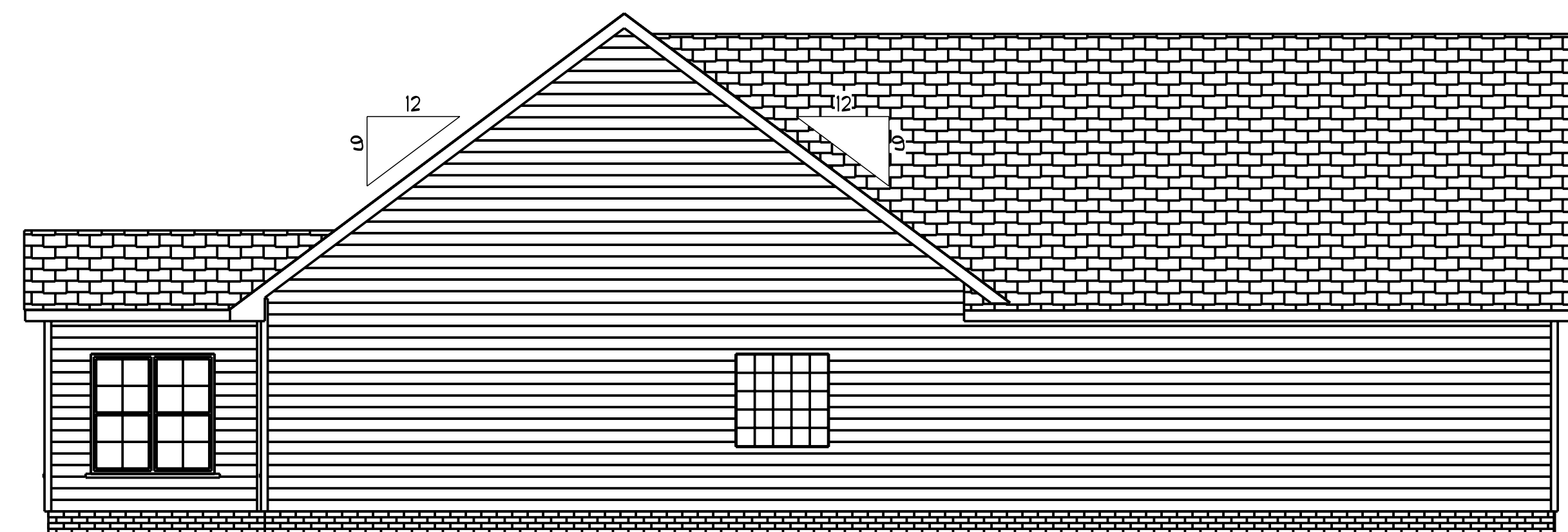




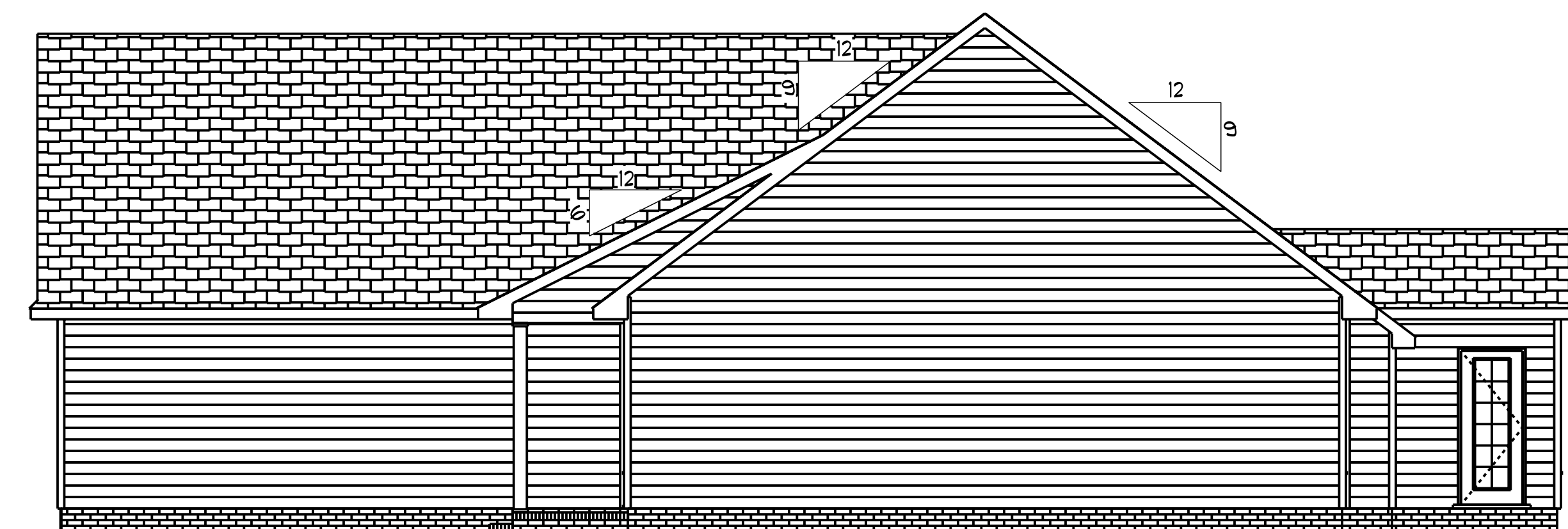
Front Elevation
Scale: 1/4" = 1'0"



Rear Elevation
Scale: 3/16" = 1'0"



Left Elevation
Scale: 3/16" = 1'0"



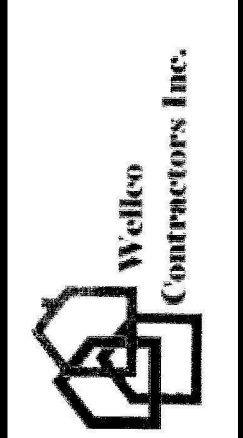
Right Elevation
Scale: 3/16" = 1'0"

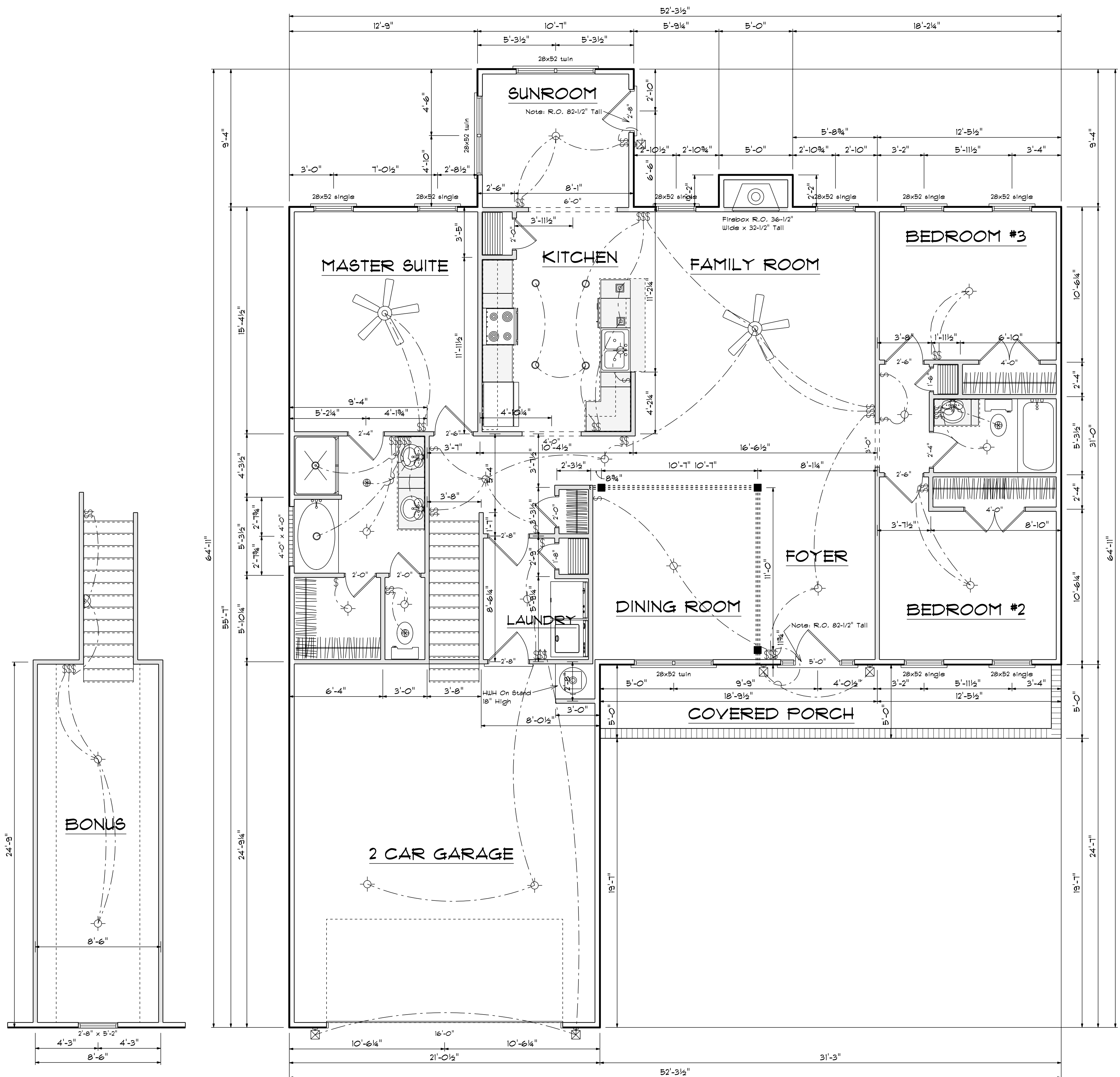


DATE: 11/29/2022
REVISED
DRAWING*

SCALE: 1/4"
DRAWN BY
APPROVED

Plan #9





First Floor Plan
Scale: 1/4" = 1'-0"

OPENING SCHEDULE				
PRODUCT CODE	SIZE	HINGE	REVERSED	COUNT
60X80 LH ENTRY - 2 SL	5'-0"	L	NA	1
32X80 FRENCH A 1	2'-8"	L	NO	1
192X84 - GARAGE DOOR	16'-0"	U	NO	1
1-6 Door Unit	1'-6"	L	NO	1
1-8 Door Unit	1'-8"	R	NO	1
2-0 Door Unit	2'-0"	R	NO	3
2-0 Door Unit	2'-0"	L	NO	1
2-4 Door Unit	2'-4"	L	NO	2
2-6 Door Unit	2'-6"	R	NO	1
2-6 Door Unit	2'-6"	L	NO	2
2-8 Door Unit	2'-8"	L	NO	1
2-8 Door Unit	2'-8"	R	NO	1
4-0 Doublehung Door Unit	4'-0"	LR	NO	2
28x52 single	2'-8" x 5'-2"	N	NA	9
28x52 twin	5'-4" x 5'-2"	NN	NA	3
4X8 GLASS BLOCK	4'-0" x 4'-0"	N	NA	1

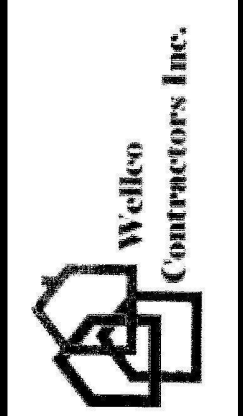
Areas

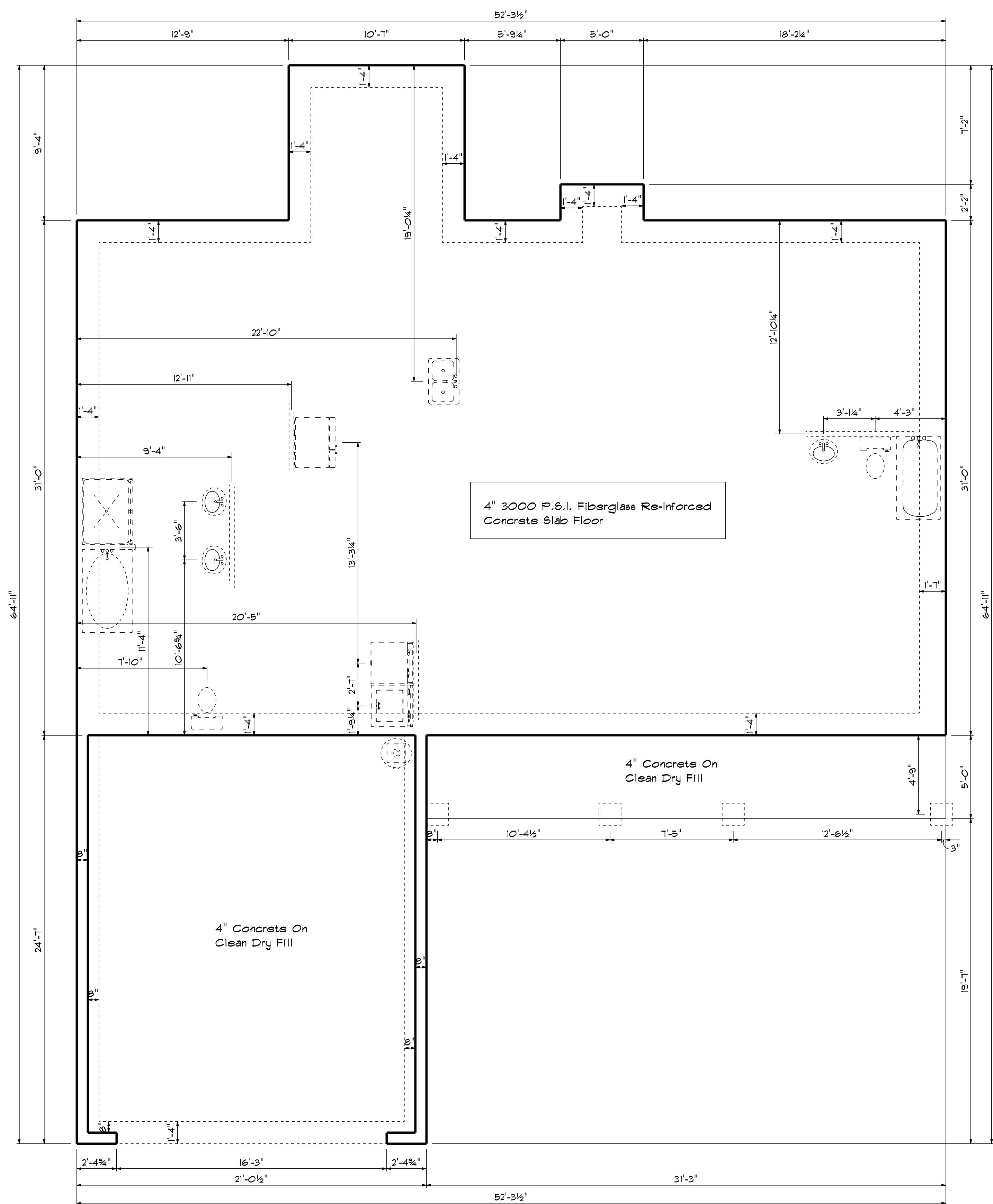
Main Floor	1728
Bonus Room	219
=====	
Total	1947
Garage	520
Front Porch	155

DATE: 11/29/2022
REVISED
DRAWING#

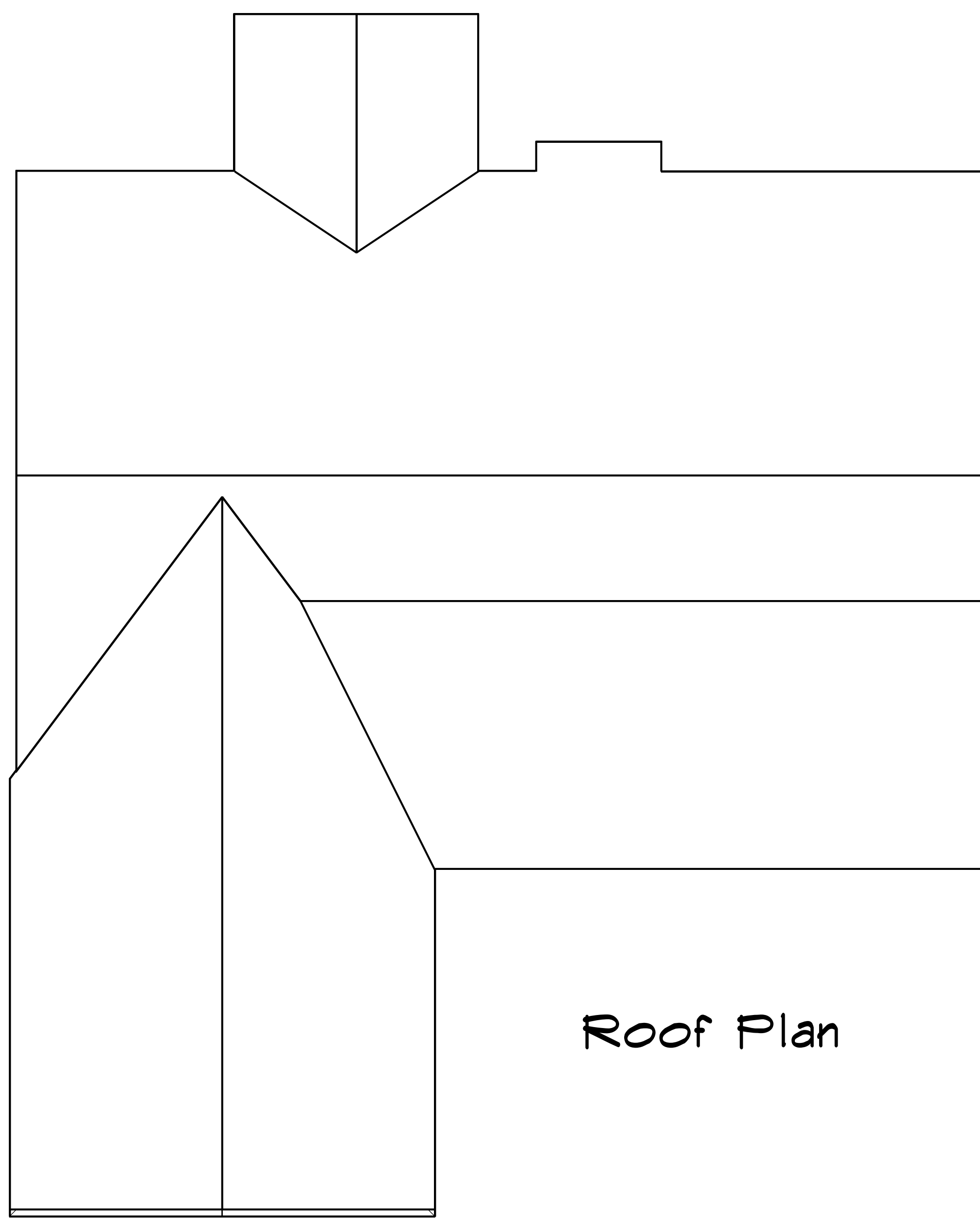
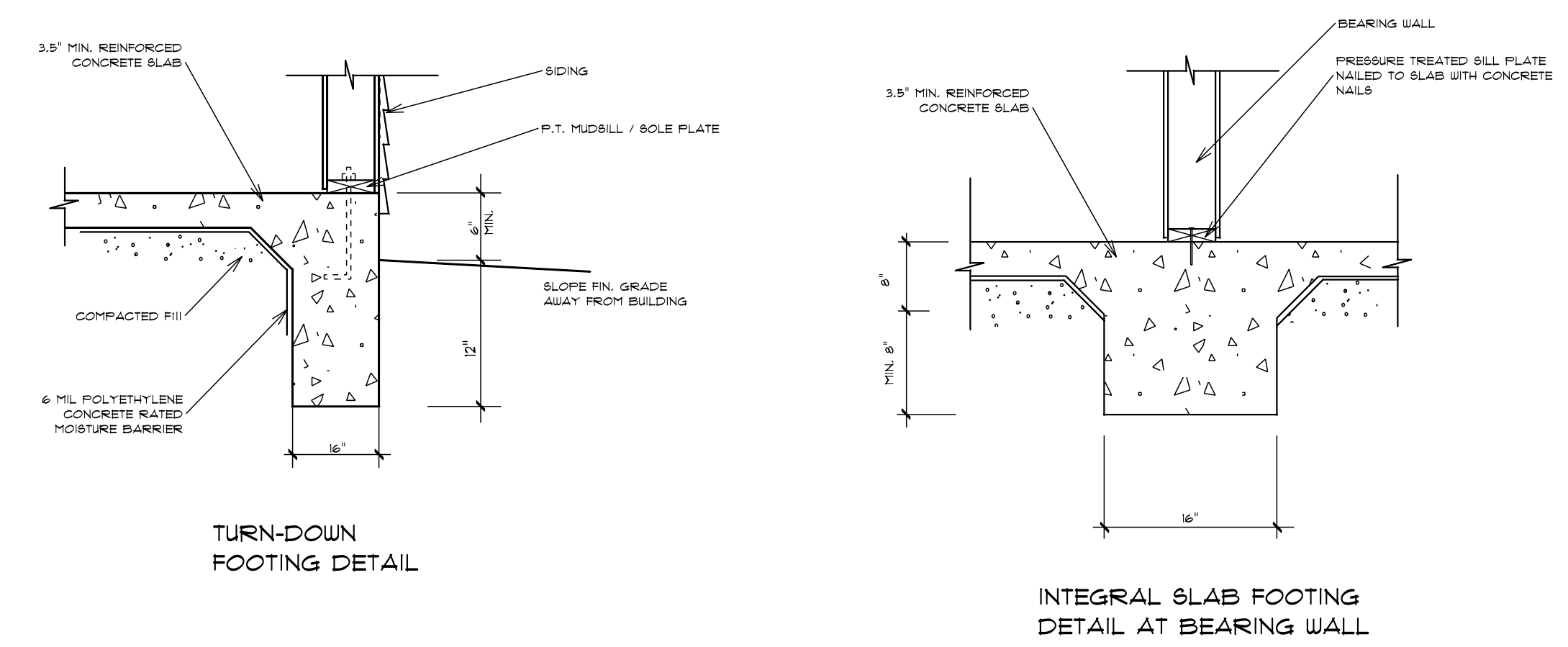
SCALE: 1/4"
DRAWN BY
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Plan #9





Foundation Plan
Scale: 1/4" = 1'-0"

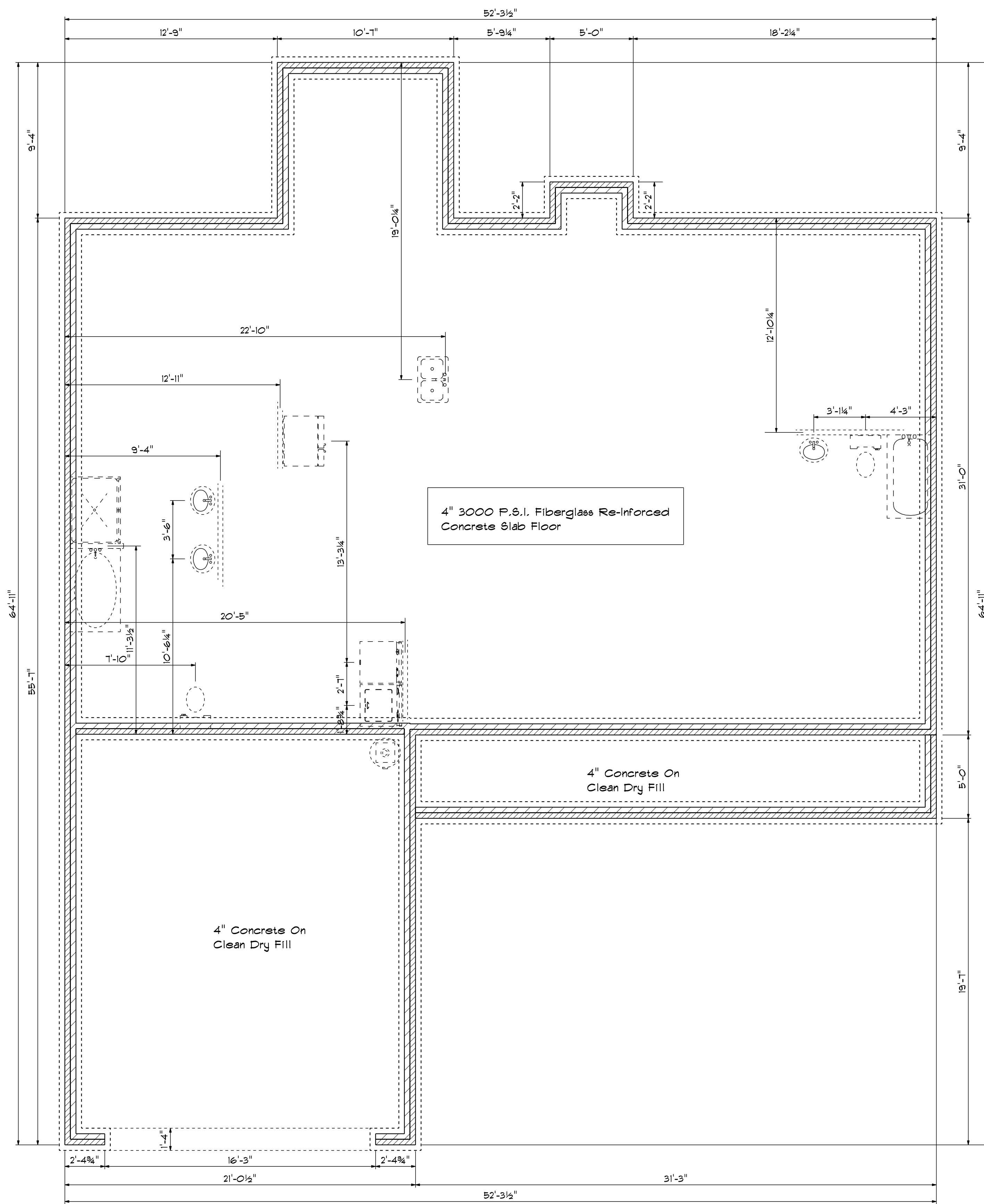


Roof Plan

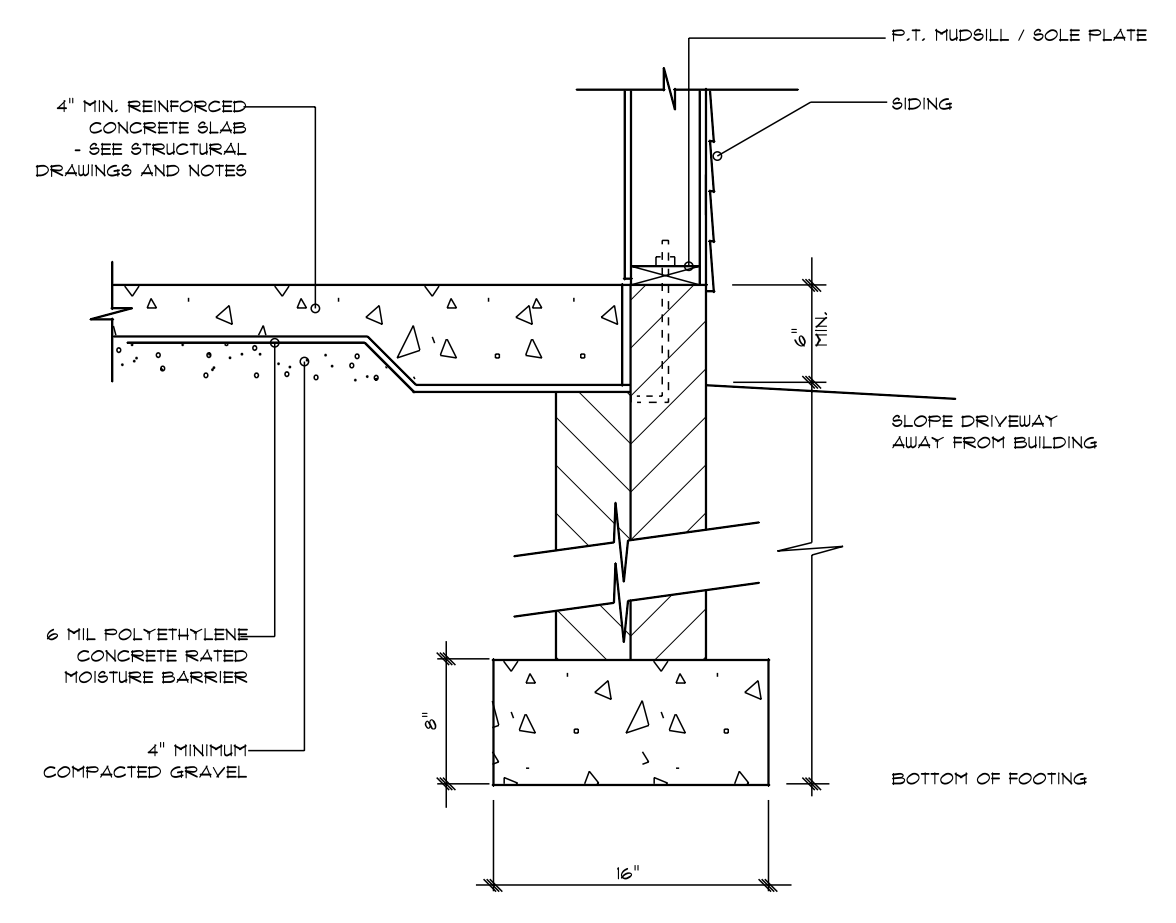
DATE: 11/29/2022
REVISED
DRAWING*

SCALE: 1/4"
DRAWN BY
APPROVED

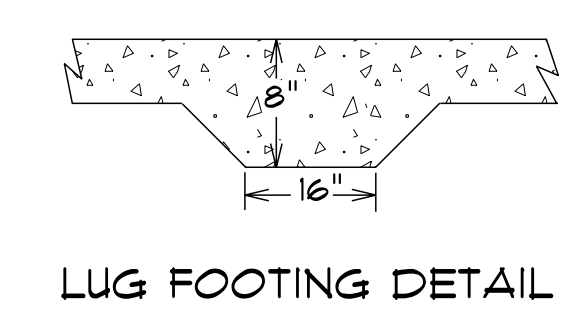
Plan #9



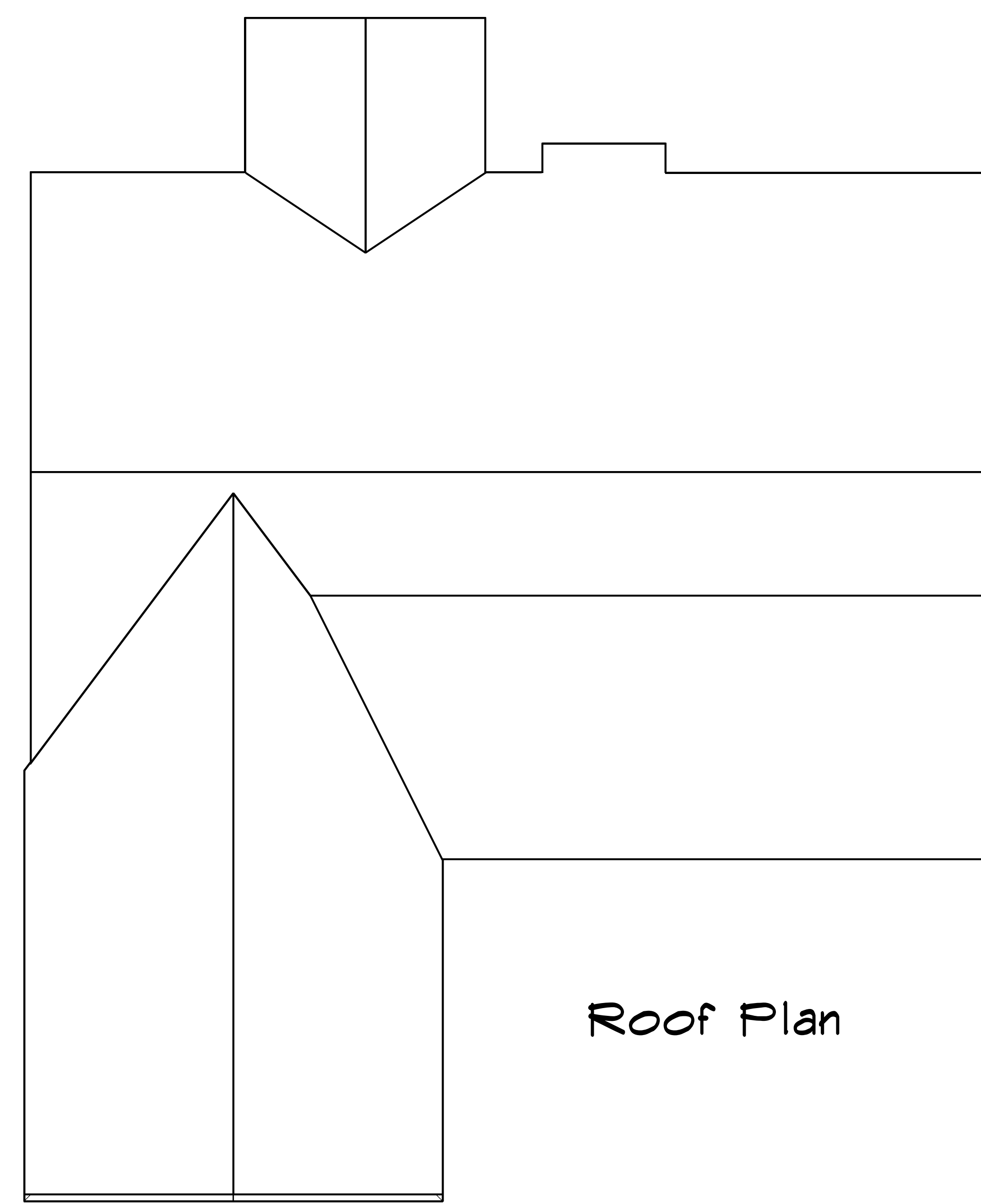
Foundation Plan
Scale: 1/4" = 1'-0"



STEM WALL FOOTING DETAIL



LUG FOOTING DETAIL

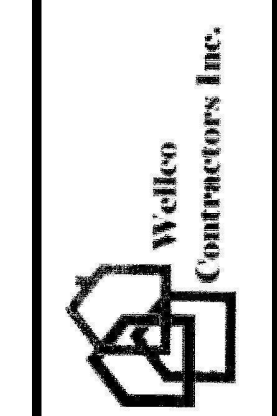


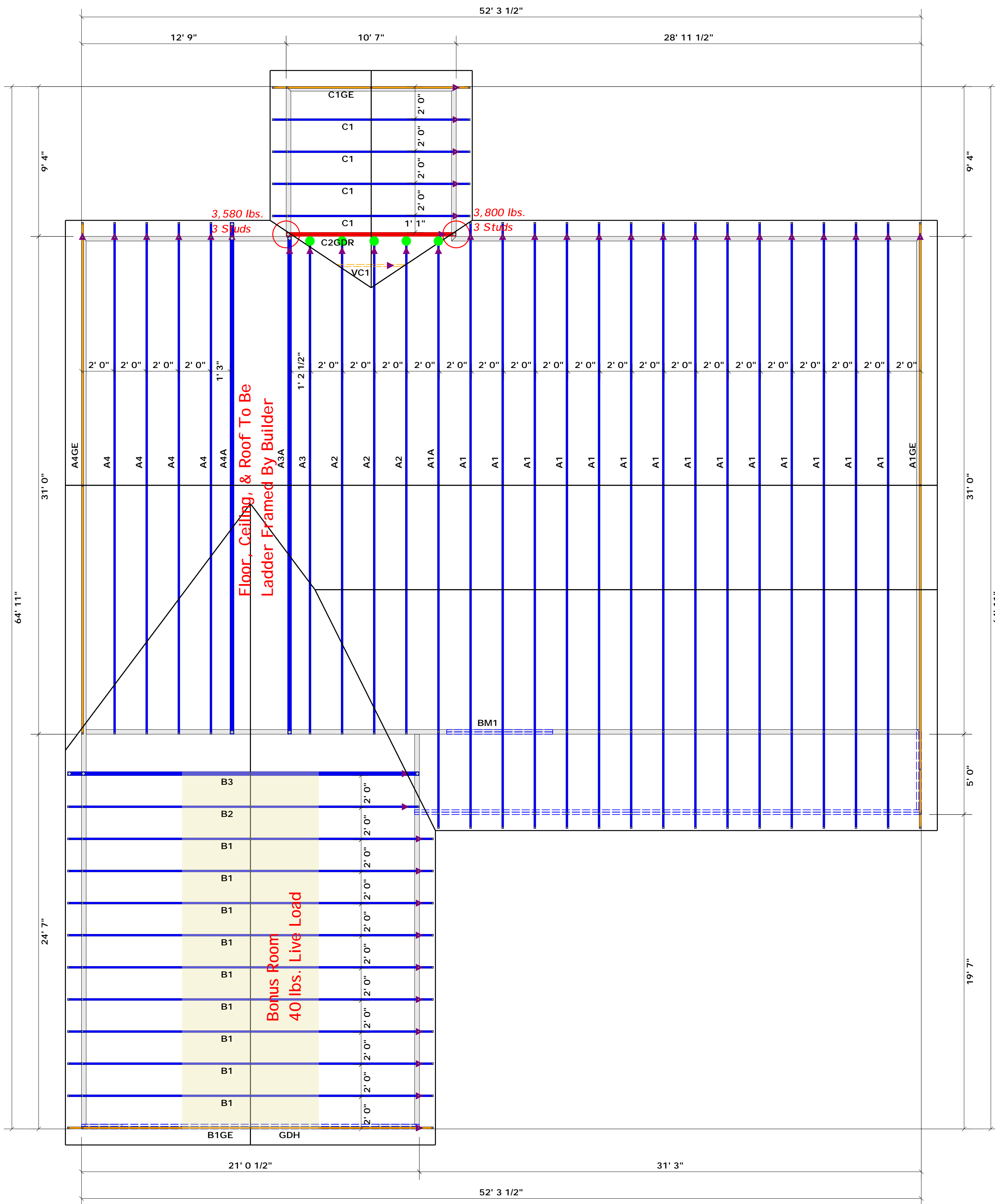
Roof Plan

DATE: 11/29/2022
REVISED
DRAWING*

SCALE: 1/4"
DRAWN BY
APPROVED

Plan #9





Floor, Ceiling, & Roof To Be Ladder Framed By Builder

Bonus Room
40 lbs. Live Load

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing)
Do Not Erect Trusses Backwards

Truss Placement Plan
SCALE: 1/4" = 1'

HANGER LEGEND	
●	= USP HUS26 / Single 2x Hanger

Beam Legend					
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH	22' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF

LOAD CHART FOR JACK STUDS

MEMBER ID	SPACING	REACTION
1700	1	2560
3400	2	5100
5100	3	7650
6800	4	10200
8500	5	12750
10200	6	15300
11900	7	
13600	8	
15300	9	

BUILDER	Wellco Contractors
JOB NAME	Lot 133 Hidden Lakes
PLAN	Plan 9
SEAL DATE	Seal Date
QUOTE #	Quote #
JOB #	J1122-5624

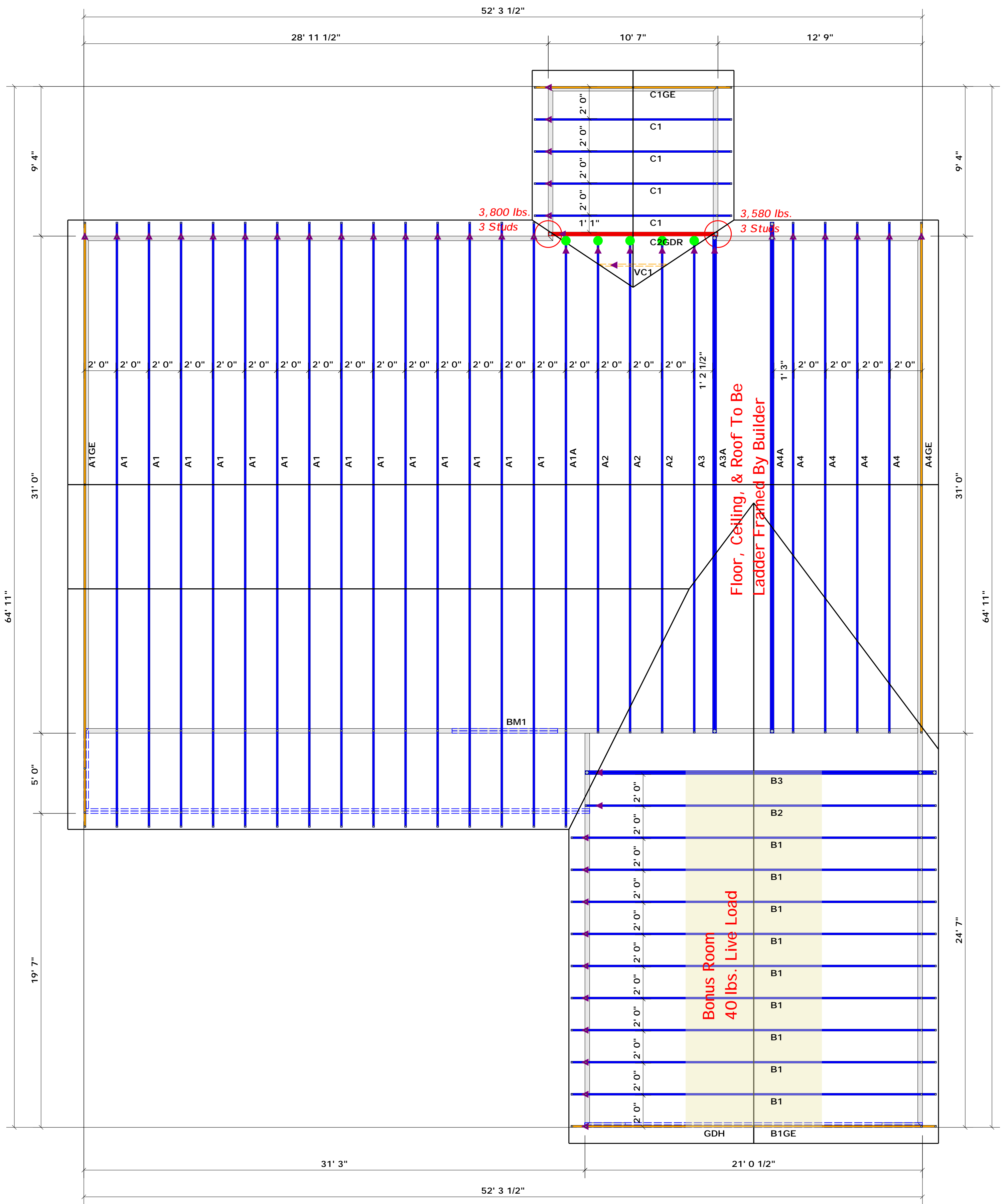
CITY / CO.	Spring Lake / Harnett
ADDRESS	Lot 133 Hidden Lakes
MODEL	Roof
DATE REV.	11/09/22
DRAWN BY	Curtis Quick
SALES REP.	Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSH-B1 and BCSH-B3 provided with the truss delivery package or online @ sbcindustry.com

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature: Curtis Quick
Curtis Quick

ROOF & FLOOR TRUSSES & BEAMS
Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444



HANGER LEGEND	
●	= USP HUS26 / Single 2x Hanger

Beam Legend					
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH	22' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

○ -- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan
SCALE: 1/4" = 1'

▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing)
Do Not Erect Trusses Backwards

LOAD CHART FOR JACK STUDS		
MEMBER	UP TO	DOWN FROM
1700	1	2550
3400	2	5100
5100	3	7650
6800	4	10200
8500	5	12750
10200	6	15300
11900	7	
13600	8	
15300	9	

BUILDER	Wellco Contractors	CITY / CO.	Spring Lake / Harnett
JOB NAME	Lot 133 Hidden Lakes	ADDRESS	Lot 133 Hidden Lakes
PLAN	Plan 9	MODEL	Roof
SEAL DATE	Seal Date	DATE REV.	11/09/22
QUOTE #	Quote #	DRAWN BY	Curtis Quick
JOB #	J1122-5624	SALES REP.	Lenny Norris

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSH-B1 and BCSH-B3 provided with the truss delivery package or online @ sbcindustry.com

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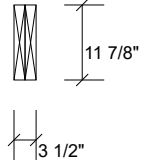
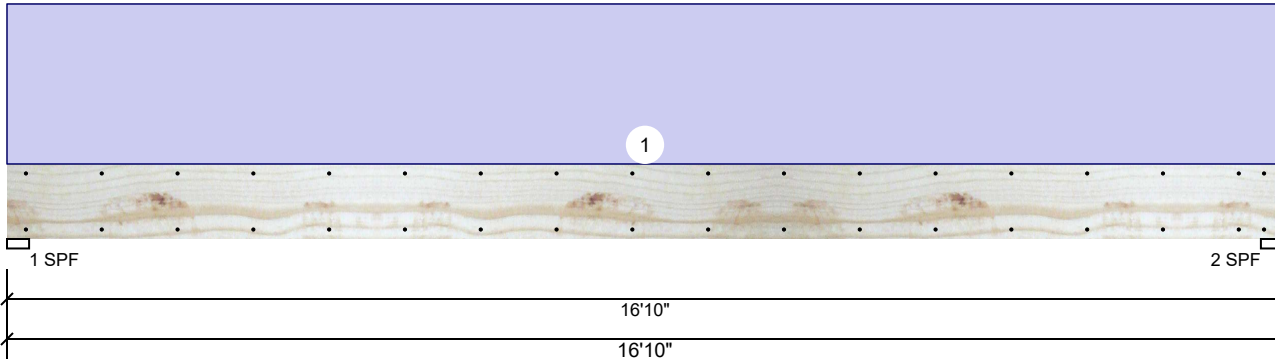
Signature: Curtis Quick
Curtis Quick

ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	2182	0	0	0
2	Vertical	0	2182	0	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	42%	2182 / 0	2182	Uniform	D
2 - SPF	3.500"	Vert	42%	2182 / 0	2182	Uniform	D

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	8689 ft-lb	8'5"	17919 ft-lb	0.485 (48%)	D	Uniform
Unbraced	8689 ft-lb	8'5"	8702 ft-lb	0.998 (100%)	D	Uniform
Shear	1859 lb	15'6 5/8"	7980 lb	0.233 (23%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.453 (L/433)	8'5 1/16"	0.546 (L/360)	0.831 (83%)	D	Uniform

Design Notes

- Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- Refer to last page of calculations for fasteners required for specified loads.
- Girders are designed to be supported on the bottom edge only.
- Top loads must be supported equally by all plies.
- Top must be laterally braced at a maximum of 10'8 15/16" o.c.
- Bottom must be laterally braced at end bearings.
- Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform Self Weight			Top	250 PLF 9 PLF	0 PLF	0 PLF	0 PLF	0 PLF	

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
- LVL not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

- LVL beams must not be cut or drilled
- Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
- Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

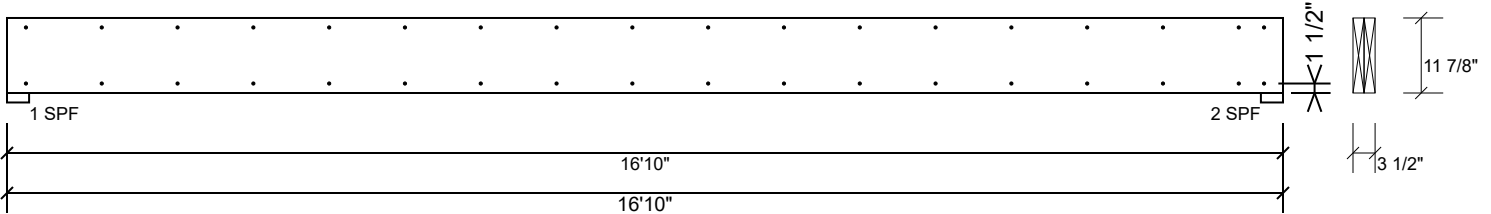
Metsä Wood
 301 Merritt 7 Building, 2nd Floor
 Norwalk, CT 06851
 (800) 622-5850
www.metsawood.com/us

Comtech, Inc.
 1001 S. Reilly Road, Suite #639
 Fayetteville, NC
 USA
 28314
 910-864-TRUS



GDH Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

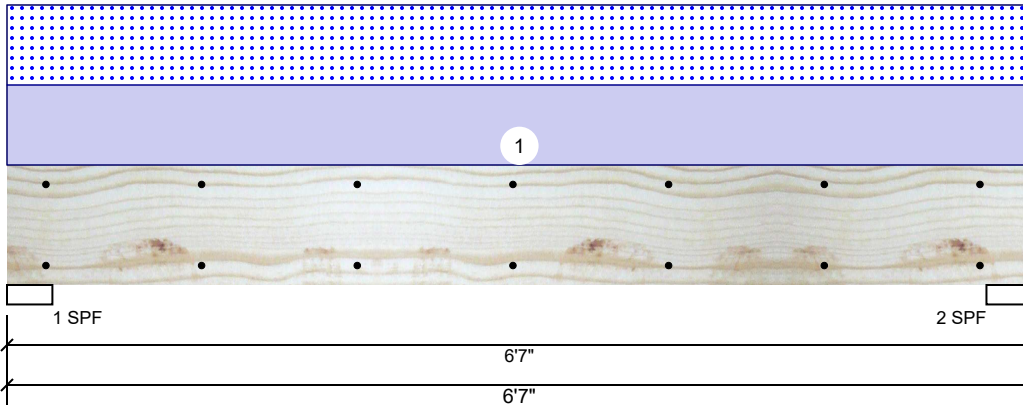
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 1001 S. Reilly Road, Suite #639
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 28314
 910-864-TRUS



BM1 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED Level: Level



Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application:	Floor
Design Method:	ASD
Building Code:	IBC/IRC 2015
Load Sharing:	No
Deck:	Not Checked

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1442	1419	0	0
2	Vertical	0	1442	1419	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	55%	1442 / 1419	2861	L	D+S
2 - SPF	3.500"	Vert	55%	1442 / 1419	2861	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4076 ft-lb	3' 3/2"	14423 ft-lb	0.283 (28%)	D+S	L
Unbraced	4076 ft-lb	3' 3/2"	10451 ft-lb	0.390 (39%)	D+S	L
Shear	1943 lb	1' 3/4"	7943 lb	0.245 (24%)	D+S	L
LL Defl inch	0.037 (L/2000)	3' 3/2"	0.153 (L/480)	0.240 (24%)	S	L
TL Defl inch	0.074 (L/992)	3' 3/2"	0.204 (L/360)	0.363 (36%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	431 PLF	0 PLF	431 PLF	0 PLF	0 PLF	A1
	Self Weight				7 PLF					

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

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2. LVL not to be treated with fire retardant or corrosive

chemicals

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2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 11/3/2024

Manufacturer Info

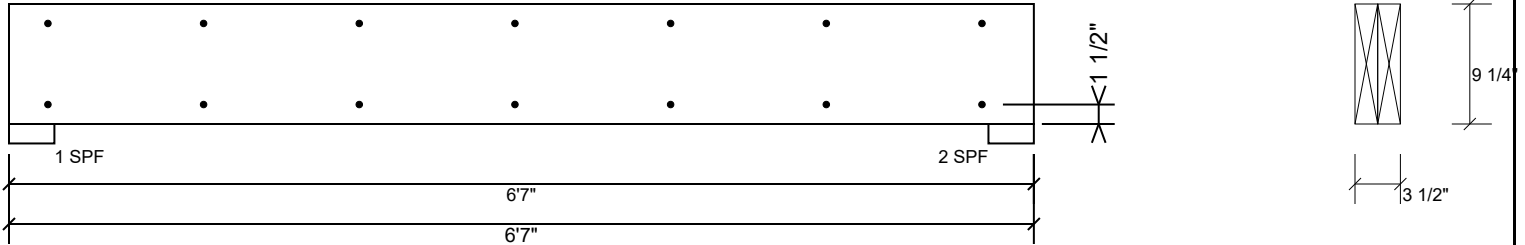
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BM1 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

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chemicals

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