BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE CODE B-PORCEPHEN OFFICIAL.

 4. FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000 PSF. CONTRACTOR IS SOLELY RESPONSIBLE FOR VERFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.

 5. FOOTINGS AND PIERS SHALL BE CENTERED INDER THEIR RESPECTIVE ELEMENTS, PROVIDE 2" WINNINGTH FOOTING FROOTING FOR THE FACE OF MASONRY.

 6. MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN SECTION RROAD OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE

- SPECIFIED IN SECTION RADAL OF THE 2008 NORTH CAPOLINA RESIDENTIAL BUILDING CODE.

 PILASTERS TO BE BONDED TO PERINETER FOUNDATION WALL.

 PROVIDE FOADDATION WINEPROCOPING, AND DRAIN WITH POSITIVE SLOPE TO
 OUTLET AS REQUIRED BY SITE CONDITIONS.

 PROVIDED FERMETER INSULATION FOR ALL FOUNDATIONS PER 2018 NORTH
 CAROLINA RESIDENTIAL BUILDING CODE.

 CORREL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK
 VENERS.

- VENEERS,

 1. CRAILL SPACE TO BE GRADED LEVEL, AND CLEARED OF ALL DEBRIS.

 12. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED FER THE 20% NORTH CARCLINA RESIDENTIAL CODE SECTION REGISLEN HITMIN MY DIA BOLTS SPACED AT 6-0" ON CENTER WITH A 1" MINIMIN EMBEDMENT INTO MASONITY OR CONCRETE. ANCHOR BOLTS SHALL BE 1" FROM THE END OF EACH PLATE SECTION MINIMIN (2) ANCHOR BOLTS FER PLATE SECTION, ANCHOR BOLTS SHALL BE LOCATED IN THE CENTER THIRD OF THE PLATE.

 13. ABBREVIATIONS.
- SJ = SINGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER TR = TRIPLE RAFTER DJ = DOUBLE JOIST GT = GIRDER TRUSS 9C = 9TUD COLUMN EE = EACH END TJ = TRIPLE JOIST CL = CENTER LINE OC = ON CENTER PL = POINT LOAD

- ALL PIERS TO BE 16"x16" MASONRY AND ALL PILASTERS TO BE 8"x16" MASONRY, TYPICAL. (INKO)

 5. WALL FOOTINGS TO BE CONTINUOUS CONCRETE, SIZES PER STRUCTURAL PLAN.

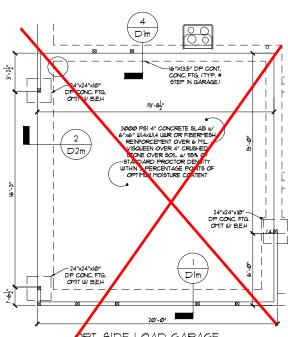
 6. A FOUNDATION EXCANATION DESERVATION SHOULD BE CONDUCTED BY A PROFESSIONAL GEOTECHNICAL. ENGINEER, OR HIS GUALIFIED EXPRESSIONAL GEOTECHNICAL. ENGINEER, OR HIS GUALIFIED EXPRESSIONAL GEOTECHNICAL. ENGINEER, OR HIS GUALIFIED EXPRESSIVATIVE. FINGOLATED AREAS OF YIELDING MATERIALIS AND/OR POTENTIALLY EXPANSIVE SOLIS ARE OBSERVED IN THE FOOTING EXCANATIONS AT THE TIME OF CONSTITUTING TO SMITHERING, LABORATORY AT TIME OF CONSTITUTING TO SMITHERING, LABORATORY AT TISTING, P.C. MUST BE PROVIDED THE OPPORTINITY TO REVIEW THE FOOTING DESIGN PRIOR TO CONCRETE IT ALCHEMIT.

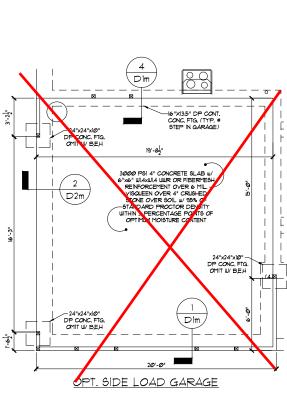
 17. ALL FOOTINGS 4 SLASS ARE TO BEAR ON WINDISTREED SOIL OR 95% COMPACTED FILL, VERRIED BY ENGINEER OR CODE OFFICIAL.

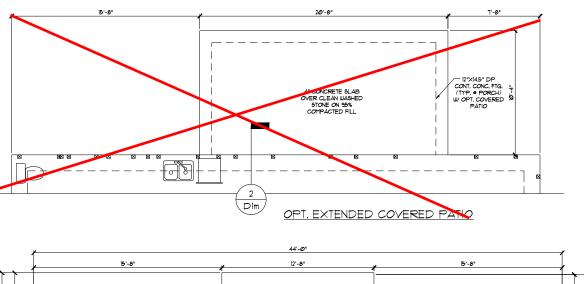
NOTE: ALL EXTERIOR FOUNDATION DIMENSIONS ARE TO FRAMING AND NOT BRICK VENEER, UNO

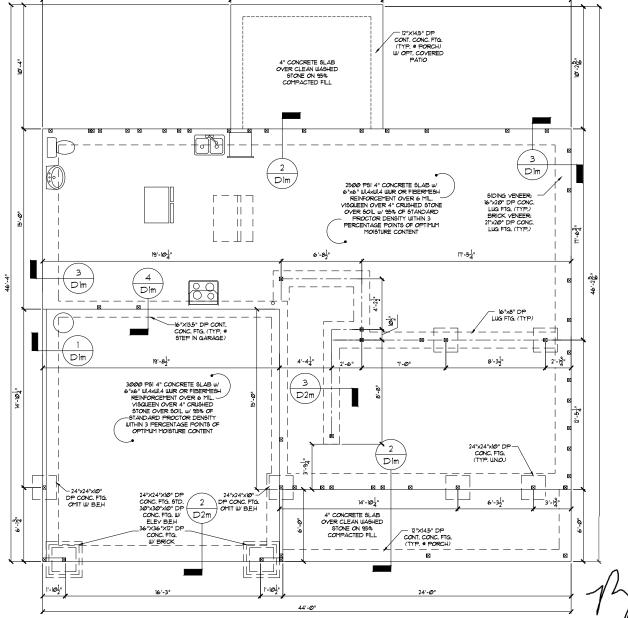
THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS HOMES COMPLETED REVISED ON <u>822/15</u>. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SIMPLIFIED REVISED ARE HADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION SUMMIT PENINERING, LADATORY 4 TESTIN, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS WEN DESIGNED ARCHITECTURAL PLANS WEN DESIGNED ARCHITECTURAL PLANS WEN DESIGNED ARCHITECTURAL PLANS WEN DESIGNED ARCHITECTURAL PLANS WEN DATE LISTED ABOVE.

NOTE: A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED WHEN SLAB IS INSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP I PER TABLE R405.









ORIGINAL DRAWING DATE PROJECT * 10/29/2019 3832.T0548

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CURRENT DRAWING

DATE: Ø1/1/2Ø21

SCALE: 1/8"=1'-@"

DRAWN BY: DGT

CHECKED BY: BCP

PROJECT *: 3832.TØ548

Douglas Homes . Reliance Ave x, NC 21539

Smith 2520

Apex,

STRUCTURAL MEMBERS ONLY

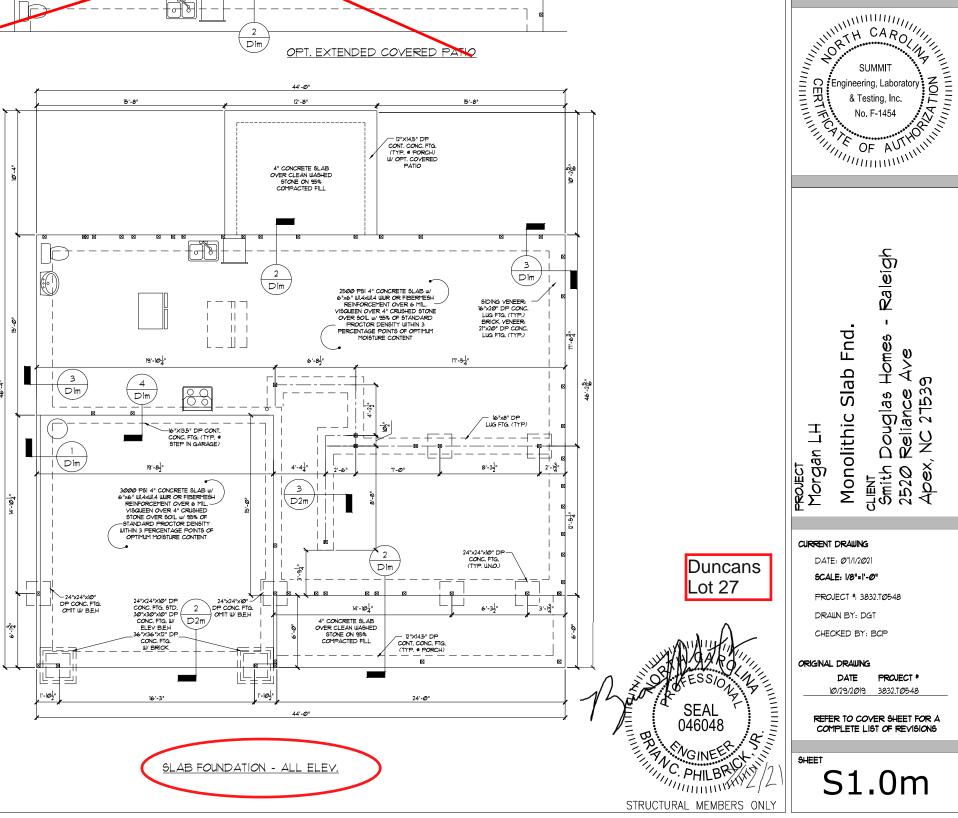
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ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

MONOLITHIC SLAB FOUNDATION



GENERAL STRUCTURAL NOTES:

- GENERAL STRUCTURAL NOTES

 1. CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AMENOPERINS.
 2. CONTRACTOR SHALL NERRY ALL DIMENSIONS, CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT, ENSINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS PROFIT HIS PLAN.
 3. CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING RECUIRED TO RESIST ALL PORCES BOCANTIFEED DURING EFECTION.
 4. PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS.
 4. PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS.
 6. MICROLLAM (LUX.PS. 2600 PS), Fr. 292 PS), E. 15540* PS!
 6. PARALLAM (PSI), Fs. 2930 PSI, Fr. 292 PSI, E. 15540* PS!
 7. PARALLAM (PSI), Fs. 2930 PSI, Fr. 292 PSI, E. 15540* PS!
 7. ALL WOOD PETBERS SHALL BE 2° SPE (INO).
 7. ALL BEAMS SHALL BE 192 SPE (INO).
 7. ALL BEAMS SHALL BE 193 PHOPORTED WITH A 2°) 254 PSI, E. 15540* PSI
 7. ALL WOOD PETBERS SHALL BE 2° SPE (INO).
 7. ALL BEAMS SHALL BE 193 HAVE SENDED FOR THE 2018 NORTH CARDINA SESDENTIAL CODE SECTION RESISTANT OF PLAN ALL STUD COLUMN AT EACH BID UNLESS NOTED OTHERWISE.
 7. ALL RESINFORCING STELL SHALL BE GRADE 60 BARS CONFORMING TO ASTM ASIS AND SHALL HAVE A MINIMAM COVER OF 3°.
 7. FOUNDATION ANCHORAGE SHALL BE CONSTRUCTED PER THE 2018 NORTH CARDINA RESIDENTIAL CODE SECTION REGISTING THE END OF EACH PLATE SECTION.
 7. MINIMAM (2) ANCHOR DOLTS SHALL BE 197 FROM THE END OF EACH PLATE SECTION.
 7. MINIMAM (2) ANCHOR DOLTS SHALL BE 2° FROM THE END OF EACH PLATE SECTION.
 7. MINIMAM (2) ANCHOR DOLTS SHALL BE 2° FROM THE END OF EACH PLATE SECTION.
 7. MINIMAM (2) ANCHOR DOLTS SHALL BE 100 FIRE PLATE.
 7. CONTRACTOR TO PROVIDED LOCKOLIS WHEN CELLING JOISTS SHALL BE DOLTED TOGETHER WITH 10° DIA THRU BOLTS SHALD BE DOLTED TOGETHER WITH 10° DIA THRU BOLTS SHALD BE POLTACE SHALL BE 10° HAVE SHALD BE DOLTED TOGETHER WITH 10° DIA THRU BOLTS SHALD BE 10° AND 10° SHALL BE 10° AND 10° SHALL BE 10° AND 10° SHALL BE 10° AND 10° OF CRIPTED.
 7. AND 10° OF CRIPTE LAULAL ADDRESS EXCEDING 8° ON 10° MIDTH AND/OR WITH MORE THAN 2° OF OF CRIPTED LAULAL BAD

- DJ = DOUBLE JOIST GT = GIRDER TRUSS SC = STUD COLUMN EE = EACH END TJ = TRIPLE JOIST CL = CENTER LINE SJ = SINGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER TR = TRIPLE RAFTER OC = ON CENTER PL = POINT LOAD

NOTE: NOTE:

DESIGNATES JOIST SUPPORTED LOAD
BEARING WALL ABOVE, PROVIDE BLOCKING UNDER
JOIST SUPPORTED LOAD BEARING WALL.

NOTE: SHADED WALLS INDICATE LOAD BEARING WALLS

JOIST 4 BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL HESE PLANS AND LESIONED IN ACCOMMENDE WITH ARCHITECTURAL PLANS PROVIDED BY SHITL POXIGLAS HOTEL COMPLETED PROPERTIED ON <u>2020</u>19. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SHITLE THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION SHITLE THE ADELIANCE AND ACCOMPLETED FOR A THE AND SHITLE THE ADELIANCE OF THESE STRUCTURAL PLANS WEN USED WITH ACHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

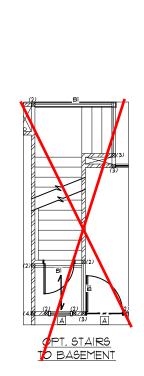
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SCALE: 1/8"=1"

FIRST FLOOR FRAMING PLAN



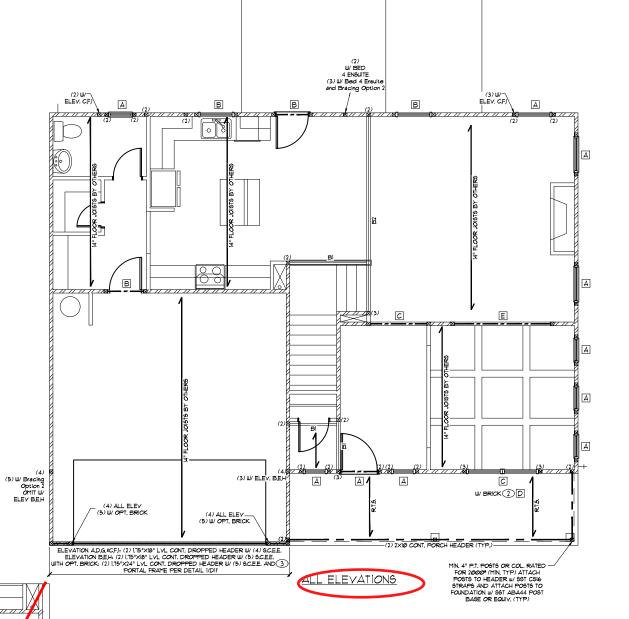
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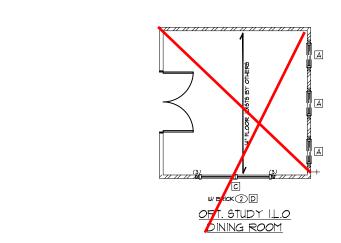
OPT. SIDE LOAD

GARAGE

(2) (15×18" LVL CONT. DROPPED HEADER BEH. (2) (15×11875" LVL W. (2) SCEE. PORTAL FRAME PER DETAIL I/DIF

ADG, (CF)





HEADER/BEAM SCHEDULE				
HEADER TAG	BEAM TAG	SIZE	JACKS (EACH EN	
-	BI	(1) 14" FLOOR JOIST	(2)	
-	B2	(2) 14" FLOOR JOIST	(2)	
Α	B3	(2) 2x6	(I)	
В	B4	(2) 2x8	(2)	
С	B5	(2) 2x1Ø	(2)	
D	B6	(2) 2xl2	(2)	
E	BT	(2) 9-1/4" LVL	(3)	
F	B8	(2) II-7/8" LVL	(3)	
G	B9	(2) 14" LVL	(3)	
н	BIØ	(2) 16" LVL	(3)	
I	BII	(2) 18" LVL	(3)	
J	B12	(2) 24" LVL	(4)	
K	B13	(3) 9-1/4" LVL	(3)	
L	B14	(3) 11-7/8" LVL	(3)	
М	B16	(3) I4" LVL	(3)	
N	BIT	(3) 16" LVL	(3)	
0	BIS	(3) 18" LVL	(3)	
P	BIS	(3) 24" LVL	(4)	
HEADER/BEAM SITES SHOUN ON PLANS ARE MINIMUMS GREATER				

HEADEN/BEAM SIZES SHOWN ON PLANS ARE MINIMUMS, GREATER HEADEN/BEAM SIZES MAY BE USED FOR EASE OF CONSTRUCTION. ALL HEADENS TO BE DROPPED UNLESS NOTED OTHERWISE. ALL BEAMS TO BE FLUSH UNLESS NOTED OTHERWISE. ALL

 to to be reading the test of the trainer.				
LINTEL SCHEDULE				
TAG	SIZE	OPENING SIZE		
Θ	L3×3×1/4"	LESS THAN 6'-0"		
2	L5x3x1/4"	6'-0" TO 10'-0		
3	L5x3-1/2"x5/16"	GREATER THAN 10'-0"		
4	L5x3-1/2"x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS		
SECURE I NITEL TO HE ARED (2) 101				

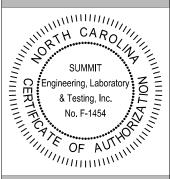
SECURE LINTEL TO HEADER W/ (2) 1/2"
DIAMETER LAG SCREWS STAGGERED @ 16"
O.C. (TYP FOR 3) ALL HEADERS WITH BRICK ABOVE: (1)(UNO)

WALL STUD SCHEDULE 1ST & 2ND FLOOR LOAD BEARING WALLS: 2x6 STUDS @ 24" O.C. OR 2x4 STUDS @ 16" O.C. 2X6 STUDS * 14" O.C. OR 2X4 STUDS * 16" O.C. 1ST FLOOR LOAD BEARING WALLS SUPPORTING: 2ND FLOOR * WALK-UP ATTIC: 2X6 STUDS * 16" O.C. OR 2X4 STUDS * 12" O.C. 246 STUDS 6 (6) O.C. OR 744 STUDS 6 (7) O.C.
BASETHENT LOAD ERRANG MULLS:
246 STUDS 6 (6) O.C. OR 744 STUDS 6 (7) O.C.
NON-LOAD BERANG MULLS (ALL FLOORS):
244 STUDS 6 24 O.C.
110 STORY MULLS:
244 STUDS 6 (7) O.C. OR 724 STUDS 6 (6) O.C.
110 STORY MULLS:
244 STUDS 6 (7) O.C. OR 724 STUDS 6 (6) O.C.
110 X STORY MULLS:
(AKA "BALLOON FRAMING")

KING STUD REQUIREMENTS			
OPENING WIDTH KINGS (EACH END)		ACH END)	
(FT)	16" O.C.	24" O.C.	
LESS THAN 3'-0"	(1)	(1)	
3'-Ø TO 4'-Ø"	(2)	(1)	
4'-0" TO 8'-0"	(3)	(2)	
8'-0" TO 12'-0"	(5)	(3)	
12'-0" TO 16'-0" (6) (4)			
KING STUD REQUIREMENTS ABOVE DO NOT APPLY TO PORTAL FRAMED OPENINGS			



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<u>g</u> <u>o</u> 0<u>7</u> Douglas Homes . Reliance Ave x, NC 21539 Framing loor PROJECT Morgan 正 Дрех, Smith 2520 First

CURRENT DRAWING

DATE: Ø7/1/2Ø21

SCALE: 1/8"=1'-@"

PROJECT *: 3832.TØ548

DRAWN BY: DGT

CHECKED BY: BCP

ORIGINAL DRAWING

DATE PROJECT * 10/29/2019 3832.T0548

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S3.0

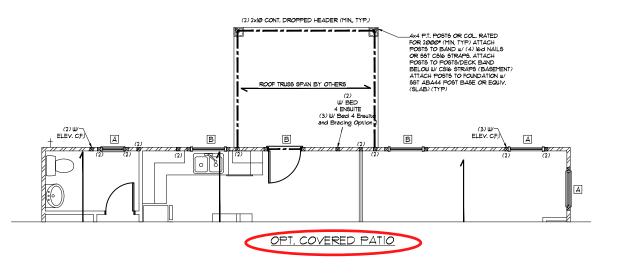


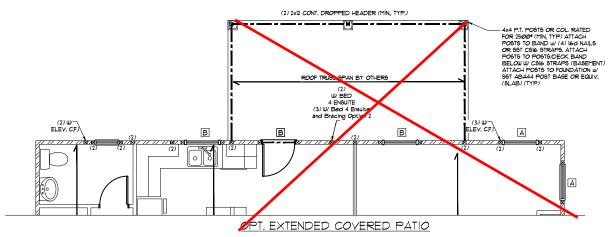
Duncans _ot 27

046048 NGINEER. NO PHILBRAND PHILDING

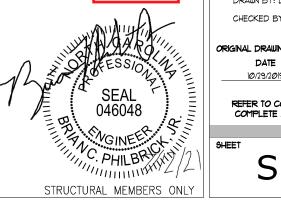
STRUCTURAL MEMBERS ONLY

SEE SHEET 53.0 FOR NOTES AND MORE INFORMATION





Duncans Lot 27



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Regineering, Laboratory

No. F-1454

OF AUTHORITICAL

OF A

Raleigh Douglas Homes . Reliance Ave x, NC 21539 Framing First Floor PROJECT Morgan LH CLIENT Smith I 2520 R Apex, 1

CURRENT DRAWING

DATE: Ø7/1/2Ø21

SCALE: 1/8"=1'-@"

PROJECT *: 3832.TØ548

DRAWN BY: DGT

CHECKED BY: BCP

ORIGINAL DRAWING

DATE PROJECT * 10/29/2019 3832.T0548

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S3.1

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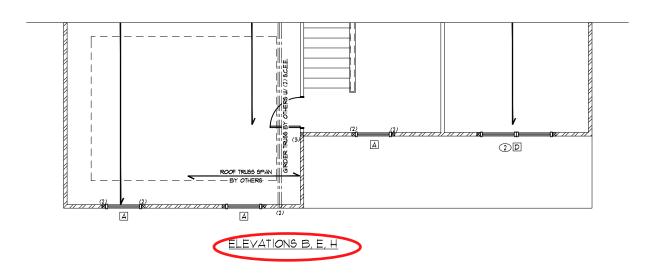
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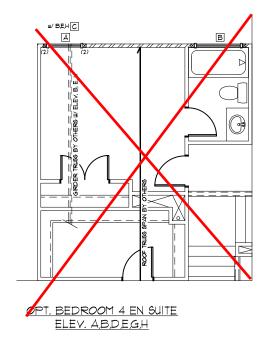
STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN

SCALE: 1/8"=1"

SEE SHEET S4.0 FOR NOTES AND MORE INFORMATION





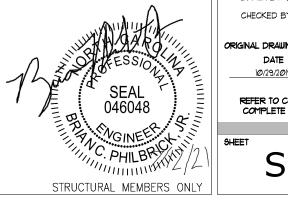
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SECOND FLOOR FRAMING PLAN SCALE: 1/8"=1"







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Regineering, Laboratory

No. F-1454

OF AUTHORITICAL

OF A

Raleigh Second Floor Framing Plan client Smith Douglas Homes -2520 Reliance Ave Apex, NC 21539 PROJECT Morgan LH

CURRENT DRAWING

DATE: Ø1/1/2Ø21

SCALE: 1/8"=1'-@"

PROJECT *: 3832.TØ548

DRAWN BY: DGT

CHECKED BY: BCP

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DATE PROJECT * 10/29/2019 3832.T0548

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S4.1

TRUSS UPLIFT CONNECTOR SCHEDULE				
MAX. UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FND	
535 LB6	H2.5A	PER WALL SHEATHIN	IG 4 FASTENERS	
1070 LB6	(2) H2.5A	CSI6 (END = 13")	DTT2Z	
1245 LBS	HTS2Ø	C5I6 (END = 13")	DTT2Z	
172Ø LB6	(2) MT52Ø	(2) C6l6 (END = 13")	DŤŤ2Z	
249Ø LB6	(2) HT52Ø	(2) CSI6 (END = 13")	HTT4	
2365 LB6	LGT3-9D62.5	(2) CSI6 (END = 13")	HTT4	
		(2) CSI6 (END = 13")		

L ALL PRODUCTS LISTED ARE SIMPSON STRONG-TIE. EQUIVALENT PRODUCTS MAY BE USED FIR MANIFACTURERS SPECIFICATIONS.

2. IMPLIFY VALUES LISTED ARE FOR SHF 2 (RADDE HEPGERS).

3. RETER TO TRUSS LATOUT TEER MANIF FOR WILLT VALUES AND TRUSS TO TRUSS CANCILORS SPECIFIED BY TRUSS MANIFACTURER.

I RUSS CONNECTIONS CONNECTORS SPECIFIED BY TRUSS FRANCACTURER OVERRIDE THOSE LISTED ABOVE.

4. CONTACT SUMMIT FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.

NOTE: 1ST PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

NOTE: TRUSS UPLIFT LOADS SHALL BE DETERMINED PER TRUSS MANUFACTURER IN ACCORDANCE WITH SECTION REQUIIJ. WALL SHEATHING AND FASTINERS HAVE BEEN DESIGNED TO RESIST THE WID UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION REQUISS OF THE 2018 NORC. REFER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER REQUIREMENTS.

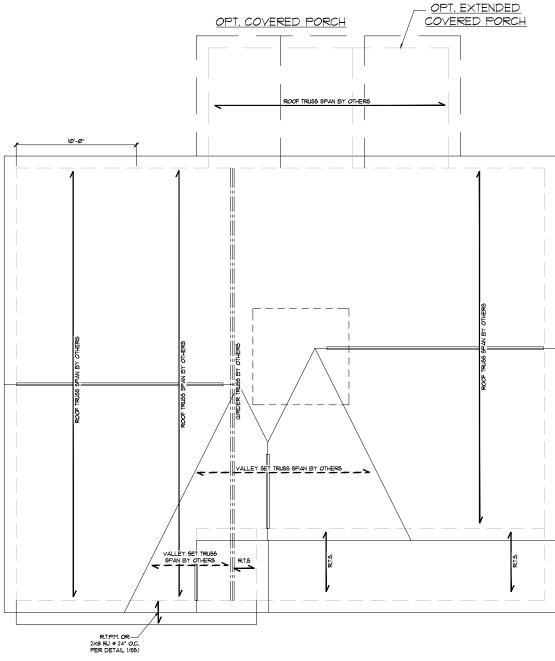
THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS MONES COMPLETED REVISED ON \$238/5, IT IS THE RESPONSIBILITY OF THE CLENT TO NOTIFY SUMMIT REMAINERING, LABORATORY 4 TESTING, P.C. PLANY CHANGES ARE TADE TO THE ARCHITECTURAL PLANS PRIOR OF CONSTRUCTION SUMMIT ENGINEERING, LABORATORY 4 TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS HERD USED WITH ARCHITECTURAL PLANS UNED MITHER RECHITECTURAL PLANS UNED WITH ARCHITECTURAL PLANS UNED WITH ARCHITECTURAL PLANS UNED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

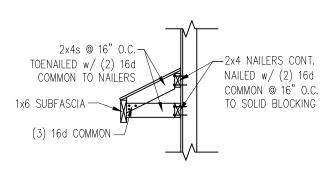
STRUCTURAL MEMBERS ONLY

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

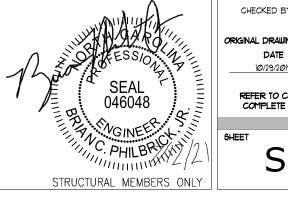
ROOF FRAMING PLAN SCALE: 1/8"=1"





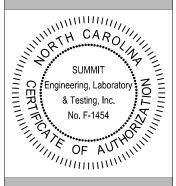


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<u>6</u> 0<u>x</u> Douglas Homes . Reliance Ave x, NC 21539 Framing Plan PROJECT Morgan L Smith D 2520 A Apex, I Roof

CURRENT DRAWING

DATE: Ø7/1/2Ø21

SCALE: 1/8"=1'-@"

PROJECT *: 3832.TØ548

DRAWN BY: DGT

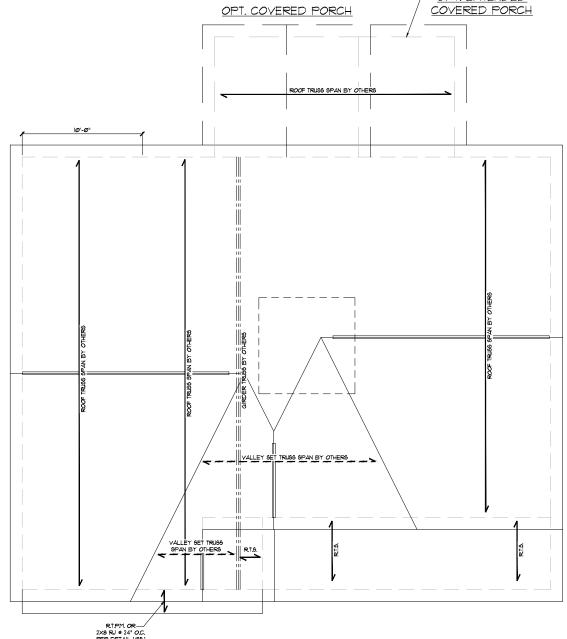
CHECKED BY: BCP

ORIGINAL DRAWING

DATE PROJECT * 10/29/2019 3832.T0548

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S5.1





REQ	REQUIRED BRACED WALL PANEL CONNECTIONS			
		MIN.	REQUIRED CONNECTION	
METHOD	MATERIAL	THICKNESS	# PANEL EDGES	# INTERMEDIATE SUPPORTS
C5-W5P	STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS © 12" O.C.
GB	GYPSUM BOARD	1/2"	5d COOLER NAIL5** # 7" O.C.	5d COOLER NAILS** # 7" O.C.
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS # 6" O.C.	6d COMMON NAILS © 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.1	PER FIGURE R6/02.10.1
"OR EQUIVALENT PER TABLE RT@2.3.5				

BRACED WALL NOTES:

- I. WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602/0
 FROM THE 2019 NORTH CAROLINA RESIDENTIAL CODE.

 2. WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND
 SPEEDS UP TO 300 MPH.

 3. REFER TO ARCHITECTIRAL PLAN FOR DOORWINDOW OPENING SIZES.

 4. BRACKING MATERIALS, METHODS AND FASTENIERS SHALL BE IN
 ACCORDANCE WITH TABLE R602/30.

 5. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL
 NOT EXCEED OF HEIT FOR ISOLATED PANEL METHOD AND 12 HEET FOR
 CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING
 CALCULATIONS.
- NOT EXCEED ID FIET FOR ISOLATED PANEL INTETHOD AND IS FIET FOR CONTINUOUS SHEATHING RETHOD WITHOUT ADDITIONAL PRISHERING CALCULATIONS.

 CALCULATIONS.

 (A) MINIMUM PANEL LENGTH SHALL BE FER TABLE R602/0/1.

 1. THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHATHED CONTINUOUS! Y WITH MINIMUM IS' MYPSIM BOARD (IND).

 FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHING METHOD, EXTERIOR SHILLS SHALL BE SHEATHING METHOD, EXTERIOR SHILL AREAS SHEWARD RACKED WALL PANELS, AND ON GABLE END WALL.

 FIN. GOODS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARNS WALL BELOW WITHOUT ADDITIONAL DISINEERING CALCULATIONS.

 (B) A BRACED WALL PANEL SHALL BELOCATED WITHIN IS FEET OF EACH END OF A PRACED WALL LINE.

 11. THE MAXIMIT BOOR DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED I HELD.

 12. MASONRY OR CONCRETE SITEM WALLS WITH A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANELS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMAN.

 13. BRACED WALL PANEL CONNECTIONS OF THE 200 NORCH.

 14. BOSTRICTED IN ACCORDANCE WITH SECTION REQUIPMAN.

 15. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMAN.

 16. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REPORTS OF THE FIGURE REGULES OF THE SECTION RACKED WAS AS A PREVENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REGULES OF THE PANELS.

 16. PORTAL WALL SHALD BE DESIGNED IN ACCORDANCE WITH FIGURE REGULES OF THE FIGURE REGULES OF THE FIGURE REGULES OF THE PANELS.

 16. PORTAL WALL SHALD BE DESIGNED IN ACCORDANCE WITH FIGURE REGULES OF THE PANELS.

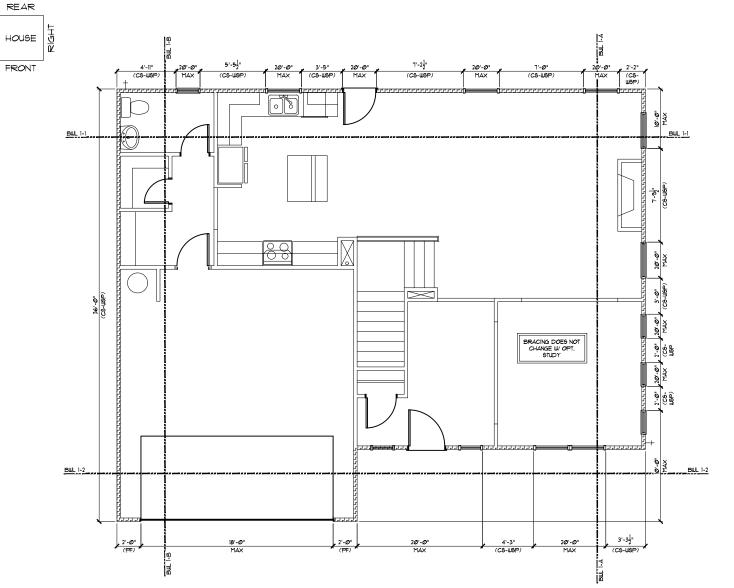
- ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.
 ABBREVIATIONS:

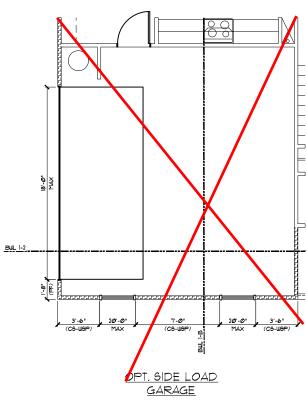
GB = GYP9UM BOARD
C5-XXX = CONT, SHEATHED
FF = PORTAL FRAME
FF = PORTAL FRAME
FF-ENG = ENG, PORTAL FRAME

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SHITH DOUGLAS HOTES COMPLETED REVISED ON 2028/S. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT EXAMERENS, LABORATORY (1 TESTING, P.C. FANY CHANGES ARE MODE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION, SUMMIT EXAMERENS, LABORATORY (1 TESTING, P.C. CANNOT GLARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

FIRST FLOOR BRACING (FT)					
con	CONTINUOUS SHEATHING METHOD				
	REQUIRED PROVIDED				
BWL 1-1	10.5	3Ø2			
BWL 1-2	BUL 1-2 105				
BWL 1-A 13.2 14.8					
201 1 B	10.0	24.0			

INSTALL HOLD-DOWNS PER SECTION R602.10.4 AND FIGURE R602.10.3(4) OF THE 2018 NCRC.





FIRST FLOOR BRACING (FT)				
CON	TINUOUS SHEATHING ME	THOO		
REQUIRED PROVIDED				
BWL 1-1	10.5	3Ø2		
BWL 1-2	10.5	21.5		
BWL 1-A	13.2	14.8		
BWL 1-B	13.2	20.8		

Duncans Lot 27

STRUCTURAL MEMBERS ONLY

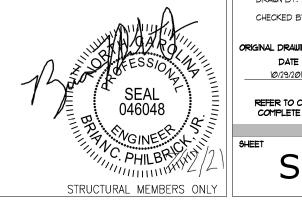
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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR BRACING PLAN SCALE: 1/8"=1"

<u>ALL ELEV.</u>





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ξ <u>o</u> 0<u>7</u> Douglas Homes . Reliance Ave x, NC 21539 Bracing Floor PROJECT Morgan Smith D 2520 A Apex, First

CURRENT DRAWING

DATE: Ø7/1/2Ø21

SCALE: 1/8"=1'-@"

PROJECT *: 3832.TØ548

DRAWN BY: DGT

CHECKED BY: BCP

ORIGINAL DRAWING

DATE PROJECT * 10/29/2019 3832.T0548

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S7.0

SEE SHEET ST.Ø FOR NOTES
AND MORE INFORMATION

REQ	REQUIRED BRACED WALL PANEL CONNECTIONS			
		MIN	REQUIRED (CONNECTION
METHOD	MATERIAL	THICKNESS	# PANEL EDGES	# INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS © 6" O.C.	6d COMMON NAILS @ 12" O.C.
GB	GYP9UM BOARD	1/2"	5d COOLER NAILS** 9 7" O.C.	5d COOLER NAILS…
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS # 6" O.C.	6d COMMON NAILS # 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.1	PER FIGURE R602,10,1
	**OR EQUIVALENT PER TABLE R102.3.5			

BRACED WALL NOTES:

- III MALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R6/02/0
 ROOM THE 2019 NORTH CAROLINA RESIDENTIAL CODE.
 IIII ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND SPEEDS UP TO 190 MPH.
 SPEEDS UP TO 190 MPH.
 SPEEDS TO ARCHITECTURAL PLAN FOR DOORWINDOW OPENING SIZES.
 RERACING MATERIALS, NETHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R6/02/10.
 ALL DRACED UALL PARELS SHALL BE FULL MALL HEIGHT AND SHALL NOT EXCEED 10 PEET FOR ISOLATED PAREL METHOD AND IT PEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CAI CILI ATOMS.
- NOT EXCEED ID FEET FOR IBOLATED PANEL METHOD AND IZ HEET FOR CONTINUOUS SHEATHING PETHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.

 (ALCULATIONS)

 (B. MINIMM PANEL LENGTH SHALL BE FER TABLE REGOLID.)

 (C. MINIMM PANEL LENGTH SHALL BE FER TABLE REGOLID.)

 (C. MINIMM PANEL LENGTH SHALL BE FER TABLE REGOLID.)

 (C. MINIMM PANEL LENGTH SHALL BE SHEAD BOTH SIDES OF INTERIOR WALLS SHALL BE S

- II. ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.

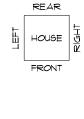
 18. ABBREVIATIONS:

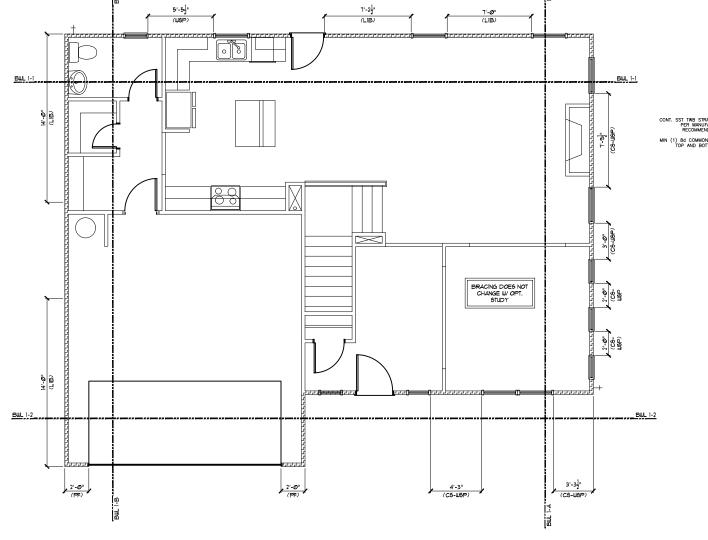
GB = GYP9UM BOARD
C5-XXX = CONT. 9HEATHED
FF = PORTAL FRAME
USP = WOOD STRUCTURAL PANEL
FNG = ENGINEERED SOLUTION
FF-ENG = ENG. PORTAL FRAME

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS HOTES COMPLETED REVISED ON BOADS. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY UNITHING THE CLIENT ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION SWITHIT ENGINEERING, LADORATORY 4 TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ACHITECTURAL PLANS WHEN DESIGNATION ACHITECTURAL PLANS WED

INSTALL HOLD-DOWNS PER SECTION R602.10.4 AND FIGURE R602.10.3(4) OF THE 2018 NCRC.

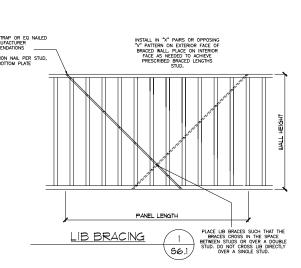
FIRST FLOOR BRACING (FT)			
CONTINUOUS SHEATHING METHOD			
REQUIRED PROVIDED			
BWL 1-1	10.5	12.5	
BWL 1-2	10.5	13.5	
BWL 1-A	13.2	14.8	
BWL 1-B	13.2	140	



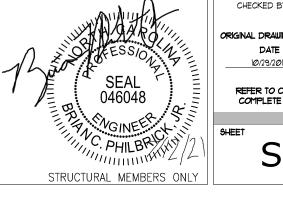


ALL ELEV.

BRACING OPTION 2



Duncans



ORIGINAL DRAWING

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DATE PROJECT *

10/29/2019 3832.T0548

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<u>(Q)</u> <u>o</u> 0<u>7</u>

Bracing

Floor

First

CURRENT DRAWING DATE: Ø7/1/2Ø21

> SCALE: 1/8"=1'-@" PROJECT *: 3832.TØ548 DRAWN BY: DGT CHECKED BY: BCP

PROJECT Morgan

Douglas Homes . Reliance Ave x, NC 21539

Smith D 2520 A Apex,

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR BRACING PLAN

REQUIRED BRACED WALL PANEL CONNECTIONS				
		. MIN.	REQUIRED (CONNECTION
METHOD	MATERIAL	THICKNESS	# PANEL EDGES	# INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS © 12" O.C.
GB	GYPSUM BOARD	1/2"	5d COOLER NAIL5** # 7" O.C.	5d COOLER NAILS # 7" O.C.
WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS # 6" O.C.	6d COMMON NAILS © 12" O.C.
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.1	PER FIGURE R602.10.1
"OR EQUIVALENT PER TABLE RT02.3.5				

REAR

BRACED WALL NOTES:

- I. WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602/0
 FROM THE 2019 NORTH CAROLINA RESIDENTIAL CODE.

 2. WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND
 SPEEDS UP TO 300 MPH.

 3. REFER TO ARCHITECTIRAL PLAN FOR DOORWINDOW OPENING SIZES.

 4. BRACKING MATERIALS, METHODS AND FASTENIERS SHALL BE IN
 ACCORDANCE WITH TABLE R602/30.

 5. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL
 NOT EXCEED OF HEIT FOR ISOLATED PANEL METHOD AND 12 HEET FOR
 CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING
 CALCULATIONS.
- NOT EXCEED ID FIET FOR ISOLATED PANEL INTETHOD AND IS FIET FOR CONTINUOUS SHEATHING RETHOD WITHOUT ADDITIONAL PRISHERING CALCULATIONS.

 CALCULATIONS.

 (A) MINIMUM PANEL LENGTH SHALL BE FER TABLE R602/0/1.

 1. THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHATHED CONTINUOUS! Y WITH MINIMUM IS' MYPSIM BOARD (WAD).

 FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHING METHOD, EXTERIOR SHILL AREAS SHEWARD RACKED WALL PANELS, AND ON GABLE END WALL.

 FIN. ORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARNS WALL BELOW WITHOUT ADDITIONAL DISINEERING CALCULATIONS.

 (B) A BRACED WALL PANEL SHALL BELOCATED WITHIN IS FEET OF EACH END OF A BRACED WALL INTO.

 10. THE MAXIMIT BOOR DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED I FEET.

 11. THE MAXIMIT BOOR DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED I FEET OF THE WALLS WITH A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANELS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMENT.

 12. BRACED WALL PANEL CONNECTIONS OF THE 2000 NORC.

 13. BRACED WALL PANEL CONNECTIONS OF THE 2000 NORC.

 14. BOSTORIETT DIS ACCORDANCE WITH SECTION REQUIPMENT.

 15. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMENT.

 16. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REQUIPMENT.

 17. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REQUIPMENT.

 18. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REQUIPMENT.

 19. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMENT.

 19. CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION REQUIPMENT.

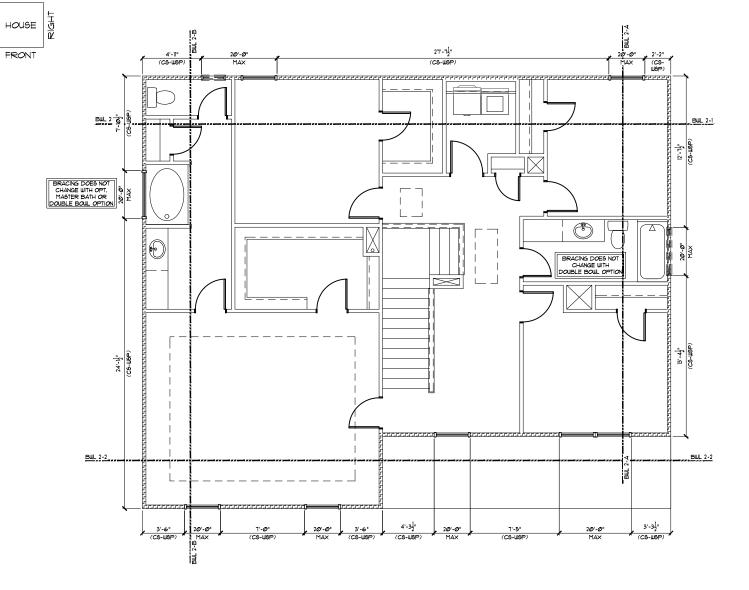
- II. ON SCHEMATIC, SHADED WALLS INDICATE BRACED WALL PANELS.
 IB. ABBREVIATIONS:

GB = GYPSUM BOARD
C5-XXX = CONT. SHEATHED
FF = PORTAL FRAME
WSP = WOOD STRUCTURAL PANEL
ENG = ENG.NEERED SOLUTION
FF-ENG = ENG. PORTAL FRAME

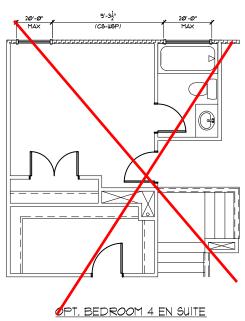
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INSTALL HOLD-DOWNS PER SECTION R602.10.4 AND FIGURE R602.10.3(4) OF THE 2018 NCRC.

SECOND FLOOR BRACING (FT)				
CON	TINUOUS SHEATHING ME	MOD		
REQUIRED PROVIDED				
BWL 2-1	5.5	34.1		
BUL 2-2	5.5	29.0		
BWL 2-A	6.8	26.0		
BIII 2-B	6.9	32.00		

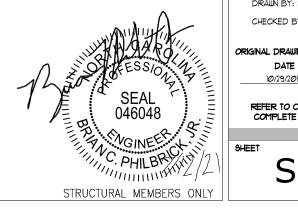






SECOND FLOOR BRACING (FT)							
CONTINUOUS SHEATHING METHOD							
	PROVIDED						
BWL 2-1	5.5	3Ø.T					
BWL 2-2	BWL 2-2 5.5 29.Ø						
BWL 2-A	-A 6.8 26.0						
BWL 2-B	6.8	32.0					

Duncans Lot 27





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OF AUTHOR

<u>6</u> 0<u>x</u> Bracing Douglas Homes . Reliance Ave x, NC 21539 Floor Second PROJECT Morgan Smith D 2520 R Apex,

CURRENT DRAWING

DATE: Ø7/1/2Ø21

SCALE: 1/8"=1'-@"

PROJECT *: 3832.TØ548

DRAWN BY: DGT

CHECKED BY: BCP

ORIGINAL DRAWING

DATE PROJECT * 10/29/2019 3832.T0548

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STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SECOND FLOOR BRACING PLAN SCALE: 1/8"=1"

REQUIRED BRACED WALL PANEL CONNECTIONS							
		MIN.	REQUIRED (CONNECTION			
METHOD	MATERIAL	THICKNESS	@ PANEL EDGES	 INTERMEDIATE SUPPORTS 			
C5-W5P	STRUCTURAL PANEL	3/8"	6d COMMON NAILS 6 6" O.C.	6d COMMON NAILS @ 12" O.C.			
GB	GYP9UM BOARD	1/2"	5d COOLER NAIL5** # 7" O.C.	5d COOLER NAILS** # T" O.C.			
WSP	STRUCTURAL PANEL	3/8"	6d COMMON NAILS # 6" O.C.	6d COMMON NAILS # 12" O.C.			
PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.1	PER FIGURE R602.10.1			
	"OR EQUIVALENT PER TABLE RT0235						

BRACED WALL NOTES:

PRACED WALL NOTES

I. WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R6/02/0
FRONT THE 20/8 NORTH CAROLINA RESIDENTIAL CODE.

2. WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND ULTIMATE WIND SPEEDS UP TO 180 MIPH.

3. REFER TO ARCHITECTURAL PLAN FOR DOORWINDOW OPENING SIZES.

4. BRACKING MATERIALS, PIEMPOS AND FASTINERS SHALL BE IN ACCORDANCE WITH TABLE R6/02/01.

5. ALL PRACED WALL PARILS SHALL BE FILL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.

6. MINISTRUM FANEL LENGTH SHALL BE FER TABLE R6/02/10.1

7. THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHALL BE SHALL BE SHALDED ON THIRDING MICHOLISM.

1. THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUS! WITH MINIMIM 1/2" GYPBUM BOARD (UNO).

FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL SHEATHING METHOD, EXTERIOR WALLS SHALL BE SETWERN BRACED WALL PARELS, AROO'S AND DELOWALL AREAS ETWERN BRACED WALL PARELS, AROO'S AND EXCURPINGS, AND ON GABLE FIND WALLS.

1. ELOOPS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL DEGINEERING CALCULATIONS.

10. A BRACED WALL FANEL SHALL BELOCATED WITHIN 17 FEET OF EACH BID OF A BRACED WALL PANEL SHALL BE LOCATED WITHIN 17 FEET OF EACH BID OF A BRACED WALL LINE.

11. THE MAXIMUM BODE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 31 FEET.

12. MASONRY OR CONCRETE STEM WALLS WITH A LENGTH OF 48" OR LESS SUPPOSTING A BRACED WALL PANELS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REGISORS OF THE 20% NORCE.

13. BRACED WALL PANEL CONNECTIONS TO FLOORICELING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION REGISORAL BE CONSTRUCTED IN ACCORDANCE WITH SECTION REGISORAL BE DESIGNED WALL DESIGNED WALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION REGISORAL BE DESIGNED IN ACCORDANCE WITH SECTION REGISORAL BE DESIGNED IN ACCORDANCE WITH SECTION REGISORAL BE DESIGNED IN ACCORDANCE WITH FIGURE REGISORAL WALL BE ASSETTED SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REGISORAL WALL BE ASSETTED. SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE REGISORAL WALL BE DESIGNED IN ACCORDANCE WITH FIGURE REGISORAL BE SOME WALL BE SOME PANELS.

18. BEREVIALTIONS.

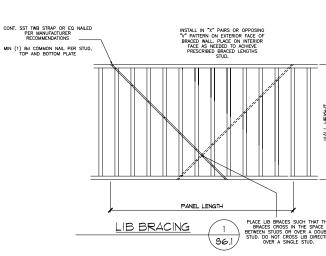
GB = GYP9UM BOARD
C3-XXX = CONT, SHEATHED
FF = PORTAL FRAME
FF = PORTAL FRAME
FF-ENG = ENG, PORTAL FRAME

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY SMITH DOUGLAS HAMES COMPLETED REVISED ON <u>2020/9</u>. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SMITH INTERPRISE, LADRATORY (1 TESTING, P.C. FANY CHANGES ARE THOSE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION SMITH INTERPRISE, LADRATORY (1 TESTING, P.C. CANNOT GLIARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WEN USED WITH ARCHITECTURAL PLANS OF THE DESIGNATION OF THE STRUCTURAL PLANS WEN DESIGNATION TO THE STRUCTURAL PLANS THE DATE LISTED ABOVE.

INSTALL HOLD-DOWNS PER SECTION R602.10.4 AND FIGURE R602.10.3(4) OF THE 2018 NORC.

SECOND FLOOR BRACING (FT)						
CONTINUOUS SHEATHING METHOD						
	REQUIRED PROVIDED					
BWL 2-1	5.5	11.9				
BUL 2-2	5.5	7.2				
BWL 2-A	6.8	13.0				
DIII 2 D	6.0	7.4				

REAR HOUSE FRONT (WSP) BRACING DOES NOT CHANGE WITH OPT. MASTER BATH OR DOUBLE BOWL OPTION



ALL ELEV. BRACING OPTION 2

STRUCTURAL MEMBERS ONLY

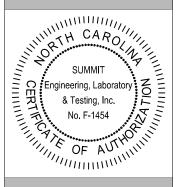
ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SECOND FLOOR BRACING PLAN SCALE: 1/8"=1"

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SEE SHEET SS.Ø FOR NOTES AND MORE INFORMATION



<u>6</u> 0<u>x</u> Floor Bracing Douglas Homes . Reliance Ave x, NC 21539 Second PROJECT Morgan Smith D 2520 R Apex, I

CURRENT DRAWING

Duncans

Lot 27

DATE: Ø7/1/2Ø21

SCALE: 1/8"=1'-@"

PROJECT *: 3832.TØ548

DRAWN BY: DGT

CHECKED BY: BCP

ORIGINAL DRAWING

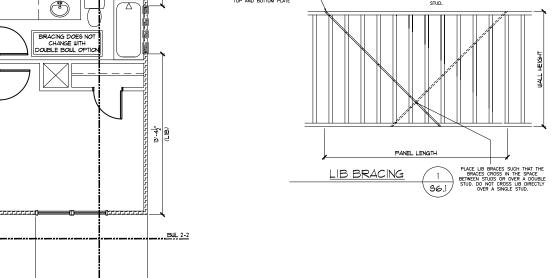
DATE PROJECT * 10/29/2019 3832.T0548

SEAL 046048

REFER TO COMP

RAL MEMBERS REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

S8.1



GENERAL STRUCTURAL NOTES:

- 1. The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.
- The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
- 3. The SER is not responsible for construction sequences. methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
- 4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
- 5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
- 6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
- This structure and all construction shall conform to all applicable sections of the international residential code.
- This structure and all construction shall conform to all applicable sections of the 2018 North Carolina Residential Code (NCRC) and any local codes or restrictions

FOUNDATIONS:

- 1. Foundations shall be constructed in accordance with chapter 4 of the 2018 NC Residential Building Code (Special consideration shall be given to Chapter 45 in wind zones above 130mph)
- 2. Footing sizes based on a presumptive soil bearing capacity of 2000 PSF. Contractor is solely responsible for verifying the suitability of the site soil conditions at the time of construction
- Maximum depth of unbalanced fill against masonry walls to be as specified in section R404.1 of the 2018 NCRC
- The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
- 6. Any fill shall be placed under the direction or recommendation of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
- 7. Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
- 8. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.
- 9. Each crawl space pier shall bear in the middle third of its respective footing and each girder shall bearing in the middle third of the piers. Pilasters to be bonded to perimeter foundation wall
- 10. Crawl spaced to be graded level and clear of all debris
- 11. Provide foundation waterproofing and drain with positive slope to outlet as required by site conditions
- 12. Energy efficiency compliance and insulation of the structure to be in accordance with chapter 11 of the 2018 NCRC

- 1. Concrete shall have a normal weight aggregate and a minimum compressive strength (f'c) at 28 days of 3000 psi, unless otherwise noted on the plan.
- 2. Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- 3. Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of target values as follows: 3.1. Footings: 5%
 - 3.2. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER
- 5. Concrete slabs—on—grade shall be constructed in accordance with ACI 302.1R-96: "Guide for Concrete Slab and Slab Construction".
- 6. The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions
- 7. Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.
- 8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint.
- 10. All welded wire fabric (W.W.F.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The W.W.F. shall be securely supported during the concrete pour. Fibermesh may be used in lieu of W.W.F.

CONCRETE REINFORCEMENT:

- 1. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strenath.
- 2. Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement
- 3. Application of fibermesh per cubic yard of concrete shall egual a minimum of 0.1% by volume (1.5 pounds per cubic yard)
- 4. Fibermesh shall comply with ASTM C1116, any local building code requirements, and shall meet or exceed the current industry standard.
- 5. Steel Reinforcing bars shall be new billet steel conforming to ASTM A615, grade 60.
- 6. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Manual of Standard Practice for Detailing Concrete Structures"
- Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same size/spacing as the horizontal reinforcement with a class B tension splice.
- 8. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.
- 9. Where reinforcing dowels are required , they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
- 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

WOOD FRAMING:

- 1. Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Spruce-Pine-Fir (SPF) #2.
- 2. LVL or PSL engineered wood shall have the following minimum design values:
 - 2.1. E = 1.900.000 psi
 - 2.2. Fb = 2600 psi
 - 2.3. Fy = 285 psi
- 2.4. Fc = 700 psi
- 3. Wood in contact with concrete, masonry, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance with AWPA standard C-2
- 4. Nails shall be common wire nails unless otherwise noted.
- 5. Lag screws shall conform to ANSI/ASME standard B18.2.1-1981. Lead holes for lag screws shall be in accordance with NDS specifications.
- 6. All beams shall have full bearing on supporting framing members unless otherwise noted.
- 7. Exterior and load bearing stud walls are to be 2x4 SPF#2 @16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall he continuous
- 8. Individual studs forming a column shall be attached with one 10d nail @6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be fully blocked at all floor levels to ensure proper load transfer
- 9. Multi-ply beams shall have each ply attached wth (3)10d nails @ 24" O.C.
- 10. Flitch beams and four and five ply beams shall be bolted together with (2) rows of 1/2" dia. through bolts staggered @24" O.C. w/ 2" edge distance and (2) bolts located at 6" from each end, unless noted otherwise.

- 1. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- 2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- 3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction." (NDS) and "Design" Specification for Metal Plate Connected Wood Trusses
- 4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing. both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
- 5. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

WOOD STRUCTURAL PANELS:

- 1. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA standards.
- All structurally required wood sheathing shall bear the mark of the APA.
- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
- Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
- Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshank nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building
- 6. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

STRUCTURAL FIBERBOARD PANELS:

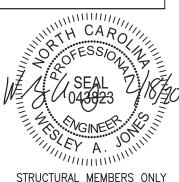
- 1. Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable AFA standards
- 2. Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.

EXTERIOR WOOD FRAMED DECKS:

1. Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

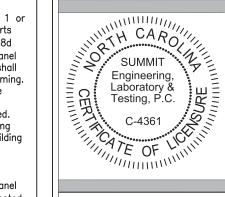
STRUCTURAL STEEL:

- 1. Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and of the manual of Steel Construction "Load Resistance Factor Design" latest editions.
- All steel shall have a minimum yield stress (Fy) of 36 ksi unless otherwise noted.
- Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D1.1. Electrodes for shopt and field welding shall be class E70XX. All welding shall be performed by a certified welder per the above standards.





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2 Specifications 21. glas Homes Trail, Suite , GA 30188 110 Village T Woodstock, (Dougl and Notes Smith |

CURRENT DRAWING

Details

Standard

DATE: 2/18/20

SCALE: NTS PROJECT #: 3832

DRAWN BY: LBV

CHECKED BY: WAI

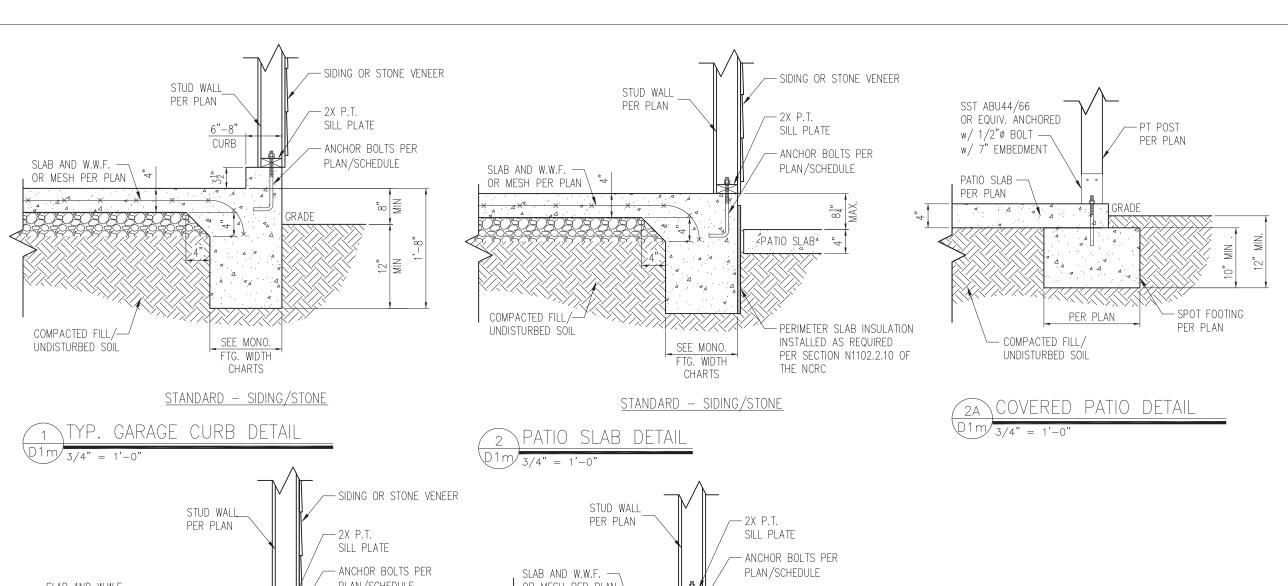
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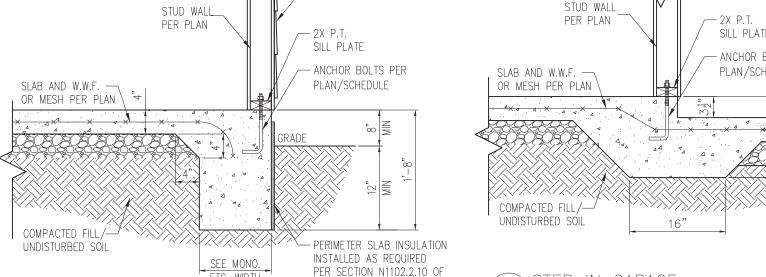
DATE PROJECT # 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

CS₂





CHARTS STANDARD - SIDING/STONE

THE NCRC

TVD CLAD DETAIL	WALL ANCHOR SCHEDULE
3 TYP. SLAB DETAIL	TYPE OF ANCHOR
$01m\sqrt{3/4"} = 1'-0"$	
	1/2"ø A307 BOLTS w/
TES:	STD. 90° BEND
	II

FTG. WIDTH

- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.

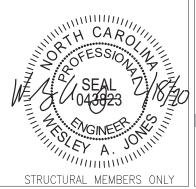
WALL ANCHOR SCHEDULE				
TYPE OF ANCHOR	MIN. CONC.	SPACING	INTERIOR	EXTERIOR
	EMBEDMENT	EMBEDMENT	WALL	WALL
1/2"ø A307 BOLTS w/	7"	6'-0"	YES	YES
STD. 90° BEND				
SST - MAS	4"	5'-0"	NO	YES
HILTI KWIK BOLT KBI 1/2-2-3/4	2-1/4"	6'-0"	YES	NO
1/2"ø HILTI THREADED ROD	7"	6'-0"	YES	YES
w/ HIT HY150 ADHESIVE				

NOTE: INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.

MONOLITHIC FOOTING WIDTH

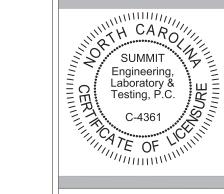
# OF STORIES	WIDTH BASED ON SOIL BEARING CAPACITY				
	1500 PSF	2000 PSF	2500 PSF		
1 STORY - STD.	16"	16"	16"		
1 STORY - BRICK VENEER	21"*	21"*	21"*		
2 STORY - STD.	20"	16"	16"		
2 STORY - BRICK VENEER	25"*	21"*	21"*		
*5" BRICK LEDGE HAS BEEN ADDED TO THE MONOLITHIC					

FOOTING WIDTH FOR BRICK SUPPORT





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2 21 Slab Details Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Standard Details Monolithic

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PRO1ECT # · 3832

DRAWN BY: LBV

CHECKED BY: WAJ

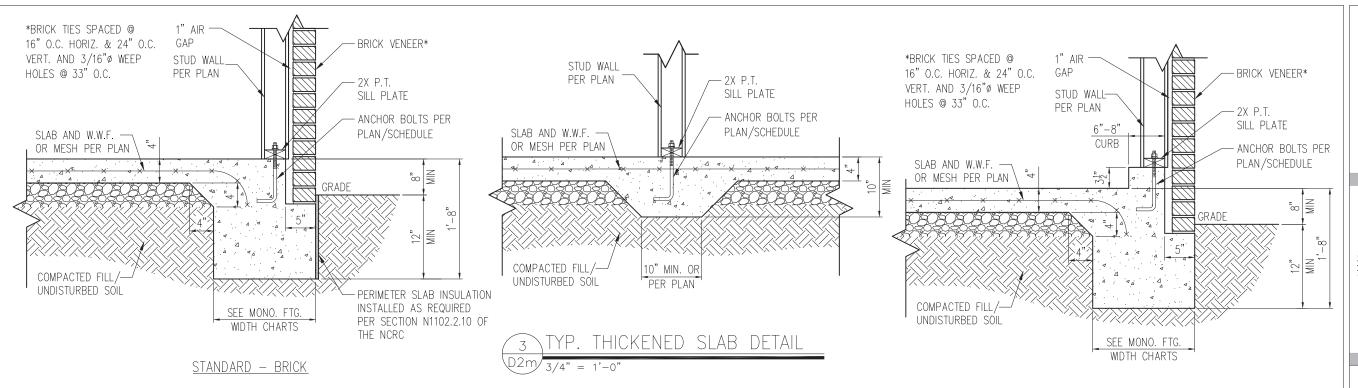
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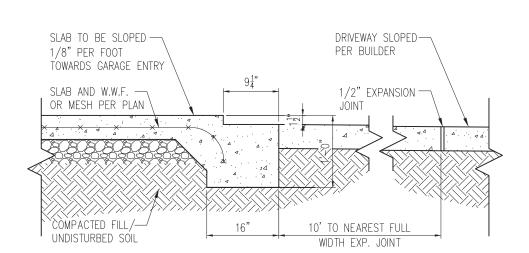
DATE PROJECT # 1/7/16

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

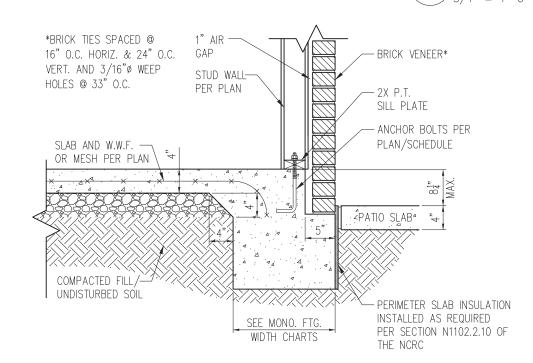
D₁m





YP. SLAB DETAIL W/ BRICK VENEER



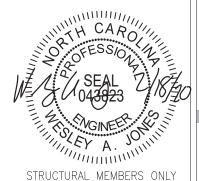


STANDARD - BRICK

PATIO SLAB DETAIL W/BRICK VENEER

NOTES

- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.



STANDARD - BRICK

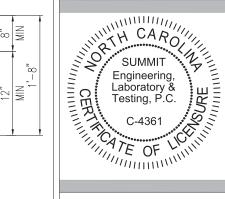
TYP. GARAGE CURB DETAIL

W/ BRICK VENEER

SUMMIT
ENGINEERING LABORATORY TESTING
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SUITE 171, RALEIGH, NC 27603
OFFICE: 919.380.9991

FAX: 919.380.9993

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Standard Details Monolithic Slab Details Smith Douglas Homes 110 Village Trail, Suite 215 Woodstock, GA 30188

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PROJECT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

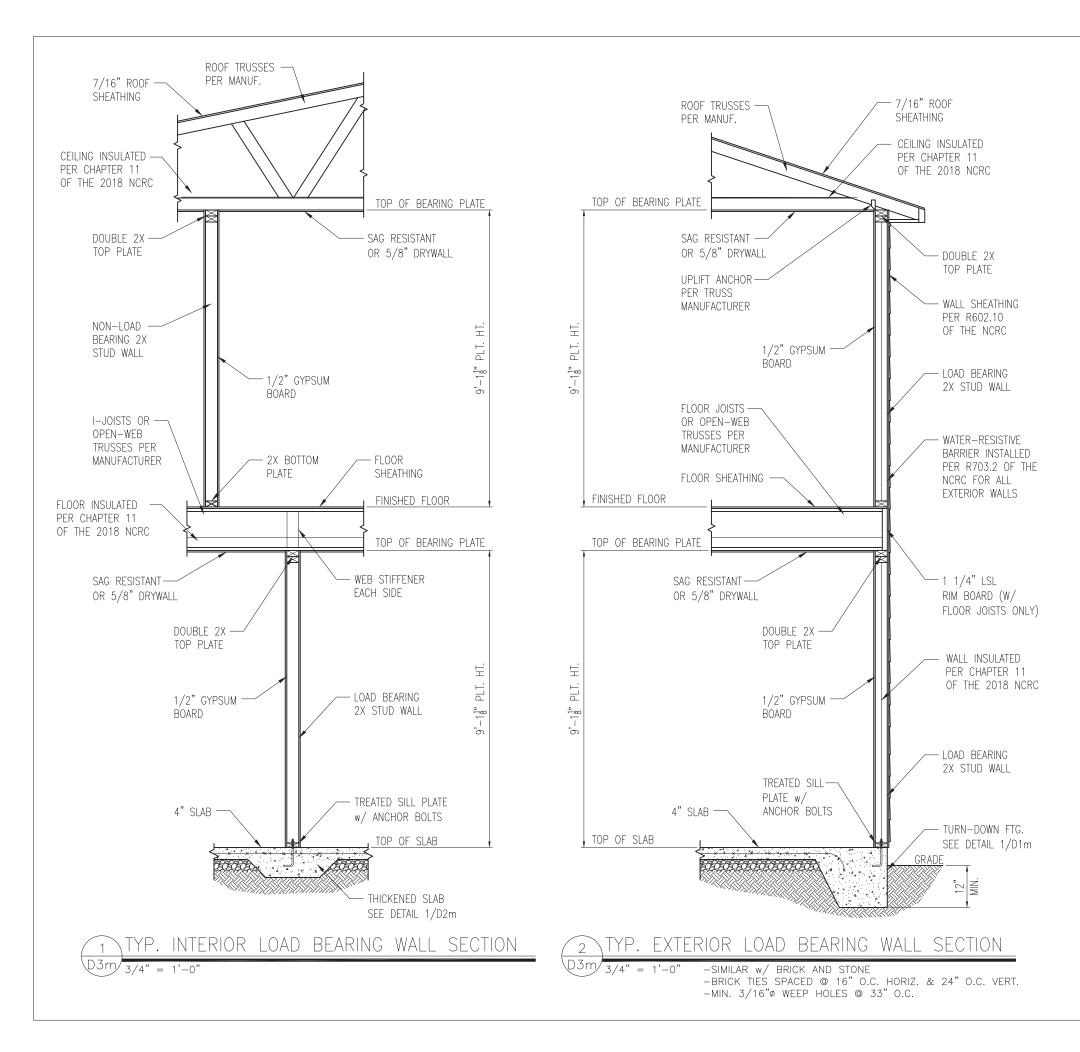
ORIGINAL DRAWING

NO. DATE PROJECT # 0 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

D₂m





Slab Details Smith Douglas Homes 110 Village Trail, Suite 2 Woodstock, GA 30188 Standard Details Monolithic

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SUITE 171, RALEIGH, NC 27603

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FAX: 919.380.9993 WWW.SUMMIT-COMPANIES.COM

THE CAROLLING

2

21

SUMMIT Engineering, Laboratory & Testing, P.C.

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PRO1FCT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

ORIGINAL DRAWING

DATE PROJECT # 1/7/16

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

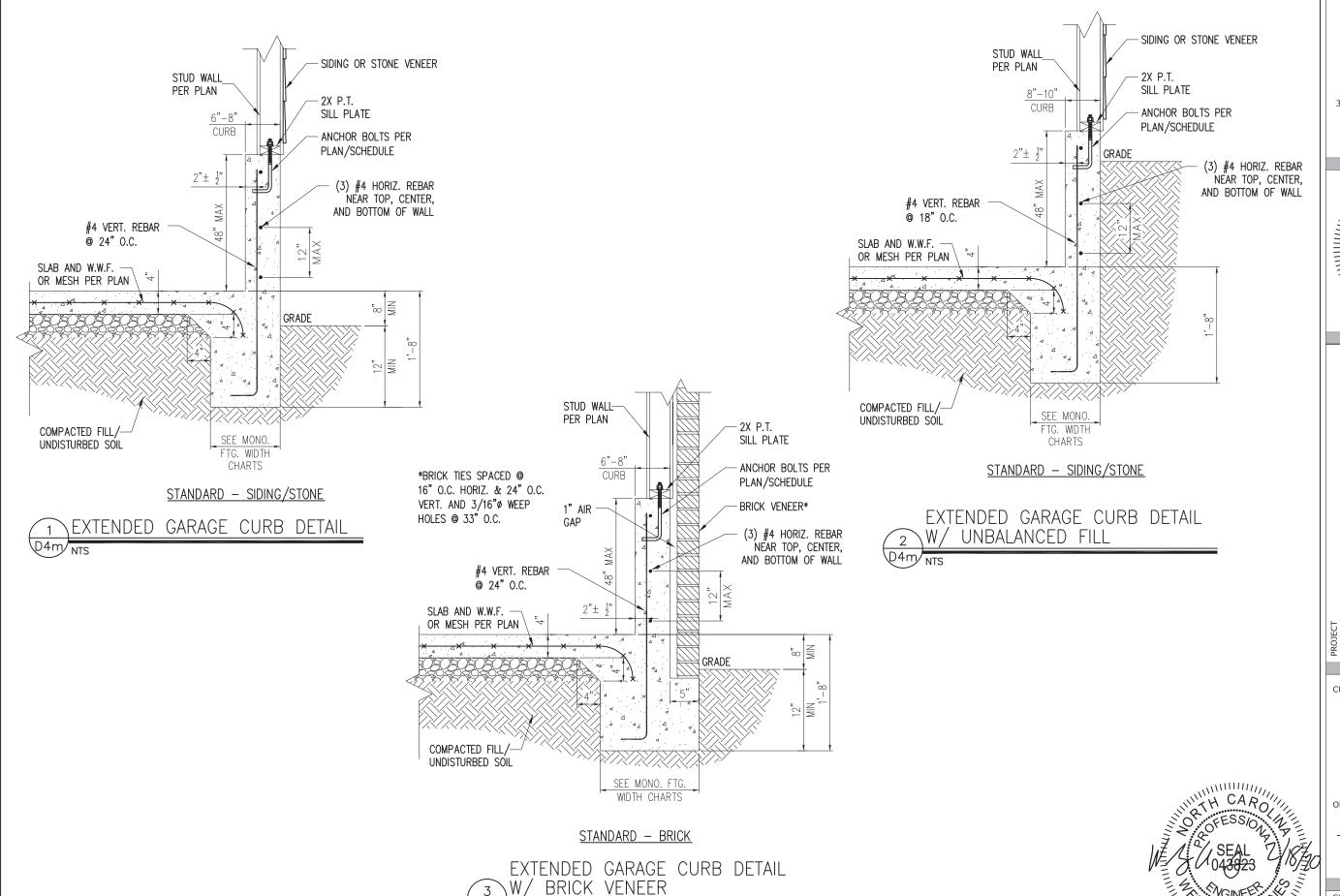
D₃m

NOTES:
1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.

2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.

3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.

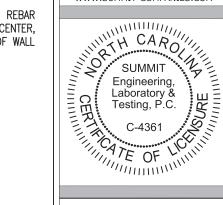




D4m/NTS



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Standard Details

Monolithic Slab Details

Smith Douglas Homes
110 Village Trail, Suite 21!
Woodstock, GA 30188

2

CURRENT DRAWING

DATE: 2/18/20

SCALE: NTS

PROJECT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

ORIGINAL DRAWING

D. DATE PROJECT #
0 1/7/16 3832

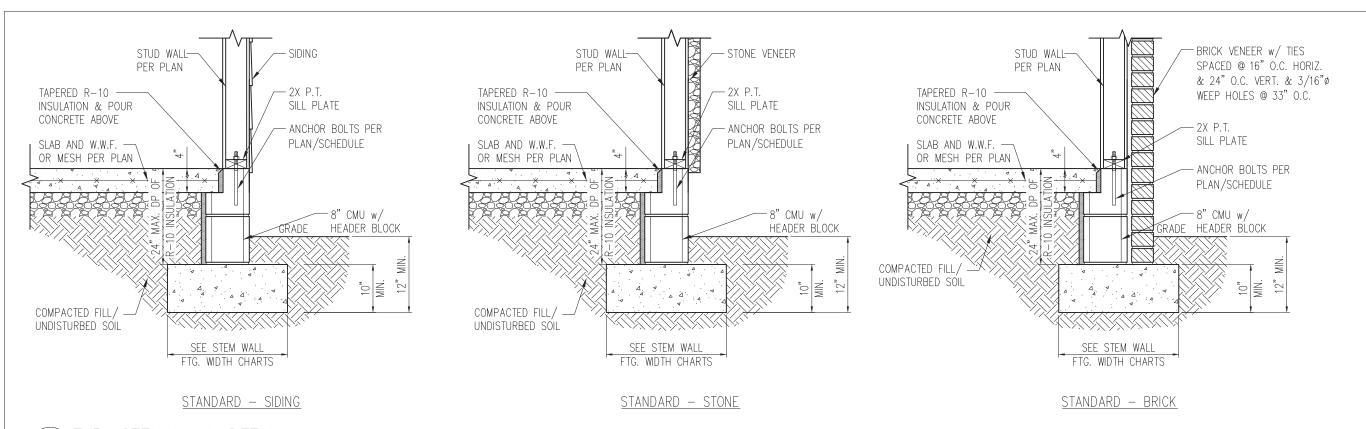
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

THEY A. JOHN

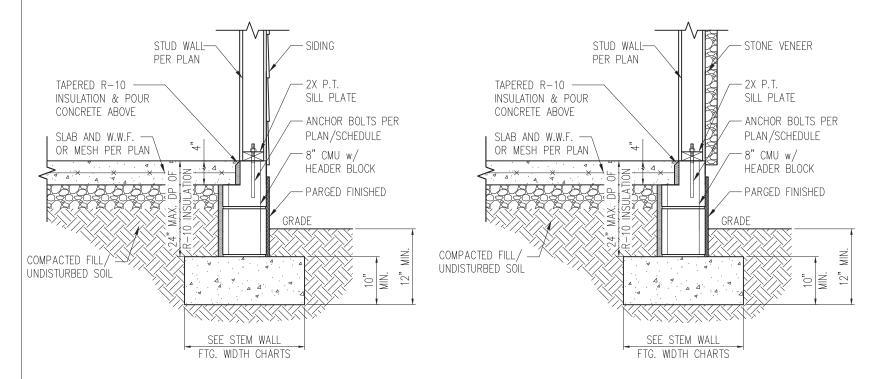
STRUCTURAL MEMBERS ONLY

D4m



STANDARD - STONE

TYP. STEM WALL DETAIL D1s 3/4" = 1'-0"



STANDARD - SIDING

1a STEM WALL DETAIL W/ PARGED FINISH

3/4" = 1'-0"

STEM WALL FOOTING WIDTH

# OF STORIES	WIDTH BASED ON SOIL BEARING CAPACITY					
	1500 PSF	2000 PSF	2500 PSF			
1 STORY - STD.	16"	16"	16"			
1 STORY - BRICK VENEER	21"*	21"*	21"*			
2 STORY - STD.	20"	16"	16"			
2 STORY – BRICK VENEER	25"*	21"*	21"*			
*5" BRICK LEDGE HAS BEEN ADDED TO THE STEM WALL						
FOOTING WIDTH FOR BRICK S	UPPORT					

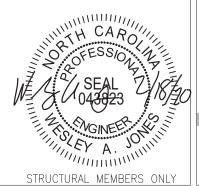
WALL ANCHOR SCHEDULE

TYPE OF ANCHOR	MIN. CONC.	SPACING	INTERIOR	EXTERIOR
	EMBEDMENT	EMBEDMENT	WALL	WALL
1/2"ø A307 BOLTS w/	7"	6'-0"	YES	YES
STD. 90° BEND				
SST - MAS	4"	5'-0"	NO	YES
HILTI KWIK BOLT KBI 1/2-2-3/4	2-1/4"	6'-0"	YES	NO
1/2"ø HILTI THREADED ROD	7"	6'-0"	YES	YES
w/ HIT HY150 ADHESIVE				

NOTE: INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.

NOTES

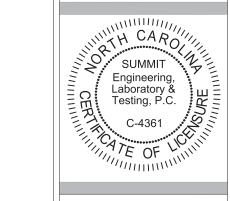
- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPS AND DEPRESSIONS.





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Stemwall Details
CLIENT
Smith Douglas Homes
110 Village Trail, Suite 21!
Woodstock, GA 30188

2

CURRENT DRAWING

Standard Details

DATE: 2/18/20

SCALE: NTS

PROJECT #: 3832

DRAWN BY: LBV
CHECKED BY: WAJ

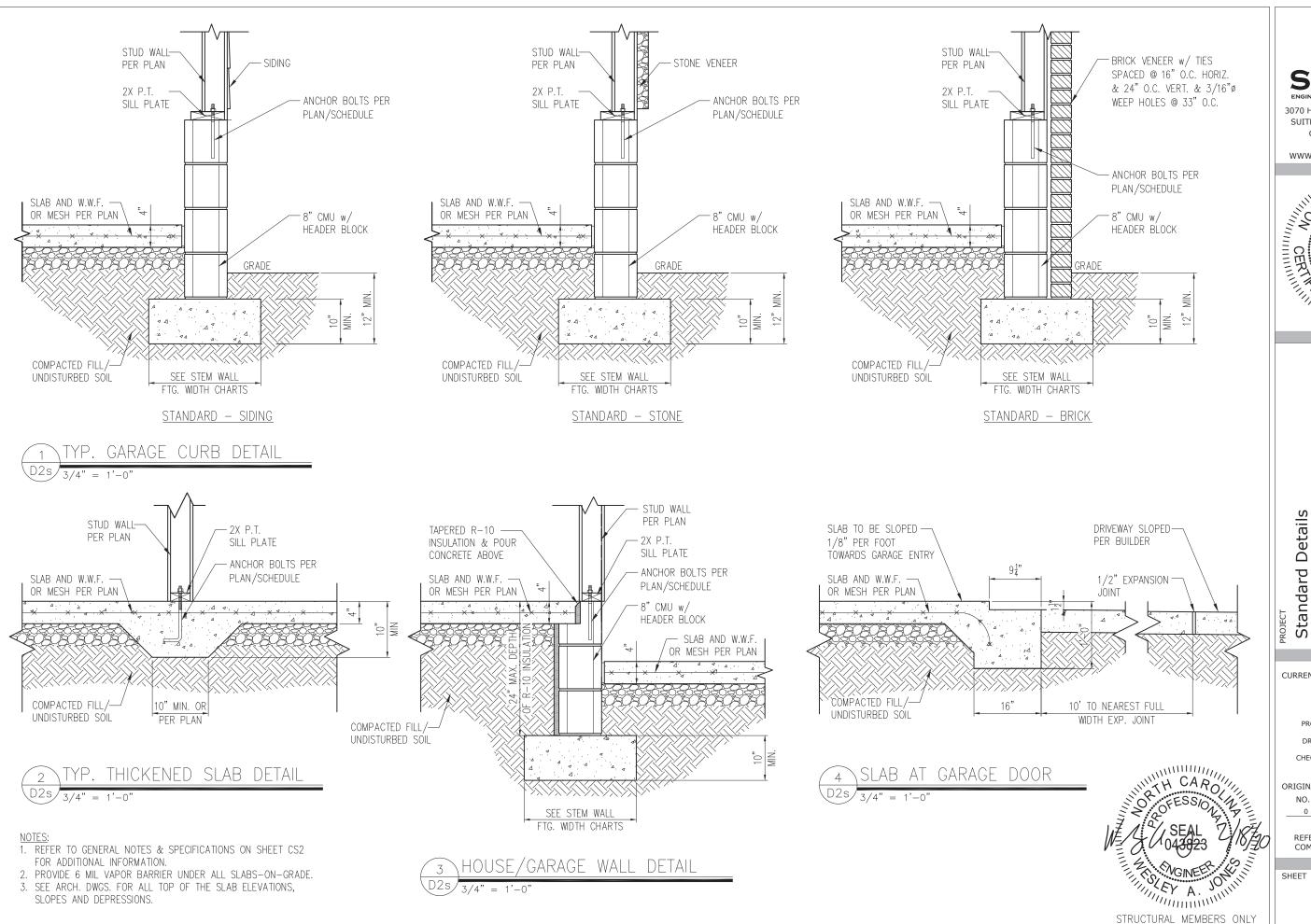
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NO. DATE PROJECT # 0 1/7/16 3832

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

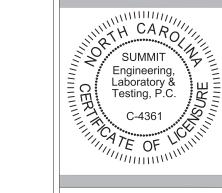
SHEET

D1s





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PRO1ECT # · 3832

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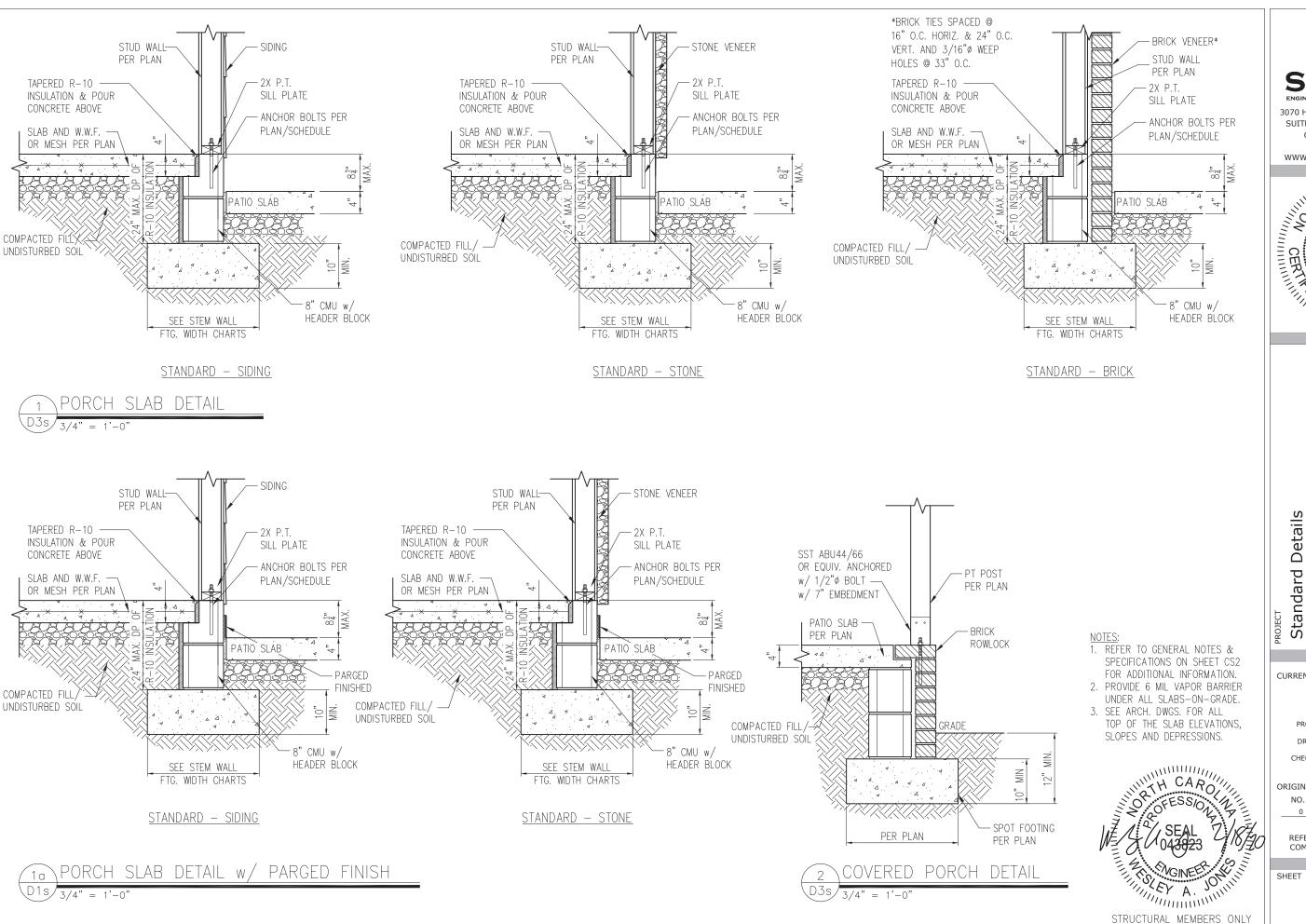
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DATE PROJECT # 1/7/16 3832

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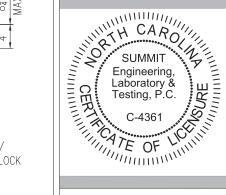
SHEET

D2s





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PRO1FCT #: 3832

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CHECKED BY: WAJ

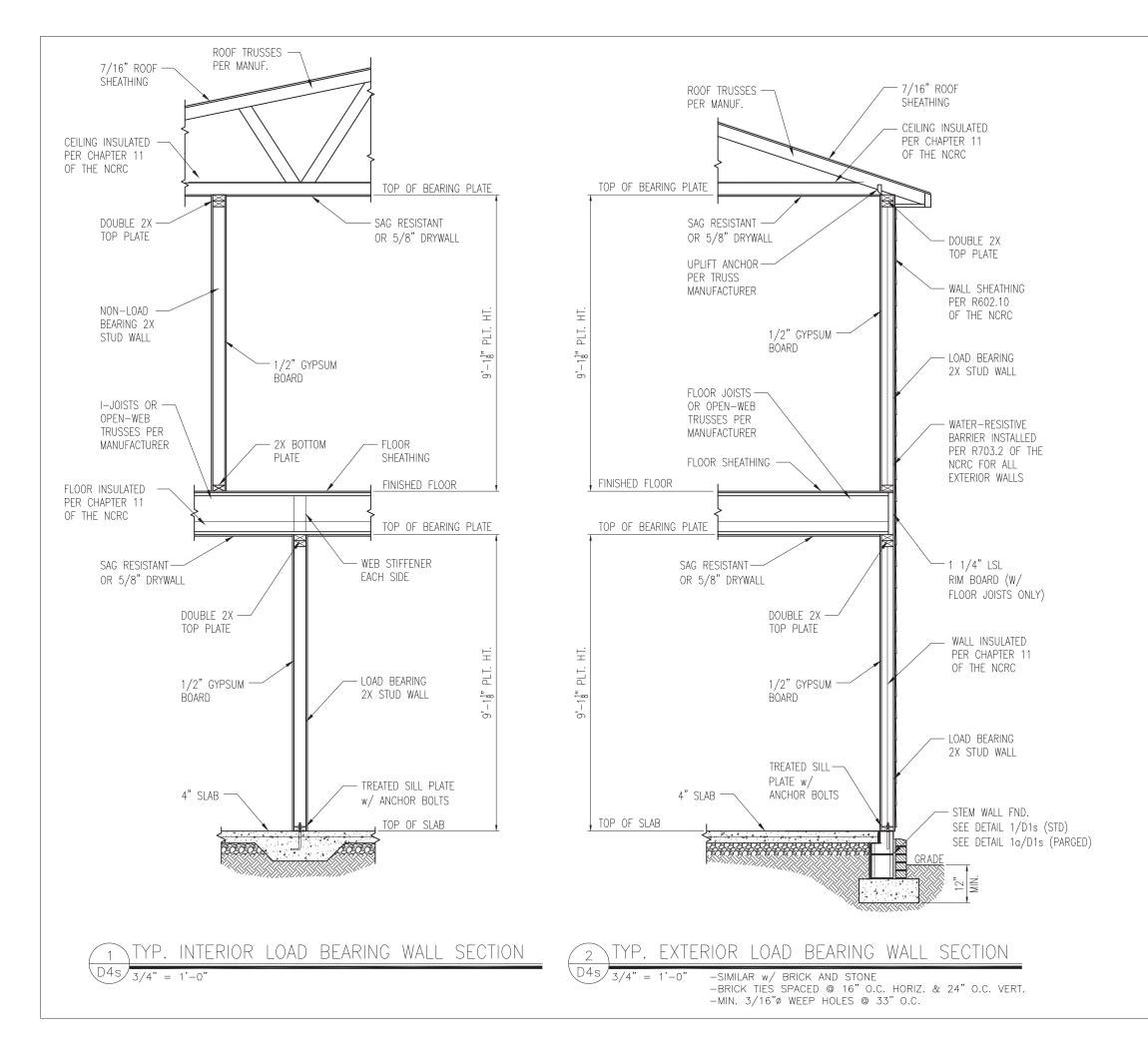
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DATE PROJECT # 1/7/16

REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

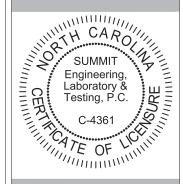
SHEET

D3s





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CURRENT DRAWING

Standard Details

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SCALE: NTS

PRO1ECT #: 3832

DRAWN BY: LBV

CHECKED BY: WAJ

ORIGINAL DRAWING

DATE PROJECT # 1/7/16 3832

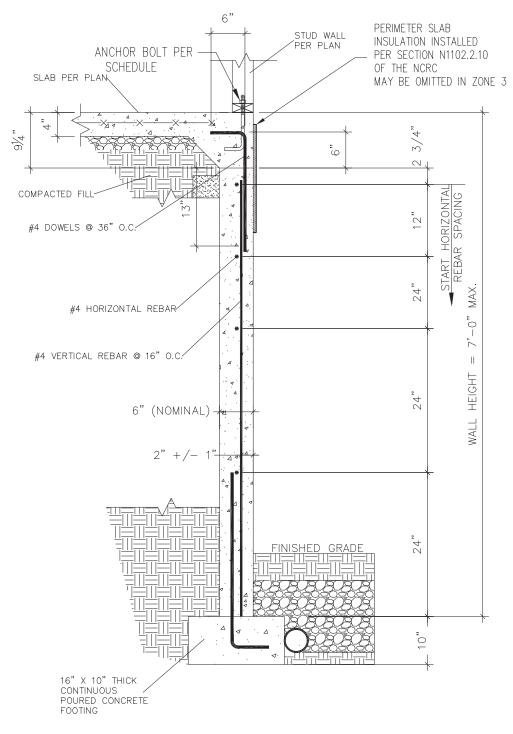
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SHEET

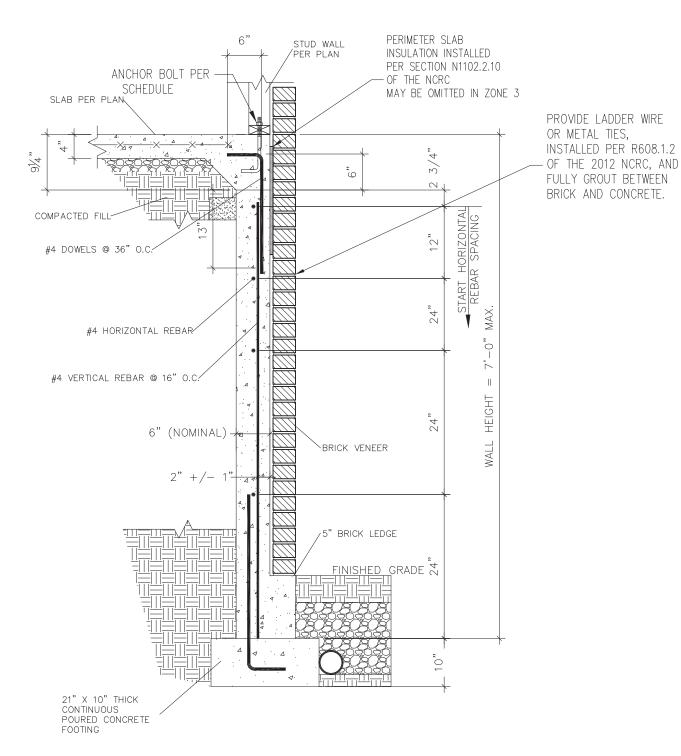
D4s

- 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON SHEET CS2 FOR ADDITIONAL INFORMATION.
- 2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
- 3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.









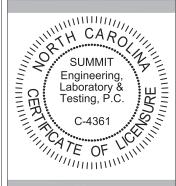
SUBWALL FOUNDATION W/ BRICK VENEER

3/4" = 1'-0"





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21

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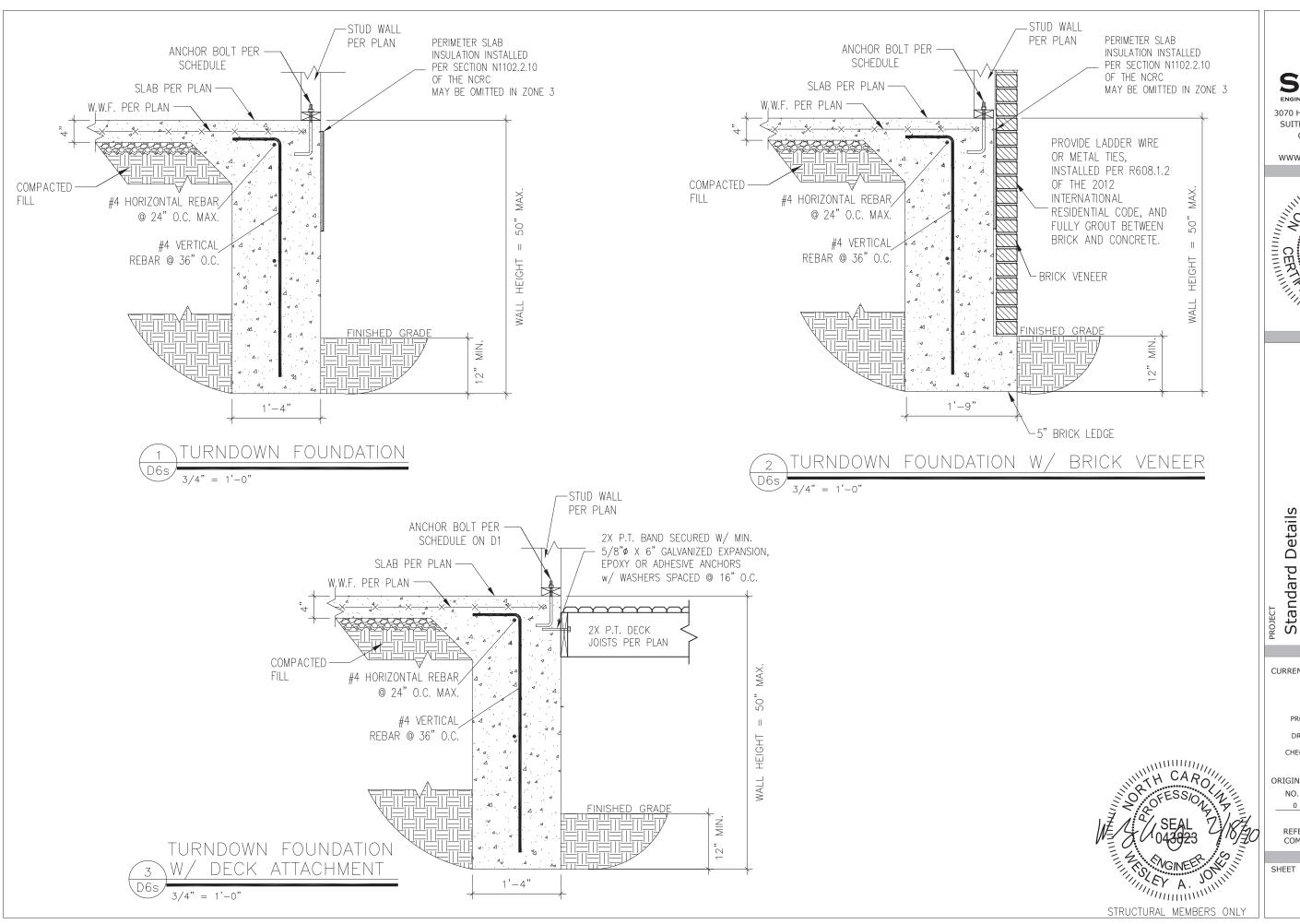
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NO. DATE PROJECT #
0 1/7/16 3832

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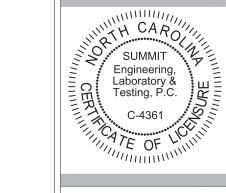
SHEET

D5s





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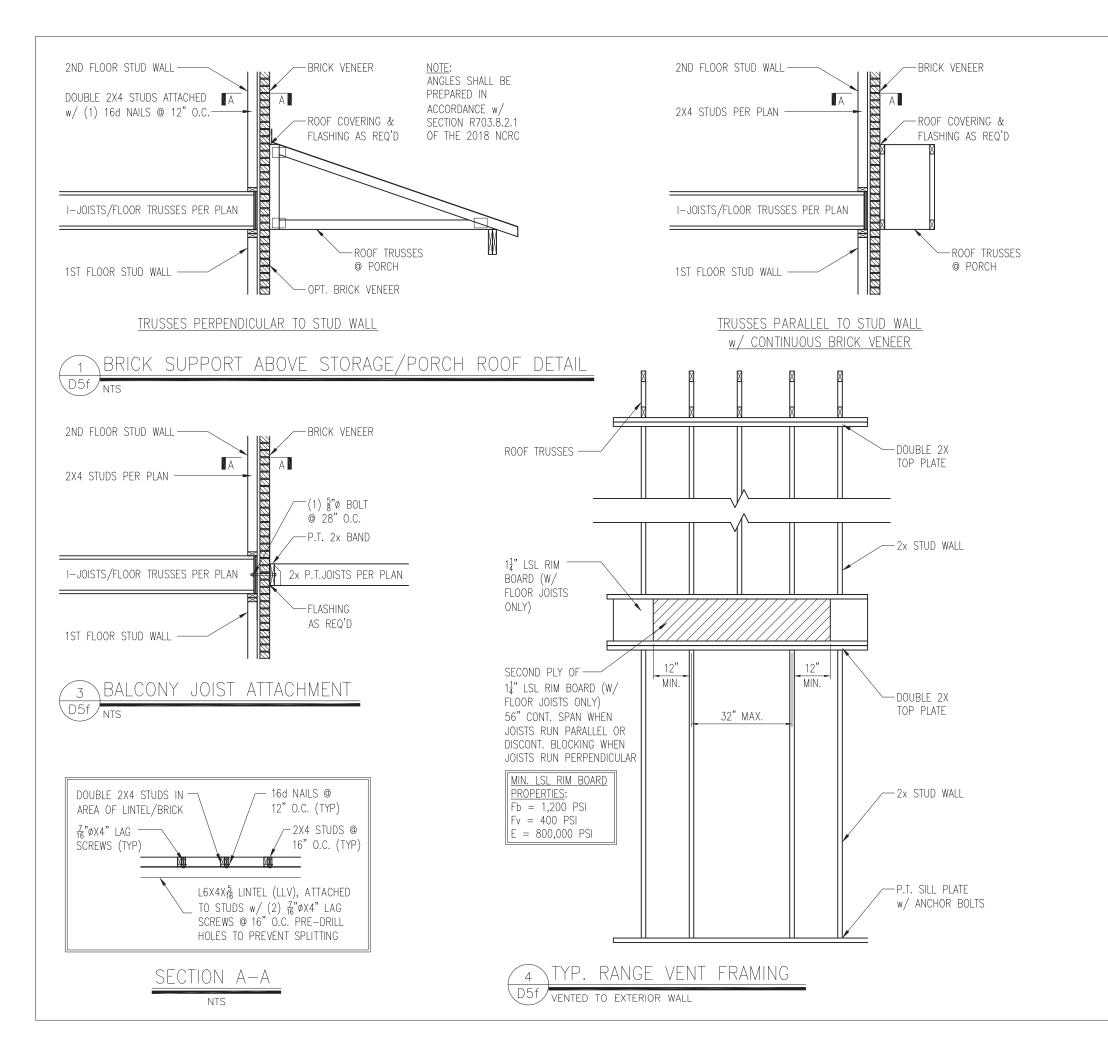
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DATE PROJECT # 1/7/16 3832

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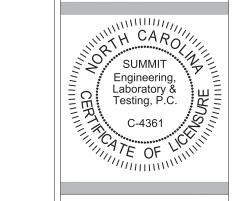
SHEET

D6s





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Woodstock, GA 30188

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PROJECT #: 3832

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ORIGINAL DRAWING

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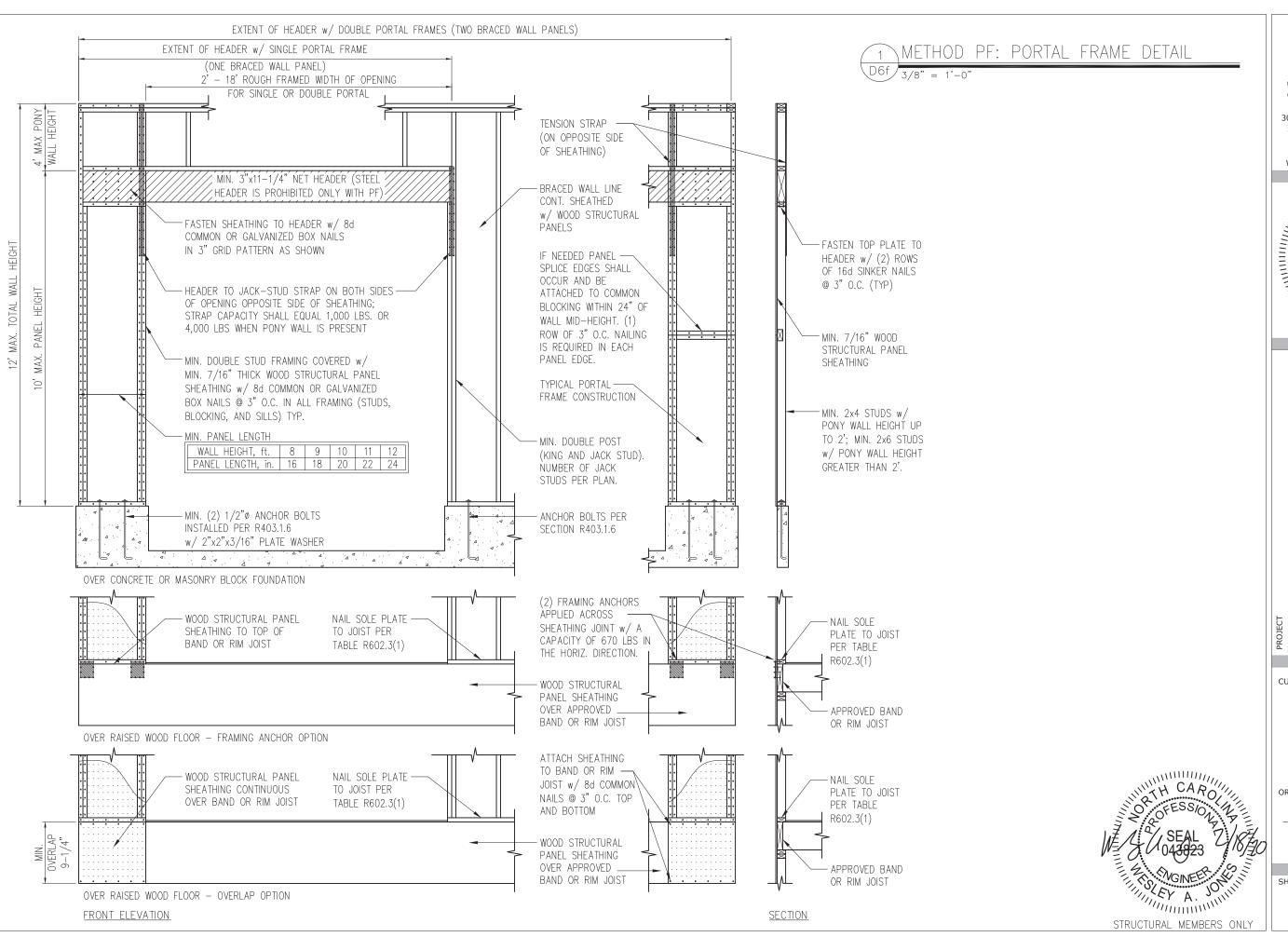
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

THEY A. JOHN

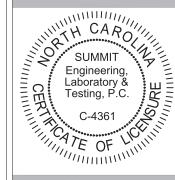
STRUCTURAL MEMBERS ONLY

D5f





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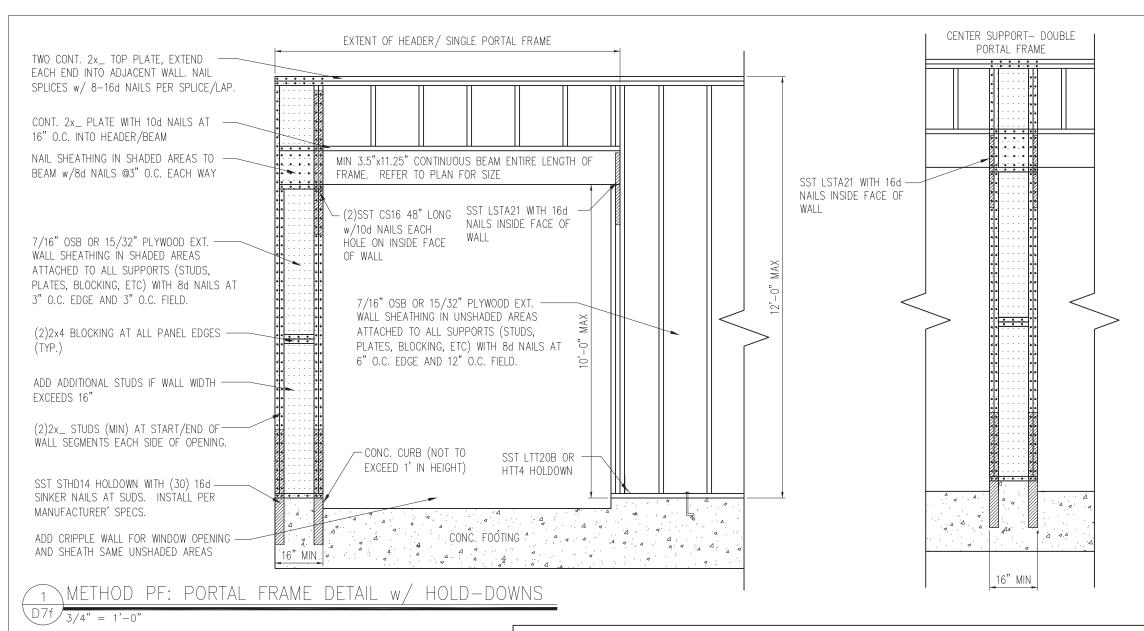
ORIGINAL DRAWING

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SHEET

D6f



SPACING PER SCHEDULE MIN. 2... MIN.

ELEVATION VIEW

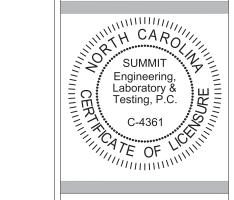
MULTI-PLY BEAM CONNECTION DETAIL

MINIMUM FASTE	NING	3¹/₂″ WIDE	51/4" WIDE		7" WIDE		
REQUIREMENTS TOP- AND SIDE- MEMBERS	OIL						
FASTENER TYPE	LVLDEPTH	2-Ply 13/4"	3-Ply 13/4"	13/4" + 31/2"	4-Ply 13/4"	2-Ply 13/4" + 31/2"	2-Ply 31/2"
10d (0.128" x 3")	7¼"≤d<14"	3 rows @ 12" o.c.	3 rows @ 12" o.c. (ES)	3 rows @ 12" o.c.		3 rows @ 12" o.c. (ES)	-
Nails	d≥14″	4 rows @ 12" o.c.	4 rows @ 12" o.c. (ES)	4 rows @ 12" o.c.		4 rows @ 12" o.c. (ES)	-
16d (0.162" x 31/2")	7¼"≤d<14"	2 rows @ 12" o.c.	2 rows @ 12" o.c. (ES)	2 rows @ 12" o.c.	-	2 rows @ 12" o.c. (ES)	-
Nails	d≥14″	3 rows @ 12" o.c.	3 rows @ 12" o.c. (ES)	3 rows @ 12" o.c.	-	3 rows @ 12" o.c. (ES)	-
½" Through Bolts		2 rows @ 24" o.c.	2 row	s @ 24" o.c.		2 rows @ 24" o.c.	
SDS ¼" x 3½", WS35, 3¾" TrussLok	3.71///	2 rows @ 24" o.c.	2 rows @ 24" o.c. (ES)	2 rows @ 24" o.c.	•	2 rows @ 24" o.c. (ES)	-
SDS 1/4" x 6", WS6	d≥7¼″	-	-			2 rows @ 24" o.c. (ES)	
5" TrussLok		-	2 rows @ 24" o.c.		-		
6¾" TrussLok			- 2 rows @ 24" o.c.				

- multiple-ply members must meet the minimum fastening and side-loading capacity requirements given on page 48.
- . Minimum fastening requirements for depths less than $7\frac{1}{4}$ require special consideration. Please contact your technical representative
- (1) if staggering or offsetting is not referenced, then none is required;
- (2) if staggering is referenced, then fasteners installed in adjacent rows on the front side are to be staggered up to one-half the o.c. spacing, but maintaining the fastener clearances above; and
- (3) if "ES" is referenced, then the fastener schedule must be repeated on each side, with the fasteners on the back side offset up to one-half the o.c. spacing of the front side (whether or not it is staggered).



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DATE PROJECT # 1/7/16 3832

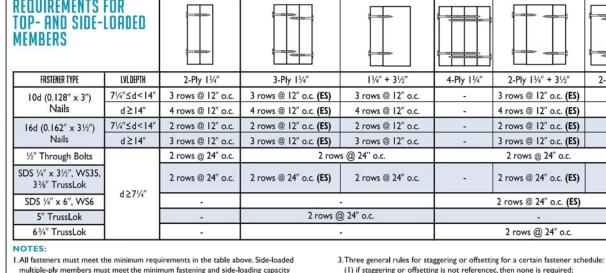
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

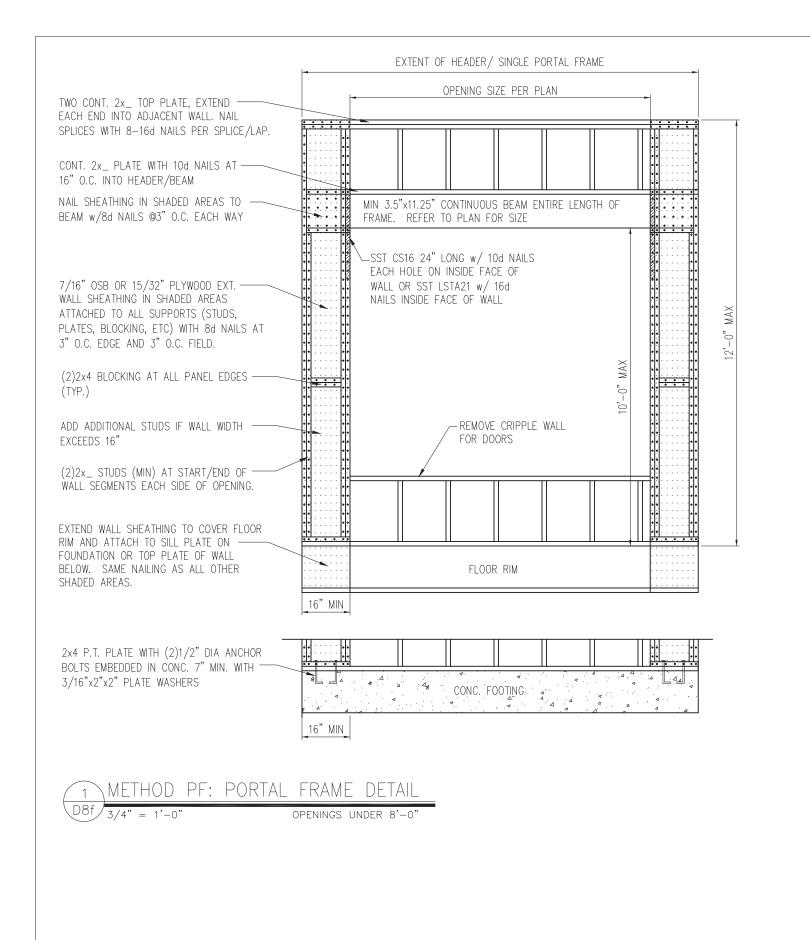
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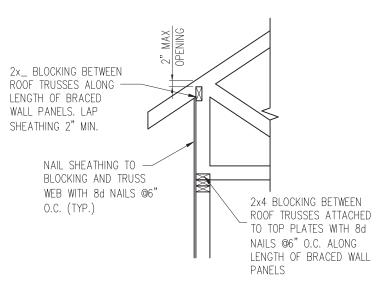
THEY A. JOHN

STRUCTURAL MEMBERS ONLY

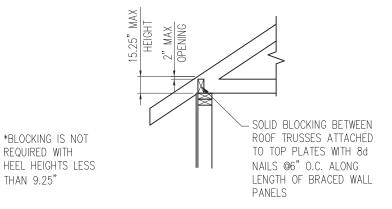
D7f







HEEL HEIGHT GREATER THAN 15.25"



HEEL HEIGHT LESS THAN 15.25" *

YP. WALL PANEL TO ROOF TRUSS CONNECTION

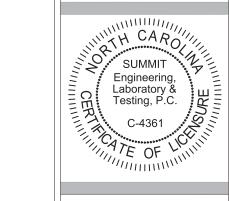
REQUIRED WITH

THAN 9.25"





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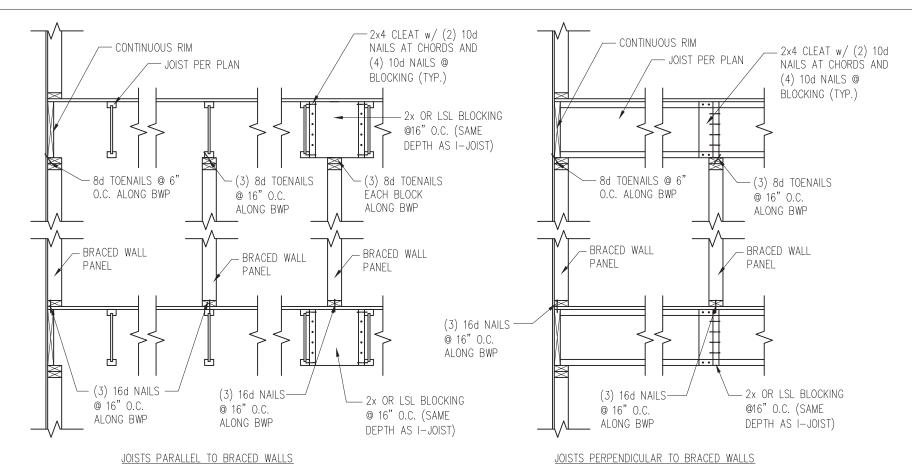
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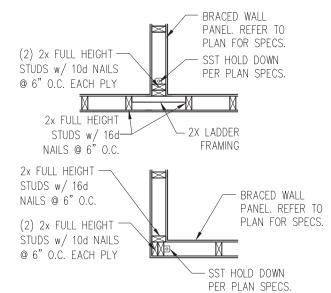
DATE PROJECT # 1/7/16 3832

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SHEET

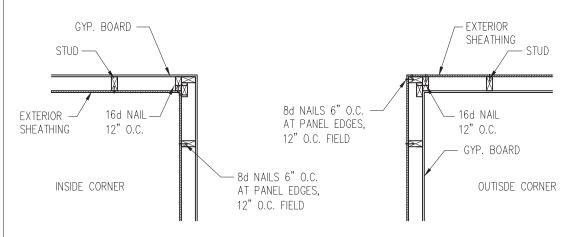
D8f

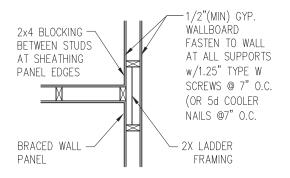




TYP. HOLD DOWN DETAIL
D9f

1 TYP. WALL PANEL TO FLOOR/CEILING CONNECTION D9f/1" = 1'-0"

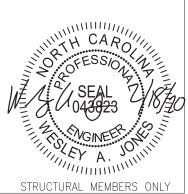




2 TYP. EXTERIOR CORNER FRAMING

3 INTERIOR 3-STUD WALL INTERSECTION

D9f 1" = 1'-0"





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C-4361

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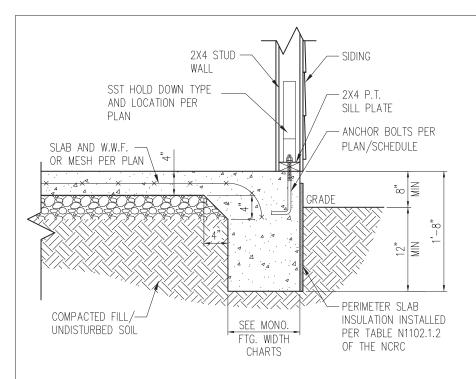
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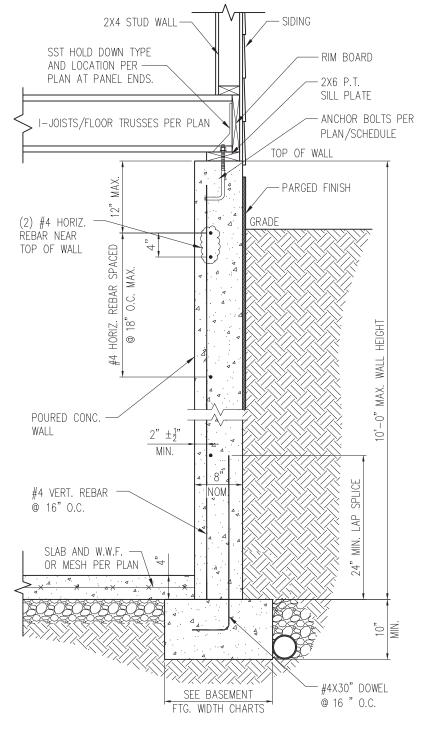
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D9f

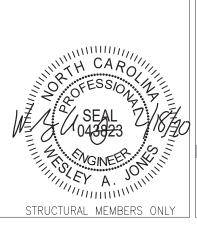


SLAB DETAIL w/ HOLD-DOWN



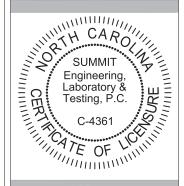
STANDARD - SIDING

BASEMENT FOUNDATION WALL DETAIL W/ HOLD-DOWN





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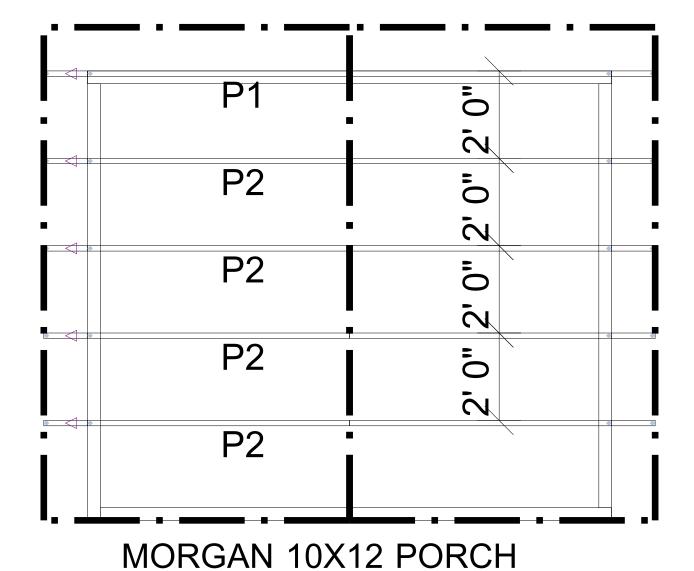
REFER TO COVER SHEET FOR A COMPLETE LIST OF REVISIONS

SHEET

D10f

44' 0" Indicates Left End of Truss 2' 01 2 **A**2 B2 **B**2 HVAC / STORAGE HIP LINES:6.08 V06 **VALLEY LINES:** V05 V04 V03 Hatch Legend OPT MSTR TRAY 2025.36_RIDGE LINE: 62.23 HVAC / STORAGE **MORGAN BEH** V09 2' 0'|| 2' 0'|| 2' 0'|| 2' 0'|| 2' 0'|| 2' 0'|| 2' 0'|| 2' 0'|| 2' 0'|| 2' 0'|| 2' 0 20' 0" 24' 0" 44' 0"

MASTER



LINE: 11.29 ft 166.63 ft²_RIDGE

			Products		
Fab Type	Net Qty	Plies	Product	Length	PlotID
MFD	8	1	14" TJI® 110	30' 0"	J1
MFD	1	1	14" TJI® 110	15' 0"	J2
MFD	5	1	14" TJI® 110	13' 0"	J3
MFD	1	1	14" TJI® 110	7' 0"	J4
MFD	2	1	14" TJI® 110	5' 0"	J5
MFD	1	1	14" TJI® 110	4' 0"	J6
MFD	2	2	14" TJI® 210	17' 0"	J7
MFD	10	1	14" TJI® 360	36' 0"	J8
MFD	2	2	1 3/4" x 9 1/4" 2.0E Microllam® LVL	9' 0"	2H-1
FF	12	1	1 1/8" x 14" TJ® Rim Board	16' 0"	RIM-1
MFD	2	1	14" TJI® 110	2' 0"	Bk1

Cor	nnector Si	ummary	/
Product	Manuf	Qty	PlotID
TFL1714	USP	7	H1

PLAN LEGEND *INDICATES BEAM ABOVE TOP PLATE (FLUSH WITH FLOOR SYSTEM) 1B-, 2B-INDICATES BEAM BY BBO-

H-, 1H-, GDH-W FLUDIC SYSTEM)

**BEAMS MAY PROTRUDE ABOVE OR BELOW

DECKING OR TOP PLATE RESPECTIVELY, REFER

TO DETAIL IF BEAM IS A DIFFERENT DEPTH THAN

FLOOR SYSTEM

SINGLE PLY BEAM (ADD LINE FOR EACH ADDITIONAL

Avoid Plumbing Drops •

FIELD TRIM NON RED END TO KEEP HOLES ALIGNED CONTAR EL LADO DE SIN MARCA ROJA RARA HOYOS ALINEADOS

FIELD LOCATE PLUMBING DROPS/CAN **LIGHTS, ETC... PRIOR TO JOIST SECUREMENT TO** AVOID INTERFERENCE

GENERAL NOTES:

1.) TOP CHORD OF JOISTS ARE PAINTED RED AT NUMBERED END. PLACE PAINTED END AS NOTED ON PLAN.

2.) FOLLOW SPECIAL SPACING AND LOCATION

2.) POLICUM SPECIAL SPACING AND LOCATION
DIMENSIONS FOR EXTRAS OR SHIFTED JOISTS
AS SHOWN ON PLAN.
3.) ALL INTERIOR WALL PLATES MUST BE LEVEL
WITH OUTSIDE WALL TOP PLATES.
4.) DO NOT STACK CONSTRUCTION LOADS ON
UN-BRACED JOISTS.

5.) PROVIDE SOLID SUPPORT BELOW ALL BEAM AND HEADER BEARING POINTS IN WALL AND JOIST SPACES CONTINUOUS DOWN TO THE

FOUNDATION.
6.) LOCATE CRIPPLE STUDS IN JOIST SPACE DIRECTLY BELOW HEADER JACKS AT ALL FIRST FLOOR EXTERIOR DOOR LOCATIONS.
7.) INSTALL NAILS IN ALL HOLES PROVIDED IN JOIST HANGERS EXCEPT AT BOTTOM CHORD SEAT. PLACE A DAB OF GLUE IN THE HANGER SEAT BEFORE SETTING JOISTS. 8.) IMPORTANT NOTE! NO STRUCTURAL

ANALYSIS OF CONVENTIONAL HEADERS HAS BEEN CONDUCTED IF NOT NOTED. THEY ARE CONSIDERED TO BE ADEQUATE TO SUPPORT

FRAMER NOTE ALL DIMENSIONS TO CENTERLINE UNLESS

DOUBLE

FRAMER NOTE

DENOTES DUCT HOLE RUNS

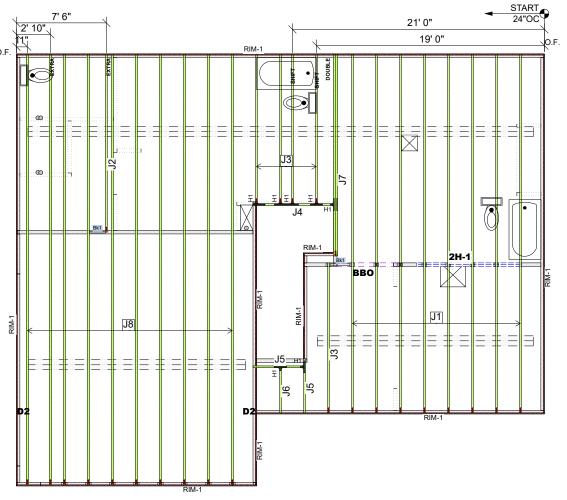
1. GLUE AND NAIL PLYWOOD SUBFLOOR TO BEAMS AND GIRDERS AT 6" O/C WHERE NO WALL IS ABOVE. 2. FILL HANGER SEAT WITH GLUE BEFORE SETTING JOIST IN HANGER. FILL ROUND HOLES WITH

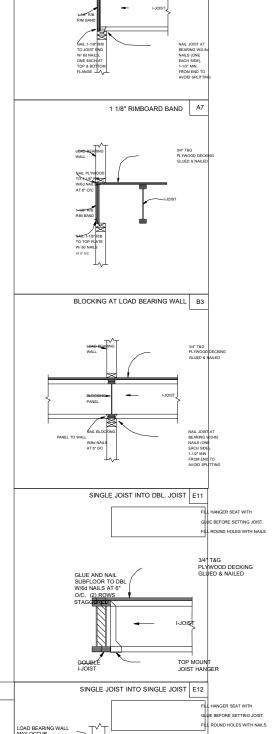
CRITICAL !! INSTALL 2X4 SQUASH BLOCKS

IN FLOOR TRUSS SPACE BELOW ALL EXTERIOR DOOR HEADER JACKS. CUT 1/16" TALLER THAN TRUSS.

FIELD VERIFY DIMENSIONS TO **JOISTS LOCATED UNDER WALLS!!** 2ND FLOOR LAYOUT

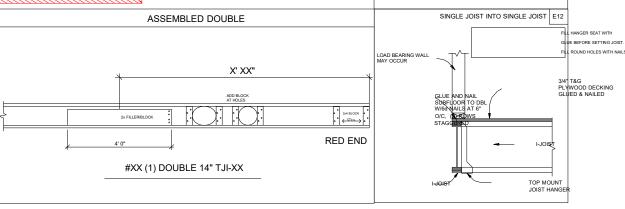
NAILS.





1 1/8" RIMBOARD BAND





UFP MID-ATLANTIC, LLC

| Customer | SMITH DOUGLAS | SMITH DOUGLAS | SMITH DOUGLAS | MORGAN | Date:3/25/21 | Scale:NTS | Revision Date:2.

Drawn By: CP3 Drawing Number

21032980F2

MSTR