



AVAILABLE WITH OPTIONAL  
9'-1" FIRST FLOOR PLATE

- NOTES AT OPT 9'-1" FLT:
- W/DW HT SET AT 7'-6"
  - INTERIOR SOFFITS AT 8'-0"
  - EXTERIOR SOFFITS AT 8'-0"

**NOTES:**

- GRADE CONDITIONS MAY VARY FOR INDIVIDUAL SITE FROM THAT SHOWN. BUILDER SHALL VERIFY AND CORRECTIVE PER ACTUAL SITE CONDITIONS.
- WINDOW HEAD HEIGHTS:
- 1ST FLOOR - 4'-0" UNCL. ON ELEVATIONS.
- 2ND FLOOR - 7'-0" UNCL. ON ELEVATIONS.
- ROOFING: FINISHED SHINGLES PER DEVELOPER.
- WINDOWING: MANUFACTURER PER DEVELOPER. DIVIDED LITES AS SHOWN ON THE EXTERIOR ELEVATIONS.
- ENTRY DOOR: AS SELECTED BY DEVELOPER.
- GARAGE DOORS: AS SELECTED BY DEVELOPER. PAIRED PANEL AS SHOWN.
- ALL EXTERIOR MATERIALS TO BE INSTALLED PER MANUFACTURER'S WRITTEN RESTRICTIONS.
- PROTECTION AGAINST DECAY: ALL PORTIONS OF A FURROW, SCREEN, PULPUS OR DECK FROM THE BOTTOM OF THE HEADER DOWN, INCLUDING POSTS, RALES, PICKETS, STEPS AND FLOOR STRUCTURE.)
- INSULATION: PER TABLE R602.2.2.
- EXTERIOR WALLS: R-5 BATT INSULATION, VERIFY.
- CEILING WITH ATTIC ABOVE: R-30 BATT INSULATION, VERIFY.
- FLOOR OVER GARAGE: R-10 BATT INSULATION, VERIFY.
- ATTIC DEEPWALL: R-10 BATT INSULATION, VERIFY.
- CRANE SPACE FLOORING: R-10 BATT INSULATION, VERIFY.

**KEY NOTES:**

**MASONRY:**

- [1] ADHERED STONE VENEER AS SELECTED BY DEVELOPER. HEIGHT AS NOTED.
- [2] MASONRY FULL BRICK AS SELECTED BY DEVELOPER. HEIGHT AS NOTED.
- [3] MASONRY FULL STONE AS SELECTED BY DEVELOPER. HEIGHT AS NOTED.
- [4] 1/2" SOLID PIER COURSE.
- [5] ROWLOCK COURSE.
- [6] NA.

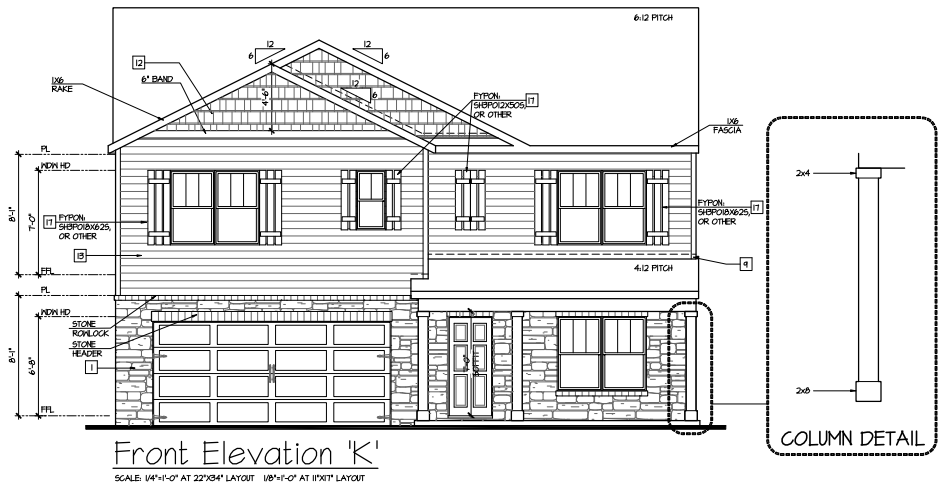
**TYPICALS:**

- [7] CORROSION RESISTANT SCREEN COVERED VENTS, SIZE AS NOTED.
- [8] CODE APPROVED TERMINATION GIMNEY CAP.
- [9] CORROSION RESISTANT ROOF TO WALL FLASHING, CODE COMPLIANT FLASHING PER MANUFACTURER'S INSTRUCTIONS.
- [10] STANDING SEAM METAL ROOF, INSTALL PER MANUFACTURER'S WRITTEN RESTRICTIONS.
- [11] DECORATIVE BROUGHT IRON, SEE DETAILS.

**SIDING:**

- [12] VINYL SHAKE SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS).
- [13] FIBER CEMENT SHAKE SIDING PER DEVELOPER (1/4" CORNER TRIM BOARD).
- [14] VINYL LAP SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS).
- [15] FIBER CEMENT LAP SIDING PER DEVELOPER (1/4" CORNER TRIM BOARD).
- [16] VINYL HAVY SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS).
- [17] FIBER CEMENT HAVY SIDING PER DEVELOPER (1/4" CORNER TRIM BOARD).
- [18] VINYL BOARD AND BATT SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS).
- [19] FIBER CEMENT PANEL SIDING (1/2" BATT) AT 12" O.C. PER DEVELOPER (1/4" CORNER TRIM BOARD).
- [20] VINYL TRIM SIZE AS NOTED. (AT SPECIFIC LOCATIONS).
- [21] FIBER CEMENT TRIM OR EQUAL, UNCL. UNCL. SIZE AS NOTED. (AT SPECIFIC LOCATIONS).
- [22] PLYCON SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED. (AT SPECIFIC LOCATIONS). FALSE VINYL SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED. (AT SPECIFIC LOCATIONS).

ALL WINDOW HEADS OPENING IS LESS THAN 24" ABOVE THE FLOOR. AND WINDOW OPENING IS GREATER THAN 12" ABOVE THE FINISHED FINISHING SURFACE MUST HAVE WINDOW OPENING LIMITING DEVICES COMPLYING WITH THE IBC, SECTION R602.2.1 AND R602.2.2.



Front Elevation 'K'  
SCALE: 1/4"=1'-0" AT 22'x34' LAYOUT 1/8"=1'-0" AT 11'x11' LAYOUT

NO.	DATE	REVISION
1	01.20.22	

PROFESSIONAL SEAL

PROJECT TITLE:  
**40' Series**

CLIENT NAME:  
**Express HOMES**

PROJECT NO.: GMD11049

SHEET TITLE:  
**'HAYDEN' EXTERIOR ELEVATIONS '4EPF-K'**

PRINT DATE:  
January 22, 2021

SHEET NO.:  
**1K**

FOR CONSTRUCTION

ATTIC VENT CALCULATION FOR PLAN 'HAYDEN': 1:150 RATIO.

THE NET FREE VENTILATING AREA SHALL NOT BE LESS THAN 1/500 OF THE AREA OF THE SPACE VENTILATED, PROVIDED THAT AT LEAST 50 PERCENT AND NOT MORE THAN 80 PERCENT OF THE REQUIRED VENTILATING AREA IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED AT LEAST 3 FEET ABOVE THE EAVE OR CORNICE VENTS WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY EAVE OR CORNICE VENTS.

EXCEPTIONS:  
1. ISOLATED ATTIC/RAFTER SPACES REQUIRING LESS THAN 1.50 FT<sup>2</sup> OF VENTILATION MAY BE VENTED WITH CONTINUOUS SOFFIT VENTILATION ONLY.

2. ENCLOSED ATTIC/RAFTER SPACES OVER UNCONDITIONED SPACE MAY BE VENTED WITH CONTINUOUS SOFFIT VENT ONLY.

GENERAL CONTRACTOR SHALL VERIFY THE NET FREE VENTILATION OF THE VENT PRODUCT SELECTED BY OWNER. VERIFY WITH MANUFACTURER OF HIGH AND LOW VENTS TO BE USED FOR MINIMUM CALCULATED VENTS REQUIRED. THE REQUIRED VENTILATION SHALL BE MAINTAINED. PROVIDE INSULATION STOP SUCH THAT INSULATION DOES NOT OBSTRUCT FREE AIR MOVEMENT AS REQUIRED BY THE BUILDING OFFICIAL.

ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF SHEATHING (AS ALLOWED BY THE STRUCTURAL ENGINEER) TO ALLOW PASSAGE AND ATTIC VENTILATION.

BETWEEN THE TWO OR ISOLATED ATTIC SPACES SHALL BE VENTED INDEPENDENTLY TO CBC REQUIREMENTS.

PER DEVELOPER, AT ALL CANTILEVERED FLOORS, CANTILEVERED ARCHITECTURAL POP-UPS, AND ANY DOUBLE FRAMING PROJECTIONS THAT ARE SEPARATED FROM THE VENTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2" CORROSION RESISTANT SOFFIT VENT AT UNDERSIDE OF FRAMED ELEMENT.

ALL ROOF DRAINAGE SHALL BE PIPED TO STREET OR APPROVED DRAINAGE FACILITY.  
- DASHED LINES INDICATE WALL BELOW.  
- LOCATE BUTTER AND DOWNSPOUTS PER BUILDER.  
- PITCHED ROOFS AS NOTED.

(PER SECTION R306.2)  
1 SQUARE INCH VENT FOR EVERY 150 SQUARE INCHES OF CEILING  
144 SQ. IN. = 1 SQ. FT.  
BLDG. CEILING (9'9" X 144' = 1426.40 SQ. IN.)  
BLDG. (50. IN.) / 150 = 50. IN. OF VENT REQUIRED

ROOF AREA 1 = 1488 SQ. FT. X 144' = 214272 SQ. IN.  
214272 SQ. IN. / 150 = 1428.48 SQ. IN. OF VENT REQ'D

ROOF AREA 2 = 39 SQ. FT. X 144' = 5616 SQ. IN.  
5616 SQ. IN. / 150 = 37.44 SQ. IN. OF VENT REQ'D

ROOF AREA 3 = 180 SQ. FT. X 144' = 25920 SQ. IN.  
25920 SQ. IN. / 150 = 172.80 SQ. IN. OF VENT REQ'D

NOTES:

- TRUSS MANUFACTURER SHALL SUBMIT STRUCTURAL CALCS AND SHOP DRAWINGS TO THE BUILDER'S GENERAL CONTRACTOR AND BUILDING DEPARTMENT FOR REVIEW PRIOR TO FABRICATIONS.  
- ALL RUNNING VENTS SHALL BE CORNERED INTO A MINIMUM ANCHOR OF ROOF PENETRATIONS. ALL ROOF PENETRATIONS SHALL OCCUR TO THE REAR OF THE MAIN RIDGE.

ATTIC VENT CALCULATION FOR PLAN 'HAYDEN': 1:300 RATIO.

AS AN ALTERNATE TO THE 1/500 RATIO LISTED ABOVE, THE NET FREE CROSS-VENTILATION AREA MAY BE REDUCED TO 1/300 WHEN A GLASS OR II VAPOR RETARDER IS INSTALLED ON THE WARM - IN - WINTER SIDE OF THE CEILING.

GENERAL CONTRACTOR SHALL VERIFY THE NET FREE VENTILATION OF THE VENT PRODUCT SELECTED BY OWNER. VERIFY WITH MANUFACTURER OF HIGH AND LOW VENTS TO BE USED FOR MINIMUM CALCULATED VENTS REQUIRED. THE REQUIRED VENTILATION SHALL BE MAINTAINED. PROVIDE INSULATION STOP SUCH THAT INSULATION DOES NOT OBSTRUCT FREE AIR MOVEMENT AS REQUIRED BY THE BUILDING OFFICIAL.

ALL OVERLAP FRAMED ROOF AREAS SHALL HAVE OPENINGS BETWEEN THE ADJACENT ATTICS IN THE ROOF SHEATHING (AS ALLOWED BY THE STRUCTURAL ENGINEER) TO ALLOW PASSAGE AND ATTIC VENTILATION.

BETWEEN THE TWO OR ISOLATED ATTIC SPACES SHALL BE VENTED INDEPENDENTLY TO CBC REQUIREMENTS.

PER DEVELOPER, AT ALL CANTILEVERED FLOORS, CANTILEVERED ARCHITECTURAL POP-UPS, AND ANY DOUBLE FRAMING PROJECTIONS THAT ARE SEPARATED FROM THE VENTING CALCULATIONS SHOWN ABOVE, PROVIDE A CONTINUOUS 2" CORROSION RESISTANT SOFFIT VENT AT UNDERSIDE OF FRAMED ELEMENT.

(PER SECTION R306.2)  
1 SQUARE INCH VENT FOR EVERY 300 SQUARE INCHES OF CEILING  
144 SQ. IN. = 1 SQ. FT.  
BLDG. CEILING (9'9" X 144' = 1426.40 SQ. IN.)  
BLDG. (50. IN.) / 300 = 50. IN. OF VENT REQUIRED

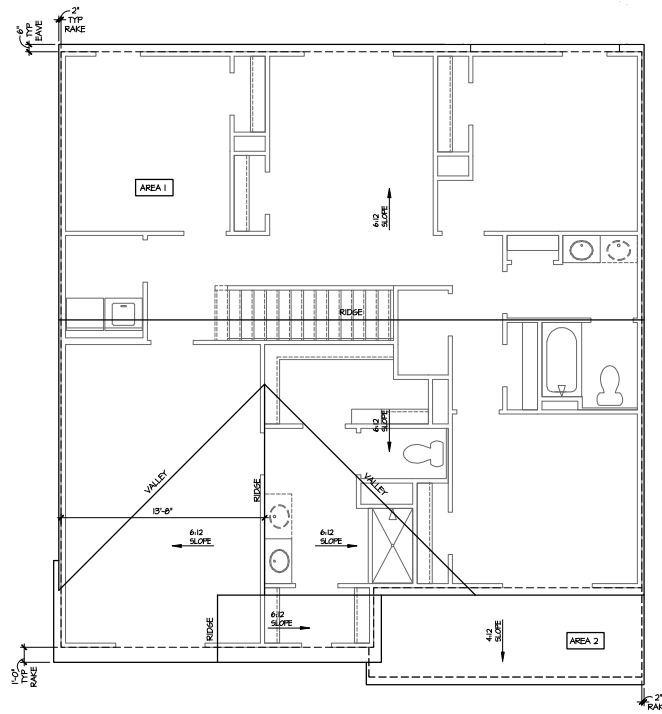
ROOF AREA 1 = 1488 SQ. FT. X 144' = 214272 SQ. IN.  
214272 SQ. IN. / 300 = 714.24 SQ. IN. OF VENT REQ'D  
714.24 SQ. IN. / 2 = 357.12 SQ. IN. OF VENT AT HIGH & 357.12 SQ. IN. OF VENT AT LOW REQUIRED.

ROOF AREA 2 = 39 SQ. FT. X 144' = 5616 SQ. IN.  
5616 SQ. IN. / 300 = 18.72 SQ. IN. OF VENT REQ'D  
18.72 SQ. IN. / 2 = 9.36 SQ. IN. OF VENT AT HIGH & 9.36 SQ. IN. OF VENT AT LOW REQUIRED.

ROOF AREA 3 = 180 SQ. FT. X 144' = 25920 SQ. IN.  
25920 SQ. IN. / 300 = 86.40 SQ. IN. OF VENT REQ'D  
86.40 SQ. IN. / 2 = 43.20 SQ. IN. OF VENT AT HIGH & 43.20 SQ. IN. OF VENT AT LOW REQUIRED.

BUILDER TO PROVIDE (2) LAYERS OF UNDERLAYMENT AT ANY ROOF W/ A SLOPE FROM 2:12 TO LESS THAN 4:12

AT SINGLE FAMILY DETACHED PLANS:  
- PRE-FINISHED VENTED SOFFIT AT EAVE PER MANUFACTURER.  
- (VERIFY FIRE SEPARATION DISTANCE FOR SOFFIT PROTECTION PER NICC. SECTION R302.1.1 AND TABLE R302.1)



Roof Plan 'K'  
SCALE: 1/4"=1'-0" AT 22'x34' LAYOUT 1/8"=1'-0" AT 11'x17' LAYOUT

NO.	DATE	REVISION
1	01.20.21	

PROFESSIONAL SEAL:

PROJECT TITLE:

40' Series

CLIENTS NAME:



PROJECT NO: GMD170491

SHEET TITLE:  
'HAYDEN'  
ROOF PLAN  
'4EPF-K'

PRINT DATE:  
January 22, 2021

SHEET NO:  
1.1 K

FOR CONSTRUCTION

**NOTES:**

- GRADE CONDITIONS MAY VARY FOR INDIVIDUAL SITE FROM THAT SHOWN. BUILDER SHALL VERIFY AND COORDINATE PER ACTUAL SITE CONDITIONS.
- WINDOW HEAD HEIGHTS.
- 1ST FLOOR = 6'-0" UNO, ON ELEVATIONS.
- 2ND FLOOR = 1'-0" UNO, ON ELEVATIONS.
- ROOFING, PITCHED SHINGLES PER DEVELOPER.
- WINDOWS, MANUFACTURER PER DEVELOPER, DIVIDED LITES AS SHOWN ON THE EXTERIOR ELEVATIONS.
- ENTRY DOOR, AS SELECTED BY DEVELOPER.
- GARAGE DOORS, AS SELECTED BY DEVELOPER, RAISED PANEL AS SHOWN.
- ALL EXTERIOR MATERIALS TO BE INSTALLED PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
- PROTECTION AGAINST DECAY:
  - (ALL PORTIONS OF A NORMAL SCREEN FLOOR OR DECK FROM THE BOTTOM OF THE HEADER DOWN, INCLUDING POST, RAILS, PICKETS, STEPS AND FLOOR STRUCTURE)
- INSULATION, PER TABLE N1021.2:
  - EXTERIOR WALLS: R-5 BATTIS MINIMUM, VERIFY
  - CELLING WITH ATTIC ABOVE: R-30 BATTIS MINIMUM, VERIFY
  - FLOOR OVER GARAGE: R-18 BATTIS MINIMUM, VERIFY
  - ATTIC KNEEWALL: R-18 BATTIS MINIMUM, VERIFY
  - GRANL. SPACE FLOORING: R-18 BATTIS MINIMUM, VERIFY

**KEY NOTES:**

**MASONRY:**

- ADHERED STONE VENEER AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.
- MASONRY FULL BRICK AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.
- MASONRY FULL STONE AS SELECTED BY DEVELOPER, HEIGHT AS NOTED.
- 1" SOLID CER. COURSE.
- ROCKLACK COURSE.
- N/A

**TYPICALS:**

- CORROSION RESISTANT SCREEN LOUVERED VENTS, SIZE AS NOTED.
- CODE APPROVED TERMINATION GARNEY CAP.
- CORROSION RESISTANT ROOF TO WALL FLASHING, CODE COMPLIANT FLASHING PER NRC R105.2.8.5
- STANDING SEAM METAL ROOF, INSTALL PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
- DECORATIVE WROUGHT IRON, SEE DETAILS.

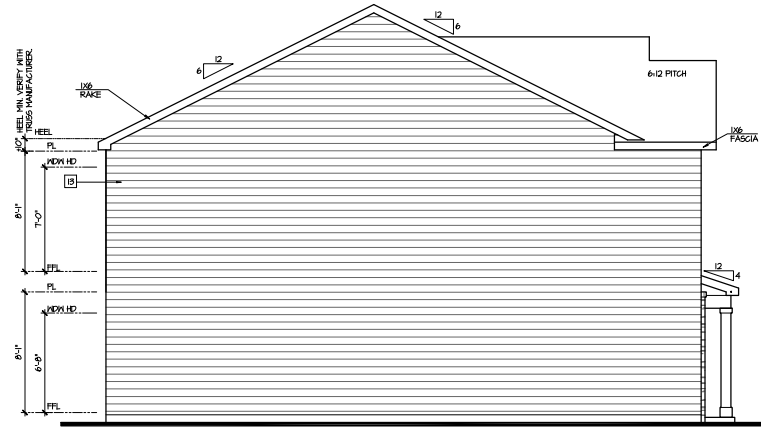
**SIDING:**

- VINYL SHAKE SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS. FIBER CEMENT SHAKE SIDING PER DEVELOPER W/ 1/4" CORNER TRIM BOARD.)
- VINYL LAP SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS. FIBER CEMENT LAP SIDING PER DEVELOPER W/ 1/4" CORNER TRIM BOARD.)
- VINYL NAVY SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS. FIBER CEMENT NAVY SIDING PER DEVELOPER W/ 1/4" CORNER TRIM BOARD.)
- VINYL BOARD AND BATT SIDING PER DEVELOPER WITH VINYL CORNER TRIM PER DEVELOPER. (AT SPECIFIED LOCATIONS. FIBER CEMENT PANEL SIDING W/ 1/2" BATTIS AT 12" O.C. PER DEVELOPER W/ 1/4" CORNER TRIM BOARD.)
- VINYL TRIM SIZE AS NOTED. (AT SPECIFIC LOCATIONS: IX FIBER CEMENT TRIM OR EQUAL, UNO, SIZE AS NOTED)
- PLYM SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED. (AT SPECIFIC LOCATIONS: FALSE VINYL SHUTTERS, TYPE AS SHOWN, SIZE AS NOTED)

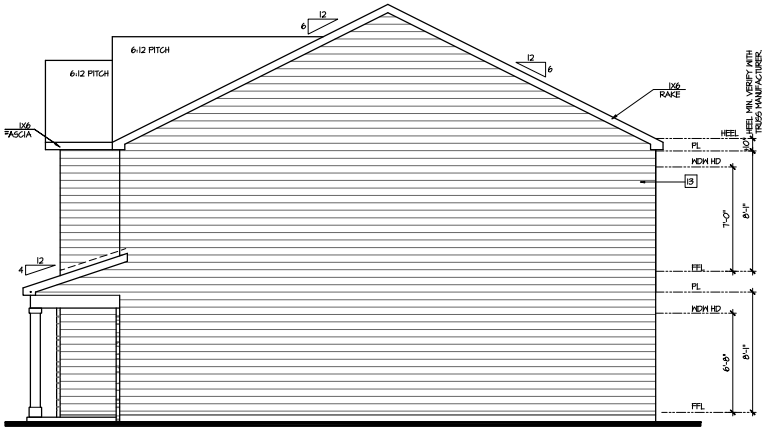
ALL WINDOWS WHOSE OPENING IS LESS THAN 24" ABOVE THE FINISH FLOOR AND WHOSE OPENING IS GREATER THAN 12" ABOVE THE FINISH FLOORING SURFACE MUST HAVE WINDOW OPENING LIMITING DEVICES COMPLYING WITH THE NRC RC SECTION N1021.2 AND N1021.2.2.

AVAILABLE WITH OPTIONAL  
9'-1" FIRST FLOOR PLATE

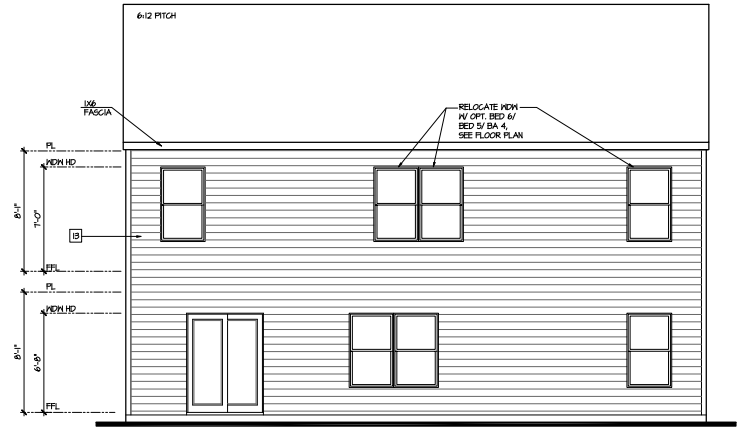
NOTES AT OPT 9'-1" PLT:  
 - WINDOW HT SET AT 7'-6"  
 - INTERIOR SOFFITS AT 8'-0"  
 - EXTERIOR SOFFITS AT 8'-0"



Left Elevation 'K'  
SCALE: 1/4"=1'-0" AT 22'x34' LAYOUT 1/8"=1'-0" AT 11'x11' LAYOUT



Right Elevation 'K'  
SCALE: 1/4"=1'-0" AT 22'x34' LAYOUT 1/8"=1'-0" AT 11'x11' LAYOUT



Rear Elevation 'K'  
SCALE: 1/4"=1'-0" AT 22'x34' LAYOUT 1/8"=1'-0" AT 11'x11' LAYOUT

NO.	DATE	REVISION
1	01.20.22	

PROFESSIONAL SEAL:

PROJECT TITLE:  
40' Series

FOR CONSTRUCTION



PROJECT NO: GMD11044

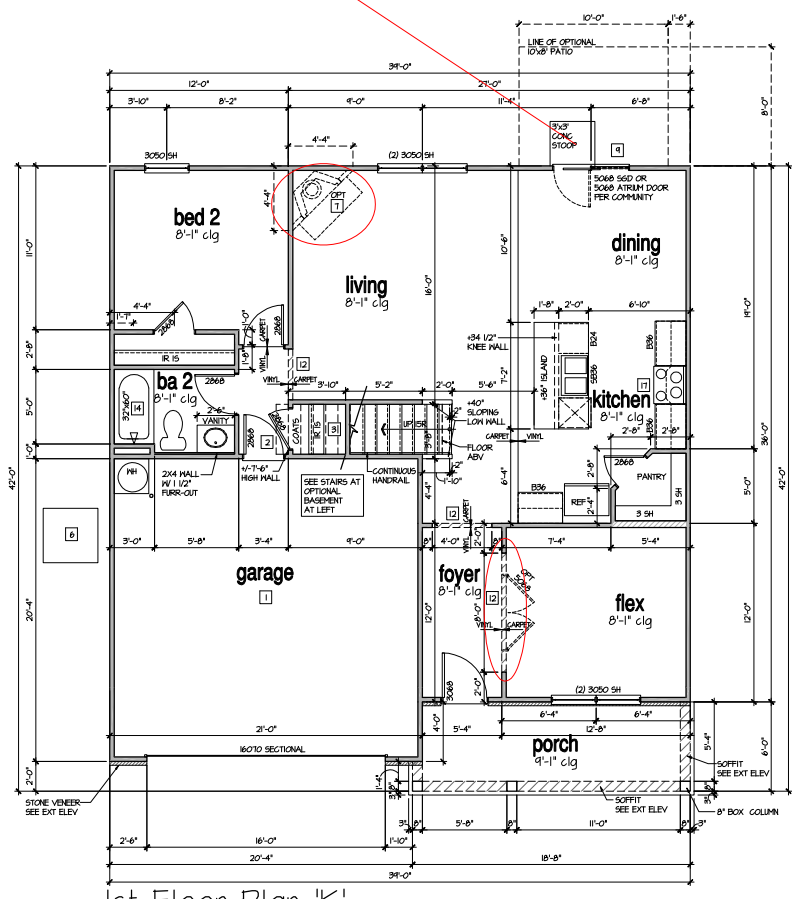
SHEET TITLE:  
'HAYDEN'  
EXTERIOR  
ELEVATIONS  
'4EPF-K'

PRINT DATE:  
January 22, 2021

SHEET NO:  
2 K



10' X 12' PATIO



1st Floor Plan 'K'

SCALE: 1/4"=1'-0" AT 22'x34' LAYOUT 1/8"=1'-0" AT 11'x11' LAYOUT

- 8'-4 1/2" STAIR NOTE:**  
USE 14" T.J WITH 3/4" PLYWOOD SUBFLOOR.  
15 TREADS AT 10" EACH VERIFY.  
16 RISERS AT 4" - 150" = 120 1/4" TOTAL RISE VERIFY.
- 4'-1" STAIR NOTE:**  
USE 14" T.J WITH 3/4" PLYWOOD SUBFLOOR.  
16 TREADS AT 10" EACH VERIFY.  
11 RISERS AT 4" - 121" = 123 3/4" TOTAL RISE VERIFY.
- 8'-1" STAIR NOTE:**  
USE 14" T.J WITH 3/4" PLYWOOD SUBFLOOR.  
14 TREADS AT 10" EACH VERIFY.  
15 RISERS AT 4" - 145" = 11 3/4" TOTAL RISE VERIFY.

- FOR ADDITIONAL NOTES SEE GENERAL NOTES ON TITLE SHEET AND DETAILS.  
- WINDOW HEAD HEIGHTS:  
1ST FLOOR = 6'-8" UNO. ON ELEVATIONS.  
2ND FLOOR = 7'-0" UNO. ON ELEVATIONS.  
ALL DIMENSIONS TO WINDOWS AND DOORS ARE TO CENTERLINE.

**WALL LEGEND:**

FULL HEIGHT 2x4 HOOD 5/16\"/> FULL HEIGHT 2x4 HOOD 5/16\"/> PARTITION	FULL HEIGHT 2x6 HOOD 5/16\"/> PARTITION
BRICK / STONE VENEER	DRYWALL OPENING HEIGHT AS NOTED ON PLAN
LOW GYPSUM BOARD WALL HEIGHT AND STD SIZE AS NOTED	DRYWALL OPENING HEIGHT AS NOTED ON PLAN

**KEY NOTES FOR NORTH CAROLINA:**

- FIRE PROTECTION:**
- [1] HOUSING TO GARAGE FIRE SEPARATION, GARAGEHOUSE SEPARATION AT VERTICAL SURFACES SHALL BE PROTECTED WITH ONE (1) LAYER 1/2" GYPSUM BOARD. (PER NCRC TABLE R302.6.) GARAGEHOUSE SEPARATION AT HORIZONTAL SURFACES SHALL BE PROTECTED WITH ONE (1) LAYER 5/8" TYPE 'X' GYPSUM BOARD. (PER NCRC TABLE R302.6.)
  - [2] HOUSING TO GARAGE DOOR SEPARATION PROVIDE 1-3/8" SOLID CORE DOOR OR APPROVED 20 MINUTE RATED DOOR. (PER NCRC SECTION R302.5.1)
  - [3] BENEATH STAIRS AND LANDINGS, 1/2" GYPSUM BOARD ON WALLS AND CEILING OF ENCLOSED ACCESSIBLE AREAS. (PER NCRC SECTION R302.1) IN CONCEALED SPACES BETWEEN STAIR STRINGERS PROVIDE FIREBLOCKING PER R302.11 MEPS.
  - [4] GAS WATER HEATER ON 8' HIGH PLATFORM. (PER CHAPTER 5 NCRC-PLUMBING)
  - [5] PAU 8"x8" PLATFORM. VERIFY WITH TRUSS MANUFACTURER. 16'-4" MIN. CLEAR HEIGHT TO HORIZONTAL MEMBERS. 2"x6" OVER 2"x4" BOTTOM CHORD. OF TRUSS. VERIFY W/ TRUSSES.)
  - [6] A/C CONDENSER PAD. (VERIFY)
  - [7] PRE-FABRICATED METAL FIREPLATE. INSTALL PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
  - [8] ATTIC ACCESS LARGE ENOUGH TO REMOVE LARGEST PIECE OF EQUIPMENT BUT NOT LESS THAN 30"x22" FIRE RATED ACCESS AS NOTED. (PER NCRC R011) ATTIC ACCESS LARGER VERIFY LOCATION AND SIZE WITH TRUSSES. (25 1/2" X 54" SIZE) FOR GARAGE TO ATTIC SEPARATION PER NCRC 302.5) EXCEPTION: TRUSSES.
  - [9] TEMPERED SAFETY GLASS. (PER NCRC SECTION 508.4)
  - [10] PLYWOOD SHELF ABOVE WITH DRYWALL FINISH OVER HEIGHT AS NOTED.
  - [11] HALF WALL, HEIGHT AS NOTED.
  - [12] INTERIOR SOFFITS: FFL = 8'-1" UNO. SFL = 7'-6" UNO. BATHS.
  - [13] SHOWER: TEMPERED GLASS ENCLOSURE.
  - [14] TUB-SHOWER COMBO: TEMPERED GLASS ENCLOSURE.
  - [15] CERAMIC TILE SHOWER AND FLOOR, TEMPERED GLASS ENCLOSURE.
  - [16] ACRYLIC TUB W/ CERAMIC PLATFORM KITCHEN.
  - [17] 30" SLIDE-IN ELECTRICAL RANGE W/ HOOD AND MICRO ABV. VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
  - [18] 30" GAS COOKTOP AND HOOD: VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
  - [19] ELECTRIC OVEN WITH MICRO/WAVE OVEN.

NO.	DATE	REVISION
1	01.20.22	

PROFESSIONAL SEAL:

PROJECT TITLE:  
**40' Series**

CLIENTS NAME:



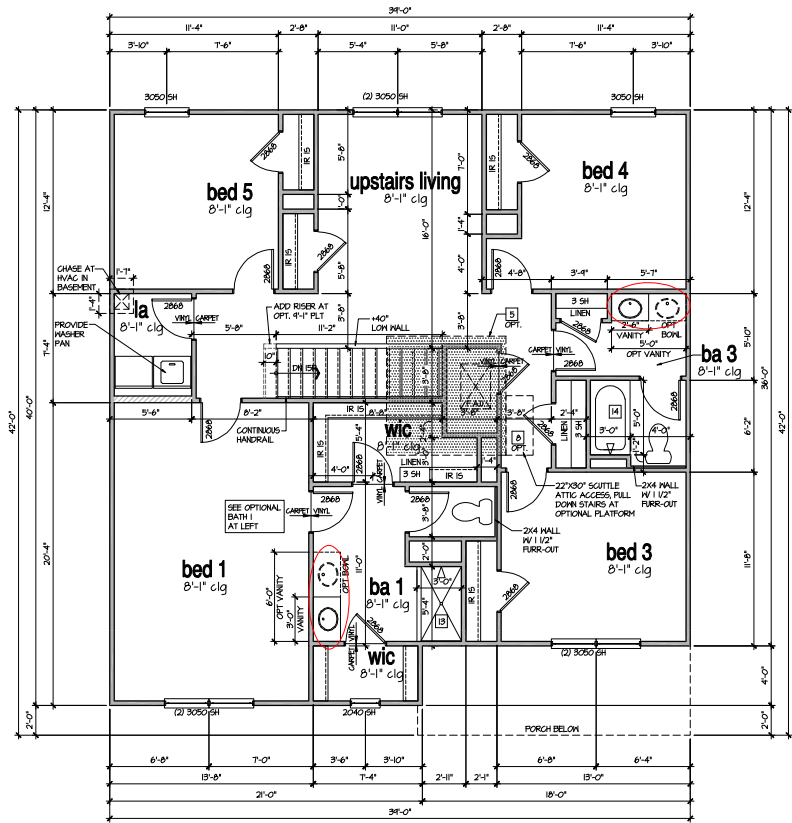
PROJECT NO: GMD110491

SHEET TITLE:  
**'HAYDEN'  
1st FLOOR  
PLAN '4EPF-K'**

PRINT DATE:  
January 22, 2021

SHEET NO:  
**4 K**

FOR CONSTRUCTION



2nd Floor Plan 'K'  
SCALE: 1/4"=1'-0" AT 22'x34" LAYOUT 1/8"=1'-0" AT 11'x11" LAYOUT

**9'-1" STAIR NOTE:**  
USE 14" T.J WITH 3/4" PLYWOOD SHELFLOOR.  
16 TREADS AT 10" EACH VERIFY.  
11 RISERS AT 4"-12" = 123 3/4" TOTAL RISE VERIFY.

**8'-1" STAIR NOTE:**  
USE 14" T.J WITH 3/4" PLYWOOD SHELFLOOR.  
14 TREADS AT 10" EACH VERIFY.  
15 RISERS AT 4"-14" = 113 3/4" TOTAL RISE VERIFY.

- FOR ADDITIONAL NOTES SEE GENERAL NOTES ON TITLE SHEET AND DETAILS.  
- WINDOW HEAD HEIGHTS:  
1ST FLOOR = 6'-8" UNO. ON ELEVATIONS.  
2ND FLOOR = 7'-0" UNO. ON ELEVATIONS.  
ALL DIMENSIONS TO WINDOWS AND DOORS ARE TO CENTERLINE.

**WALL LEGEND:**

	FULL HEIGHT 2x4 HOOD 5/16 PARTITION		FULL HEIGHT 2x6 HOOD 5/16 PARTITION
	BRICK / STONE VENEER		S/D WALL BELOW HEIGHT AND S/D SIZE AS NOTED
	LOW GYPSUM BOARD WALL HEIGHT AND S/D SIZE AS NOTED		DRYWALL OPENING HEIGHT AS NOTED ON PLAN

**KEY NOTES FOR NORTH CAROLINA:**

- FIRE PROTECTION:**
- [1] HOUSING TO GARAGE FIRE SEPARATION, GARAGEHOUSE SEPARATION AT VERTICAL SURFACES SHALL BE PROTECTED WITH ONE (1) LAYER 1/2" GYPSUM BOARD. (PER NCRC TABLE R302.6.) GARAGEHOUSE SEPARATION AT HORIZONTAL SURFACES SHALL BE PROTECTED WITH ONE (1) LAYER 5/8" TYPE 'X' GYPSUM BOARD. (PER NCRC TABLE R302.6.)
  - [2] HOUSING TO GARAGE DOOR SEPARATION PROVIDE 1-3/8" SOLID CORE DOOR OR APPROVED 20 MINUTE RATED DOOR. (PER NCRC SECTION R302.5.1.)
  - [3] BENEATH STAIRS AND LANDINGS, 1/2" GYPSUM BOARD ON WALLS AND CEILING OF ENCLOSED ACCESSIBLE AREAS. (PER NCRC SECTION R302.1.) IN CONCEALED SPACES BETWEEN STAIR STRINGERS PROVIDE FIREBLOCKING PER R302.11.
  - [4] GAS WATER HEATER ON 8' HIGH PLATFORM. (PER CHAPTER 5 NCRC-PLUMBING)
  - [5] PAU 8"x8" PLATFORM. VERIFY WITH TRUSS MANUFACTURER. 16" CLEAR HEIGHT TO HORIZONTAL MEMBERS. 2"x6" ABOVE 2"x4" BOTTOM CHORD. OF TRUSS, VERIFY W/ TRUSSES.)
  - [6] A/C CONDENSER PAD. (VERIFY)
  - [7] PRE-FABRICATED METAL FIREPLATE. INSTALL PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
  - [8] ATTIC ACCESS LARGE ENOUGH TO REMOVE LARGEST PIECE OF EQUIPMENT BUT NOT LESS THAN 30"x22" FIRE RATED ACCESS AS NOTED. (PER NCRC 901.1) ATTIC ACCESS LARGER, VERIFY LOCATION AND SIZE WITH TRUSSES. (25 1/2" X 54" SIZE) FOR GARAGE TO ATTIC SEPARATION PER NCRC 302.5.1 EXCEPTION.
  - TITLES:
  - [9] TEMPERED SAFETY GLASS. (PER NCRC SECTION 308.4)
  - [10] PLYWOOD SHELF ABOVE WITH DRYWALL FINISH OVER. HEIGHT AS NOTED.
  - [11] HALF WALL, HEIGHT AS NOTED.
  - [12] INTERIOR SOFFITS: FFL = 8'-1" UNO. SFL = 7'-6" UNO. BATHS.
  - [13] SHOWER, TEMPERED GLASS ENCLOSURE.
  - [14] TUB-SHOWER COMBO, TEMPERED GLASS ENCLOSURE.
  - [15] CERAMIC TILE SHOWER AND FLOOR, TEMPERED GLASS ENCLOSURE. KITCHEN.
  - [16] 30" SLIDE-IN ELECTRICAL RANGE W/ HOOD AND MICRO ADV. VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
  - [17] 30" GAS COOKTOP AND HOOD. VENT PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
  - [18] ELECTRIC OVEN WITH MICROVAPE OVEN.

NO.	DATE	REVISION
1	01.20.22	

PROFESSIONAL SEAL:

PROJECT TITLE:  
**40' Series**

CLIENT NAME:



PROJECT NO.: GMD110491

SHEET TITLE:  
**'HAYDEN'  
2nd FLOOR  
PLAN '4EPF-K'**

PRINT DATE:  
January 22, 2021

SHEET NO.:  
**5 K**

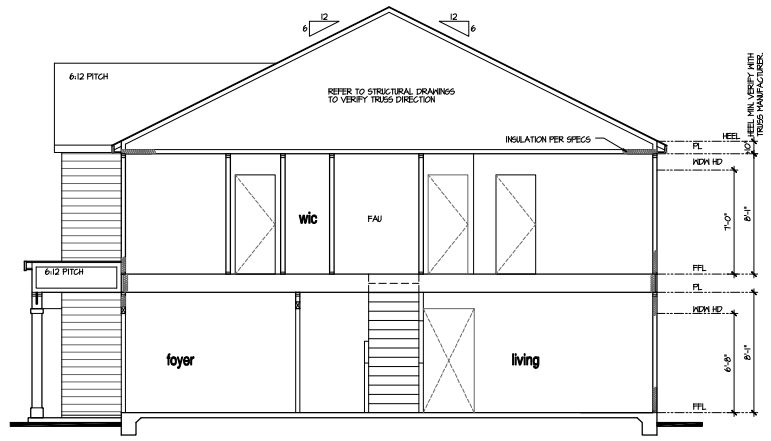
FOR CONSTRUCTION

**9'-1" STAIR NOTE:**  
 (USE 1" T.J. WITH 3/4" FLYWOOD SUBFLOOR)  
 16 TREADS AT 10" EACH VERIFY  
 11 RISERS AT 11"-12 1/2" = 125 3/4" TOTAL  
 RISE VERIFY

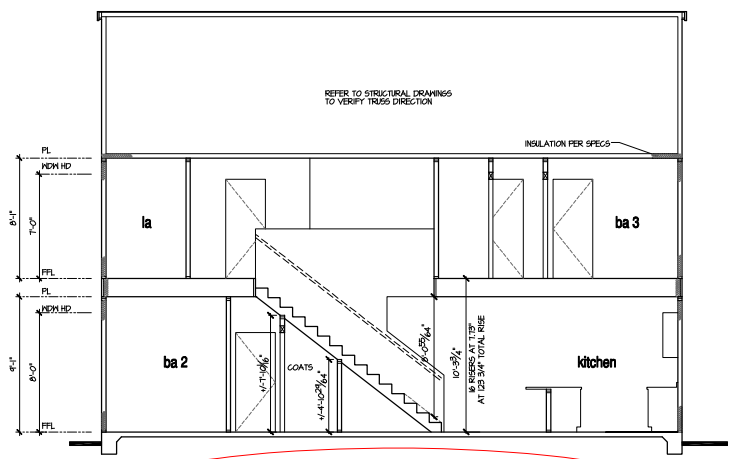
**8'-1" STAIR NOTE:**  
 (USE 1" T.J. WITH 3/4" FLYWOOD SUBFLOOR)  
 14 TREADS AT 10" EACH VERIFY  
 13 RISERS AT 11"-11 1/2" = 143 1/2" TOTAL  
 RISE VERIFY

**NOTES:**

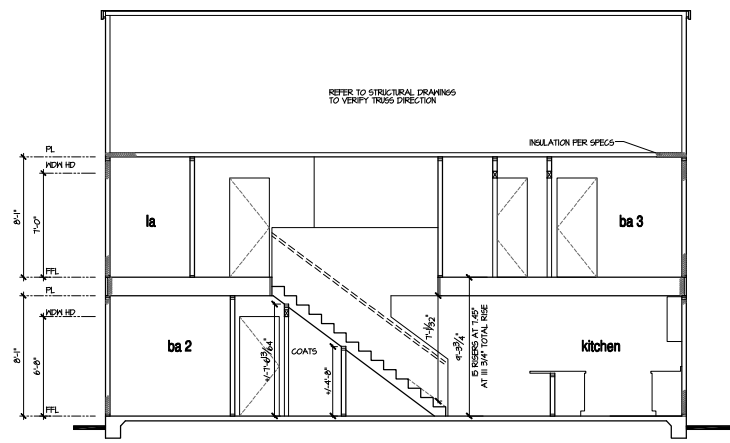
- REFER TO FLOOR PLAN NOTES FOR TYPICAL FIRE PROTECTION NOTES AND LOCATIONS.
- THESE BUILDING SECTIONS MAY VARY AT ALTERNATE ELEVATION STYLES AND AT PLAN OPTION CONDITIONS. REFER TO MAIN FLOOR PLAN AND ALTERNATE FLOOR PLANS FOR INFORMATION NOT SHOWN HERE.
- BUILDING SECTIONS SHOWN HERE DEPICT VOLUME SPACES WITHIN THE STRUCTURE. REFER TO STRUCTURAL DRAWINGS, TRUSS DRAWINGS, STRUCTURAL DETAILS AND CALCULATIONS BY OTHER FOR ALL STRUCTURAL INFO.
- ROOFING: PITCHED SHINGLE ROOF. REFER TO ROOF PLAN FOR TYPICALS.
- ROOF FLOORS: FLOOR SHEATHING OVER FLOOR JOIST. REFER TO STRUCTURAL AND TRUSS DRAWINGS BY OTHERS.
- VERIFY STAIRS MINIMUM AND MAXIMUM REQUIREMENTS FOR CONSTRUCTION CLEARANCES WITH LOCAL CODES.
- INSULATION:  
 EXTERIOR WALLS ZONE 3: R-13 BATTS MINIMUM, VERIFY  
 EXTERIOR WALLS ZONE 4: R-15 BATTS MINIMUM, VERIFY  
 CEILING WITH ATTIC ABOVE UNCOMPRESSED INSULATION: R-36 BATTS MINIMUM, VERIFY  
 PER STATE RESIDENTIAL CODE COMPLIANCE METHOD TO BE DETERMINED BY BUILDER.  
 FLOOR OVER GARAGE: R-14 BATTS MINIMUM, VERIFY  
 ATTIC FLOORWALL: R-14 BATTS MINIMUM, VERIFY  
 CRAWL SPACE FLOORING: R-11 BATTS MINIMUM, VERIFY
- WINDOW GLAZING U-Factor: 0.35



**Building Section I at Monolithic Slab**  
 SCALE: 1/4"=1'-0" AT 22'X34" LAYOUT 1/8"=1'-0" AT 11'X17" LAYOUT



**Building Section 2 at Optional 9' Plt**  
 SCALE: 1/4"=1'-0" AT 22'X34" LAYOUT 1/8"=1'-0" AT 11'X17" LAYOUT



**Building Section 2 at Monolithic Slab**  
 SCALE: 1/4"=1'-0" AT 22'X34" LAYOUT 1/8"=1'-0" AT 11'X17" LAYOUT

NO.	DATE	REVISION
1	01.28.22	

PROFESSIONAL SEAL:

PROJECT TITLE:  
**40' Series**

**FOR CONSTRUCTION**



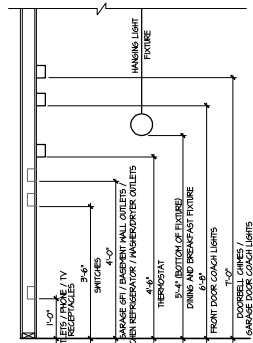
PROJECT NO: GMD1049

SHEET TITLE:  
**'HAYDEN' BUILDING SECTIONS**

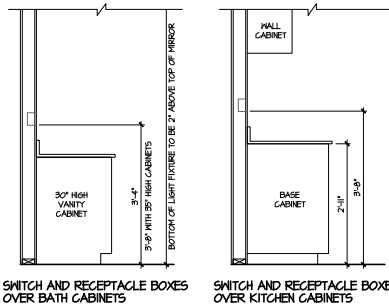
PRINT DATE:  
 January 22, 2021

SHEET NO:  
**1A S**





STANDARD ELECTRICAL BOX HEIGHTS



SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS

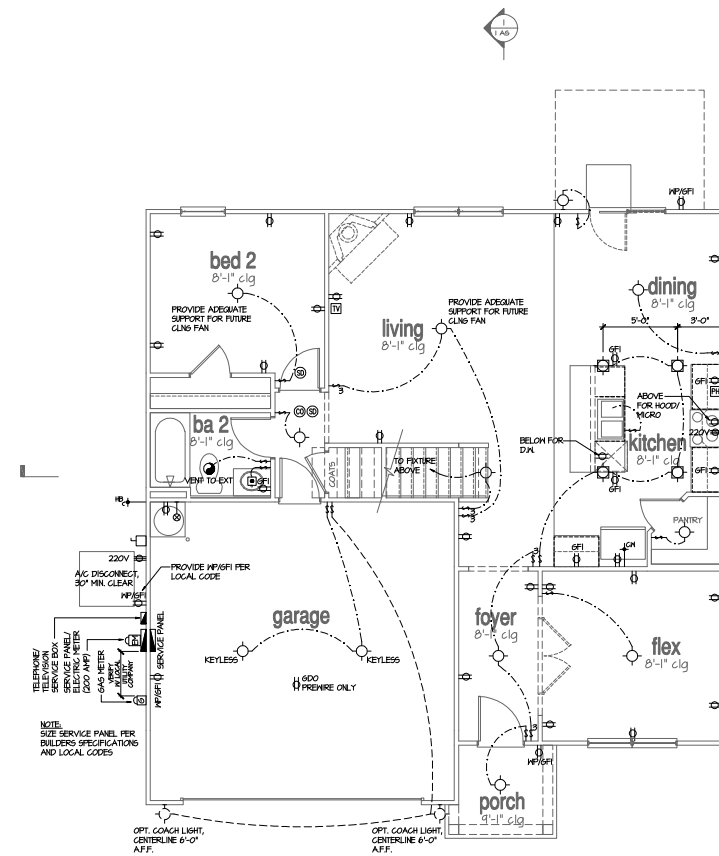
SWITCH AND RECEPTACLE BOXES OVER KITCHEN CABINETS

NOTES:

- PROVIDE GROUNDING ELECTRICAL ROD PER LOCAL CODES.
- PROVIDE AND INSTALL ARC FAULT CIRCUIT INTERRUPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- ALL EXHAUST FANS SHALL HAVE BACKDRAFT DAMPERS.
- FANLIGHTS IN NET/DAMP LOCATIONS SHALL BE LABELED "SUITABLE FOR NET OR DAMP LOCATIONS."
- ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE ENGINEERED BY OTHERS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION AND PLACEMENT.
- PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- PROVIDE AND INSTALL GROUND FAULT CIRCUIT INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS.
- HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.
- ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FRIDGES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PITS, DRAIN TILE SUMP AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.
- PROVIDE POWER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURER'S WRITTEN INSTRUCTIONS.

LEGEND:

⊕	DUPLEX OUTLET	⬠	FLUSH-MOUNT LED CEILING FIXTURE	⊞	CHIMES
⊞	HP/SPFI WEATHERPROOF GFI DUPLEX OUTLET	⬠	HANGING FIXTURE	⊞	POSITION SWITCH
⊞	GROUND-FAULT CIRCUIT-INTERRUPTER DUPLEX OUTLET	⬠	FLUSH-MOUNT LED CEILING FIXTURE (PROVIDE CEILING FAN SUPPORT)	⊞	10V SMOKE DETECTOR / VV BATTERY BACKUP
⊞	HALF-SWITCHED DUPLEX OUTLET	⬠	2-LIGHT VANITY FIXTURE	⊞	CO2 DETECTOR
⊞	220V 220 VOLT OUTLET	⬠	3-LIGHT VANITY FIXTURE	⊞	THERMOSTAT
⊞	REINFORCED JUNCTION BOX	⬠	4-LIGHT VANITY FIXTURE	⊞	TELEPHONE
⊞	WALL SWITCH	⬠	WALL HOOK FIXTURE	⊞	TELEVISION
⊞	THREE-WAY SWITCH	⬠	EXHAUST FAN (VENT TO EXTERIOR)	⊞	ELECTRIC PANEL
⊞	FOUR-WAY SWITCH	⬠		⊞	ELECTRIC METER
		⬠		⊞	DISCONNECT SWITCH



1st Floor Plan 'A'  
SCALE: 1/4"=1'-0" AT 22'X34' LAYOUT 1/8"=1'-0" AT 10'X11' LAYOUT

NO.	DATE	REVISION
1	01.20.22	

PROFESSIONAL SEAL

PROJECT TITLE:  
40' Series

CLIENTS NAME:



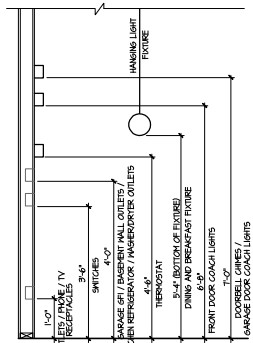
PROJECT NO: GMD110491

SHEET TITLE:  
'HAYDEN'  
1st FLOOR  
UTILITY PLAN

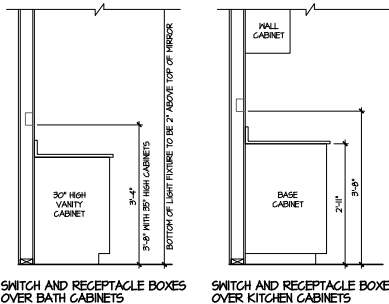
PRINT DATE:  
January 22, 2021

SHEET NO:  
7

FOR CONSTRUCTION



STANDARD ELECTRICAL BOX HEIGHTS



SWITCH AND RECEPTACLE BOXES OVER BATH CABINETS

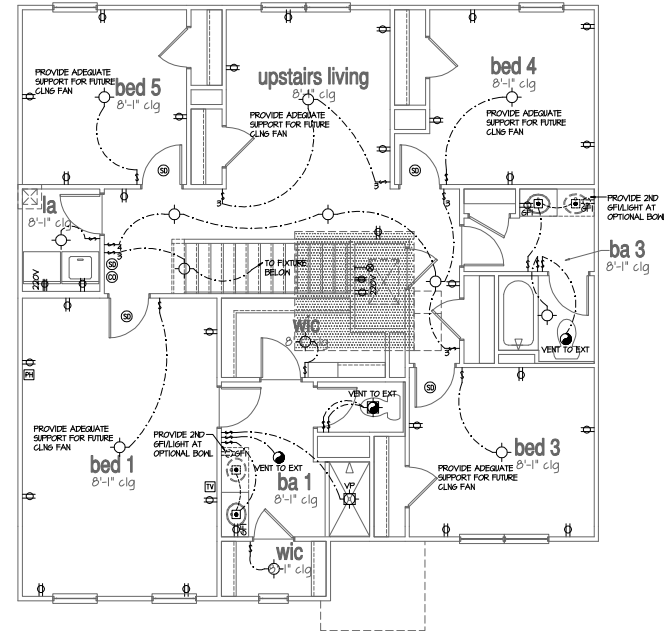
SWITCH AND RECEPTACLE BOXES OVER KITCHEN CABINETS

NOTES:

- PROVIDE GROUNDING ELECTRICAL ROD PER LOCAL CODES.
- PROVIDE AND INSTALL ARC FAULT CIRCUIT INTERRUPTERS (AFCI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- ALL EXHAUST FANS SHALL HAVE BACKDRAFT DAMPERS.
- FANLIGHTS IN NET/DAMP LOCATIONS SHALL BE LABELED "SUITABLE FOR NET OR DAMP LOCATIONS."
- ELECTRICAL SYSTEMS ARE SHOWN FOR INTENT ONLY. THESE SYSTEMS SHALL BE ENGINEERED BY OTHERS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER INSTALLATION AND PLACEMENT.
- PROVIDE AND INSTALL LOCALLY CERTIFIED SMOKE DETECTORS AND CO2 DETECTORS AS REQUIRED BY NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- PROVIDE AND INSTALL GROUND FAULT CIRCUIT INTERRUPTERS (GFI) AS REQUIRED BY NATIONAL ELECTRICAL CODE (NEC) AND MEETING THE REQUIREMENTS OF ALL GOVERNING CODES.
- ELECTRICAL CONTRACTOR TO PROVIDE REQUIRED DIRECT HOOK-UPS/CUTOFFS.
- HVAC CONTRACTOR TO VERIFY THERMOSTAT LOCATIONS.
- ALL ELECTRICAL AND MECHANICAL EQUIPMENT (FURNACES, A/C UNITS, ELECTRICAL PANELS, SANITARY SUMP PITS, DRAIN TILE SUMP, AND WATER HEATERS) ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS.
- PROVIDE POWER, LIGHT AND SWITCH AS REQUIRED FOR ATTIC FURNACE PER CODE AND MANUFACTURERS' WRITTEN INSTRUCTIONS.

LEGEND:

⊕	DUPLEX OUTLET	⬆	FLUSH MOUNT LED CEILING FIXTURE	⊞	GAMES
⊕	WEATHERPROOF GFI DUPLEX OUTLET	⬆	HANGING FIXTURE	⊞	FISHBONE SWITCH
⊕	GROUND-FAULT CIRCUIT INTERRUPTER DUPLEX OUTLET	⬆	FLUSH MOUNT LED CEILING FIXTURE (PROVIDE CEILING FAN SUPPORT)	⊞	10V SMOKE DETECTOR / V BATTERY BACKUP
⊕	HALF-SWITCHED DUPLEX OUTLET	⬆	2-LIGHT VANITY FIXTURE	⊞	CO2 DETECTOR
⊕	220V 220 VOLT OUTLET	⬆	3-LIGHT VANITY FIXTURE	⊞	THERMOSTAT
⊕	REINFORCED JUNCTION BOX	⬆	4-LIGHT VANITY FIXTURE	⊞	TELEPHONE
⊕	WALL SWITCH	⬆	WALL HOOD FIXTURE	⊞	TELEVISION
⊕	THREE-WAY SWITCH	⬆	EXHAUST FAN (VENT TO EXTERIOR)	⊞	ELECTRIC METER
⊕	FOUR-WAY SWITCH	⬆		⊞	ELECTRIC PANEL
		⬆		⊞	DISCONNECT SWITCH
		⬆		⊞	CEILING FAN (PROVIDE ADEQUATE SUPPORT)
		⬆		⊞	6AS SUPPLY WITH VALVE
		⬆		⊞	HOSE BIBB
		⬆		⊞	1/4" WATER STUB OUT
		⬆		⊞	WALL SCOFF



2nd Floor Plan 'A'  
SCALE: 1/4"=1'-0" AT 22'x34" LAYOUT 1/8"=1'-0" AT 11'x11" LAYOUT

NO.	DATE	REVISION
1	01.20.22	

PROFESSIONAL SEAL

PROJECT TITLE:  
40' Series

CLIENTS NAME:



PROJECT NO: GMD110491

SHEET TITLE:  
'HAYDEN'  
2nd FLOOR  
UTILITY PLAN

PRINT DATE:  
January 22, 2021

SHEET NO:

8

FOR CONSTRUCTION

DESIGN SPECIFICATIONS:

Construction Type: Commercial  Residential

Applicable Building Codes:

- 2008 North Carolina Residential Building Code with All Local Amendments
ABCE 1-16 Minimum Design Loads for Buildings and Other Structures

Design Loads:

- 1. Roof Live Loads
11. Conventional 2x 20 PSF
12. Truss 20 PSF
13. U/L Joist Truss 60 PSF
21. Conventional 2x 10 PSF
22. Truss 20 PSF
3. Snow 5 PSF
31. Importance Factor 1.0
4. Floor Live Loads
41. Typ. Dwelling 40 PSF
42. Sleeping Area 30 PSF
43. Deck 40 PSF
44. Passenger Garage 60 PSF
5. Floor Dead Loads
51. Conventional 2x 10 PSF
52. Joist 5 PSF
53. Floor Truss 5 PSF
6. Ultimate Design Wind Speed (3 sec. gust) 100 MPH
61. Exposure D
62. Importance Factor 1.0
63. Wind Base Shear (3.3 Vx 6.33 Vx)

1. Component and Cladding (in PSF)

Table with columns: ZONE, UP TO 30', 30'-39', 39'-49', 49'-60'. Rows: ZONE 1, ZONE 2, ZONE 3, ZONE 4, ZONE 5.

8. Seismic

- 81. Site Class C
82. Design Category D
83. Importance Factor 1.0
84. Seismic Use Group I
85. Spectral Response Acceleration SSI, Ss = .3g, SSI, Ss = .3g
86. Seismic Base Shear SSI, Vx = 8.52 Vx
87. Basic Structural System (check one)
87a. Bearing Wall
87b. Building Frame
87c. Moment Frame
87d. Dual w/ Special Moment Frame
87e. Dual w/ Intermediate RC or Special Steel
87f. Inverted Pendulum
88. Arch/High Components Anchored No
89. Lateral Design Control Seismic Wind 89
9. Assumed Soil Bearing Capacity 10000 psf



HAYDEN LH

PROJECT ADDRESS: TBD OWNER: DR Horton, Inc. 8009 Arrowidge Blvd. Charlotte, NC 28213
DESIGNER: GFD Design Group 102 Fourth Brook Circle Suite C Cary, NC 27518

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. The coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUPMIT Engineering, Laboratory 4 Testing, P.C. before construction begins.

PLAN ABBREVIATIONS:

Table with columns: ABBREVIATION, DESCRIPTION. Includes: ANCHOR BOLT, ABOVE FINISHED FLOOR, CEILING JOINT, CLEAR, DOUBLE JOIST, DOUBLE STUD POCKET, EACH END, EACH WAY, NOT TO SCALE, ON CENTER, POUNDS PER SQUARE FOOT, RIBS PER SQUARE INCH, PRESSURE TREATED, ROOF SUPPORT, STUD COLUMN, RANGLE JOIST, SPRUCE PINE FIR, SIPFORM STRONG-TIE, SIPFORM YELLOW PINE, TRIPLE JOIST, TRIPLE STUD POCKET, TYPICAL, UNLESS NOTED OTHERWISE, WELDED WIRE FABRIC.

Roof truss and floor joist layout, and their corresponding loading details, were not provided to SUPMIT Engineering, Laboratory 4 Testing, P.C. (SUPMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by DR Horton, Inc. Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUPMIT immediately.

SHEET LIST:

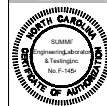
Table with columns: Sheet No., Description. Lists sheets C-1 through C-10: Cover Sheet, Specifications, Revisions, Monolithic Slab Foundation, Stem Wall Foundation, Crawl Space Foundation, Basement Foundation, Basement Framing Plan, First Floor Framing Plan, Second Floor Framing Plan, Roof Framing Plan, Basement Bracing Plan, First Floor Bracing Plan, Second Floor Bracing Plan.

REVISION LIST:

Table with columns: Revision No., Date, Project No., Description. Includes revisions 1, 2, 3: Updated elevation names, Added Cx-18 option and table for framing, Updated framing in the first floor.

DR HORTON PROJECT SIGN-OFF:

Table with columns: Manager, Operations, Operations System, Operations Product Development. Includes signature lines.



DR Horton, Inc. 8009 Arrowidge Blvd. Charlotte, NC 28213
Hayden LH
CONTRACTOR

GENERAL STRUCTURAL NOTES:

- 1. The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUPMIT Engineering, Laboratory 4 Testing, P.C. (SUPMIT) or the SER. For the purposes of these construction documents the SER and SUPMIT shall be considered the same entity.
2. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
3. The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of the structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUPMIT for review before any construction begins. The shop drawings shall be reviewed for overall compliance as it relates to the structural design of the project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUPMIT.
5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUPMIT before construction begins.
6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
7. The structure and all construction shall conform to all applicable sections of the International Residential Code.
8. The structure and all construction shall conform to all applicable sections of local building codes.
9. All structural assemblies are to meet or exceed to requirements of the current local building code.

FOUNDATION:

- 1. The structural engineer has not performed a subsurface investigation. Verification of the assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.
31. Footings: 5%
32. Exterior Slabs: 5%
4. No additional reinforcement shall be added to any structural concrete without written permission of the SER.

- 2. The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
3. Any fill shall be placed under the direction or recommendation of a licensed professional engineer.
4. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
5. Excavation of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 74 hours of excavation.
6. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

STRUCTURAL STEEL:

- 1. Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest edition.
2. Structural steel shall receive one coat of shop applied rust-inhibitive paint.
3. All steel shall have a minimum yield stress (Fy) of 36 ksi unless otherwise noted.
4. Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D1.1. Electrodes for shop and field welding shall be class E70XX. All welding shall be performed by a certified welder per the above standards.

CONCRETE:

- 1. Concrete shall have a nominal weight aggregate and a minimum compressive strength (F'c) of 28 days of 3000 psi, unless otherwise noted on the plan.
2. Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 308, "Building Code Requirements for Reinforced Concrete" and ACI 309, "Manual of Standard Practice for Concrete Construction".
3. All structural concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to 4% of target values as follows:
31. Footings: 5%
32. Exterior Slabs: 5%
4. No additional reinforcement shall be added to any structural concrete without written permission of the SER.

- 5. Concrete slab-on-grade shall be constructed in accordance with ACI 302R-96, "Guide for Concrete Slab and Slab Construction".
6. The concrete slab-on-grade has been designed using a subgrade modulus of 10500 psi and a design loading of 300 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unsupported conditions not in accordance with the above assumptions.
7. Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 18'-0" unless otherwise noted.
8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished. Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint.
10. All welded wire fabric (WWF) for concrete slab-on-grade shall be placed at mid-depth of slab. The WWF shall be securely supported during the concrete pour.

CONCRETE REINFORCEMENT:

- 1. Fibrous concrete reinforcement, or fibremesh specified in concrete slab-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength. Fibremesh reinforcing to be 90% virgin polypropylene fibers containing no reprocessed plastic materials and specifically manufactured for use as concrete secondary reinforcement. Application of fibremesh per cubic yard of concrete shall equal a minimum of 6% by volume (15 pounds per cubic yard).
2. Fibremesh shall comply with ASTM C1619, any local building code requirements, and shall meet or exceed the current industry standard.
3. Steel reinforcing bars shall be new mill steel conforming to ASTM A636, grade 60.
4. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 308, "Manual of Standard Practice for Concrete Construction".
5. Horizontal footing and wall reinforcement shall be continuous and shall have 30" bending, or corner bars with the same spacing as the horizontal reinforcement with a close B lapover splice.
6. Lap reinforcement as required a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in rebar shall be a minimum of 48 bar diameters.

- 9. Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.
10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.

WOOD FRAMING:

- 1. Solid stem wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Southern-Yellow-Pine (SYP) 2x.
2. LVL or PSL engineered wood shall have the following minimum design values:
21. E = 1,900,000 psi
22. Fv = 2400 psi
23. Fv = 285 psi
24. Fc = 100 psi
3. Unless in concrete with concrete, masonry, or earth shall be pressure treated in accordance with ALFA standard C-5. All other moisture exposed wood shall be treated in accordance with ALFA standard C-2.
4. Nails shall be common steel nails unless otherwise noted.
5. Lag screws shall conform to ANSI/APA-E standard D3821-R8L. Lag holes for lag screws shall be in accordance with NDS specifications.
6. All beams shall have full bearing on supporting framing members unless otherwise noted.
7. Exterior end load bearing stud walls are to be 2x4 SYP 2" x 4" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header.
8. Individual studs forming a column shall be attached with one 10d nail @ 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all floor levels to ensure proper load transfer.
9. Multi-ply beams shall have each ply attached with 1/4" 10d nails @ 24" O.C.
10. Four and five ply beams shall be bolted together with (2) rows of 1/2" diameter through bolts staggered @ 16" O.C. unless noted otherwise.

WOOD TRUSSES:

- 1. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit detailed shop drawings and engineering calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall not constitute an assumption of responsibility for the correctness of the structural design for the wood trusses.
2. The wood trusses shall be designed for all required loadings as specified in the local building code, the ABCE Standard "Minimum Design Loads for Buildings and Other Structures," (ABCE 1-16), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural finishes attached to the trusses.
3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction" (NDS) and "Design Specification for Metal Plate Connected Wood Trusses." The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Installing, and Bracing Metal Plate Connected Wood Trusses" (NDS-B). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
4. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.
EXTERIOR WOOD FINISHED DECKS:
1. Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

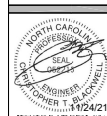
WOOD STRUCTURAL PANELS:

- 1. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial" and all other applicable APA standards.
2. All structurally required wood sheathing shall bear the mark of the APA.

- 3. Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
4. Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (U-6d) CC nail @ 6" o.c. at panel edges and at 2' o.c. in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
5. Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. Attach sheathing to its supporting framing with (U-6d) CC rightrink nail @ 6" o.c. at panel edges and at 2' o.c. in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
6. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

STRUCTURAL FIBERBOARD PANELS:

- 1. Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable APA standards.
2. All structurally required fiberboard sheathing shall bear the mark of the APA.
3. Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
4. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.



Hayden LH
CONTRACTOR
STRUCTURAL ENGINEER ONLY
DATE: 08/26/2013
TIME: 10:54:00 AM
PROJECT: 13-00000000
DRAWING: 0000



Sumit Engineering, Laboratory & Testing, Inc.  
 3075 Hammond Business  
 Black Hills, NC 27605

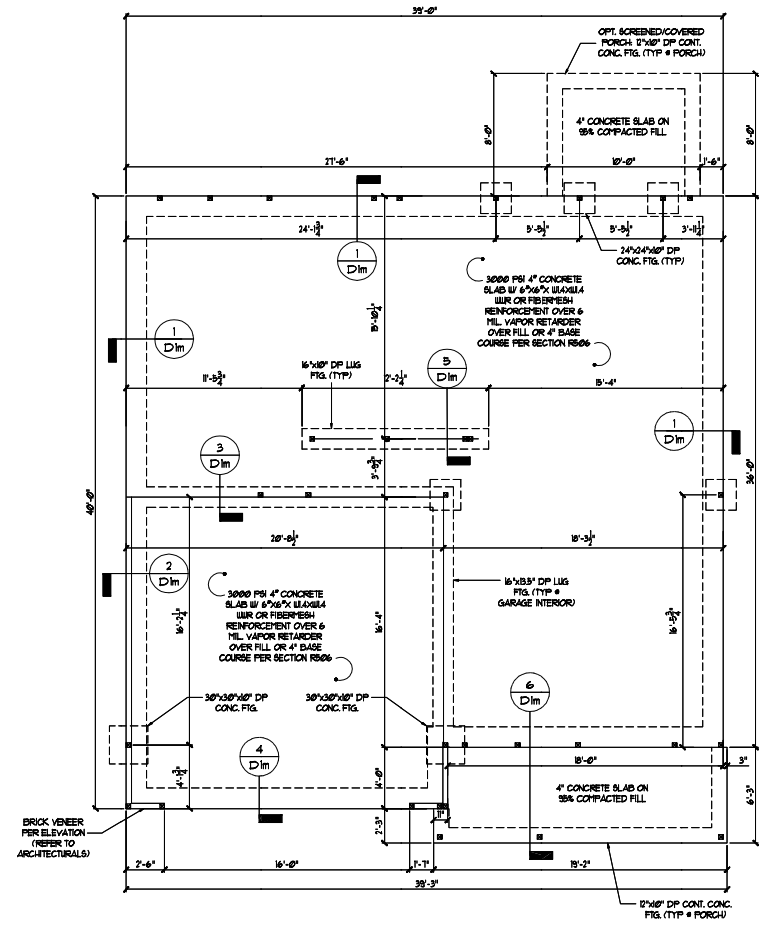
Region 14  
**Monolithic Slab Foundation**



DATE REVISION  
 REVISION NO. DATE BY  
 1 1/24/21  
 DESIGNER JSP  
 CHECKED MJC

2020 1000  
 VISIT US AT WWW.SUMMIT-CONCRETE.COM  
 CONTACT US AT 615.888.8881

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ELEVATION BFK

**STRUCTURAL MEMBERS ONLY**  
 ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT. SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.  
 STRUCTURAL ANALYSIS BASED ON 2018 NCR.

**MONOLITHIC SLAB FOUNDATION PLAN**  
 SCALE: 1/4" = 1'-0" OR 2 1/4" = 8'-0" OR 1 1/2" = 6'-0" OR 3/4" = 3'-0"

REQUIRED BRACED WALL PANEL CONNECTIONS				
METHOD	MATERIAL	MIN. THICKNESS	REQUIRED CONNECTION	
			# PANEL EDGES	# INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS # 6" O.C.	6d COMMON NAILS # 12" O.C.
GB	GYP/FRM BOARD	1/2"	5d COOLER NAILS* # 7" O.C.	5d COOLER NAILS* # 12" O.C.
USP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS # 6" O.C.	6d COMMON NAILS # 12" O.C.
PF	WOOD STRUCTURAL PANEL	1/6"	PER FIGURE R6-02.10.6.4	PER FIGURE R6-02.10.6.4

\*OR EQUIVALENT PER TABLE R102.13

**BRACED WALL NOTES:**

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R6-02.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE AMENDMENTS.
- WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS UP TO 130 MPH.
- REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING SIZES.
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R6-02.10.
- ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMUM PANEL LENGTH SHALL BE PER TABLE R6-02.10.1.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYP/FRM BOARD (IND).
- FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL WEATHABLE SURFACES INCLUDING INFL. AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.
- FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- A BRACED WALL PANEL SHALL BE LOCATED WITHIN 2 FEET OF EACH END OF A 12" DIA. PORTAL COLUMN.
- THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- MASONRY OR CONCRETE STEM WALLS w/ A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R6-02.10.4.3 OF THE 2018 NCR.
- BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R6-02.10.4.1.
- BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R6-02.10.4.2.
- CRIPPLE WALLS AND WALK OUT BRACED WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R6-02.10.4.6.
- PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R6-02.10.4.1 (IND) & REVISIONS.

GB = GYP/FRM BOARD  
CS-WSP = CONT. SHEATHED  
PF = PORTAL FRAME

USP = WOOD STRUCTURAL PANEL  
ENG = ENGINEERED SOLUTION  
PF-ENG = ENG. PORTAL FRAME

**GENERAL STRUCTURAL NOTES:**

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AND STATE AMENDMENTS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT. ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
- CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION.
- PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS:  
MICROLAR (LVL) F<sub>y</sub> = 2400 PSI, F<sub>v</sub> = 280 PSI, E = 1,500,000 PSI  
PARALLAX (PFL) F<sub>y</sub> = 2400 PSI, F<sub>v</sub> = 230 PSI, E = 1,500,000 PSI
- ALL WOOD MEMBERS SHALL BE 2" STP UNLESS NOTED ON PLAN. ALL STD COLUMNS AND JOISTS SHALL BE 2" STP (IND).
- ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 2" STP STD COLUMN AT EACH END UNLESS NOTED OTHERWISE.
- ALL REINFORCING STEEL SHALL BE GRADES 60 BARS CONFORMING TO ASTM A630 AND SHALL HAVE A MINIMUM COVER OF 3".
- CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- FITCH BEAMS, 4-PLY LVLs AND 3-PLY SIDE LOADED LVLs SHALL BE BOLTED TOGETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL U037. MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) PLAT 2x4 2" STP, DROPPED FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE. SHALL BE (2) PLAT 2x4 2" STP, DROPPED, UNLESS NOTED OTHERWISE.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DELHOPP. CONSULT/REVISED ON 4/26/21 IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING LABORATORY (SEL) TESTING P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING LABORATORY (SEL) TESTING P.C. CANNOT GUARANTEE THE ACCURACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

**STRUCTURAL MEMBERS ONLY**

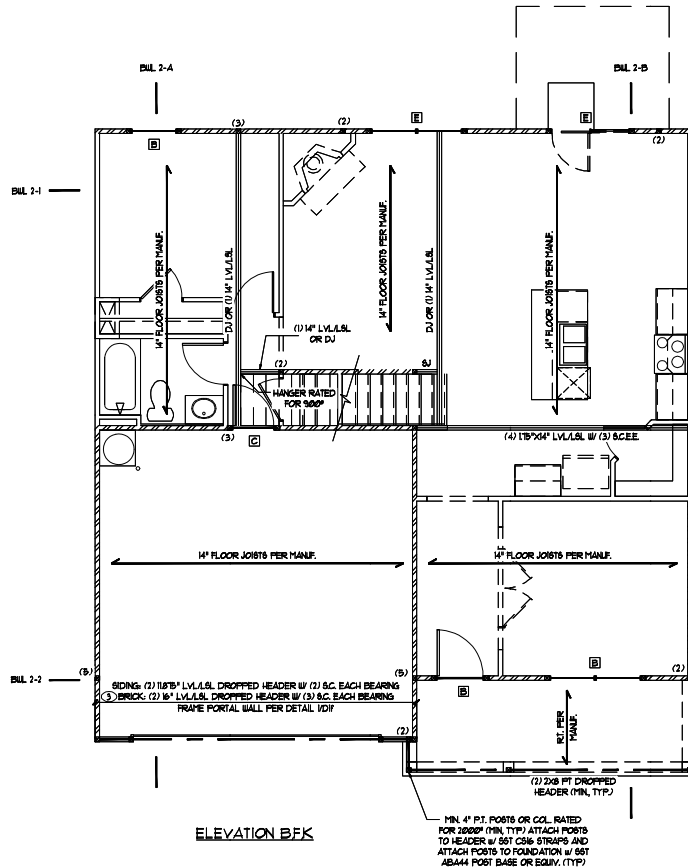
ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT. SEAL DOES NOT INCLUDE CONSTRUCTION METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE DRAWING'S ATTENTION OF SUMMIT ENGINEERING LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCR.

**FIRST FLOOR FRAMING PLAN**

SCALE: 1/4" = 1'-0" OR 3/16" = 1'-0" OR 1/8" = 1'-0" ON PLAN

	FIRST FLOOR BRACING (FT)	
	REQUIRED	PROVIDED
BILL 1-1	16	24
BILL 1-2	16	18
BILL 1-A	15	48
BILL 1-B	15	36



ELEVATION BFK

HEADER SCHEDULE		
TAG	SIZE	JACKS (EACH END)
A	(2) 2x6	(2)
B	(2) 2x6	(2)
C	(2) 2x6	(2)
D	(2) 2x6	(2)
E	(2) 3x4 LVL/LVL	(3)
F	(3) 2x6	(3)
G	(3) 2x6	(2)
H	(3) 2x6	(2)
I	(3) 2x6	(2)

- NOTES:**
- HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION.
  - ALL HEADERS TO BE DROPPED (IND).
  - STD COLUMNS NOTED ON PLAN OVERSIDE STUD COLUMNS LISTED ABOVE (IND).

KING STUD SCHEDULE	
MAXIMUM HEADER SPAN	MINIMUM KING STUDS EE
6'-0"	(2)
8'-0"	(2)
10'-0"	(3)
12'-0"	(3)
14'-0"	(3)
16'-0"	(4)

WALL STUD SCHEDULE (10 FT HEIGHT)				
WALL SIZE	STUD SPACING (O.C.)			
	ROOF + 1 FLOOR	ROOF + 2 FLOORS	NON-LOAD BEARING	
2x4	24"	18"	24"	24"
2x6	24"	24"	18"	24"

- NOTES:**
- BRACED WALL STUDS SHALL BE A MAX OF 16" O.C.
  - STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX OF 16" O.C.
  - TWO STORY WALLS SHALL BE FRAMED w/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLROOM FRAMED w/ HORIZ. BLOCKING @ 6'-0" O.C. VERTICALLY.

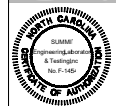
LINTEL SCHEDULE		
TAG	SIZE	OPENING SIZE
①	L3x3x1/4"	LESS THAN 6'-0"
②	L3x3x1/4"	6'-0" TO 10'-0"
③	L3x3-1/2x5/16"	GREATER THAN 10'-0"
④	L3x3-1/2x5/16"	ALL ARCHED OPENINGS

SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR )

ALL HEADERS WHERE BRICK IS USED, TO BE: (IND)

- SHADED WALLS INDICATED LOAD BEARING WALLS
- NOTE: REDUCE JOIST SPACING UNDER 1ST FLOOR, GRANITE COUNTERTOPS AND/OR ISLANDS.
- JOIST + BEAM SIZES SHOWN ARE MINIMUMS. BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.
- NOTE: --- DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.
- NOTE: MEMBERS NOTED AS PRESURE TREATED MAY BE FRAMED WITH NON-PRESURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.
- INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R6-02.10.8 + FIG. R6-02.10.7 OF THE 2018 NCR.
- NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R6-02.10.3 OF THE 2018 NCR.

MIN. 4" P.T. PORTS OR COLL. RATED FOR 2000F (MIN. TYP) ATTACH PORTS TO HEADER w/ 5/8" C&G STRAPS AND ATTACH PORTS TO FOUNDATION w/ 5/8" ANCH. POST BASE OR EQUIV. (TYP)



DELHOPP, INC.  
ARCHITECTURAL ENGINEER  
Charlotte, NC 28203

Region 1A  
First Floor Framing Plan



4/26/21  
STRUCTURAL MEMBERS ONLY

DATE: 4/26/21  
SCALE: 1/4" = 1'-0" OR 3/16" = 1'-0" OR 1/8" = 1'-0" ON PLAN

REQUIRED BRACED WALL PANEL CONNECTIONS				
METHOD	MATERIAL	MIN. THICKNESS	REQUIRED CONNECTION	
			# PANEL EDGES	# INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS # 4" O.C.	6d COMMON NAILS # 4" O.C.
GB	GYP-SUM BOARD	1/2"	5d COOLER NAILS # 7" O.C.	5d COOLER NAILS # 7" O.C.
USP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS # 6" O.C.	6d COMMON NAILS # 12" O.C.
PF	WOOD STRUCTURAL PANEL	1/6"	PER FIGURE R602.10.6.4	PER FIGURE R602.10.6.4

\*OR EQUIVALENT PER TABLE R102.13

**BRACED WALL NOTES:**

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE AMENDMENTS.
- WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS UP TO 130 MPH.
- REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING SIZES.
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R602.10.
- ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 10 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYP-SUM BOARD (IND).
- FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON ALL WEATHABLE SURFACES INCLUDING INTL. AREAS BETWEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.
- FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A BRACED WALL LINE.
- THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 71 FEET.
- MASONRY OR CONCRETE STEM WALLS w/ A LENGTH OF 48" OR LESS SUPPORTING A BRACED WALL PANEL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.4.3 OF THE 2018 NRC.
- BRACED WALL PANEL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.
- BRACED WALL PANEL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.5.
- CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10.4.6.
- PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.4.7 (IND) & ADHERE THERETO.

GB = GYP-SUM BOARD  
CS-WSP = CONT. SHEATHED  
PF = PORTAL FRAME

USP = WOOD STRUCTURAL PANEL  
ENS = ENGINEERED SOLUTION  
PF-ENS = ENCL. PORTAL FRAME

**GENERAL STRUCTURAL NOTES:**

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AND STATE AMENDMENTS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC PROJECT. ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
- CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO RESIST ALL FORCES ENCOUNTERED DURING ERECTION.
- PROPERTIES USED IN THE DESIGN ARE AS FOLLOWS:  
MICROSLIP (LVL) F<sub>y</sub> = 2400 PSI, F<sub>v</sub> = 280 PSI, E = 1.5E+6 PSI  
PARALLEL (LVL) F<sub>y</sub> = 2400 PSI, F<sub>v</sub> = 230 PSI, E = 1.5E+6 PSI
- ALL WOOD MEMBERS SHALL BE 2" STP UNLESS NOTED ON PLAN. ALL STD COLUMNS AND JOISTS SHALL BE 2" STP (IND).
- ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 2" STP STUD COLUMN AT EACH END UNLESS NOTED OTHERWISE.
- ALL REINFORCING STEEL SHALL BE GRADES 60 BARS CONFORMING TO ASTM A63 AND SHALL HAVE A MINIMUM COVER OF 3".
- CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- FITCH BEAMS 4-PLY LVLs AND 3-PLY SIDE LOADED LVLs SHALL BE BOLTED TOGETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTION PER DETAIL U02. MIN. EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM.
- ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 STP 2", DROPPED FOR NON-LOAD BEARING HEADERS EXCEEDING 8'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE. SHALL BE (2) FLAT 2x4 STP 2", DROPPED. (UNLESS NOTED OTHERWISE)

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DEL HORIZON. COMPLETED/REVISED ON 04/21/21. IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING LABORATORY & TESTING, P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ACCURACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

**STRUCTURAL MEMBERS ONLY**

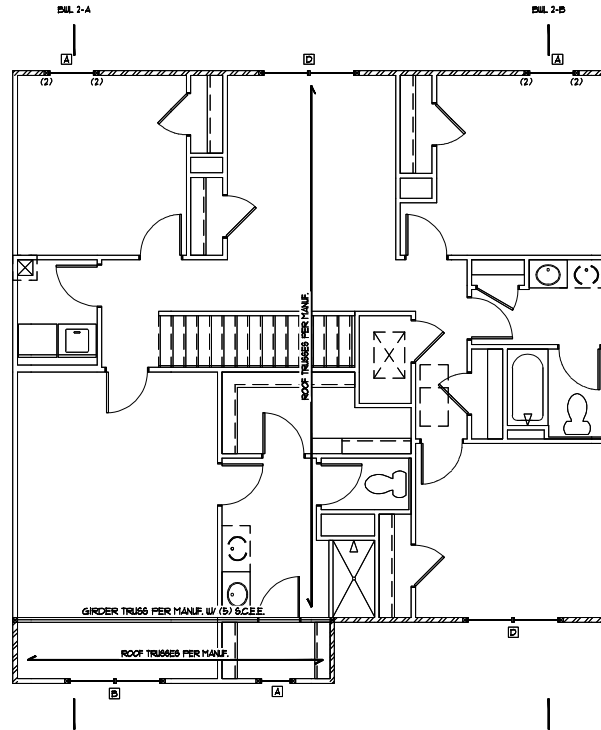
ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT. SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NRC.

**SECOND FLOOR FRAMING PLAN**

SCALE: 1/4" = 1'-0" ON 20'x20' OR 1/8" = 1'-0" ON 10'x10'

SECOND FLOOR BRACING (FT)	CONTINUOUS SHEATHING METHOD	
	REQUIRED	PROVIDED
BIL 2-1	6.0	71.0
BIL 2-2	8.0	25.0
BIL 2-4	3.8	49.0
BIL 2-5	3.8	36.0



ELEVATION BFK

HEADER SCHEDULE		
TAG	SIZE	JACKS (EACH END)
A	(2) 3x6	(2)
B	(2) 2x6	(2)
C	(2) 2x6	(2)
D	(2) 2x6	(2)
E	(2) 3x4 LVL/LVL	(3)
F	(3) 2x6	(2)
G	(3) 2x6	(2)
H	(3) 2x6	(2)
I	(3) 2x6	(2)

NOTES:  
1. HEADER SIZES SHOWN ON PLANS ARE MINIMUM. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION.  
2. ALL HEADERS TO BE DROPPED (IND).  
3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS LISTED ABOVE (IND).

KING STUD SCHEDULE	
MAXIMUM HEADER SPAN	MINIMUM KING STUDS EE
4'-0"	(1)
6'-0"	(2)
8'-0"	(2)
10'-0"	(3)
12'-0"	(3)
14'-0"	(3)
16'-0"	(4)

WALL STUD SCHEDULE (10 FT HEIGHT)				
STUD SIZE	STUD SPACING (O.C.)	ROOF ONLY	ROOF 1 FLOOR / 2 FLOORS	NON-LOAD BEARING
2x4	24"	16"	16"	24"
2x6	24"	24"	16"	24"

- NOTES:  
1. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C.  
2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX. OF 14" O.C.  
3. TWO STORY WALLS SHALL BE FRAMED w/ 2x4 STUDS # 12" O.C. OR 2x6 STUDS # 16" O.C. BALLBOON FRAMED w/ HORIZ. BLOOCKING # 6'-0" O.C. VERTICALLY.

LINTEL SCHEDULE		
TAG	SIZE	OPENING SIZE
①	L3x3x1/4"	LESS THAN 6'-0"
②	L3x3x1/4"	6'-0" TO 10'-0"
③	L4x4-1/2x1/8"	GREATER THAN 10'-0"
④	L3x3-1/2x1/8"	ALL ARCHED ROLLED OR EQUIV. OPENINGS

SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED # 16" O.C. (TYP FOR ③ & ④)

ALL HEADERS WHERE BRICK IS USED, TO BE: (IND)

SHADED WALLS INDICATED LOAD BEARING WALLS

JOIST 4 BEAM SIZES SHOWN ARE MINIMUM. BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDED THE ENTIRETY OF THE MEMBER IS WRAPPED TO PREVENT MOISTURE INTRUSION.

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602.10.6 & FIG. R602.10.7 OF THE 2018 NRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.10.3 OF THE 2018 NRC.



Del HORIZON  
ARCHITECTURE  
1000 W. HARRISON BLVD  
RALEIGH, NC 27603

Region 1A  
Second Floor Framing Plan



DATE: 04/21/21  
DRAWN: THT  
CHECKED: THT  
DESIGNED: THT  
DATE: 04/21/21

94.1

**TRUSS UPLIFT CONNECTOR SCHEDULE**

MAX UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FIN
6000 LBS	H2BA	PER WALL SHEATHING & FASTENERS	
1000 LBS	(2) H2BA	C2M6 (END x IP)	DTTZ
1400 LBS	HT5AP	C2M6 (END x IP)	DTTZ
2000 LBS	(2) HT5AP	(2) C2M6 (END x IP)	DTTZ
2500 LBS	(2) HT5AP	(2) C2M6 (END x IP)	HT14
3400 LBS	L27H-S04SD	HT22X	HT14

1. ALL PRODUCTS LISTED ARE 6000PSI STRENGTH-EQUIVALENT PRODUCTS MAY BE USED PER MANUFACTURER'S SPECIFICATIONS.
2. UPLIFT VALUES LISTED ARE FOR S1P 2 GRADE MEMBERS.
3. REFER TO TRUSS LAYOUT PER MANUF. FOR UPLIFT VALUES AND TRUSS TO TRUSS CONNECTIONS. CONNECTORS SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE.
4. CONTACT SUMMIT FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.

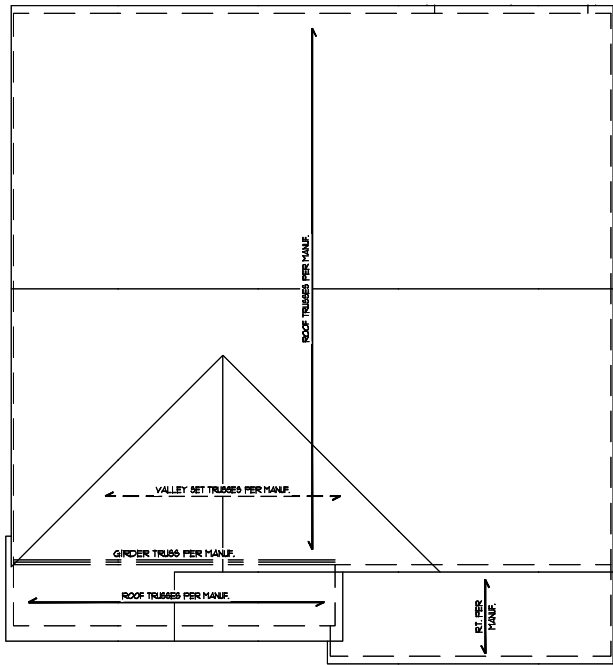
NOTE: 1ST FLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNL)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORNER WALLS (TYP, UNL)

REFER TO DETAIL S03P FOR EYEBROW RETURN OR SHED ROOF FRAMING REQUIREMENTS (TYP FOR ROOFS PROTRUDING MAXIMUM 24" FROM STRUCTURE)

NOTE: TRUSS UPLIFT LOADS SHALL BE DETERMINED PER TRUSS MANUFACTURER IN ACCORDANCE WITH SECTION R602.11.11 WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE WIND UPLIFT LOAD PER IN ACCORDANCE WITH METHOD 3 OF SECTION R602.13 OF THE 2018 NCR. REFER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER REQUIREMENTS.

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY DR. HORTON. COMPLETED/REVISED ON 4/24/21 IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERS, LABORATORY & TESTING, P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERS, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ACCURACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.



ELEVATION BFK

**STRUCTURAL MEMBERS ONLY**

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT. SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCR.

**ROOF FRAMING PLAN**

SCALE: 1/4"=1'-0" OR 3/16"=1'-0" OR 1/8"=1'-0" OR 3/32"=1'-0"



DR. HORTON, INC.  
4000 American Lane  
Charlotte, NC 28215

Region 14  
Roof Framing Plan



DATE: 04/24/21  
SCALE: 1/4"=1'-0" OR 3/16"=1'-0" OR 1/8"=1'-0" OR 3/32"=1'-0"  
PROJECT: 1408-001  
DRAWING: 20P  
DESIGNED BY: GAO

2020 1408

DESIGN TO ACCORDANCE WITH ALL APPLICABLE CODES

000

**DESIGN SPECIFICATIONS:**

Construction Type: Commercial  Residential

- Applicable Building Codes:
- 2006 North Carolina Residential Building Code with All Local Amendments
  - ASCE 7-10 Minimum Design Loads for Buildings and Other Structures

**Design Loads:**

1. Roof Live Loads
  - a. Conventional 2x \_\_\_\_\_ 20 PSF
  - b. 121. Attic Truss \_\_\_\_\_ 20 PSF
  - c. 60 PSF
2. Roof Dead Loads
  - a. Conventional 2x \_\_\_\_\_ 10 PSF
  - b. Truss \_\_\_\_\_ 20 PSF
  - c. Snow \_\_\_\_\_ 5 PSF
  - d. Importance Factor \_\_\_\_\_ 10
3. Floor Live Loads
  - a. Typ. Dwelling \_\_\_\_\_ 40 PSF
  - b. Sleeping Areas \_\_\_\_\_ 30 PSF
  - c. Dwell \_\_\_\_\_ 40 PSF
  - d. Passenger Garage \_\_\_\_\_ 30 PSF
4. Floor Dead Loads
  - a. Conventional 2x \_\_\_\_\_ 10 PSF
  - b. 1-Joist \_\_\_\_\_ 5 PSF
  - c. Floor Truss \_\_\_\_\_ 5 PSF
5. Ultimate Wind Speed (3 sec. gust) \_\_\_\_\_ PER PLAN
  - a. Exposure \_\_\_\_\_ B
  - b. Importance Factor \_\_\_\_\_ 10
  - c. Wind Base Shear \_\_\_\_\_
  - d. 63.1 Vx + \_\_\_\_\_
  - e. 63.1 Vy + \_\_\_\_\_

MEAN ROOF HT.	UP TO 30'	30'-1'-35'	35'-1'-46'	46'-1'-60'
ZONE 1	16.7-18.0	18.0-21.5	21.5-22.1	22.1-23.9
ZONE 2	16.7-21.0	21.0-22.1	22.1-22.9	22.9-23.9
ZONE 3	16.7-21.0	21.0-22.1	22.1-22.9	22.9-23.9
ZONE 4	18.7-19.0	19.0-20.0	20.0-20.1	20.1-21.3
ZONE 5	18.7-24.0	24.0-25.2	25.2-26.1	26.1-26.9

6. Seismic
  - a. Site Class \_\_\_\_\_ D
  - b. Seismic Category \_\_\_\_\_ I
  - c. Importance Factor \_\_\_\_\_ 10
  - d. Seismic Use Group \_\_\_\_\_ I
  - e. Spectral Response Acceleration \_\_\_\_\_
  - f. 0.51 S<sub>v</sub> + kg \_\_\_\_\_
  - g. 0.52 S<sub>v</sub> + kg \_\_\_\_\_
  - h. Seismic Base Shear \_\_\_\_\_
  - i. 86.1 Vx + \_\_\_\_\_
  - j. 86.2 Vy + \_\_\_\_\_
7. Basic Structural System (check one)
  - Bearing Wall
  - Building Frame
  - Moment Frame
  - Dual w/ Special Moment Frame
  - Dual w/ Intermediate RC or Special Steel
  - Inverted Pendulum
8. Arch/Chord Components Analyzed or Not
  - a. Arch/Chord Components Analyzed \_\_\_\_\_ No
  - b. Lateral Design Control: Seismic  Wind
9. Assumed Soil Bearing Capacity \_\_\_\_\_ 2,000psf



**STRUCTURAL PLANS PREPARED FOR STANDARD DETAILS**

PROJECT ADDRESS: TBD OWNER: DR Horton Carolina Division 6801 Aronogue Blvd Charlotte, NC 28213

**ARCHITECT/DESIGNER:**

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering Laboratory + Testing, P.C. before construction begins.

**PLAN ABBREVIATIONS:**

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AF	ABOVE FINISHED FLOOR	RB	ROOF SUPPORT
CI	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SFF	SFRANCE PINE FIR
DSP	DOUBLE STUD ROCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	STP	SOUTHERN YELLOW PINE
EU	EACH END	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TOP	TRIPLE STUD ROCKET
OC	ON CENTER	TYF	TYPICAL
PSF	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WUF	WELDED WIRE FABRIC

Roof truss and floor joist layouts and their corresponding loading details, were not provided to SUMMIT Engineering Laboratory + Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by DE Design, Inc. Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

**SHEET LIST:**

Sheet No.	Description
CS1	Cover Sheet, Specifications, Revisions
D1a	Monolithic Slab Foundation Details
D1b	Stem Wall Foundation Details
D1c	Crail Space Foundation Details
D1d	Basement Foundation Details
D1f	Framing Details

**REVISION LIST:**

Revision No.	Date	Project No.	Description
1	5/3/17		Added box bay detail (2017). Added deck options with basement. Revised deck options with stem wall and crawl space foundations.
2	10/2/17		Revised stem wall insulation note.
3	2/28/18		Revised garage door detail, NC only.
4	2/28/18		Added high-wind foundation details.
5	12/28/18		Revised per 2018 NCRIC.
6	2/28/19		Revised per Mecklenburg County Comments.
7	3/18/19		Revised stem wall deck attachment and roof sheathing on wall sections.
8	3/6/19		Corrected dimensions at perimeter footings.
9	3/22/19		Added tall luncheon detail.

**DR. HORTON PROJECT SIGN-OFF:**

Manager	Signature
Operations	
Operations System	
Operations Product Development	



CLIENT: DR. Horton Carolina Division 6801 Aronogue Blvd Charlotte, NC 28213

**GENERAL STRUCTURAL NOTES:**

1. The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering Laboratory + Testing, P.C. (SUMMIT) or the SER. For the purposes of these construction documents the SER and SUMMIT shall be considered the same entity.
2. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction to stabilize the structure.
3. The SER is not responsible for construction sequence, methods, or techniques in connection with the construction of this structure. The SER will not be held responsible for the contractor's failure to conform to the contract documents, should any non-conformities occur.
4. Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.
5. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins.
6. The SER is not responsible for any secondary structural elements or non-structural elements, except for the elements specifically noted on the structural drawings.
7. This structure and all construction shall conform to all applicable sections of the International Residential Code.
8. This structure and all construction shall conform to all applicable sections of local building codes.
9. All structural assemblies are to meet or exceed to requirements of the current local building code.

**FOUNDATIONS:**

1. The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.

**CONCRETE:**

1. The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade.
3. Any fill shall be placed under the direction or recommendation of a licensed professional engineer.
4. The resulting soil shall be compacted to a minimum of 95% maximum dry density.
5. Excavations of footings shall be lined temporarily with a 6 mil polyethylene membrane if placement of concrete does not occur within 24 hours of excavation.
6. No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

**STRUCTURAL STEEL:**

1. Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design" latest editions.
2. Structural steel shall receive one coat of shop applied rust-inhibitive paint.
3. All steel shall have a minimum yield stress (F<sub>y</sub>) of 36 ksi unless otherwise noted on the plans.
4. Welding shall conform to the latest edition of the American Welding Society's Structural Welding Code AWS D11. Electrodes for shop and field welding shall be class E70XX. All welding shall be performed by a certified welder per the above standards.

**CONCRETE:**

1. Concrete shall have a normal weight aggregate and a minimum compressive strength (f'<sub>c</sub>) at 28 days of 3000 psi, unless otherwise noted on the plan.
2. Concrete shall be proportioned, mixed, and placed in accordance with the latest editions of ACI 318, "Building Code Requirements for Reinforced Concrete" and ACI 309, "Specifications for Structural Concrete for Buildings".
3. Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air entrainment amount (in percent) shall be within -.3 to .0% of target values as follows:
  - 31. Footings 5%
  - 33. Exterior Walls 5%
4. No admixtures shall be added to any structural concrete without written permission of the SER.

**CONCRETE REINFORCEMENT:**

1. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction, increased abrasion resistance, and residual strength.
2. Fibermesh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed oil materials and specifically manufactured for use as concrete secondary reinforcement.
3. Application of fibermesh per cubic yard of concrete shall equal a minimum of 0.25 by volume (15 pounds per cubic yard) fibermesh shall comply with ASTM C1116, "Local Building Code Requirements, and steel mesh, or access as current industry standard.
4. Steel reinforcing bars to be hot rolled steel conforming to ASTM A63, grade 60.
5. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 318, "Manual of Standard Practice for Detailing Concrete Structures".
6. Horizontal footing and wall reinforcement shall be continuous and shall have 90° bends, or corner bars with the same spacing as the horizontal reinforcement with a class B lap splice.
7. Lap reinforcement as required, a minimum of 48 bar diameters for tension or compression unless otherwise noted. Splices in rebar shall be 48 bar diameters.

**WOOD TRUSSES:**

1. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall not constitute a review for compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
2. The wood trusses shall be designed for all required loadings as specified in the local building codes the ASCE Standard "Minimum Design Loads for Buildings and Other Structures," (ASCE 7-05), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction" (NDS) and "Design Specification for Metal Plate Connected Wood Trusses".
4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Hoisting, and Bracing Metal Plate Connected Wood Trusses" (HDS-S). This bracing both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
5. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

**EXTERIOR WOOD FRAMED DECKS:**

1. Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code requirements or construction details.

**WOOD STRUCTURAL PANELS:**

1. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA guidance.
2. All structurally required wood sheathing shall bear the mark of the APA.

**WOOD TRUSSES:**

1. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall not constitute a review for compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
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3. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction" (NDS) and "Design Specification for Metal Plate Connected Wood Trusses".
4. The truss manufacturer shall provide adequate bracing information in accordance with "Commentary and Recommendations for Handling, Hoisting, and Bracing Metal Plate Connected Wood Trusses" (HDS-S). This bracing both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments for the trusses.
5. Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacturer.

**EXTERIOR WOOD FRAMED DECKS:**

1. Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code requirements or construction details.

**WOOD STRUCTURAL PANELS:**

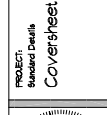
1. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA guidance.
2. All structurally required wood sheathing shall bear the mark of the APA.

**WOOD TRUSSES:**

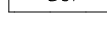
3. Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction perpendicular to framing, unless noted otherwise.
4. Roof sheathing shall be APA rated sheathing exposure 1 or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with 11-8d OC nail at 6" o/c at panel edges and at 19" in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plywood clips or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
5. Wood floor sheathing shall be APA rated sheathing exposure 1 or 2. All sheathing to be supporting framing with 11-8d OC noggins at 6" o/c at panel edges and at 19" in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall be span rating consistent with the framing spacing. Use suitable edge support by use of T&G plywood or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
6. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

**STRUCTURAL FIBERBOARD PANELS:**

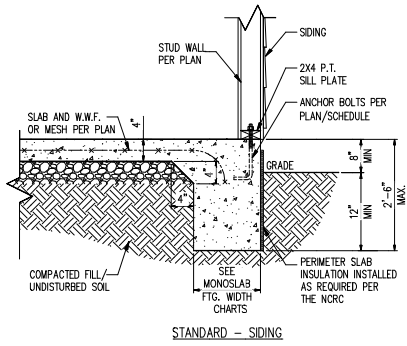
1. Fabrication and placement of structural fiberboard sheathing shall be in accordance with the applicable APA standards.
2. All structurally required fiberboard sheathing shall bear the mark of the APA.
3. Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
4. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.



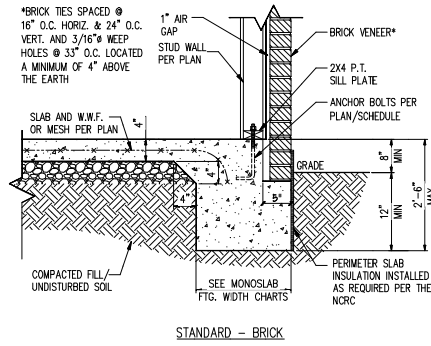
DRWNS DATE: 10/28/18 SCALE: 1/4" = 1'-0" SHEET NO.: 101 PROJECT: 1-PHASE DRAWN BY: LAB CHECKED BY: LAB



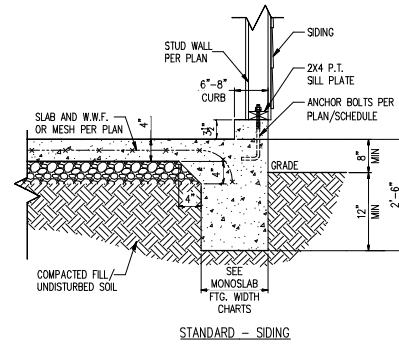




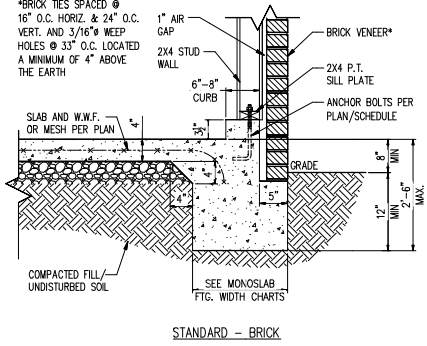
1 TYP. SLAB DETAIL  
D1m N.T.S.



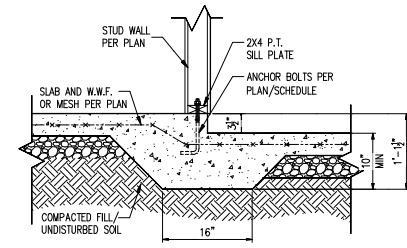
STANDARD - BRICK



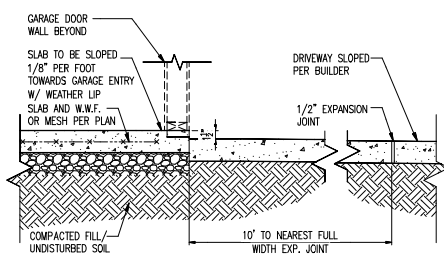
STANDARD - SIDING



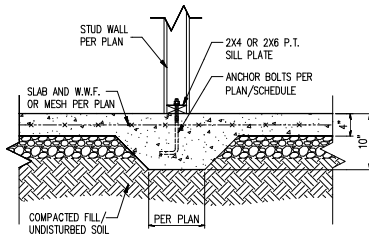
STANDARD - BRICK



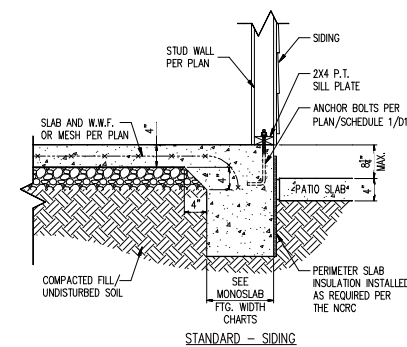
3 STEP IN GARAGE  
D1m N.T.S.



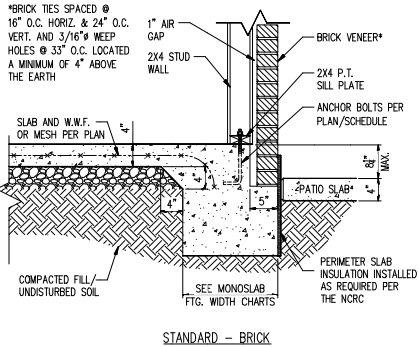
4 SLAB AT GARAGE DOOR  
D1m N.T.S.



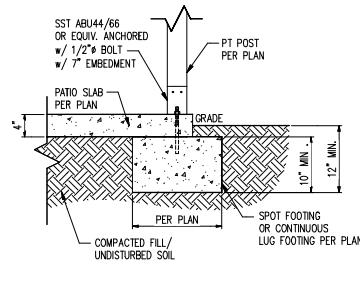
5 TYP. THICKENED SLAB DETAIL  
D1m N.T.S.



STANDARD - SIDING



STANDARD - BRICK



6A COVERED PATIO DETAIL  
D1m N.T.S.

MONOLITHIC FOOTING WIDTH

# OF STORIES	WIDTH BASED ON SOIL BEARING CAPACITY		
	1500 PSF	2000 PSF	2500 PSF
1 STORY - SID.	16"	16"	16"
1 STORY - BRICK VENEER	21"	21"	21"
2 STORY - SID.	16"	16"	16"
2 STORY - BRICK VENEER	21"	21"	21"
3 STORY - SID.	23"	18"	18"
3 STORY - BRICK VENEER	32"	24"	24"

\*5" BRICK LEDGE HAS BEEN ADDED TO THE MONOLITHIC FOOTING WIDTH FOR BRICK SUPPORT

WALL ANCHOR SCHEDULE

TYPE OF ANCHOR	MIN. CONC. EMBEDMENT	SPACING	INTERIOR WALL	EXTERIOR WALL
1/2" A307 BOLTS w/ STD. 90° BEND	7"	6'-0"	YES	YES
SST - MAS	4"	5'-0"	NO	YES
HILTI KWIK BOLT KBI 1/2-2-3/4	2-1/4"	6'-0"	YES	NO
1/2" HILTI THREADED ROD w/ HIT HY150 ADHESIVE	7"	6'-0"	YES	YES

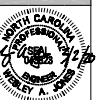
NOTE: INSTALL ALL ANCHORS 12" MAX. FROM ALL BOTTOM PLATE ENDS AND JOINTS.

- NOTES:
- REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET FOR ADDITIONAL INFORMATION.
  - PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
  - SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
  - REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS.
  - REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN.
  - PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC



CLIENT:  
Dixie-Vacation Cottages Division  
8600 Independence Blvd.  
Charlotte, NC 28210

PROJECT: Standard Details  
Monolithic Slab Foundation Details



STRUCTURAL REVISIONS ONLY

DATE	SCALE	BY	CHKD BY
12-14-20	1/4" = 1'-0"	JPD	JPD

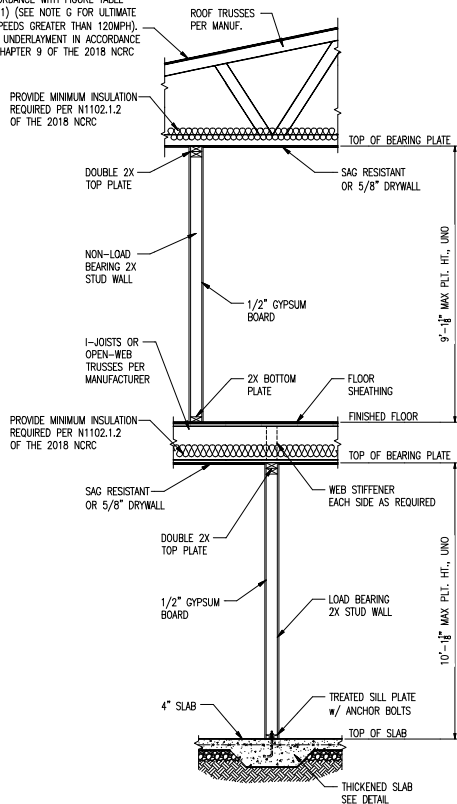
PROJECT: 14-104-000  
DRAWN BY: LAB  
CHECKED BY: JAU

DATE: 12-14-20

REVISIONS TO BE SHOWN IN RED OR A DIFFERENT COLOR FROM THE ORIGINAL

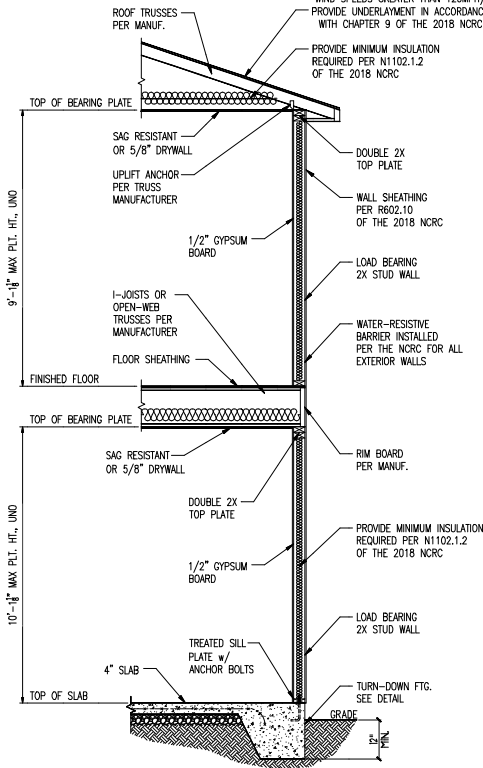
D1m

MIN. 3/8" ROOF SHEATHING SECURED IN ACCORDANCE WITH FIGURE TABLE R602.3(1) (SEE NOTE G FOR ULTIMATE WIND SPEEDS GREATER THAN 120MPH). PROVIDE UNDERLAYMENT IN ACCORDANCE WITH CHAPTER 9 OF THE 2018 NCR



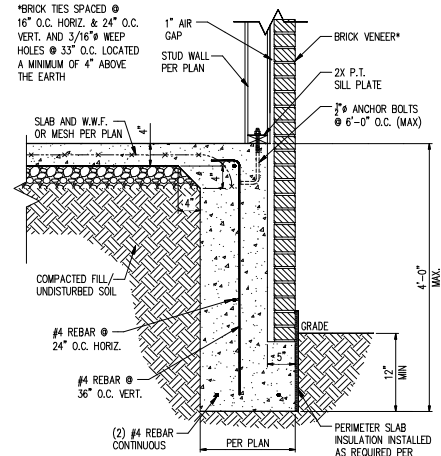
1 TYP. INTERIOR LOAD BEARING WALL SECTION  
D2m N.T.S.

MIN. 3/8" ROOF SHEATHING SECURED IN ACCORDANCE WITH FIGURE TABLE R602.3(1) (SEE NOTE G FOR ULTIMATE WIND SPEEDS GREATER THAN 120MPH). PROVIDE UNDERLAYMENT IN ACCORDANCE WITH CHAPTER 9 OF THE 2018 NCR



2 TYP. EXTERIOR LOAD BEARING WALL SECTION  
D2m N.T.S.

—SIMILAR w/ BRICK AND STONE  
—BRICK TIES SPACED @ 16" O.C. HORIZ. & 24" O.C. VERT.  
—MIN. 3/16" WEEP HOLES @ 33" O.C.



3 TALL SLAB DETAIL  
D2m N.T.S.

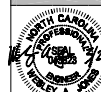
NOTES:

1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET FOR ADDITIONAL INFORMATION.
2. PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
3. SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS
5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
6. PERIMETER INSULATION SHALL AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCR



CLIENT:  
DFA-Vision Carolina Division  
8001 Innovation Blvd.  
Charlotte, NC 28219

PROJECT:  
Standard Details  
Monolithic Slab Foundation Details



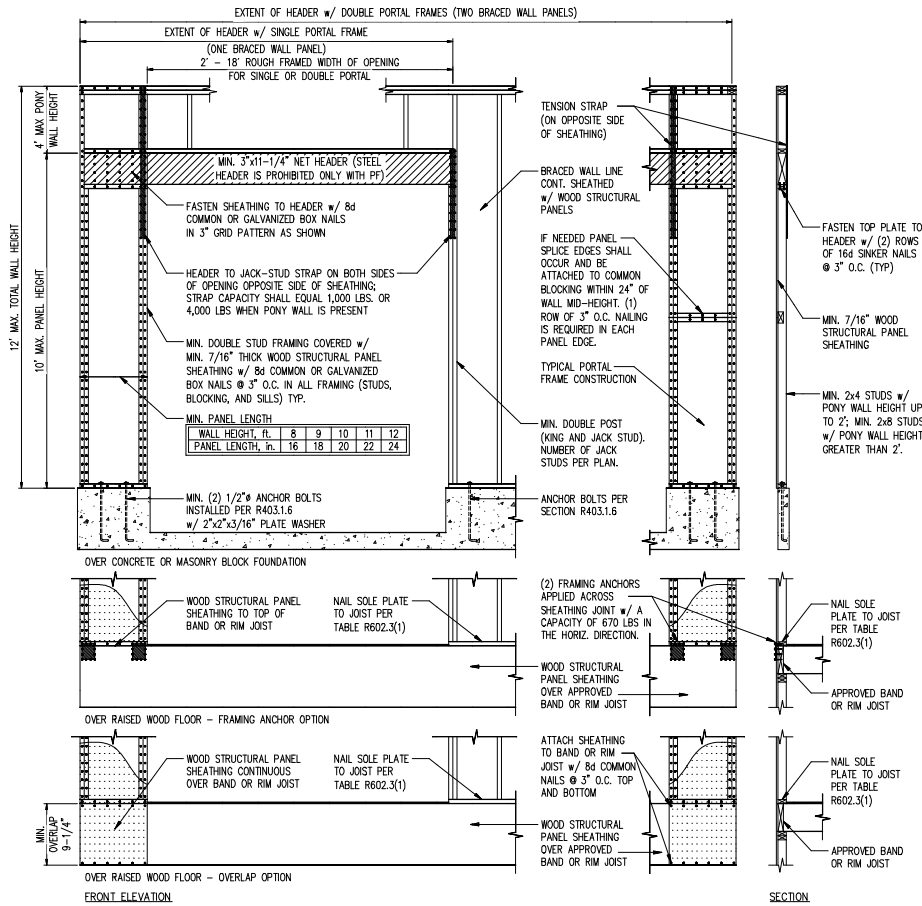
STRUCTURAL REVISIONS ONLY

DATE	BY	REVISION
08/14/2018	LAB	ISSUE FOR PERMIT
08/14/2018	LAB	ISSUE FOR PERMIT

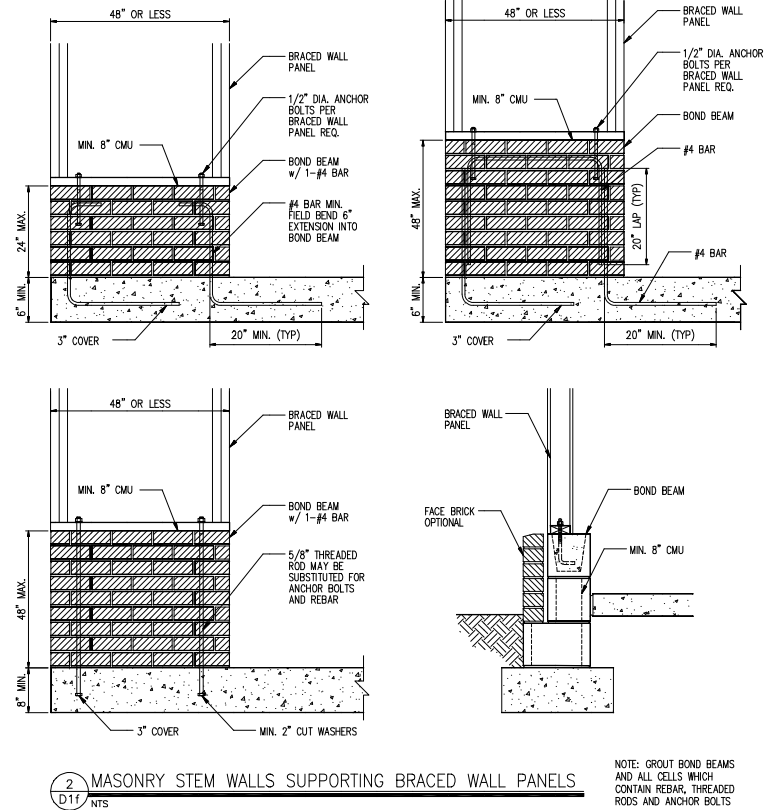
DESIGN BY: LAB  
CHECKED BY: LAB  
DATE: 08/14/2018

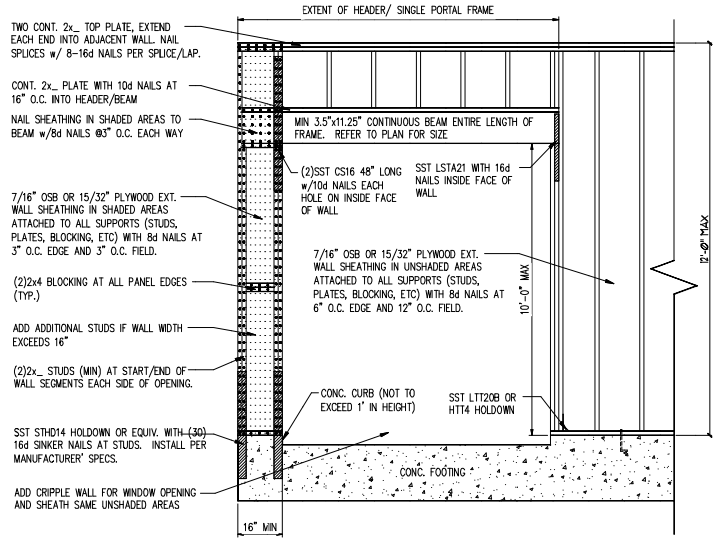
REVISIONS TO BE MADE BY: LAB  
DATE: 08/14/2018

D2m

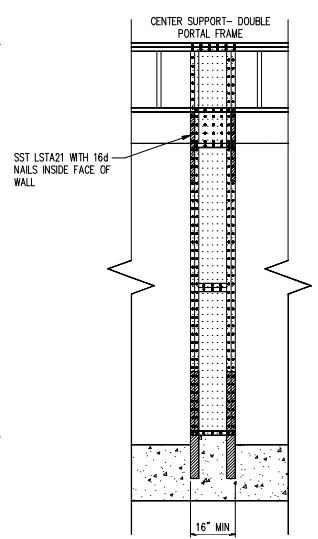


1 METHOD PF: PORTAL FRAME DETAIL  
D1f 3/8" = 1'-0"

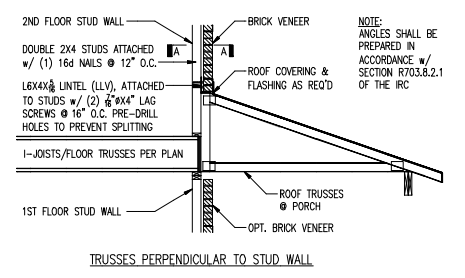
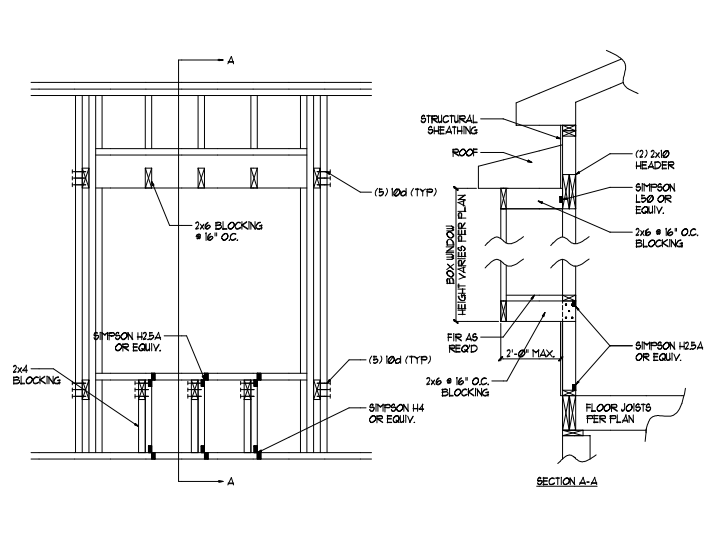




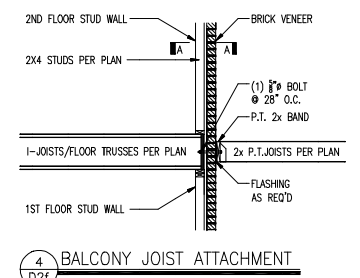
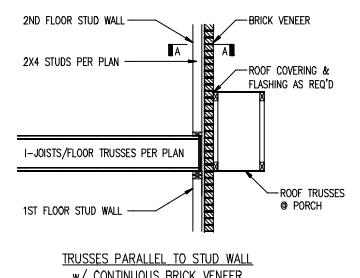
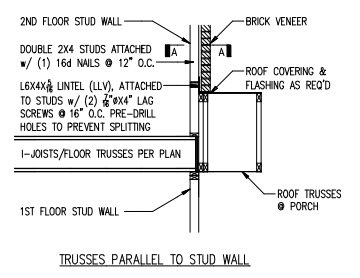
1 METHOD PF: PORTAL FRAME DETAIL w/ HOLD-DOWNS  
D2f 3/4" = 1'-0"



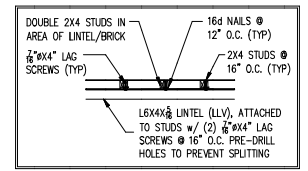
2 BOX WINDOW DETAIL  
D2f N.T.S.



3 BRICK SUPPORT ABOVE STORAGE/PORCH ROOF DETAIL  
D2f N.T.S.



4 BALCONY JOIST ATTACHMENT  
D2f N.T.S.



SECTION A-A  
N.T.S.

**SUMMIT**  
130 HARRISON BLVD. SUITE 100  
RALEIGH, NC 27601  
PHONE: 919.386.8885  
FAX: 919.386.8888  
WWW.SUMMIT-CONCRETE.COM

**NORTH CAROLINA**  
Professional Engineer  
No. 04881

CLIENT:  
D&B Vance Carolina Division  
8600 Independence Blvd.  
Charlotte, NC 28210

PROJECT:  
Standard Details  
Framing Details

**NORTH CAROLINA**  
Professional Engineer  
No. 04881

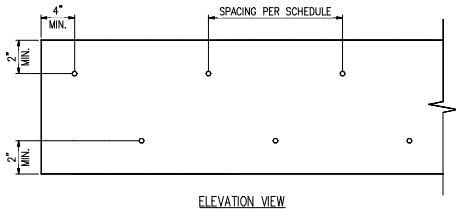
STRUCTURAL MEMBERS ONLY

DESIGNER:  
SCALE: 3/4" = 1'-0"  
DATE: 12/17/11  
PROJECT: 11-P-0018-00  
DRAWN BY: LAB  
CHECKED BY: JAU

REVISIONS:  
PROJECT: DATE:  
USER:

ISSUED TO: GROUP: SHEET FROM: A  
DATE: 12/17/11

D2f



ELEVATION VIEW

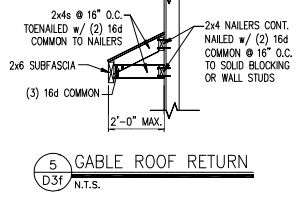
1 MULTI-PLY BEAM CONNECTION DETAIL  
D3f N.T.S.

**MINIMUM FASTENING REQUIREMENTS FOR TOP- AND SIDE-LOADED MEMBERS**

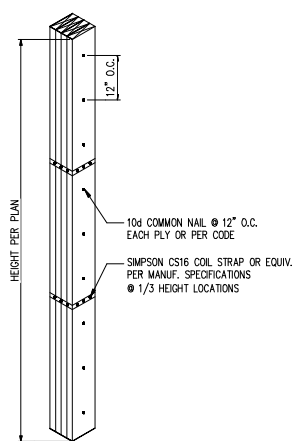
MEMBER TYPE	LVL BOTH	3 1/2\" WIDE		5 1/2\" WIDE		7\" WIDE	
		2-Ply 1 1/2\"	3-Ply 1 1/2\"	1 1/2\" + 3 1/2\"	1-Ply 1 1/2\"	2-Ply 1 1/2\" + 3 1/2\"	2-Ply 3 1/2\"
10d (0.132\" x 3\") Nails	7 1/2\" Sd < 14\" < 24\"	3 rows @ 12\" o.c.	3 rows @ 12\" s.c. (ES)	3 rows @ 12\" o.c.	-	3 rows @ 12\" o.c. (ES)	-
16d (0.142\" x 3 1/2\") Nails	7 1/2\" Sd < 14\" < 24\"	2 rows @ 12\" o.c.	2 rows @ 12\" s.c. (ES)	2 rows @ 12\" o.c.	-	2 rows @ 12\" o.c. (ES)	-
1/2\" Through Bolts		2 rows @ 24\" o.c.	2 rows @ 24\" o.c.	2 rows @ 24\" o.c.	-	2 rows @ 24\" o.c.	-
SDS 1/2\" x 3 1/2\" W635, 3 1/2\" TrussLok	Ø: 7 1/2\"	2 rows @ 24\" o.c.	2 rows @ 24\" s.c. (ES)	2 rows @ 24\" o.c.	-	2 rows @ 24\" o.c. (ES)	-
SDS 1/2\" x 6\" W66		-	-	2 rows @ 24\" o.c. (ES)	-	-	-
5\" TrussLok		-	2 rows @ 24\" o.c.	-	-	-	-
6 1/2\" TrussLok		-	-	-	-	2 rows @ 24\" o.c.	-

**NOTES:**

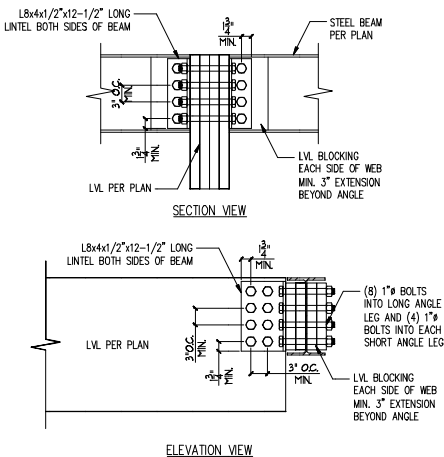
- All fasteners must meet the minimum requirements in the table above. Side-loaded multi-ply members must meet the minimum fastening and side-loading capacity requirements given on page 48.
- Minimum fastening requirements for depths less than 7 1/2\" require special consideration. Please contact your technical representative.
- Three general rules for staggering or offsetting for a certain fastener schedule:
  - If staggering or offsetting is not referenced, then none is required.
  - If staggering is referenced, then fasteners installed in adjacent rows on the front side are to be staggered up to one-half the o.c. spacing, but maintaining the fastener clearances above and
  - If "ES" is referenced, then the fastener schedule must be repeated on each side, with the fasteners on the back side offset up to one-half the o.c. spacing of the front side (whether or not it is staggered).



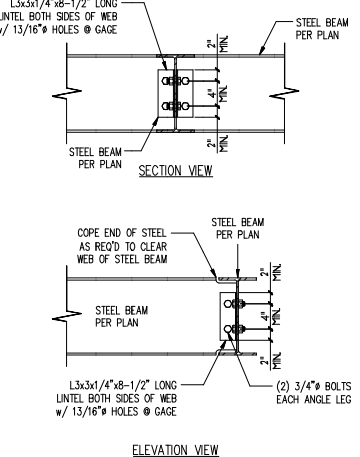
5 CABLE ROOF RETURN  
D3f N.T.S.



2 MULTI-PLY STUD CONNECTION DETAIL  
D3f N.T.S. 4+ PLYS



3 LVL TO STEEL DETAIL  
D3f N.T.S.



4 STEEL TO STEEL DETAIL  
D3f N.T.S.

**SUMMIT**  
130 INDUSTRIAL DRIVE  
RALEIGH, NC 27601  
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WWW.SUMMIT-CONCRETE.COM

**NORTH CAROLINA**  
REGISTERED PROFESSIONAL ENGINEER  
EXPIRES 12/31/2025  
04891

CLIENT:  
DFA-Vancor Carolina Division  
8600 Ironwood Blvd.  
Charlotte, NC 28219

PROJECT:  
Rosedale Details  
Framing Details

**NORTH CAROLINA**  
REGISTERED PROFESSIONAL ENGINEER  
EXPIRES 12/31/2025  
04891

STRUCTURAL MEMBERS ONLY

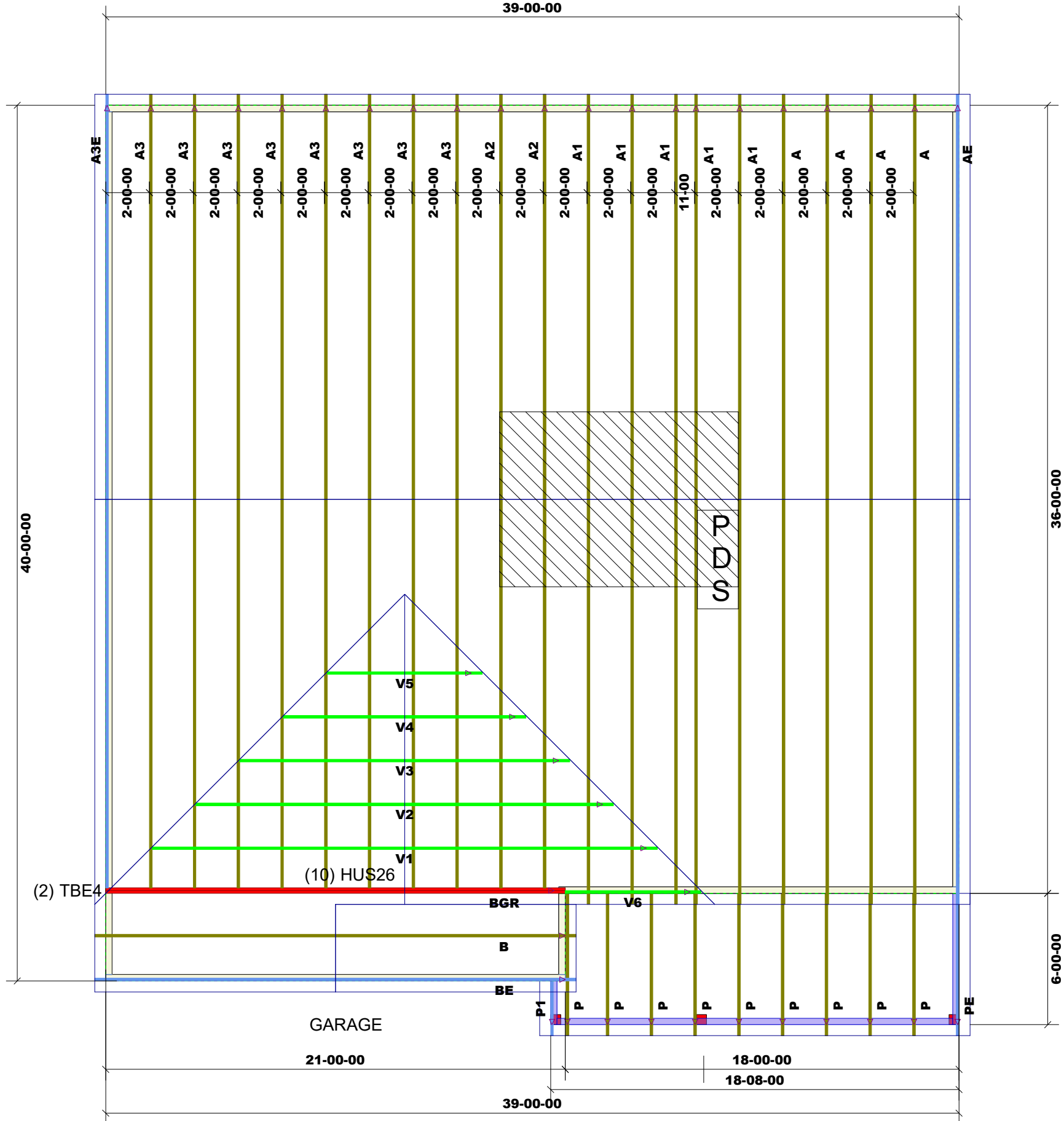
DATE: 03/08  
SCALE: 3/4\"/>



THIS LAYOUT IS INTENDED FOR THE PURPOSE OF TRUSS LOCATION AND PLACEMENT ONLY. REFER TO THE BUILDING PLANS FOR ACTUAL BUILDING CONSTRUCTION.



DEDICATED TO QUALITY AND EXCELLENCE  
 200 EMMETT ROAD  
 DUNN, NORTH CAROLINA 28334  
 PHONE: 910-892-8400



HANGERS:  
 (10) HUS26  
 (2) TBE4  
 (48) H2.5

1st Level Roof Area	2nd Level Roof Area
129.83	0

PROJECT:	
CUSTOMER:	D.R. HORTON - RAL - 055
MODEL:	HAYDEN B.F.K
QUOTE #:	31857
PRINT DATE:	4/28/2022
DRAWN BY:	Rodney Evans
SCALE:	N.T.S

TOP LIVE LOAD:	20.0 lb/ft²
TOP DEAD LOAD:	10.0 lb/ft²
BOTTOM DEAD LOAD:	10.0 lb/ft²
WIND SPEED:	130 mph

GENERAL NOTES:  
 - DO NOT CUT OR MODIFY TRUSSES  
 - TRUSSES ARE SPACED 24" ON CENTER UNLESS OTHERWISE NOTED  
 - REFER TO THE INDIVIDUAL TRUSS DESIGN DRAWINGS FOR THE LOCATION OF LATERAL BRACING AND MULTI-PLY CONNECTION REQUIREMENTS.  
 - PER ANSI TPI 1-2002 THE TRUSS ENGINEER IS RESPONSIBLE FOR TRUSS TO TRUSS CONNECTIONS AND TRUSS PLY TO PLY CONNECTIONS. THIS TRUSS PLAN RECOMMENDS TRUSS TO BEARING CONNECTIONS AND TRUSS TO BEAM CONNECTIONS WHICH SHALL BE REVIEWED BY THE BUILDING DESIGNER. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO RESOLVE ALL ROOF FORCES ADEQUATELY TO THE FOUNDATION.