

SYMBOLS + KEYS

DOOR AND WINDOW SIZE KEY
2860 = 2'-8" WIDE x 6'-0" HIGH

DRAWING NUMBER
1
3 = SECTION OR ELEVATION

SHEET NUMBER
ROOM TITLE
KITCHEN
10' CLG. WOOD
CEILING HEIGHT FLOOR FINISH

1 DRAWING TITLE

DETAIL OR ELEVATION NUMBER

14.00' = ELEVATION ABOVE SEA LEVEL
= REVISION NUMBER
= REVISION CLOUD
= PLAN REFERENCE NORTH
REF. 08

↔	SINGLE POLE SWITCH
↔↔	THREE WAY SWITCH
↔↔↔	FOUR WAY SWITCH
↔↔↔↔	DIMMER SWITCH
↔↔↔↔↔	SPEED CONTROL
⊕	DUPLEX OUTLET
⊕	1/2 HOT OUTLET
⊕ WP	WATER PROOF OUTLET
⊕ GF	GROUND FAULT OUTLET
⊕	QUADRIPLEX OUTLET
⊕	SPECIALTY OUTLET
⊕	FLOOR OUTLET
⊕	TELEPHONE JACK
⊕	THERMOSTAT
⊕	TELEVISION JACK
V	VENT
V/L	VENT w/ LIGHT
○	SURFACE MOUNTED FIXTURE
○	RECESSED FIXTURE
○	WALL MOUNTED FIXTURE
○	FLOOD LIGHT
○	FLUORESCENT FIXTURE
○	CEILING FAN
▽	STRIP LIGHTING
○	CEILING BOX
○	DOOR CHIME
○	ELECTRICAL PANEL
○	SMOKE DETECTOR
○	CARBON MONOXIDE DETECTOR



CONCEPTUAL RENDERING



DRAWING INDEX

- 0 COVER SHEET
- 1 FOUNDATION PLAN + BASEMENT PLAN
- 2 FIRST + SECOND FLOOR PLANS
- 3 ELEVATIONS
- 4 ELEVATIONS
- 5 WALL SECTIONS
- 6 ROOF PLAN + DETAILS
- 7 FIRST + SECOND ELECTRICAL LAYOUTS
- SP1 SPECIFICATIONS
- SP2 SPECIFICATIONS
- SP3 SPECIFICATIONS

GENERAL INFO.

AREA CALCULATIONS
FIRST FLOOR HEATED = 1,804 S.F. COVERED PORCH = 935 S.F.
SECOND FLOOR HEATED = 444 S.F. SCREENED PORCH = 358 S.F.
BASEMENT HEATED = 935 S.F. STORAGE = 410 S.F.
TOTAL HEATED = 3,183 S.F.

KRAKOWSKI RESIDENCE REUSE OF 19391 WHISPER CREEK

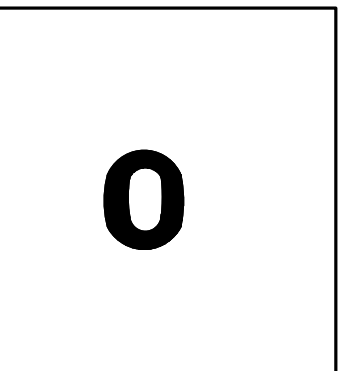
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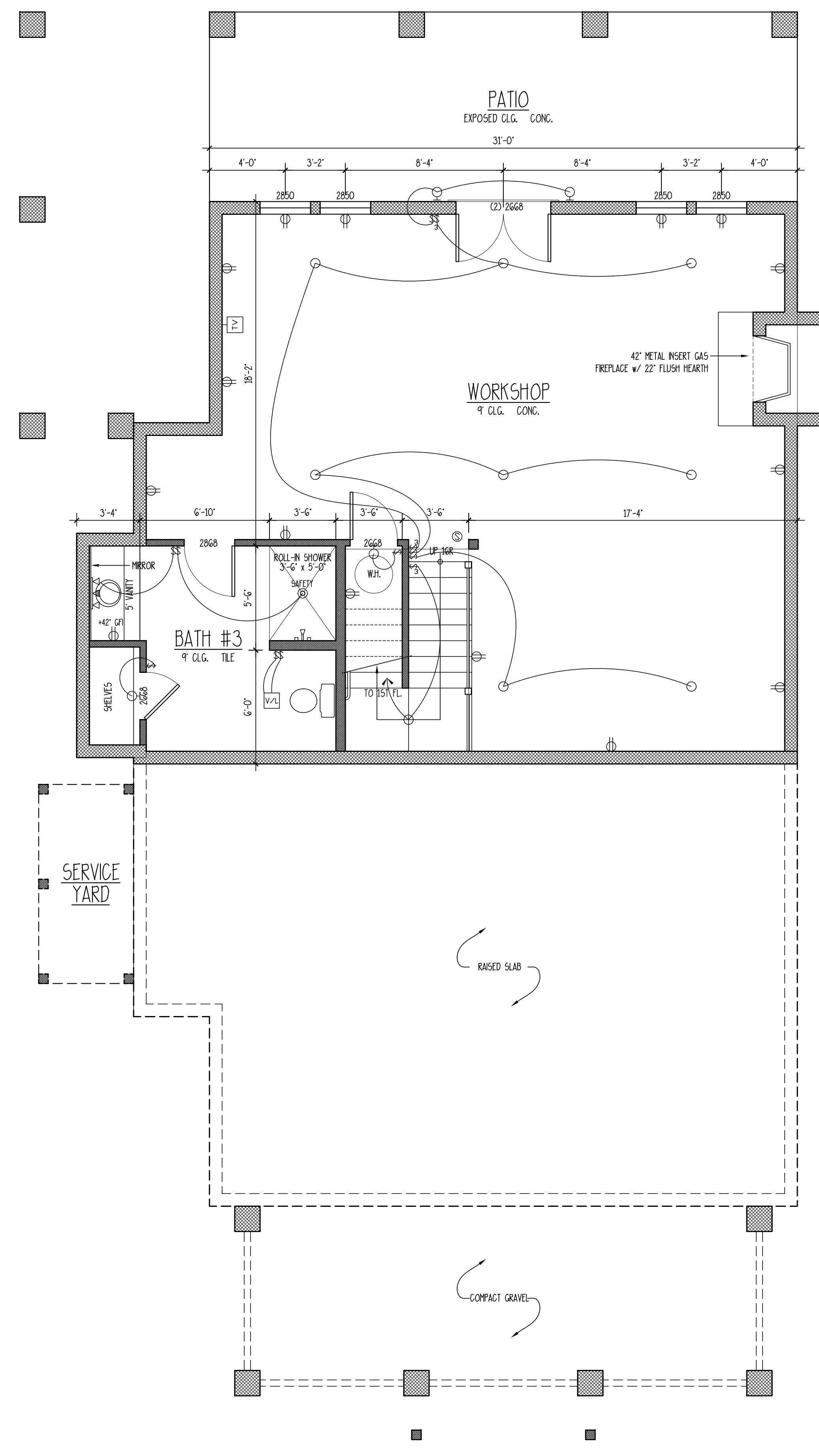
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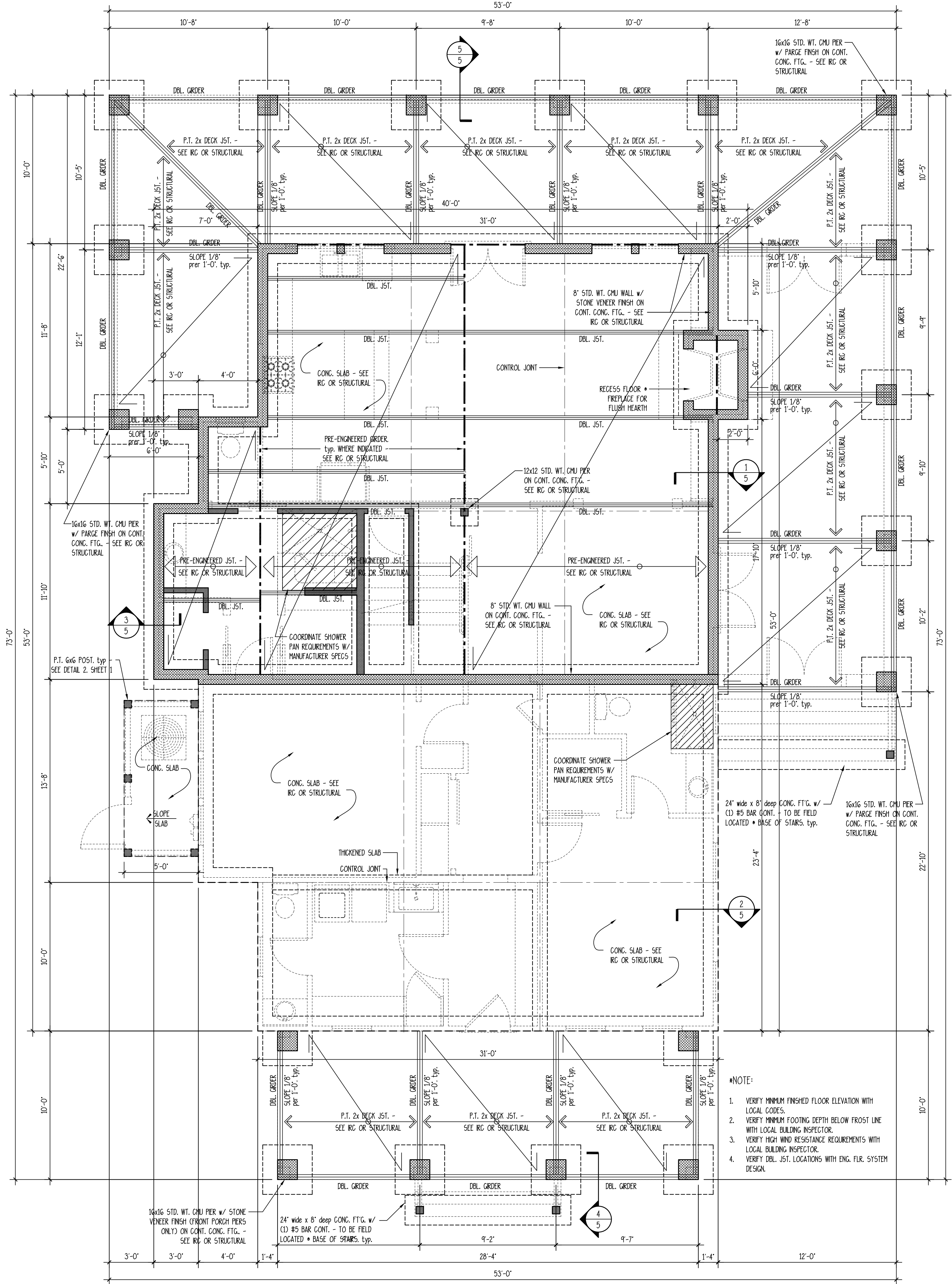
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DWG. NAME:	22304DWG





BASEMENT PLAN

SCALE: 1/4" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

- NOTE:**
1. VERIFY MINIMUM FINISHED FLOOR ELEVATION WITH LOCAL CODES.
 2. VERIFY MINIMUM FOOTING DEPTH BELOW FROST LINE WITH LOCAL BUILDING INSPECTOR.
 3. VERIFY HIGH WIND RESISTANCE REQUIREMENTS WITH LOCAL BUILDING INSPECTOR.
 4. VERIFY DBL. JST. LOCATIONS WITH ENG. FLR. SYSTEM DESIGN.

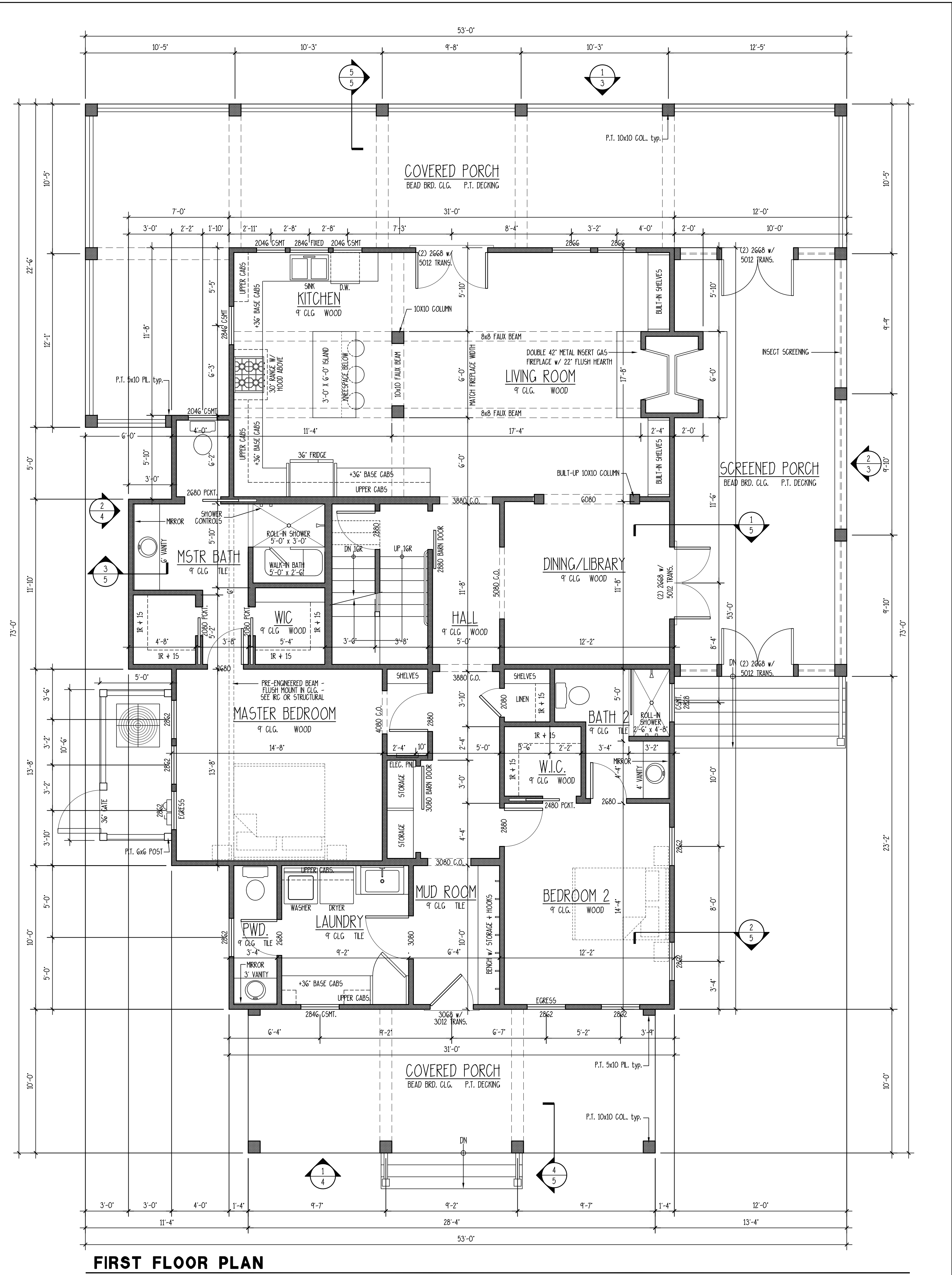
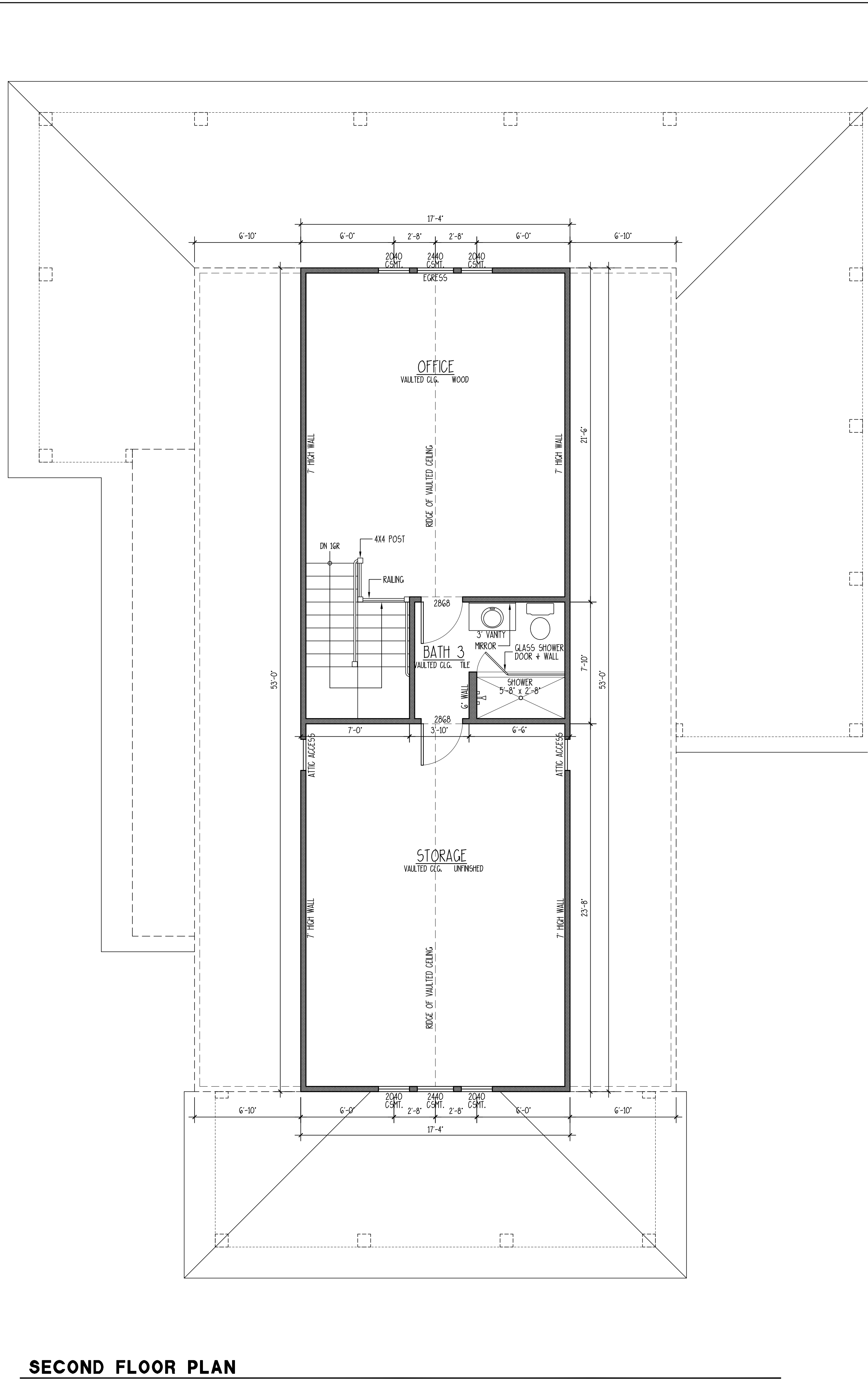
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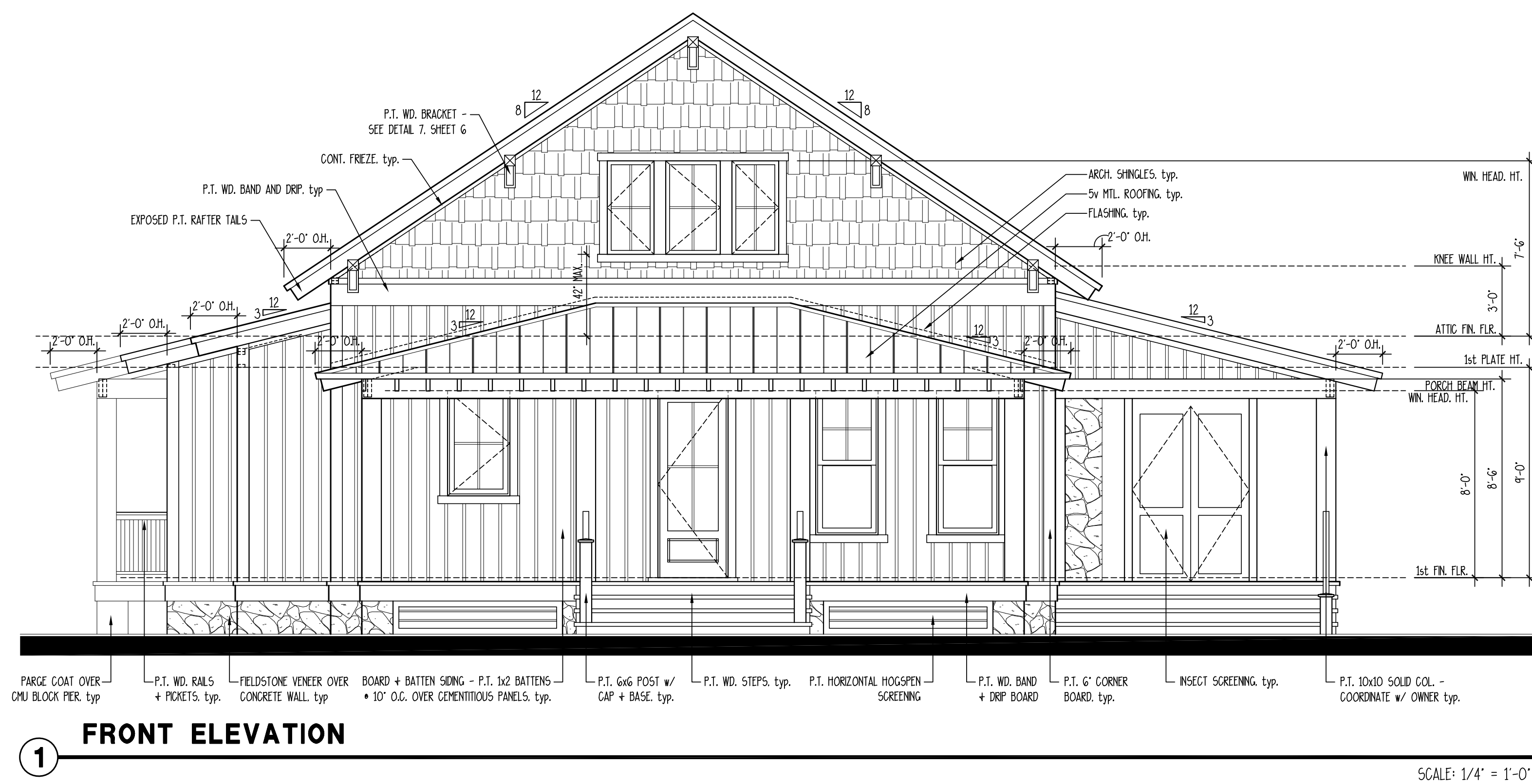


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ENGINEER:	22304DWG



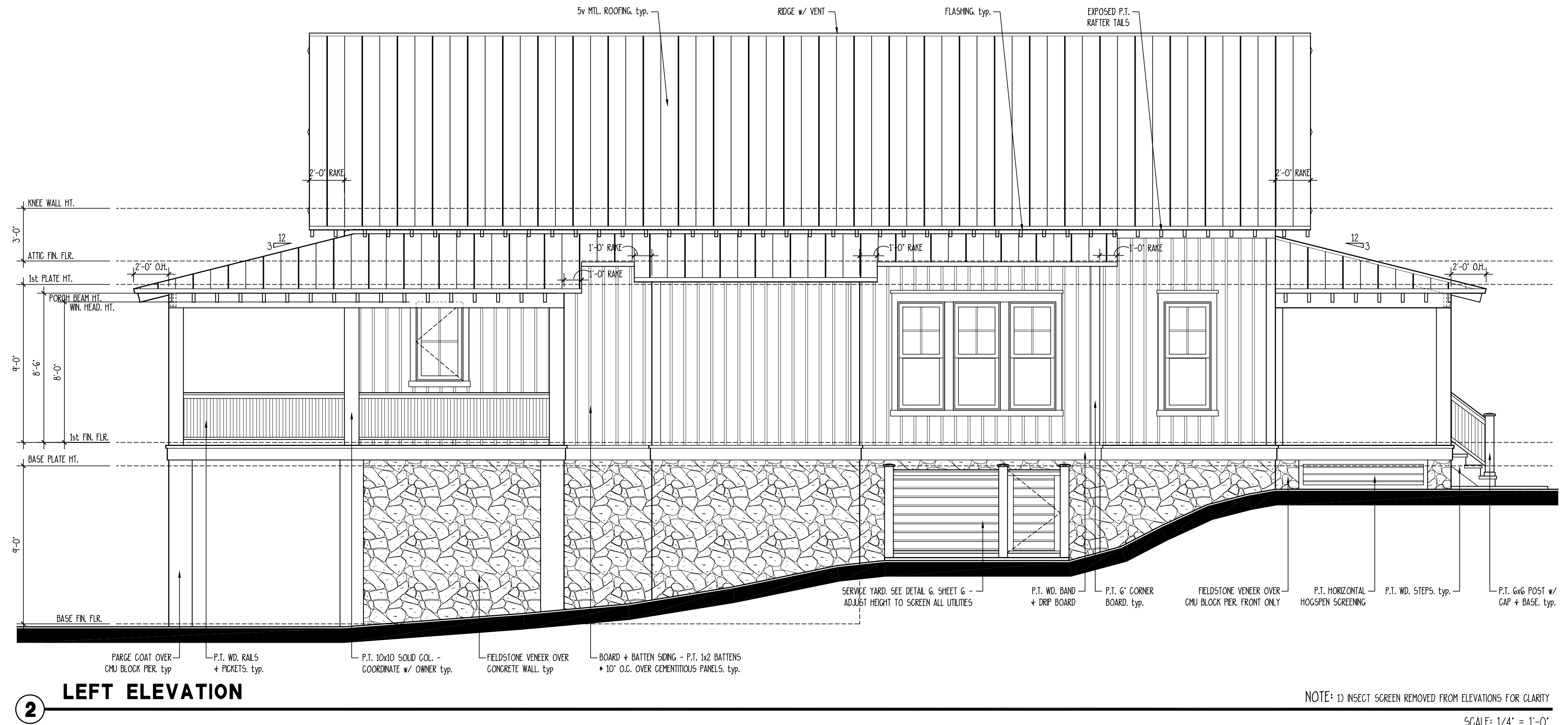
1 FRONT ELEVATION

SCALE: 1/4" = 1'-0"



3 FRONT ELEVATION w/ INSECT SCREENING

SCALE: 1/8" = 1'-0"



2 LEFT ELEVATION

SCALE: 1/4" = 1'-0"

NOTE: 1) INSECT SCREEN REMOVED FROM ELEVATIONS FOR CLARITY

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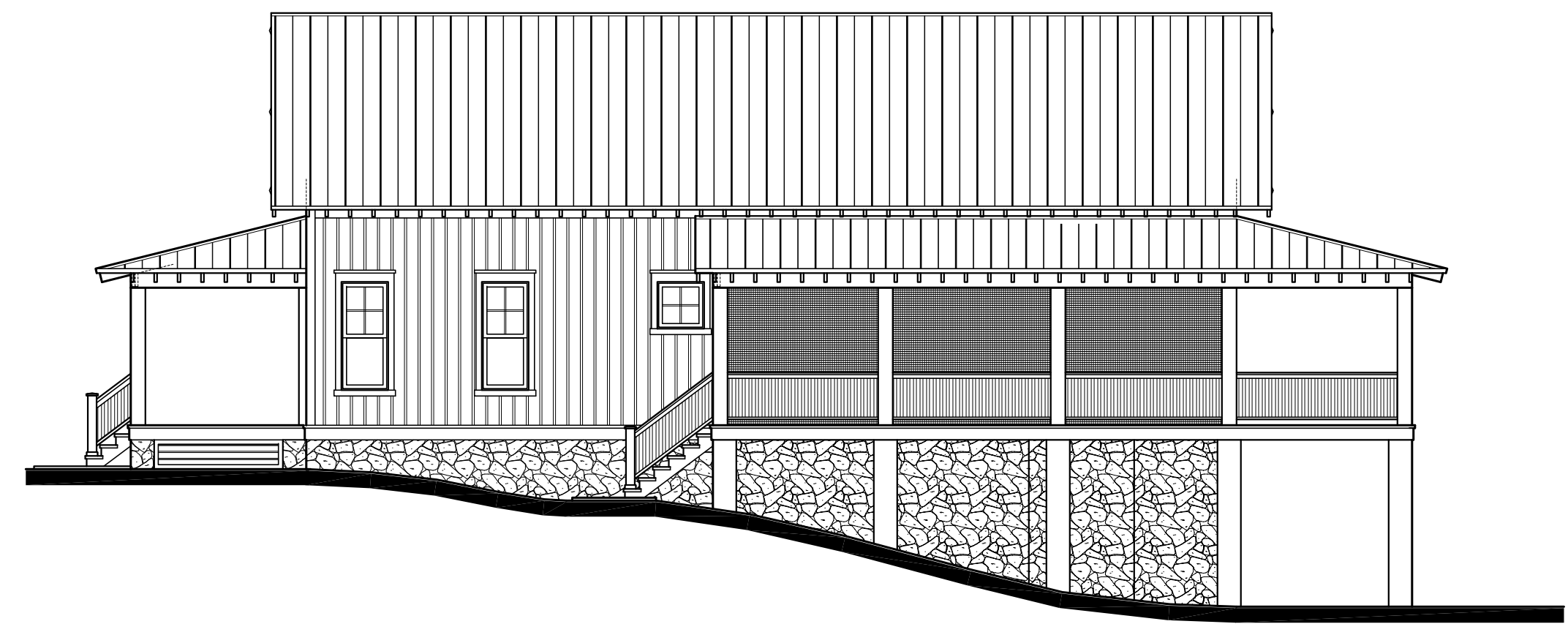
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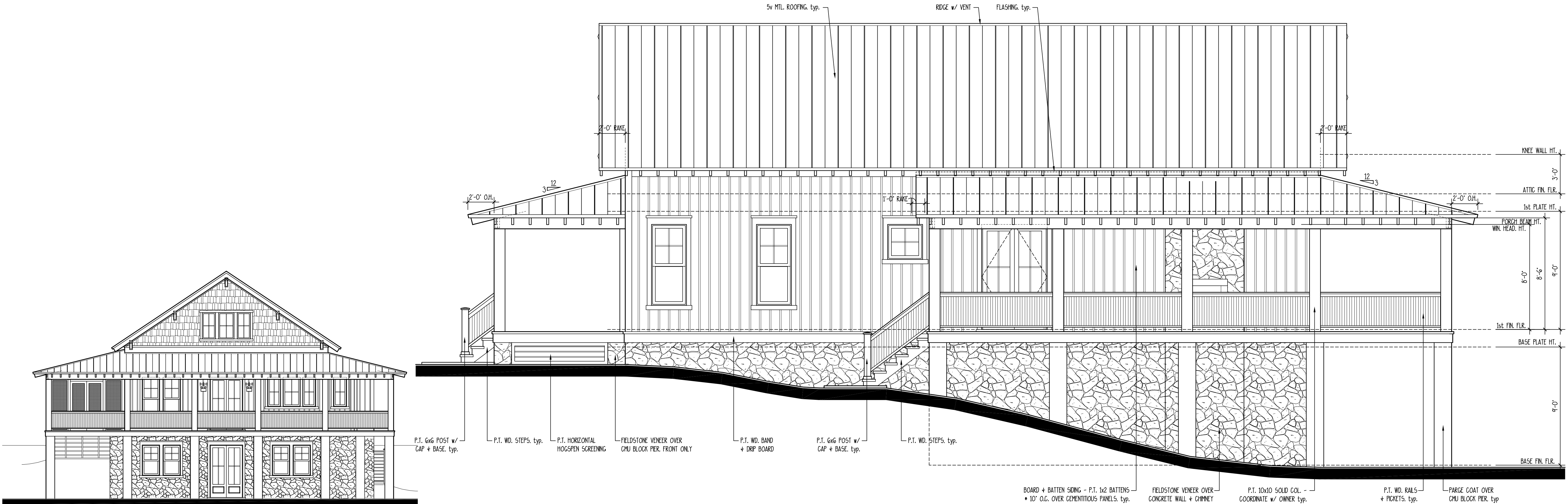
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1 REAR ELEVATION
 SCALE: 1/4" = 1'-0"



4 RIGHT ELEVATION w/ INSECT SCREENING
 SCALE: 1/8" = 1'-0"



3 REAR ELEVATION w/ INSECT SCREENING
2 RIGHT ELEVATION
 SCALE: 1/8" = 1'-0"

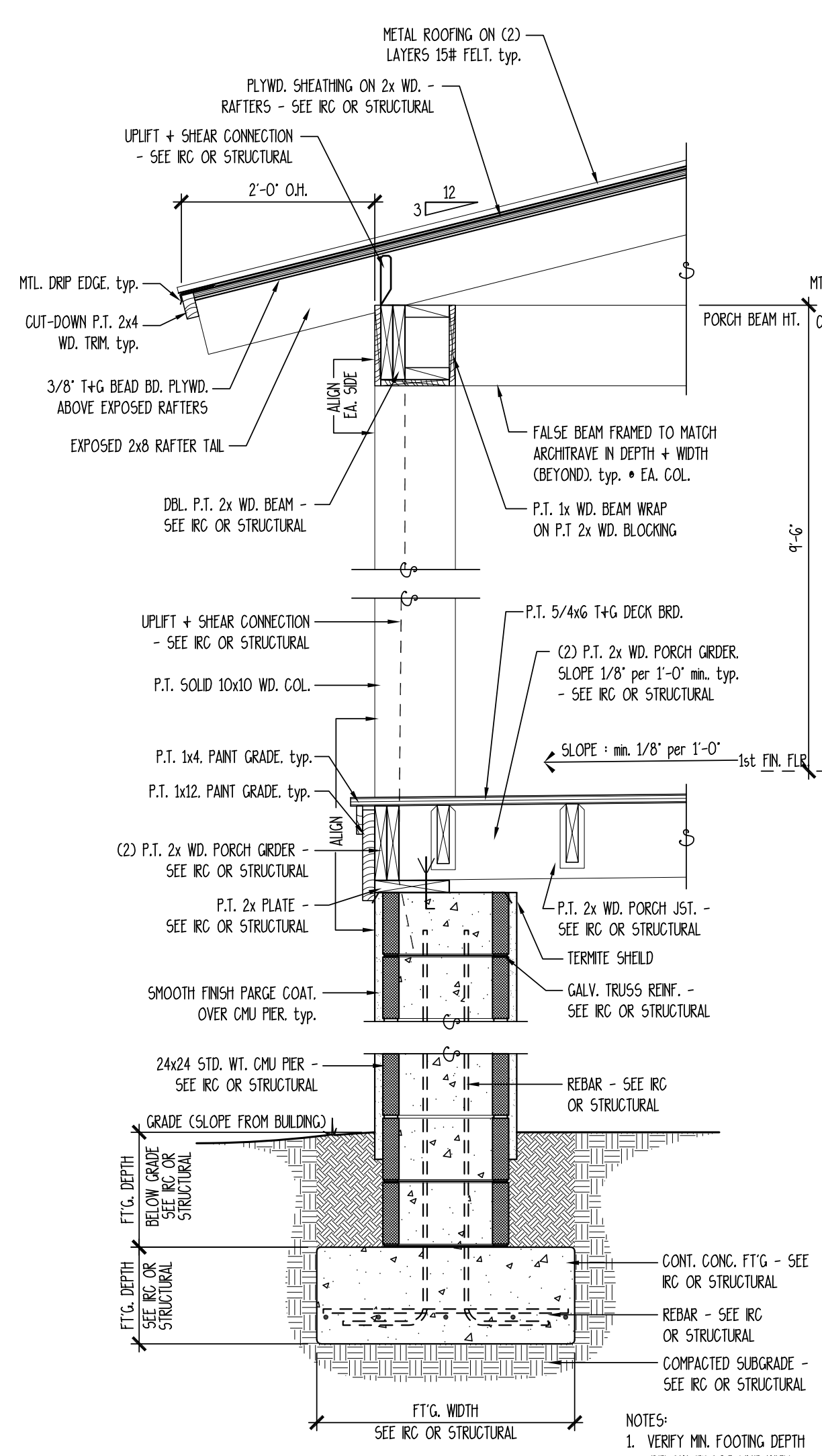
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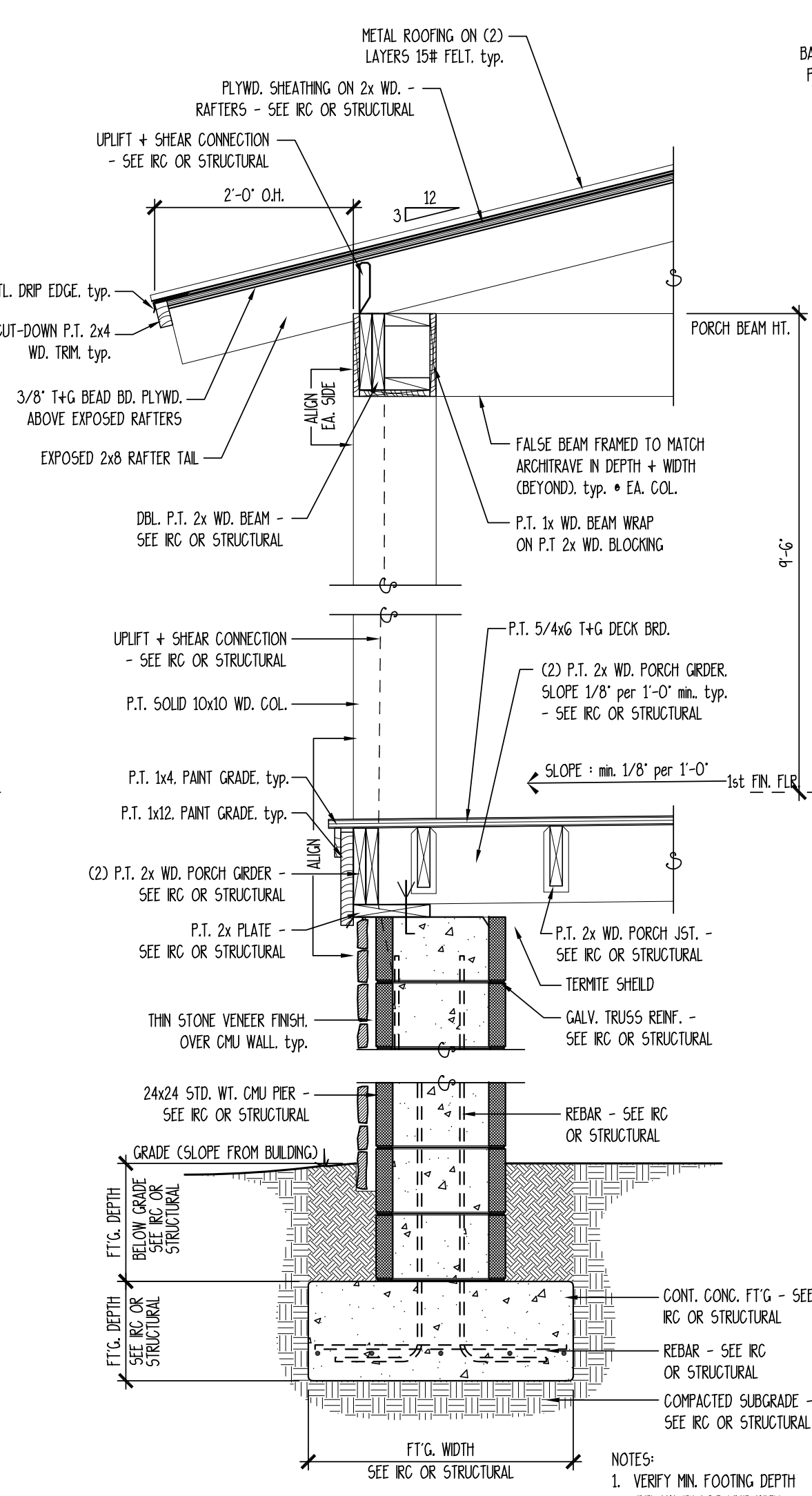
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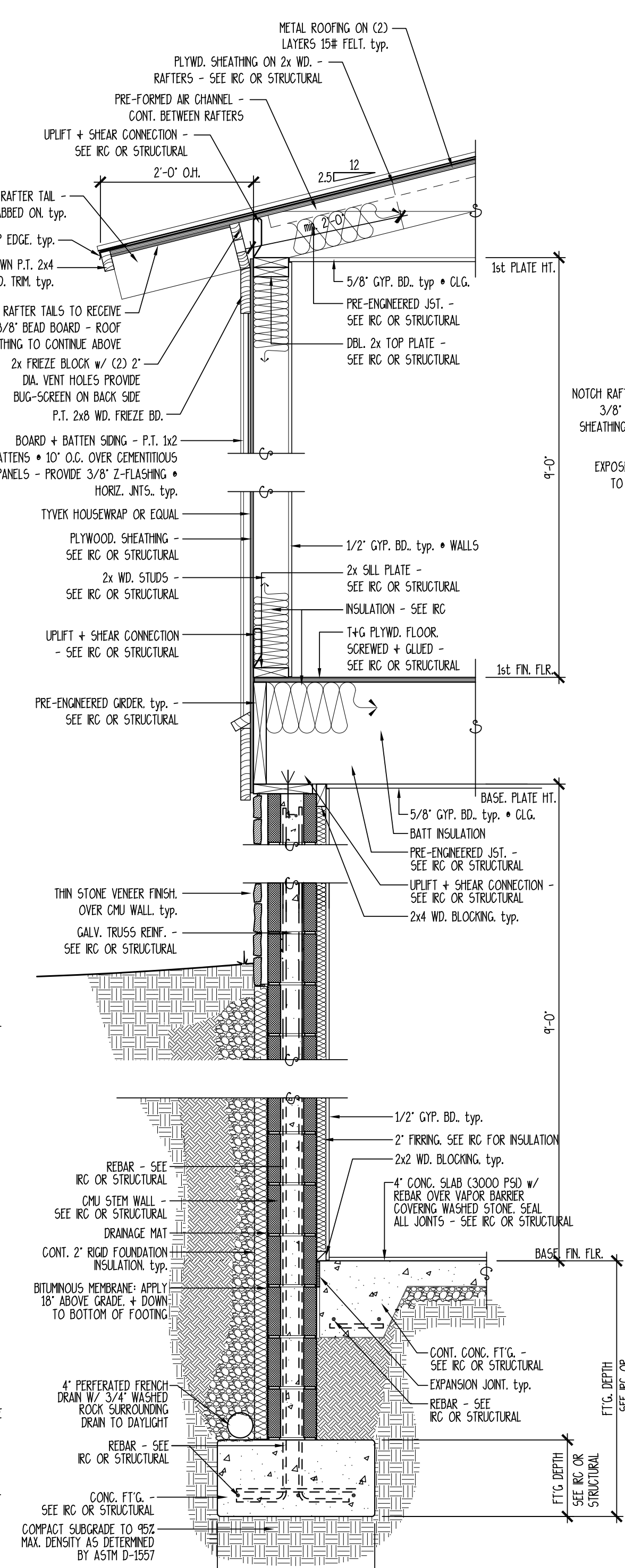
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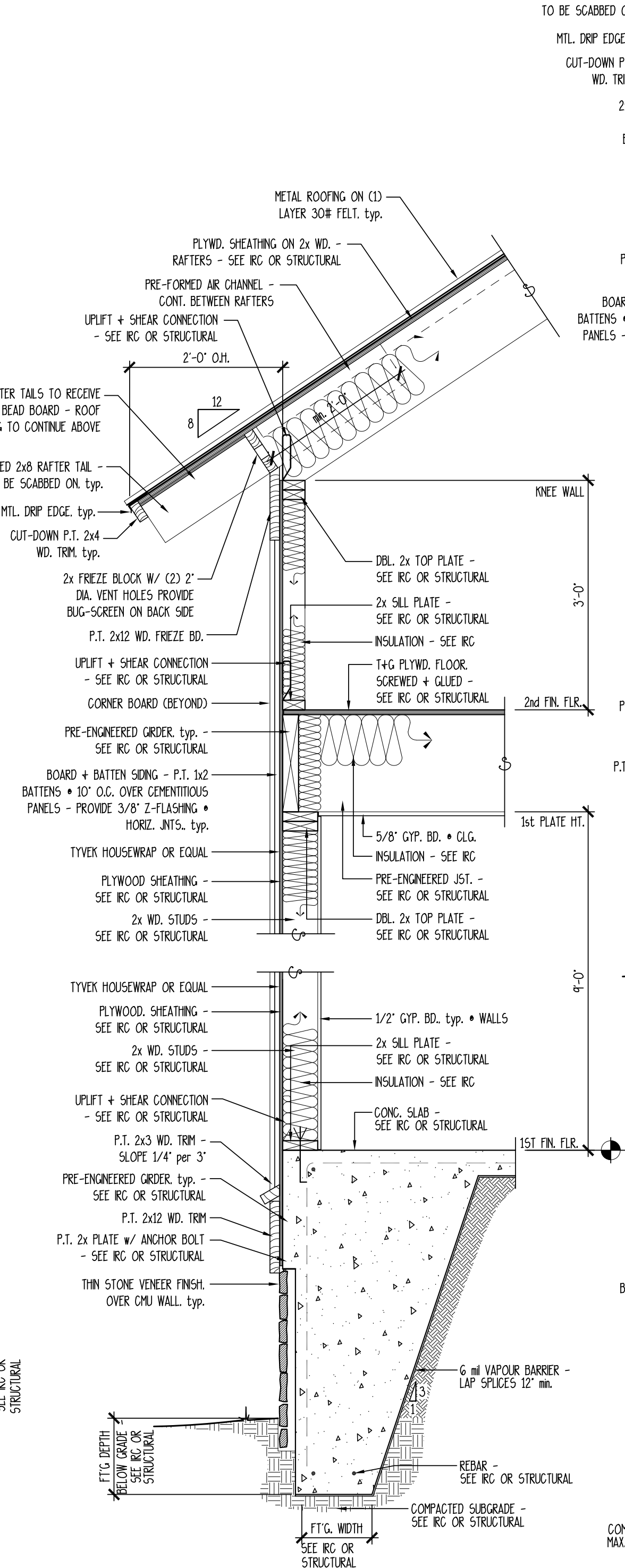
5 TYPICAL PORCH SECTION
SCALE: 3/4" = 1'-0"



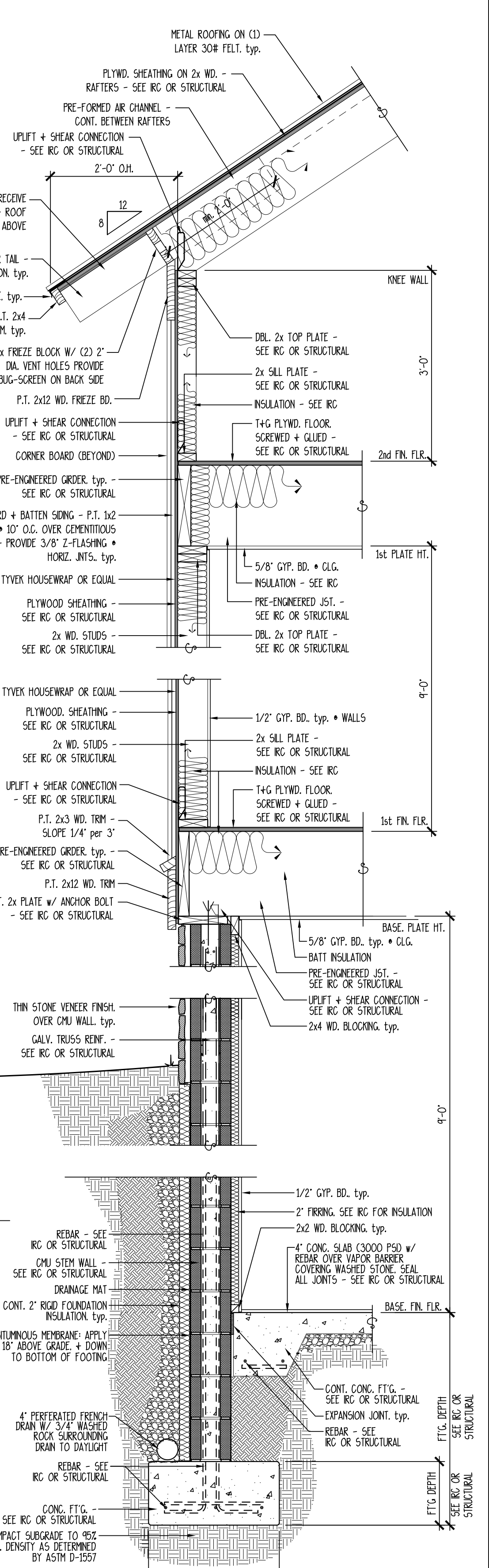
4 FRONT PORCH SECTION
SCALE: 3/4" = 1'-0"



3 ONE-STORY AT STEM WALL
SCALE: 3/4" = 1'-0"



2 TWO-STORY AT RAISED SLAB
SCALE: 3/4" = 1'-0"



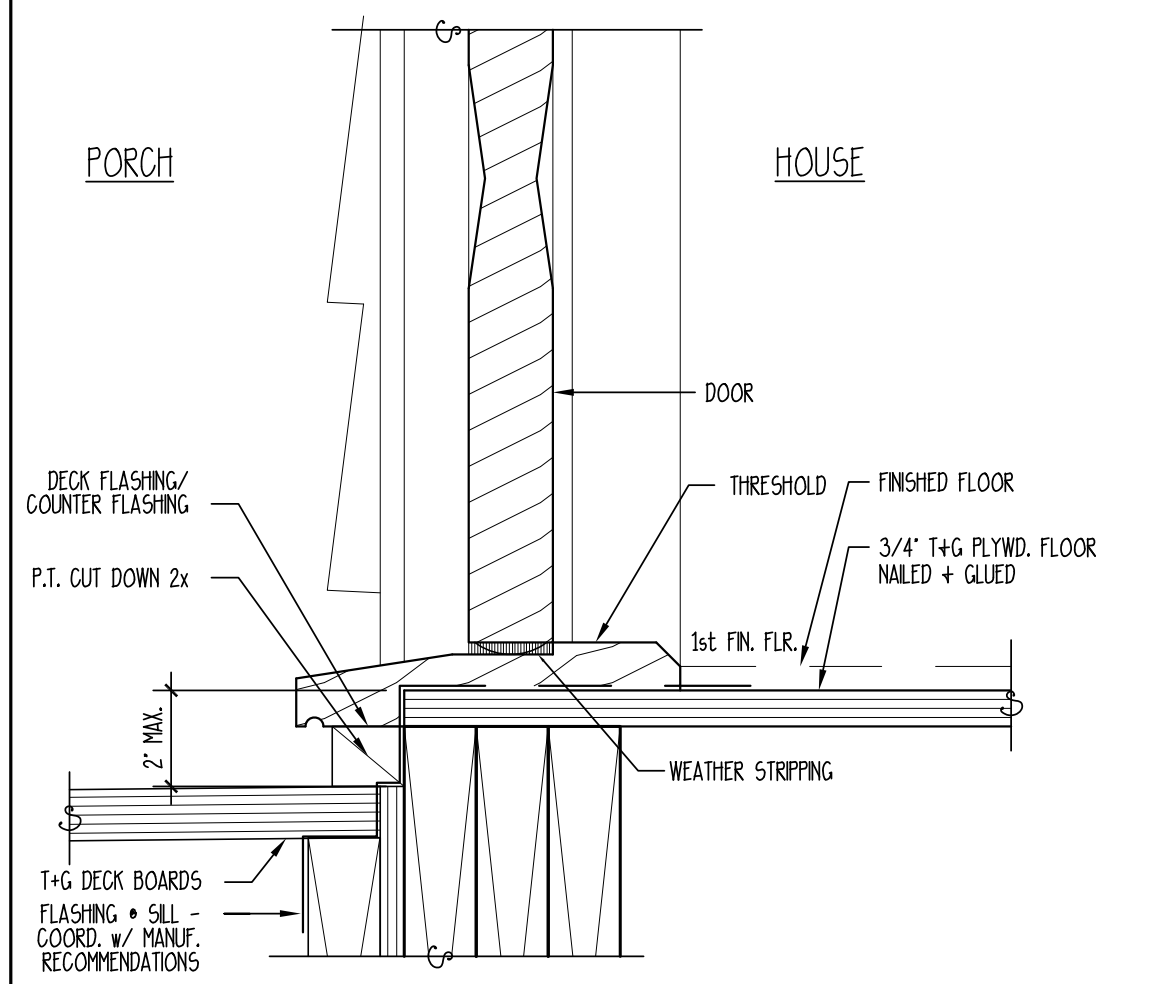
1 TWO-STORY AT STEM WALL
SCALE: 3/4" = 1'-0"

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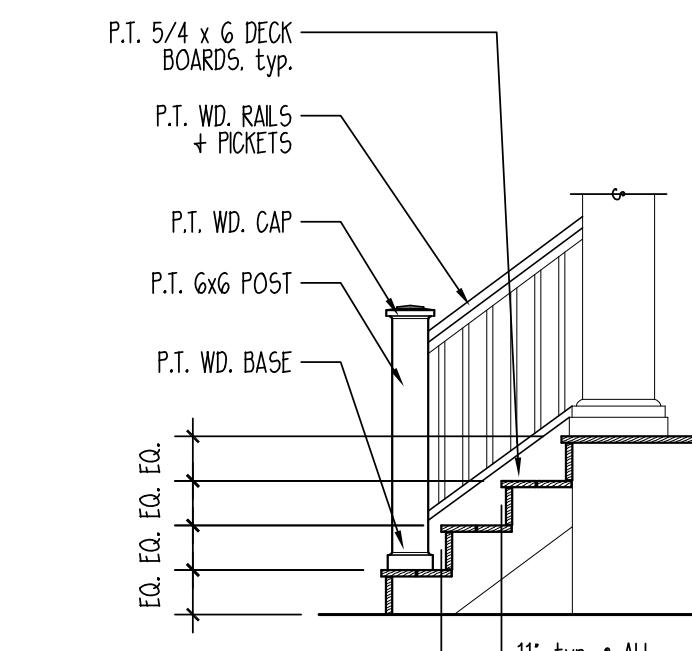
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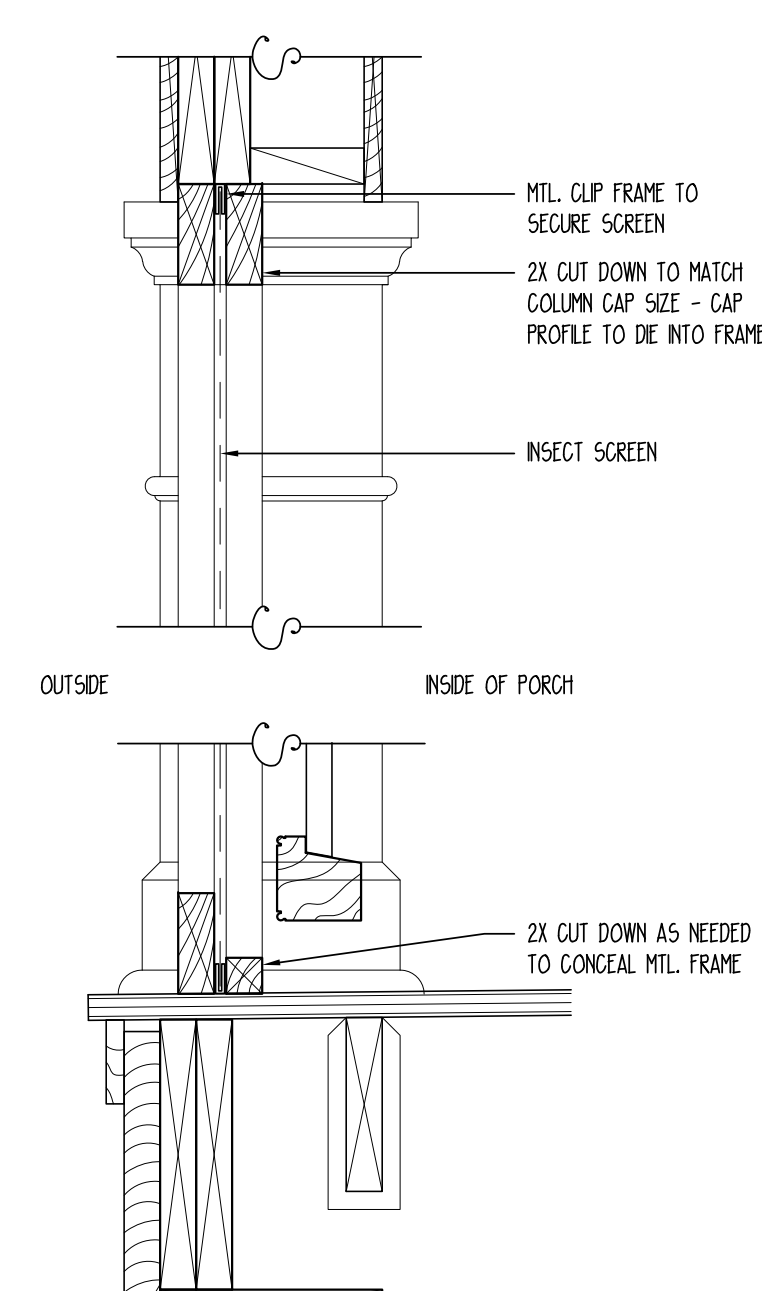
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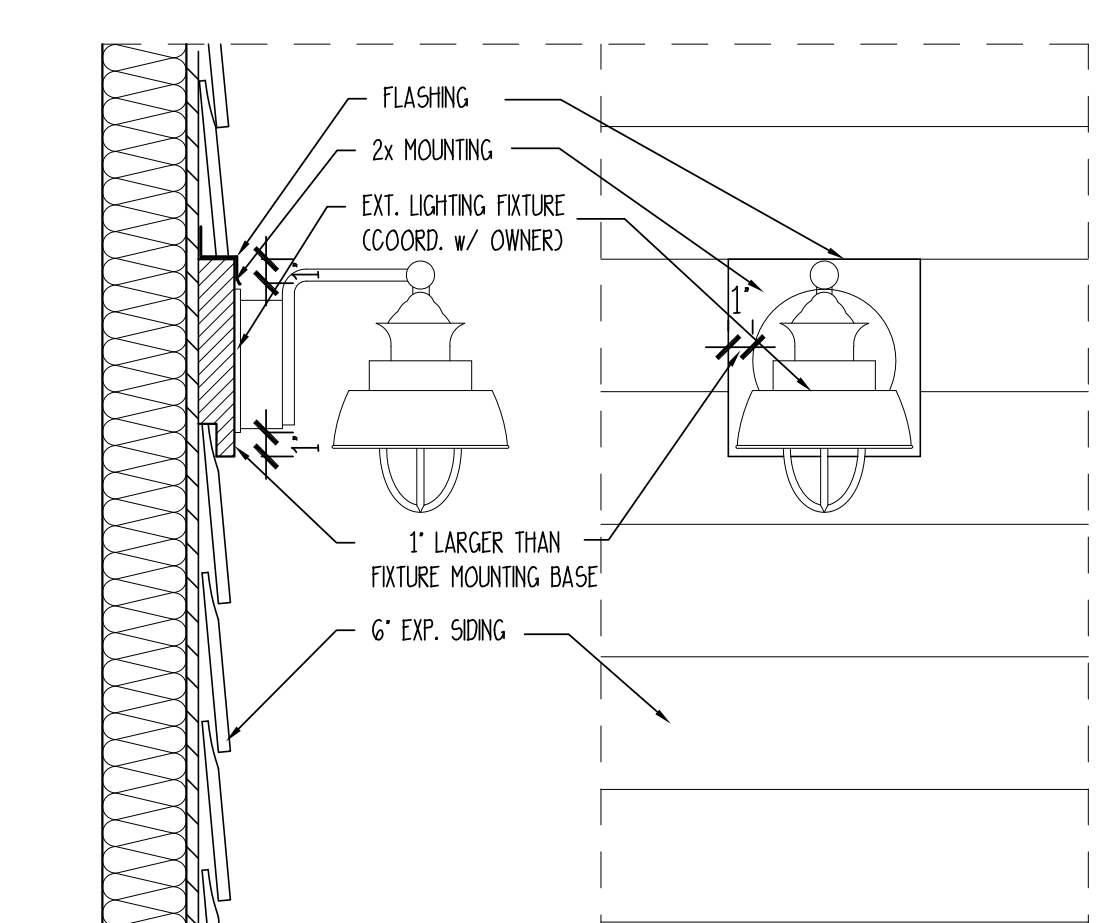
13 TYPICAL EXTERIOR DOOR SILL
SCALE: 3" = 1'-0"



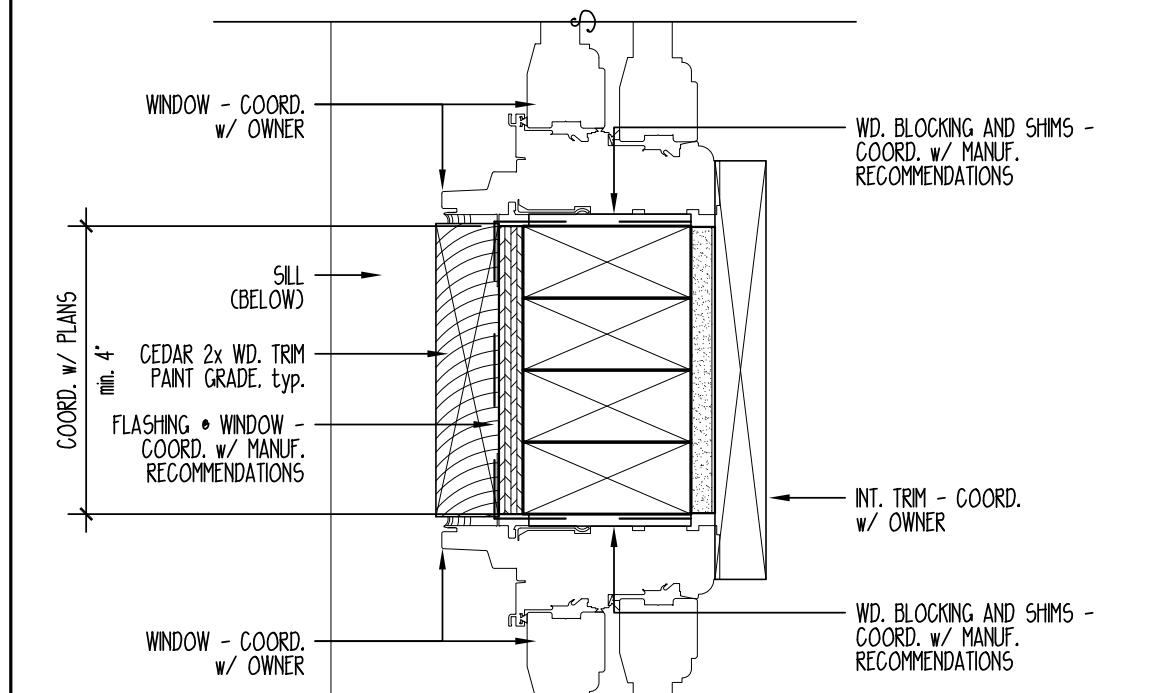
8 DETAIL • EXT. STAIRS
SCALE: 3/8" = 1'-0"



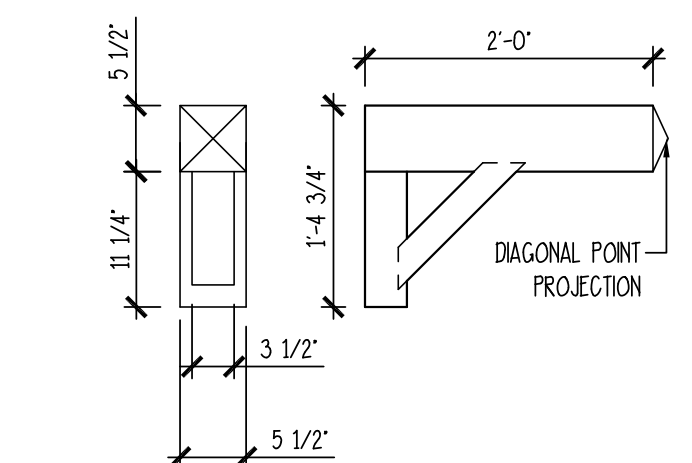
4 INSECT SCREENING DETAIL
SCALE: 1 1/2" = 1'-0"



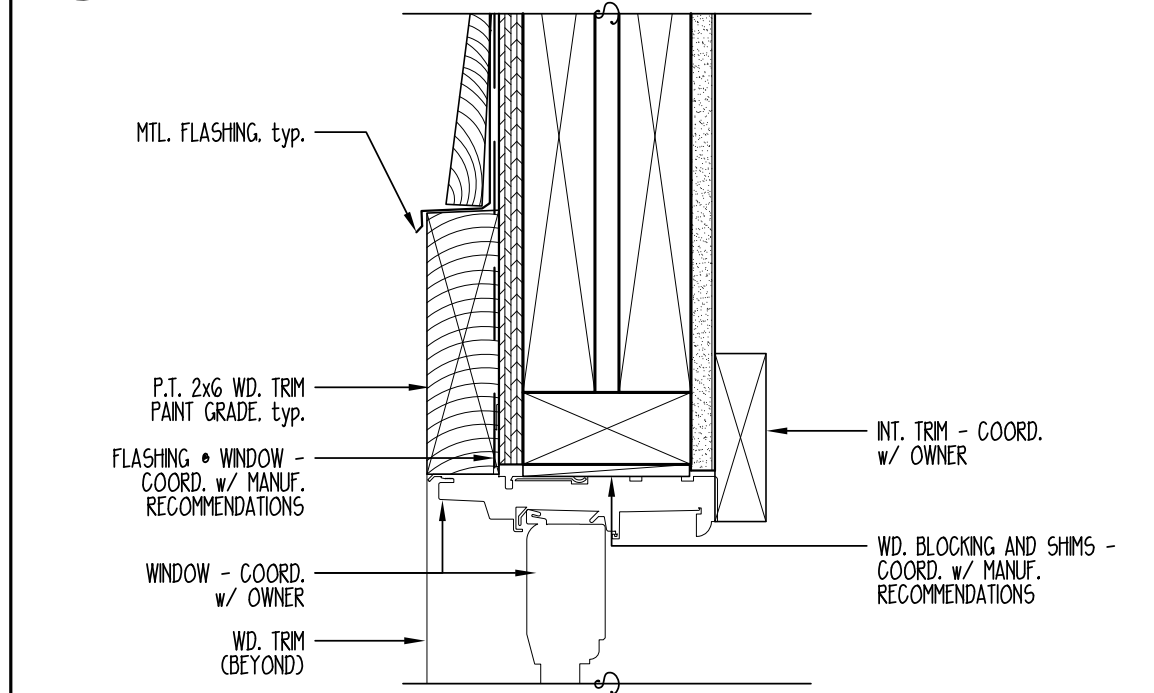
1 EXTERIOR LIGHTING MOUNT DETAIL
SCALE: 1 1/2" = 1'-0"



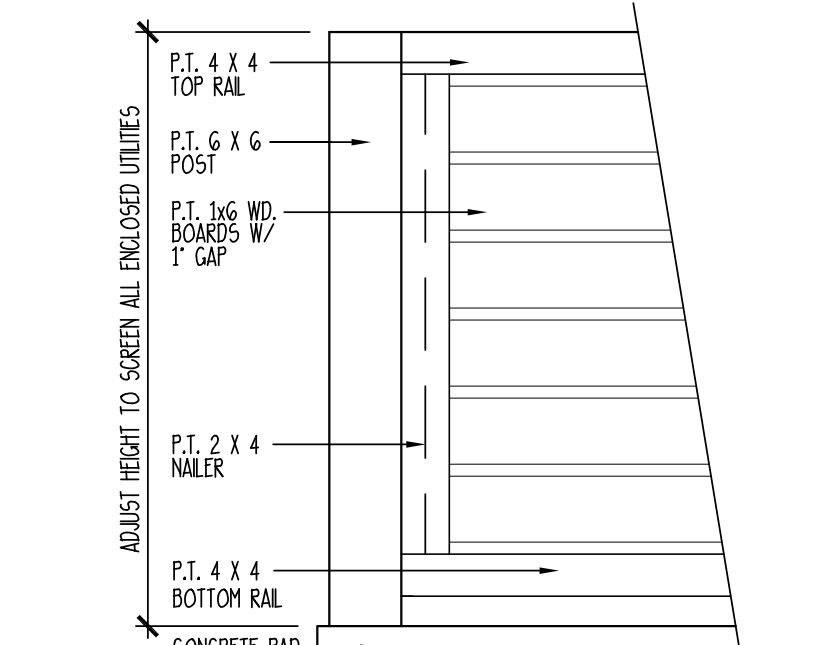
12 TYPICAL WINDOW JAMB • MULL
SCALE: 3" = 1'-0"



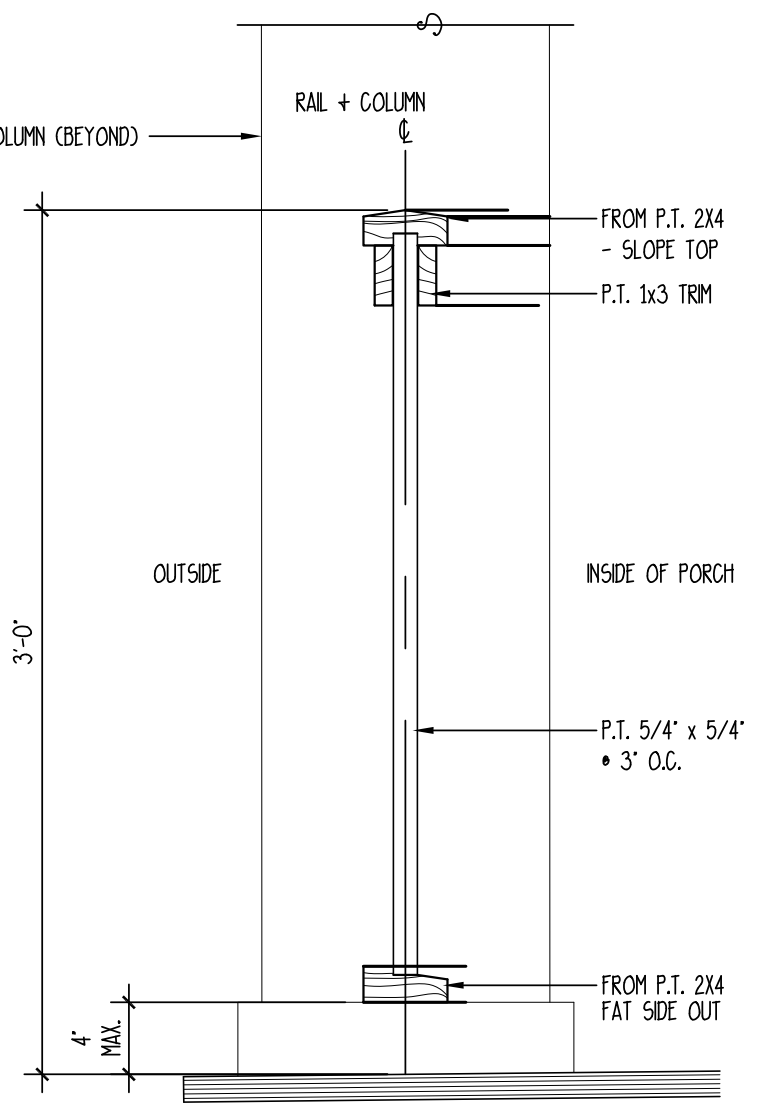
7 BRACKET DETAIL
SCALE: 3/4" = 1'-0"



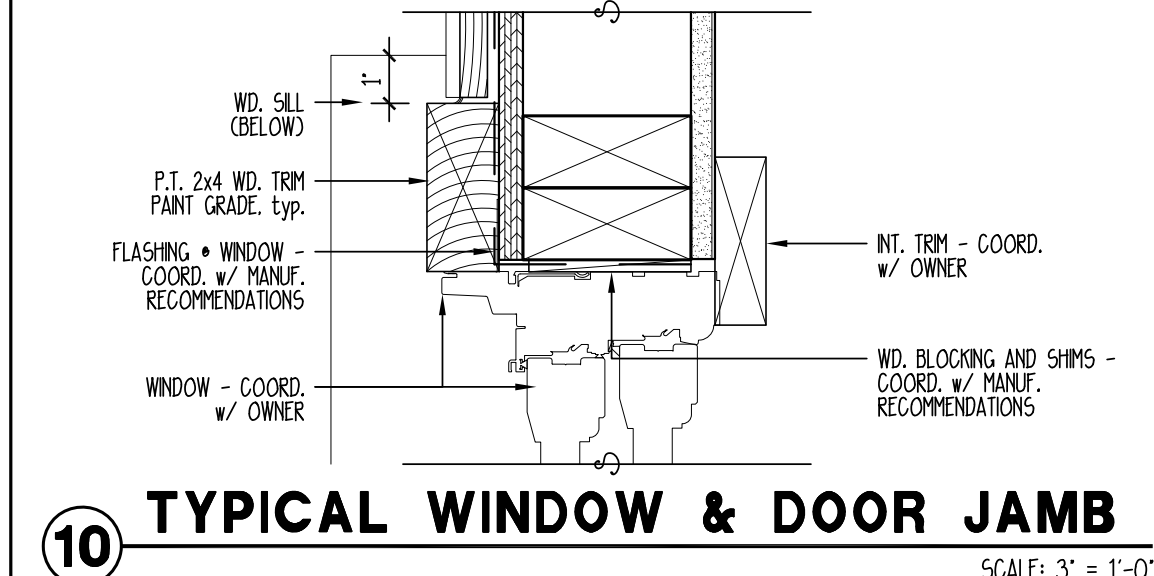
11 TYPICAL WINDOW & DOOR HEAD
SCALE: 3" = 1'-0"



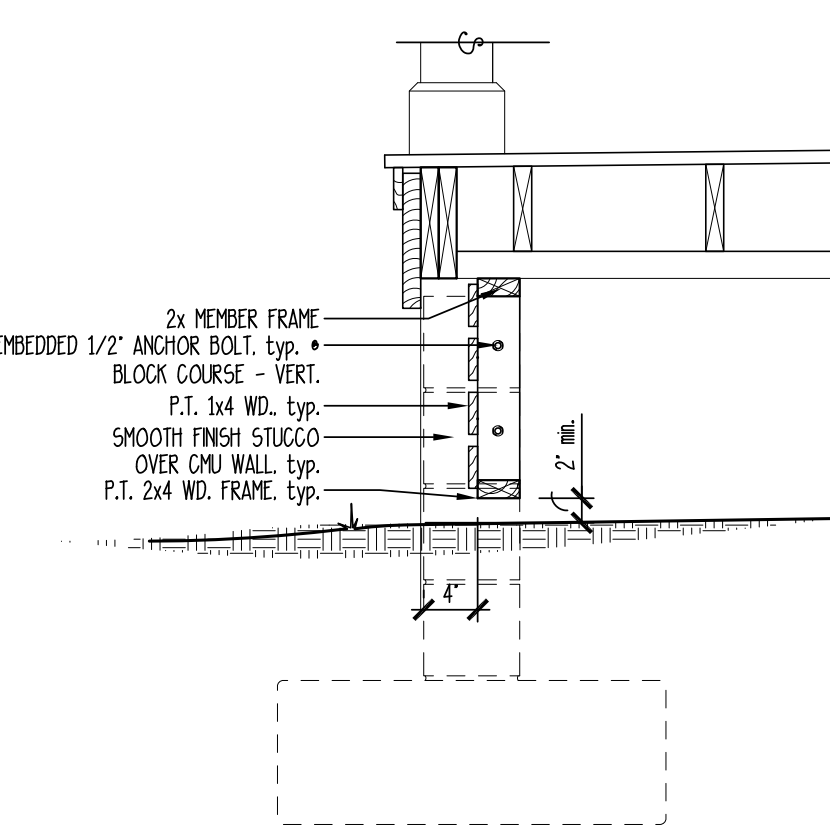
6 SERVICE COURT DETAIL
SCALE: 3/4" = 1'-0"



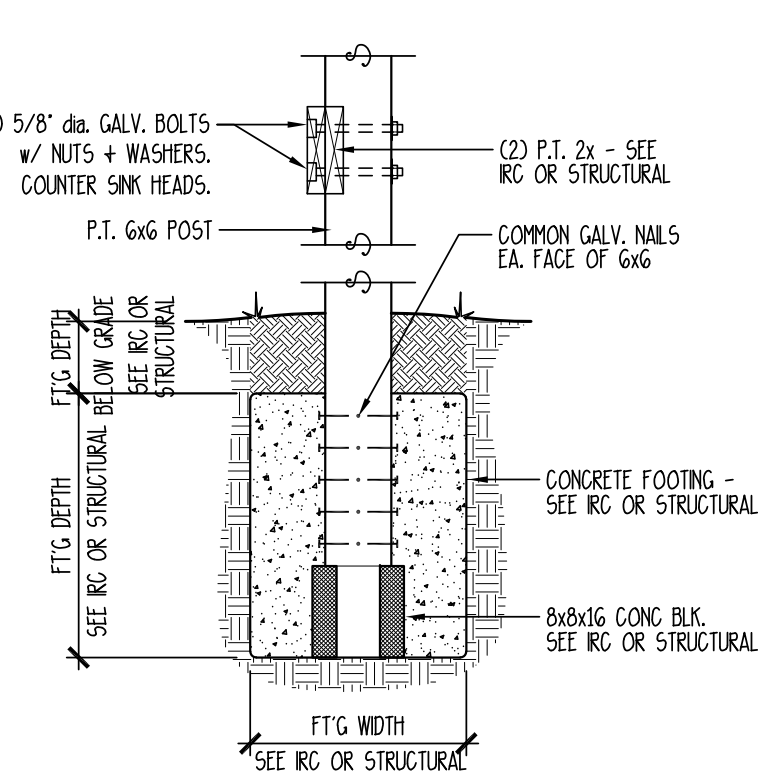
3 PICKET & RAIL DETAIL
SCALE: 1 1/2" = 1'-0"



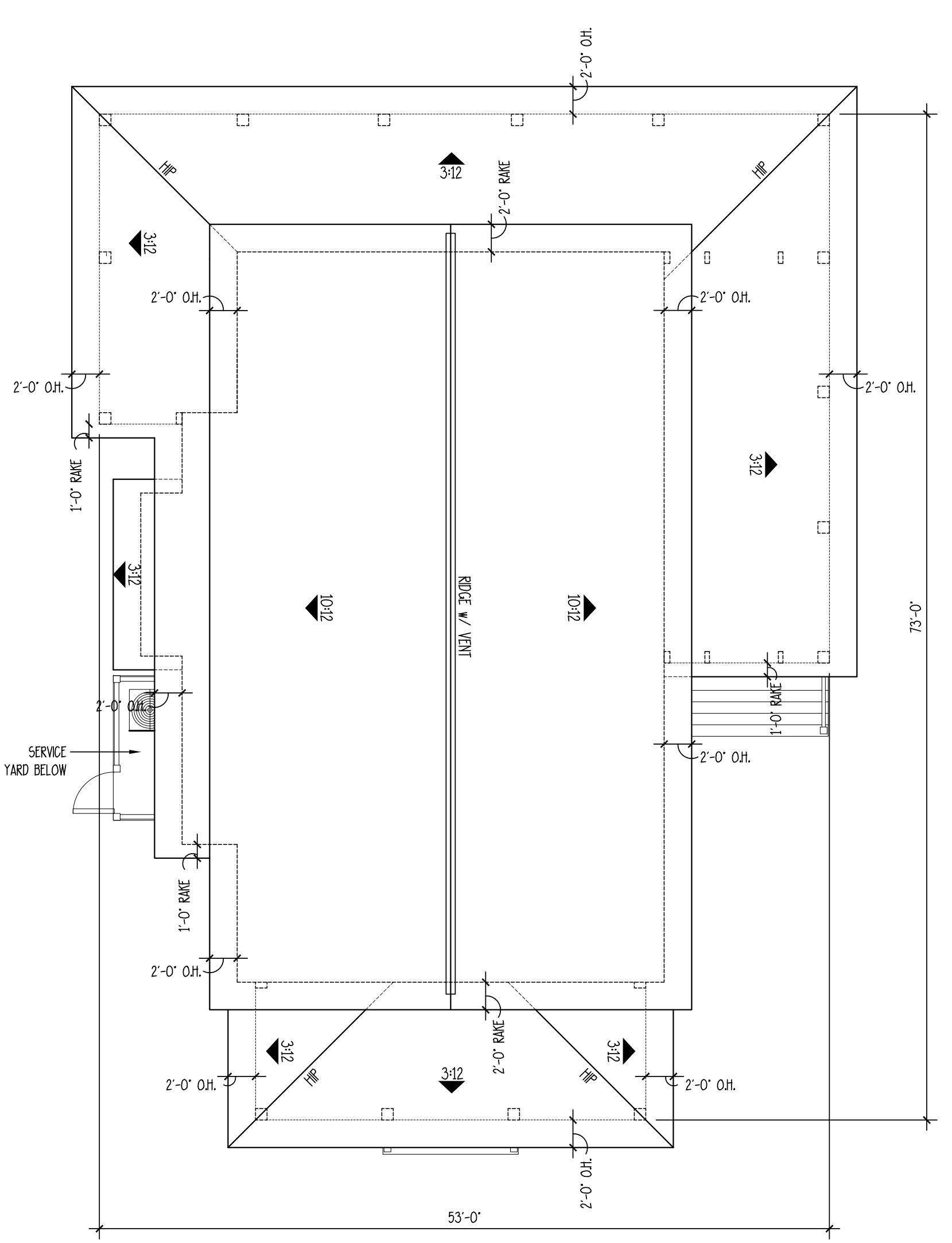
10 TYPICAL WINDOW & DOOR JAMB
SCALE: 3" = 1'-0"



5 WD. HORIZ. HOGSPEN DETAIL
SCALE: 3/4" = 1'-0"



2 6x6 POST DETAIL
SCALE: 3/4" = 1'-0"



ROOF PLAN
SCALE: 1/8" = 1'-0"

- NOTE:**
1. ALL PENETRATIONS TO BE LOCATED AS INCONSPICUOUSLY AS POSSIBLE. • REAR OR SIDES OF HOUSE AS POSSIBLE.
 2. ROOF PENETRATIONS NEED TO BE KEPT TO A MINIMUM (COMBINED WHEN POSSIBLE).
 3. ALL ROOF / WALL PENETRATIONS TO BE PAINTED TO MATCH ROOF COLOR.
 4. TWO (2) LAYERS UNDERLAYMENT REQUIRED WHEN 4:12 ROOF PITCH OR LOWER.
 5. METAL ROOF SEAMS NOT TO EXCEED 16" O.C.
 6. METAL ROOF SEAMS TO BE 1 1/2" TO 1 3/4" IN HEIGHT

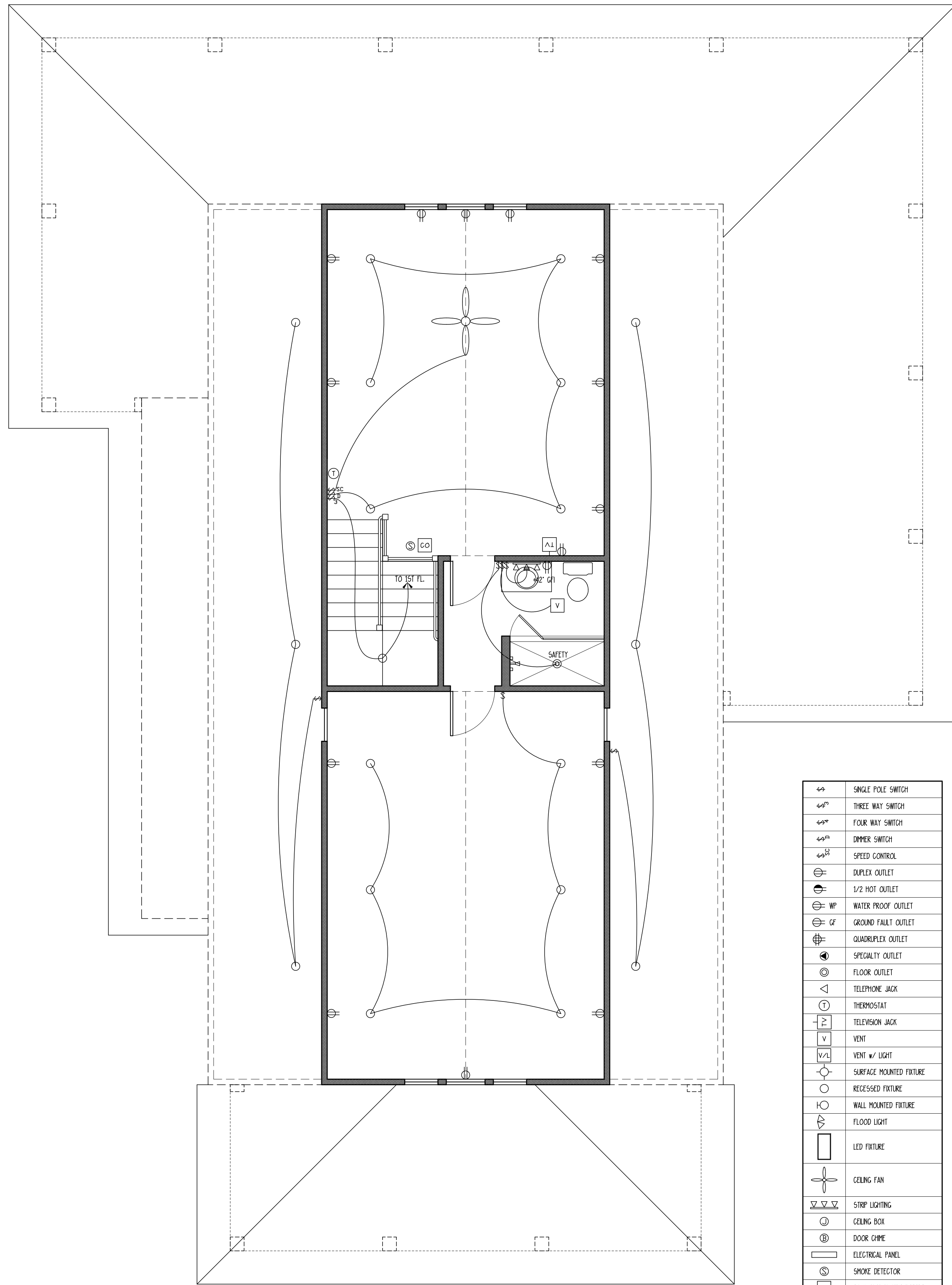
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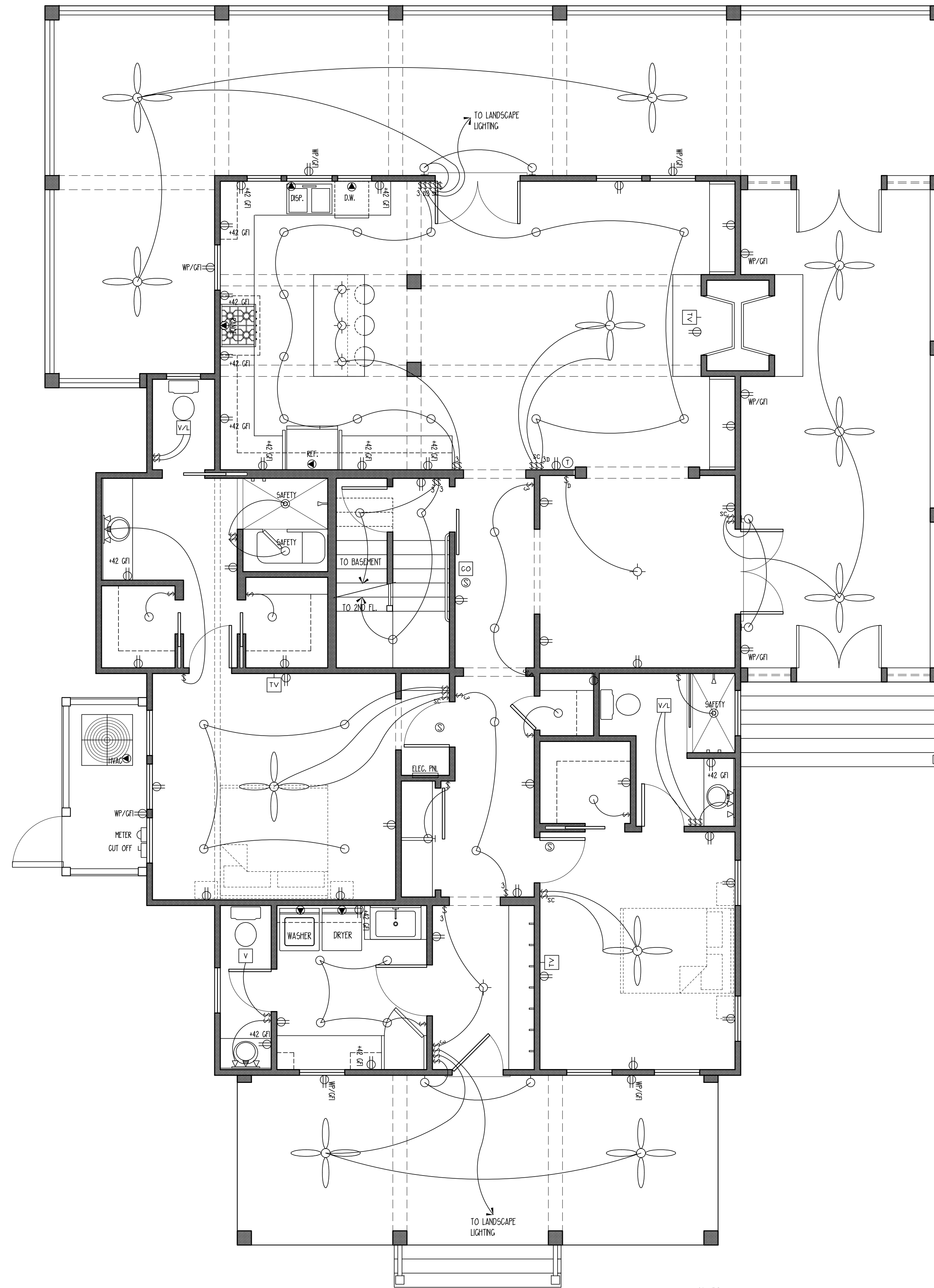
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SECOND FLOOR ELECTRICAL LAYOUT

SCALE: 1/4" = 1'-0"



FIRST FLOOR ELECTRICAL LAYOUT

SCALE: 1/4" = 1'-0"

	SINGLE POLE SWITCH
	THREE WAY SWITCH
	FOUR WAY SWITCH
	DIMMER SWITCH
	SPEED CONTROL
	DUPLEX OUTLET
	1/2 HOT OUTLET
	WATER PROOF OUTLET
	GROUND FAULT OUTLET
	QUADPLEX OUTLET
	SPECIALTY OUTLET
	FLOOR OUTLET
	TELEPHONE JACK
	THERMOSTAT
	TELEVISION JACK
	VENT
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	RECESSED FIXTURE
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	FLOOD LIGHT
	LED FIXTURE
	CEILING FAN
	STRIP LIGHTING
	CEILING BOX
	DOOR CHIME
	ELECTRICAL PANEL
	SMOKE DETECTOR
	CARBON MONOXIDE DETECTOR

COORD. ELEVATION OF UTILITIES + ELECTRICAL
OUTLETS w/ LOCAL CODES + FLOOD ELEVATION

*NOTE:

- COORDINATE LANDSCAPE LIGHTING REQUIREMENTS AND LOCATION w/ OWNER.
- COORDINATE TELECOMMUNICATIONS SYSTEM REQUIREMENTS w/ OWNER.
- COORDINATE SOUND SYSTEM REQUIREMENTS w/ OWNER.
- COORDINATE CENTRAL VACUUM REQUIREMENTS + LOCATION w/ OWNER.

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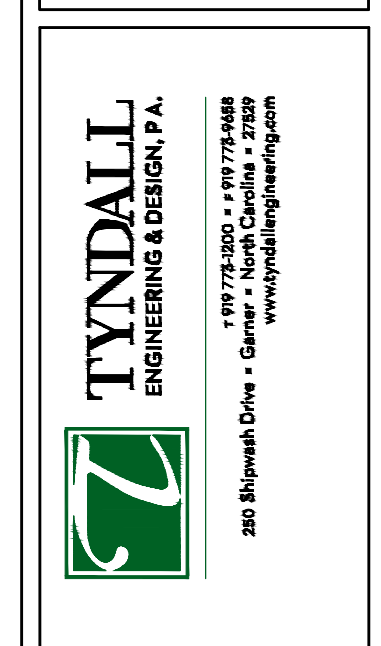
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 Any deviation or discrepancy on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.
 *Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.



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Client: **DAVID KRAKOWSKI**
 Project: **KRAKOWSKI RESIDENCE**

FOUNDATION PLAN
1ST FLOOR FRAMING

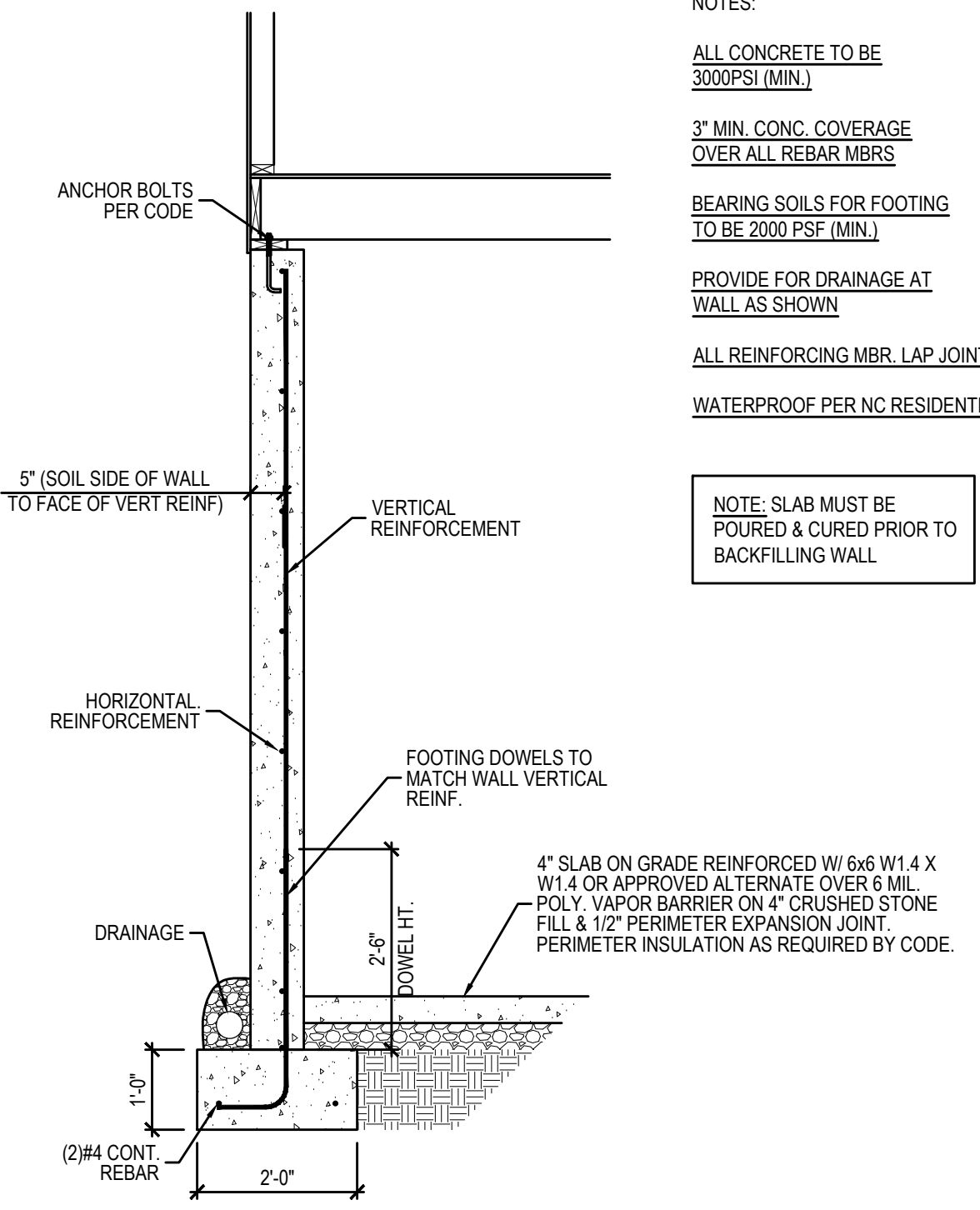
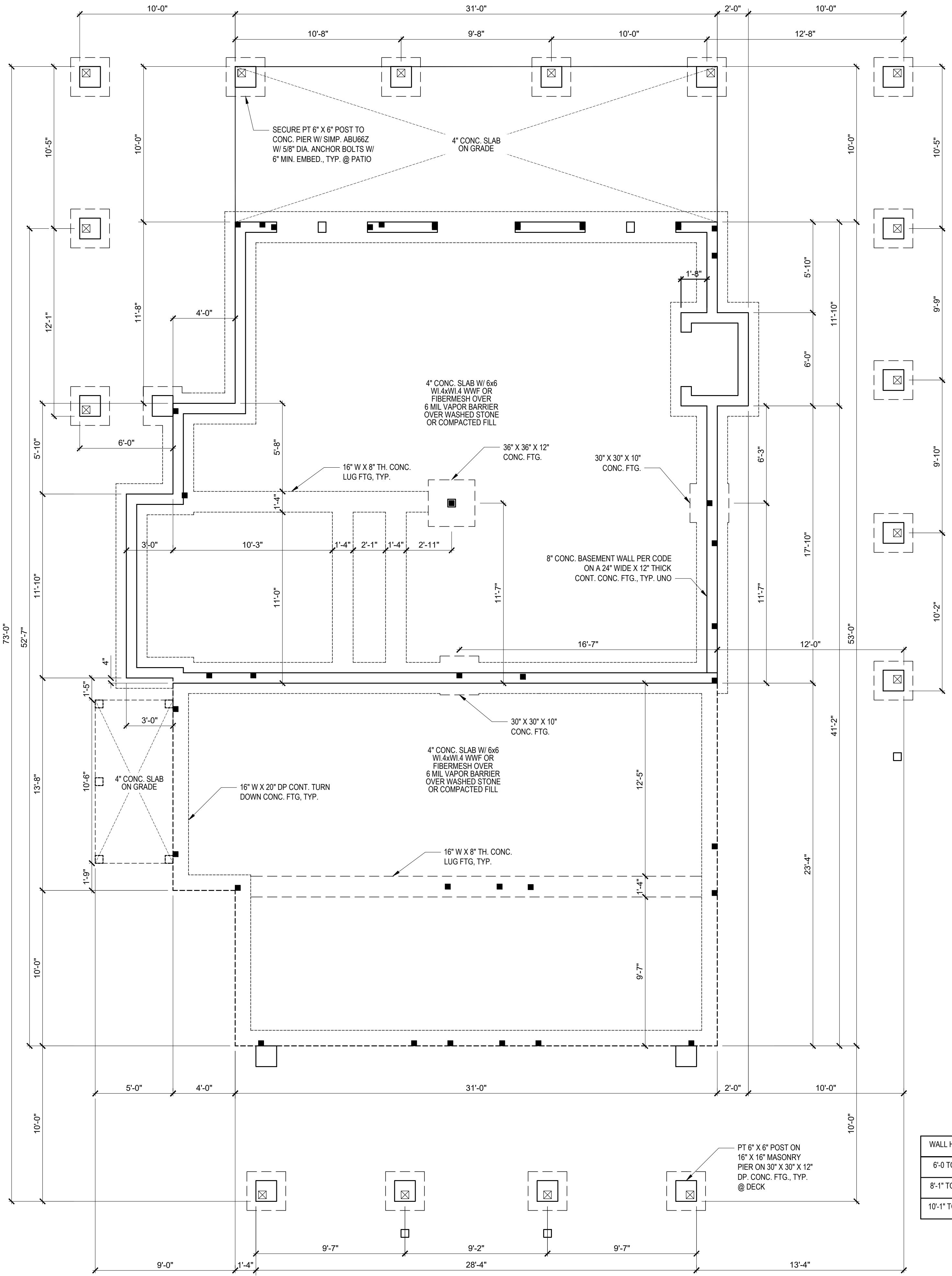
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 Date: 9/20/2022
 Engineer: HJS
 DWG. Checked By: AWL
 Scale: SEE PLAN

REVISIONS		
No.	Date	Remarks

Sheet Number
S1
 1 of 8

DESIGN LOADS	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	Tl
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (for storage)	20	10	L/240	L/180
ATTIC (for access)	15	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

- STRUCTURAL NOTES:**
- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 - IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, P.A. IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
 - ALL LUMBER SHALL BE SYP #2 (UNG).
 - ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND F_b = 2600 PSI, E = 1.9M PSI (I.E. LEVEL MICROLAM).
 - ALL LVL LUMBER IS TO BE 1.55E (F_b = 2325 PSI).
 - ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10# NAILS @ 6" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6" MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
 - ALL INTERIOR LOAD BEARING HEADERS ARE TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO).
 - REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
 - ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 F_y = 50 KSI MIN. (UNO).
 - ALL EXTERIOR LUMBER TO BE #2 SYP PT.
 - ALL CONCRETE f' = 3000 PSI MIN.
 - PRESUMPTIVE BEARING CAPACITY = 2000 PSF.
 - 1/2" Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
 - PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO).
 - PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS (U.N.O.).
 - PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC. MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
 - UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
 - METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.



TYPICAL BASEMENT WALL			BASEMENT/GARAGE COMMON WALL		
WALL HEIGHT	VERT REINF	HORIZ REINF	WALL HEIGHT	VERT REINF	HORIZ REINF
6'-0" TO 8'-0"	#4 @ 16" o.c.	#4 @ 24" o.c.	6'-0" TO 8'-0"	#5 @ 12" o.c.	#4 @ 24" o.c.
8'-1" TO 10'-0"	#5 @ 12" o.c.	#4 @ 24" o.c.	8'-1" TO 10'-0"	#5 @ 8" o.c.	#4 @ 24" o.c.
10'-1" TO 12'-0"	#5 @ 8" o.c.	#4 @ 18" o.c.	10'-1" TO 12'-0"	#5 @ 6" o.c.	#4 @ 18" o.c.

A 8" CONC. BASEMENT WALL DETAIL
 SCALE: 1/2" = 1'-0"

FOUNDATION PLAN
 1/4" = 1'-0"

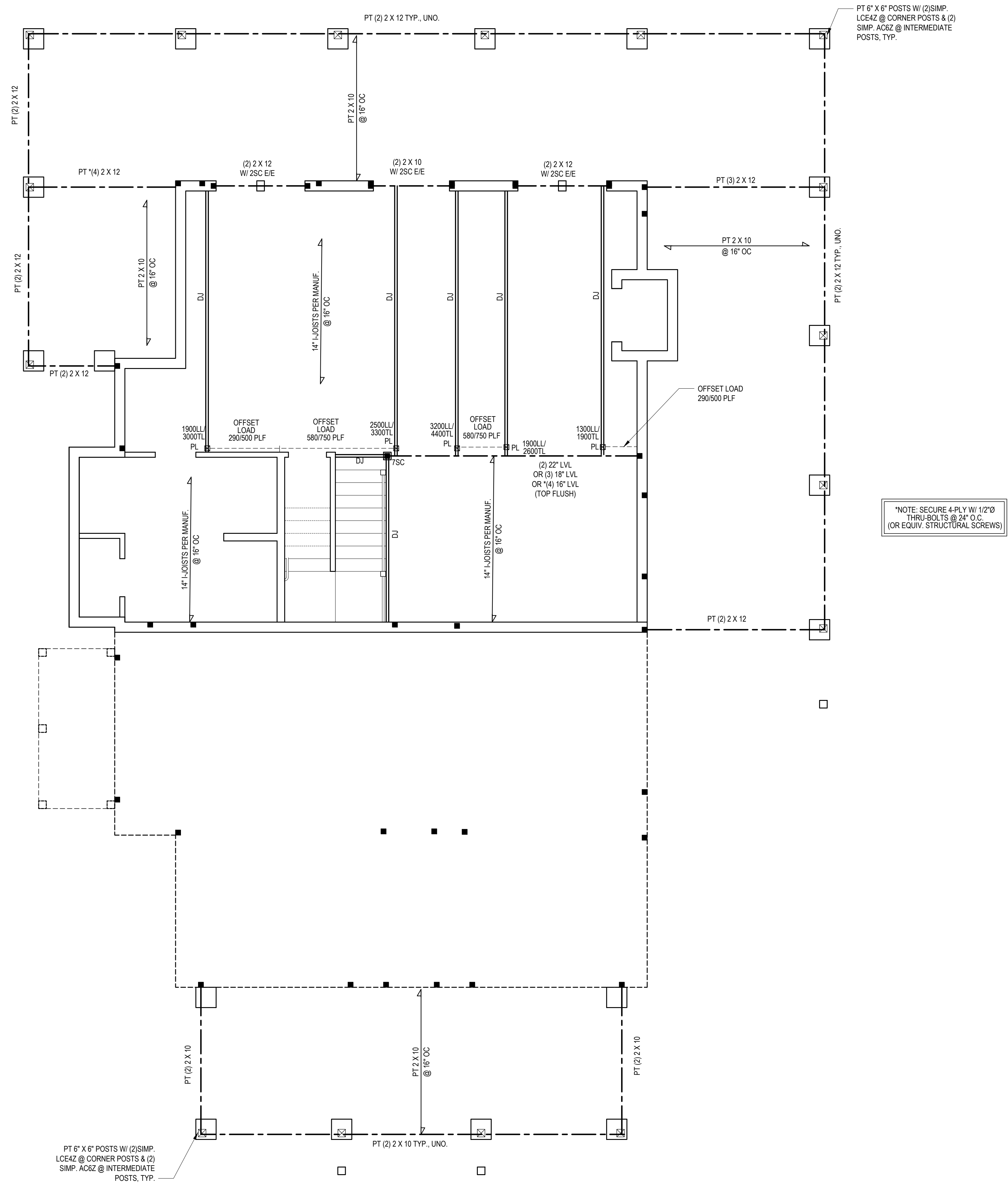
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 PROJECTS\2201-010306 - KRAKOWSKI RESIDENCE - 110306.LDW
 DATE: 9/20/2022 10:30:06 AM
 USER: HJS

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

STRUCTURAL NOTES:

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- ALL LUMBER SHALL BE SYP #2 (UNO)
ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND F_b = 2600 PSI, E = 1.8M PSI
(I.E. LEVEL MICROLAM)
ALL LSL LUMBER IS TO BE 1.5SE (F_b = 2325 PSI)
- ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (UNO) AND KING STUDS PER TABLE R602.7.5 AND TOGETHER w/ (2) 10x16 HALS @ 8' O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6", OTHERWISE REFER TO TABLE R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (UNO) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
F_y = 50 KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE f'_c = 3000 PSI MIN.
PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 10/10 ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THESE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 8'-0" (UNO)
PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PUNCH COLUMNS. (U & L)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC.
- MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.



*NOTE: SECURE 4-PLY W/ 1/2"Ø THRU-BOLTS @ 24" O.C. (OR EQUIV. STRUCTURAL SCREWS)

BASEMENT PLAN
1/4" = 1'-0"

*Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precautions. Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.
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Client: **DAVID KRAKOWSKI**
Project: **KRAKOWSKI RESIDENCE**

**BASEMENT HEADER
1ST FLOOR FRAMING**

Project #: 2201-010306
Date: 9/20/2022
Engineered by: HJS
DWG. Checked By: AWL
Scale: SEE PLAN

No.	Date	Remarks

Sheet Number
S2
2 of 8

FILENAME: \\A:\ESD\DRN\ENR\2022\STRUCTURAL PROJECTS\2201-010306 - DAVID KRAKOWSKI - KRAKOWSKI RESIDENCE\DWG FILES\2201-010306 - DAVID KRAKOWSKI - KRAKOWSKI RESIDENCE.dwg SHAD BY: SWAMESH LAST PLOT DATE: 9/21/2022 3:22 PM

DESIGN LOADS

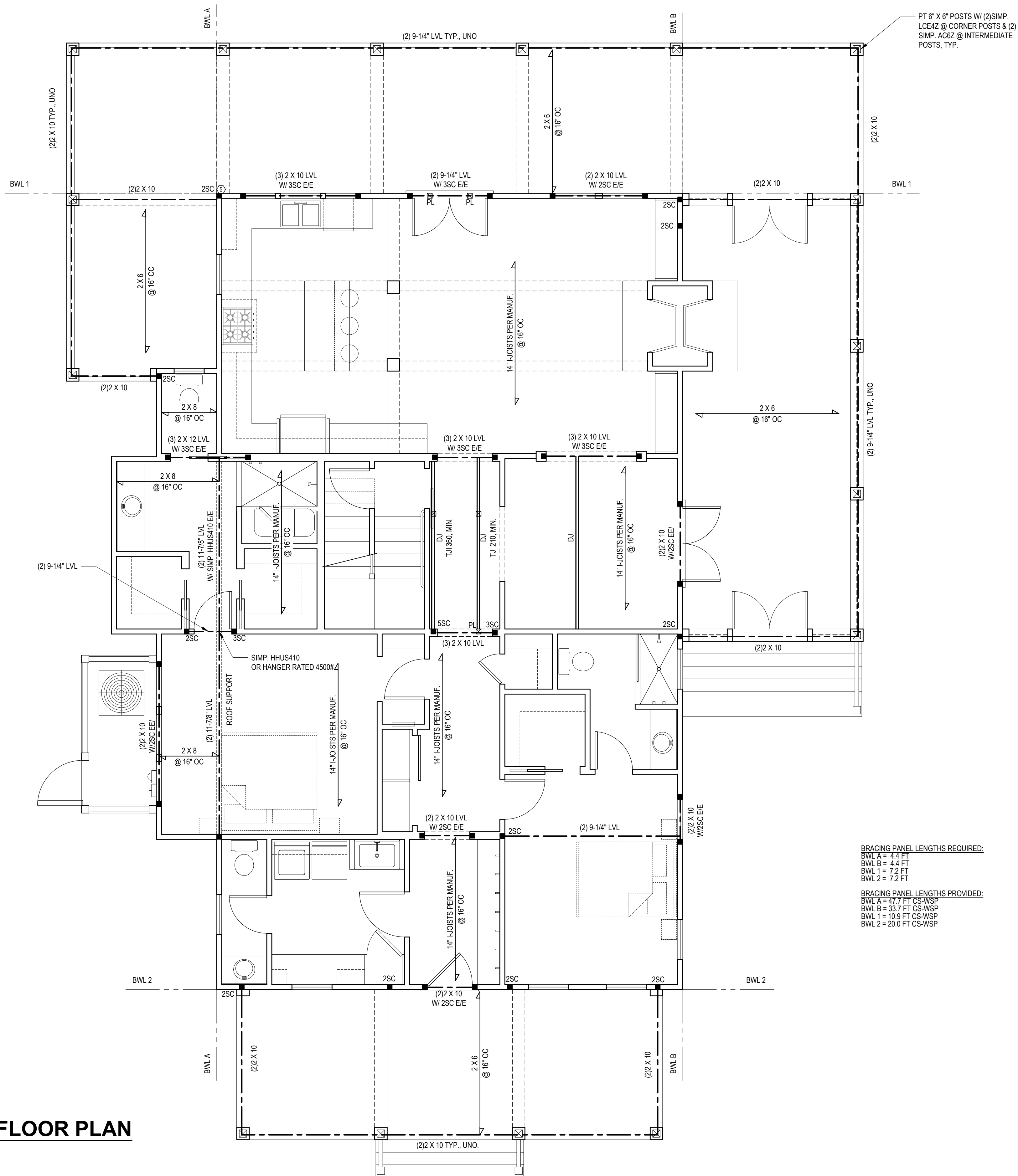
	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (w/ access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

STRUCTURAL NOTES:

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF 'NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE', IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
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- ALL LUMBER SHALL BE SYP #2 (UNO)
- ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND F_b = 2600 PSI F = 1.9M PSI (E LEVEL MICROLAM)
- ALL LVL LUMBER IS TO BE 1.55E (F_b = 2325 PSI)
- ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10# NAILS @ 8" O.C. PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 8'-0". MINIMUM BOTTOM OF THE WINDOW HEIGHT @ 1'-0". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10' IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 F_y = 50 KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, f_c = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 12"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 1'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
- PROVIDE A MINIMUM OF 300# PLUFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC.
- MAXIMUM MASONRY PER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

STRUCTURAL SHEATHING NOTES

- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
- WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- INTERIOR BRACED WALL PANELS (BNP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 12" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5# COOLER NAILS @ (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS.
- 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6# COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS.
- EXTERIOR BRACED WALL PANELS (BNP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6# COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
- MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:
 - 24" ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT
 - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT
 - 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- SHEATH INTERIOR & EXTERIOR
- FOR CS-WSP METHOD, A MINIMUM 2" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(d). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- MINIMUM 800# HOLD-DOWN DEVICE



BRACING PANEL LENGTHS REQUIRED:
 BWL A = 4.4 FT
 BWL B = 4.4 FT
 BWL 1 = 7.2 FT
 BWL 2 = 7.2 FT

BRACING PANEL LENGTHS PROVIDED:
 BWL A = 47.7 FT CS-WSP
 BWL B = 33.7 FT CS-WSP
 BWL 1 = 10.9 FT CS-WSP
 BWL 2 = 20.0 FT CS-WSP

FIRST FLOOR PLAN

1/4" = 1'-0"

*Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precautions.
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 919.778.4400 • www.tyndallengineering.com

Client: **DAVID KRAKOWSKI**
 Project: **KRAKOWSKI RESIDENCE**

1ST FLOOR HEADER
 2ND FLOOR FRAMING

Project #: 2201-010306
 Date: 9/20/2022
 Engineered by: HJS
 DWG. Checked By: AWL
 Scale: SEE PLAN

No.	Date	Remarks

Sheet Number
S3
 3 of 8

FILENAME: \\A:\ESD\DRINL_E\2022_STRUCT\PROJECTS\2201-010306 - KRAKOWSKI RESIDENCE\DWG_FILES\2201-010306_EPRG_S3.DWG BY: SWANESH LAST PLOT DATE: 9/21/2022 3:22 PM

DESIGN LOADS

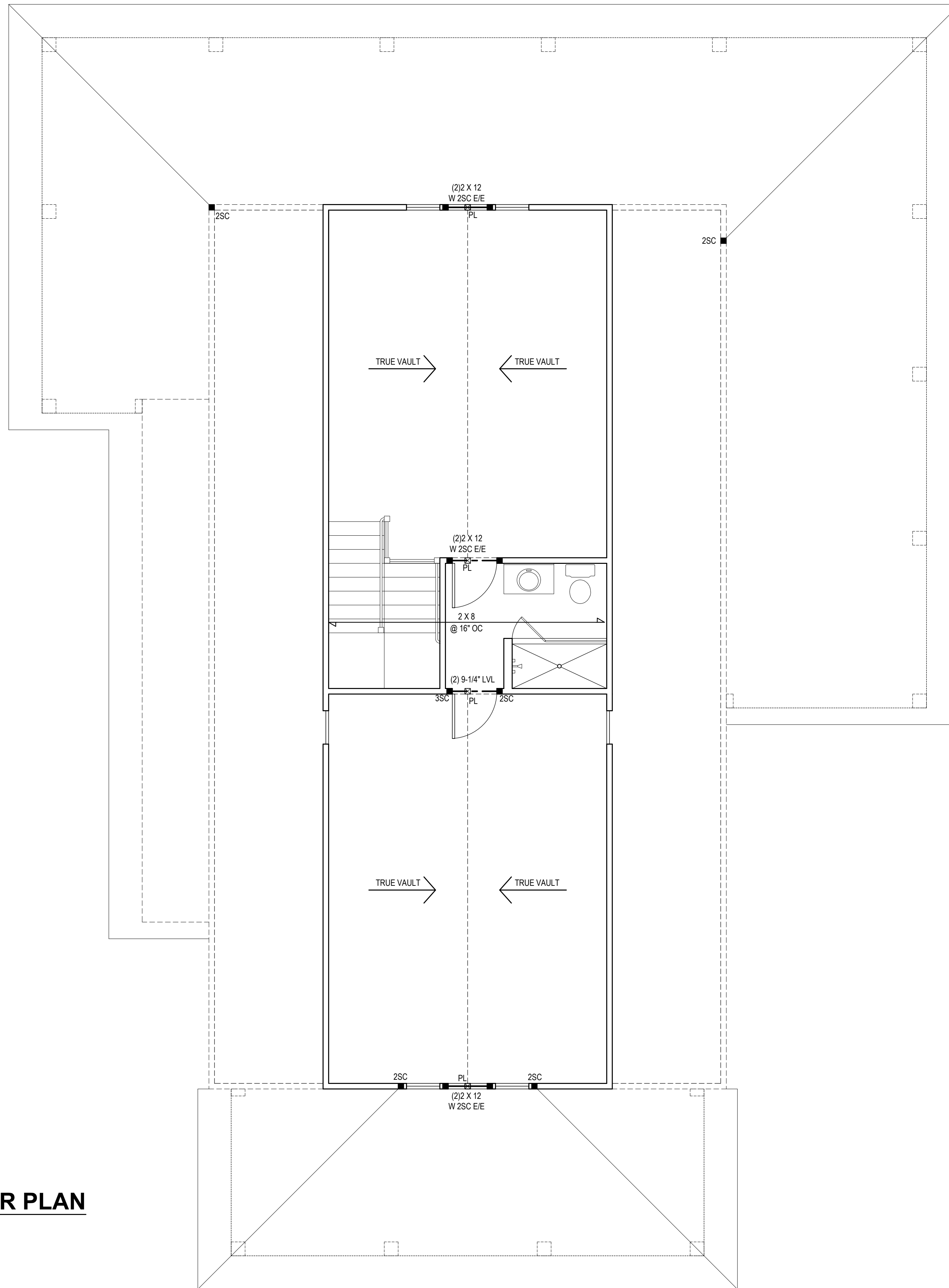
	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

STRUCTURAL NOTES:

- 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF 'NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE', IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
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- 3) ALL LUMBER SHALL BE SYP #2 (UNC)
ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND F_b = 2600 PSI, E = 1.9M PSI (I.E. LEVEL MICROLAM)
ALL LSL LUMBER IS TO BE 1.55E (F_b = 2325 PSI)
- 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/ (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 9" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 8'-0". MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-0". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- 5) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNC)
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- 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 F_y = 50 KSI MIN. (UNC)
- 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- 9) ALL CONCRETE, f_c = 3000 PSI MIN.
- 10) PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 11) 12"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNC)
- 13) PROVIDE A MINIMUM OF 300# PLUFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC.
- 15) MAXIMUM MASONRY PER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

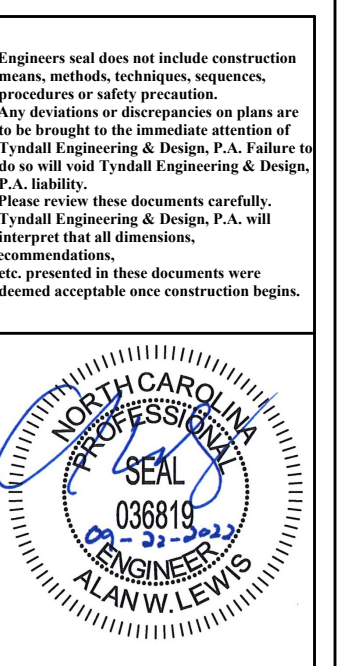
STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
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- 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
 - ① REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE QB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNC)
 - ② 12" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE W/ 5d COMMON NAILS (OR EQUAL PER TABLE R702.1.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS.
 - ③ 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE W/ 6d COMMON NAILS SPACED AT 9" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS.
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNC)
- 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 9" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
- 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:
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 - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT
 - 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- ④ SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 2x4 BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(a). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
 - ⑤ MINIMUM 800# HOLD-DOWN DEVICE



SECOND FLOOR PLAN

1/4" = 1'-0"



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919-774-1100 • 919-774-1444
www.tyndallengineering.com

Client: **DAVID KRAKOWSKI**
Project: **KRAKOWSKI RESIDENCE**

**2ND FLOOR HEADER
2ND FLR. CLG. FRAMING**

Project #: 2201-010306
Date: 9/20/2022
Engineered by: HJS
DWG. Checked By: AWL
Scale: SEE PLAN

REVISIONS		
No.	Date	Remarks
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Sheet Number
S4
4 of 8

FILENAME: \\A:\ESD\DRIVE\ENR\2022\STRUCTURAL PROJECTS\2201-010306 - DAVID KRAKOWSKI - KRAKOWSKI RESIDENCE\DWG FILES\2201-010306-CLG-FR-2209-01.dwg

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Client: **DAVID KRAKOWSKI**
Project: **KRAKOWSKI RESIDENCE**

ROOF PLAN

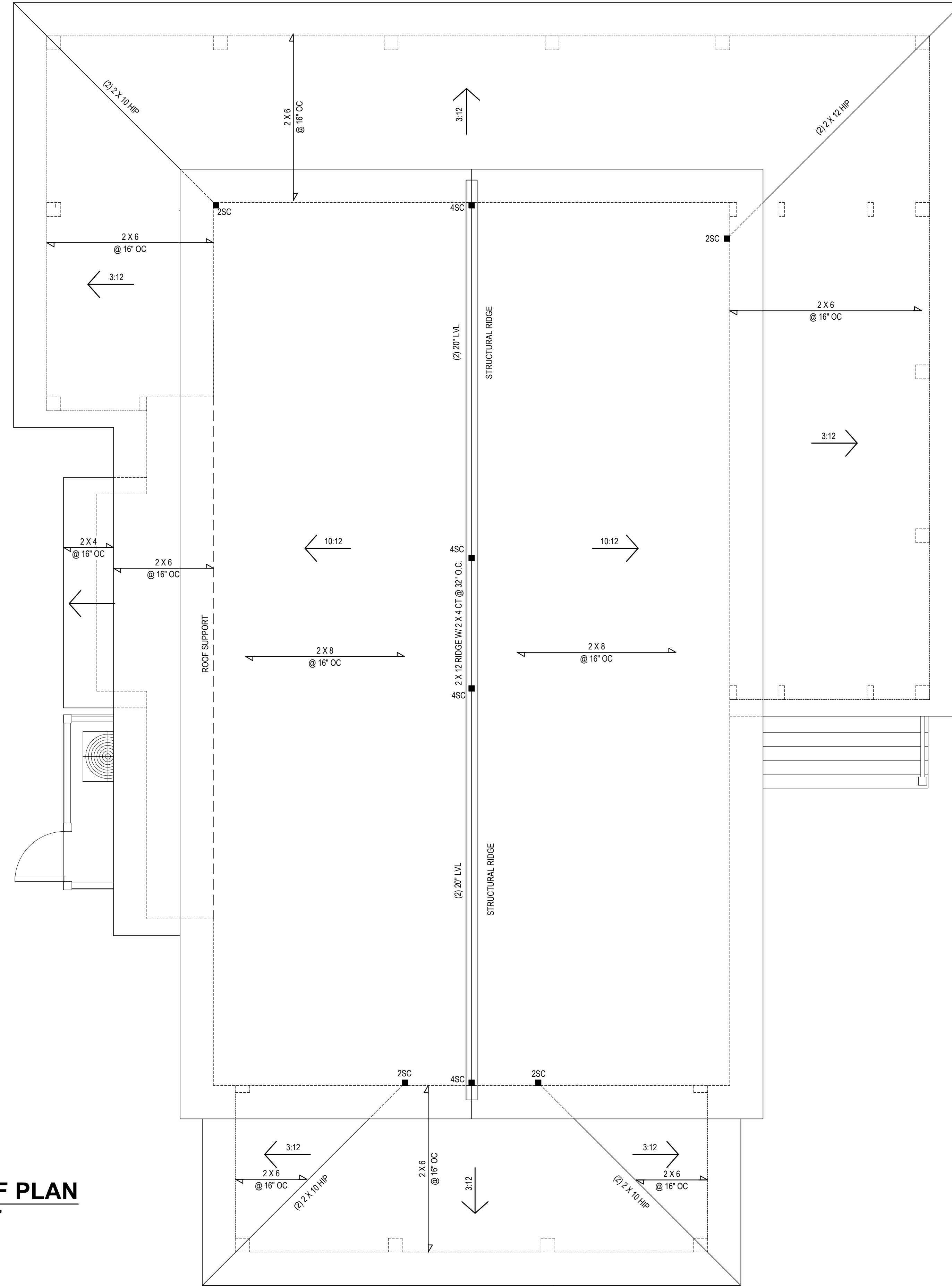
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Date: **9/20/2022**
Engineered by: **HJS**
DWG. Checked By: **AWL**
Scale: **SEE PLAN**

No.	Date:	Remarks
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Sheet Number

S5

ROOF PLAN
1/4" = 1'-0"



STRUCTURAL NOTES

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF 'NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE', IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- DESIGN LOADS:

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	TL
ALL FLOORS	40	10	L/360	L/240
ATTIC (w/ walk up stairs)	30	10	L/360	L/240
ATTIC (pull down access)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	SEISMIC ZONES A, B & C			
- MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE. (U.N.C.)
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R602.3 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- ALL FRAMING LUMBER SHALL BE SYP #2 (F_b = 800 PSI, BASED ON D x 10) (U.N.C.)
ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.
ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND F_b = 2000 PSI, E = 1.9M PSI (U.N.C.)
ALL L.S. LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND F_b = 2325 PSI, E = 1.8M PSI (U.N.C.)
ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND F_b = 2400 PSI, E = 1.8M PSI (U.N.C.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10 (U.N.C.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50.
ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36.
ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3 1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1 1/2" x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2" Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE.
- WALL AND ROOF CLADDING VALUES:
WALL CLADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE.
ROOF WALLS BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:
39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1 1/2/12
36.0 LBS/SQFT FOR ROOF PITCHES 1 1/2/12 TO 6/12
18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12
MEAN ROOF HEIGHT 3/4" OR LESS.
- FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NRC.
- UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- PSL COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.C.)
- PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.C.)
- MAXIMUM MASONRY PER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION.
TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

DEFINITIONS FOR COMMON ABBREVIATIONS

ALT = ALTERNATE	MAX = MAXIMUM
CANT = CANTILEVER	MIN = MINIMUM
CJ = CEILING JOIST	NOM = NOMINAL
CMU = CONCRETE MASONRY UNIT	O.C. = ON CENTER
COL = COLUMN	PL = POINT LOAD
CONC = CONCRETE	PT = PRESSURE TREATED
CONT = CONTINUOUS	REIN = REINFORCED
CT = COLLAR TIE	REQD = REQUIRED
DBL = DOUBLE	RJ = ROOF JOIST
DIA = DIAMETER	RS = ROOF SUPPORT
DJ = DOUBLE JOIST	SC = STUD COLUMN
DR = DOUBLE RAFTER	SCH = SCHEDULE
EA = EACH	SPEC = SPECIFIED
EE = EACH END	THK = THICK
FJ = FLOOR JOIST	TJ = TRIPLE JOIST
FND = FOUNDATION	TRTD = TREATED
FTG = FOOTING	TYP = TYPICAL
GALV = GALVANIZED	UNO = UNLESS NOTED OTHERWISE
HORIZ = HORIZONTAL	W = WIDE FLANGE BEAM
HT = HEIGHT	WWF = WELDED WIRE FABRIC
MANUF = MANUFACTURER	XJ = EXTRA JOIST

1) MAXIMUM HEIGHT OF DECK SUPPORT POSTS AS FOLLOWS:

POST SIZE	MAX. POST HEIGHT**
4 x 4	8'-0"
6 x 6	20'-0"
***	OVER 20'-0"

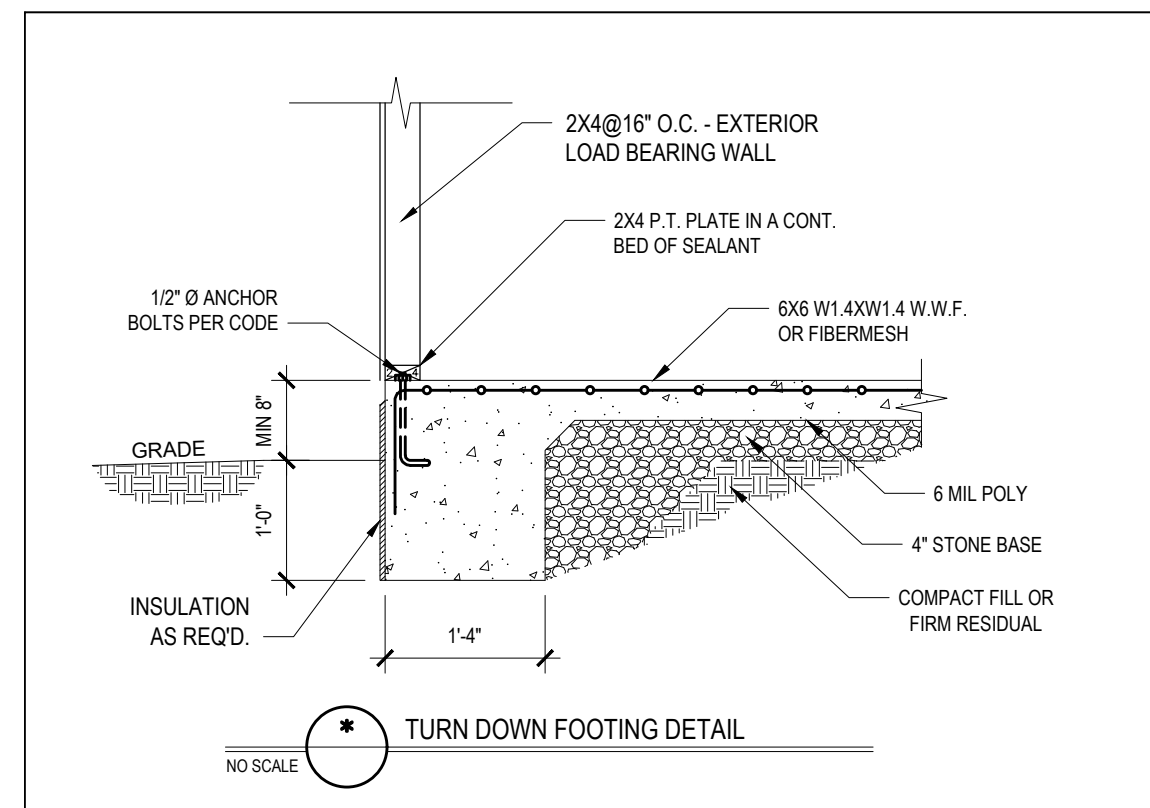
- * THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. MAXIMUM TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET WHICH MAY BE LOCATED AT DIFFERENT LEVELS.
- ** FROM TOP OF FOOTING TO BOTTOM OF GIRDER.
- *** DECKS WITH POST HEIGHTS OVER 20'-0" SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT.

2) DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THESE METHODS:

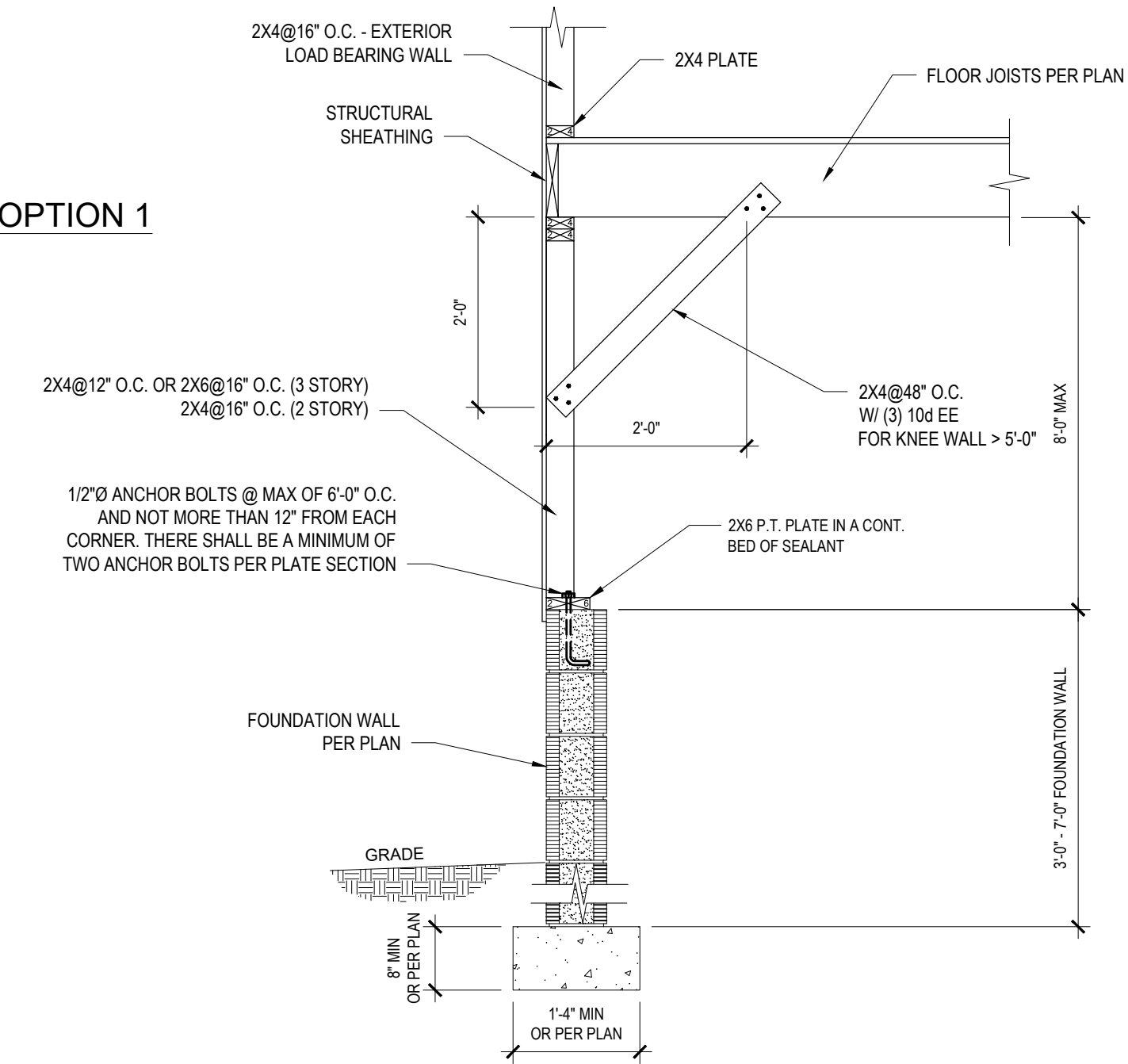
- THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION (4) ABOVE. LATERAL BRACING IS NOT REQUIRED.
- 4 x 4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE BOLTED TO THE POST AND GIRDER WITH ONE 5/8" Ø NOT DIPPED GALVANIZED BOLT AT EACH END OF THE BRACE.
- FOR FREESTANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN ACCORDANCE WITH THE FOLLOWING:

POST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER
4 x 4	48 SQ. FT.	4'-0"	2'-6"	1'-0"
6 x 6	120 SQ. FT.	6'-0"	3'-6"	1'-8"

- 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO (2) PERPENDICULAR DIRECTIONS FOR FREESTANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE 2 x 6s SHALL BE ATTACHED TO THE POSTS WITH ONE 5/8" Ø NOT DIPPED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER.
- FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.



OPTION 1



OPTION 2

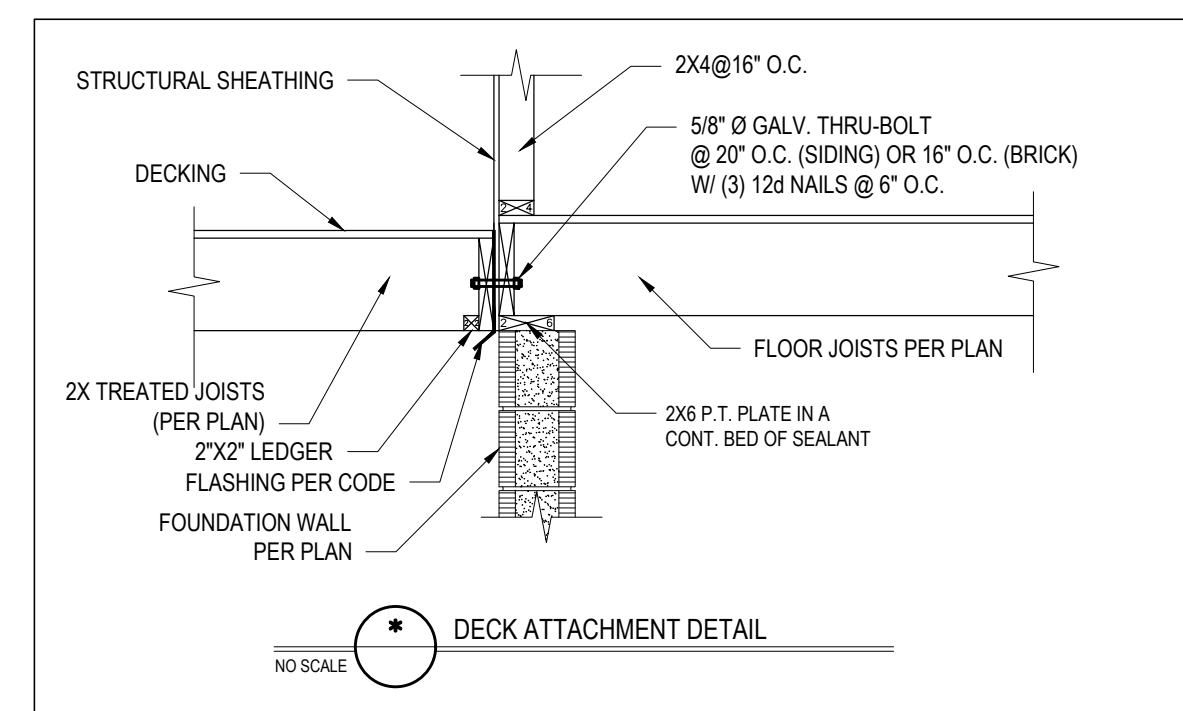
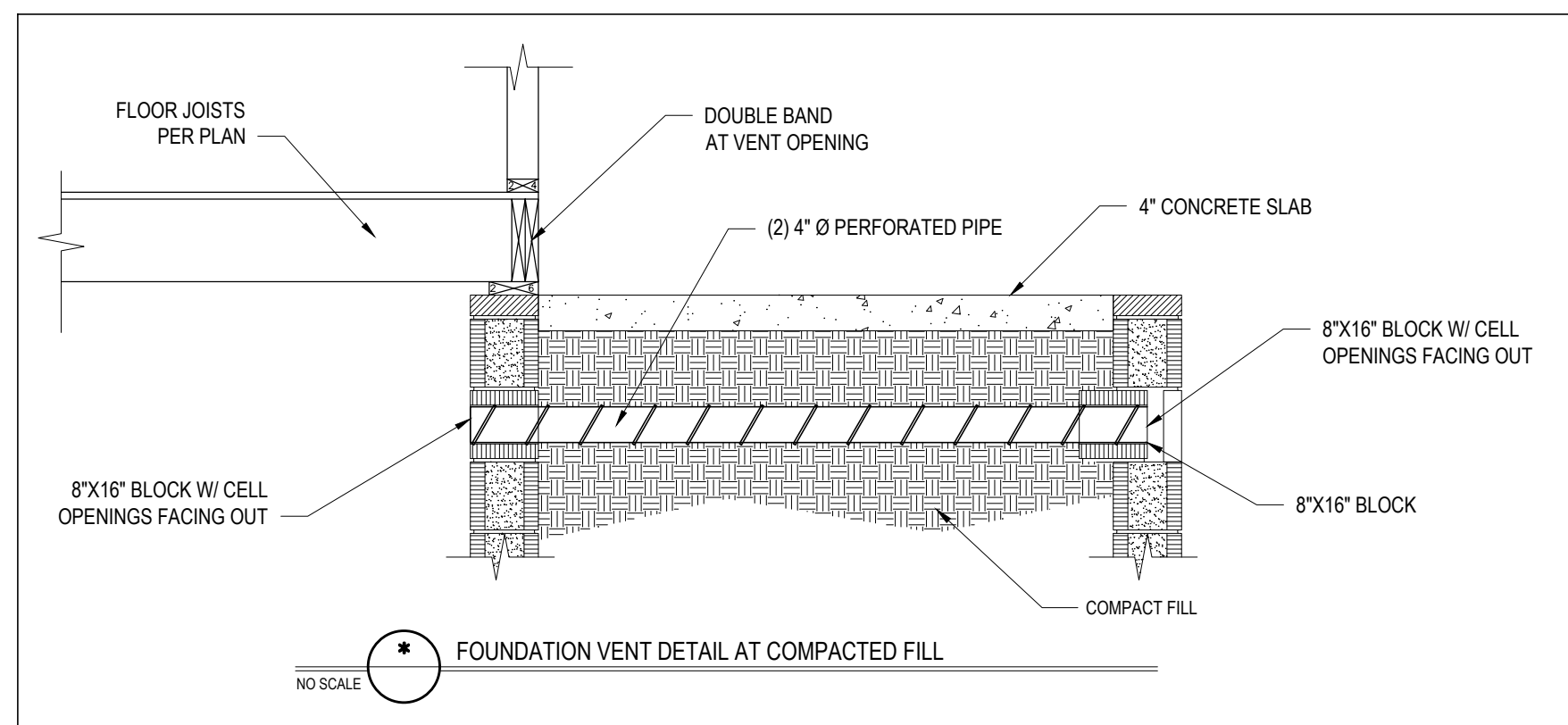
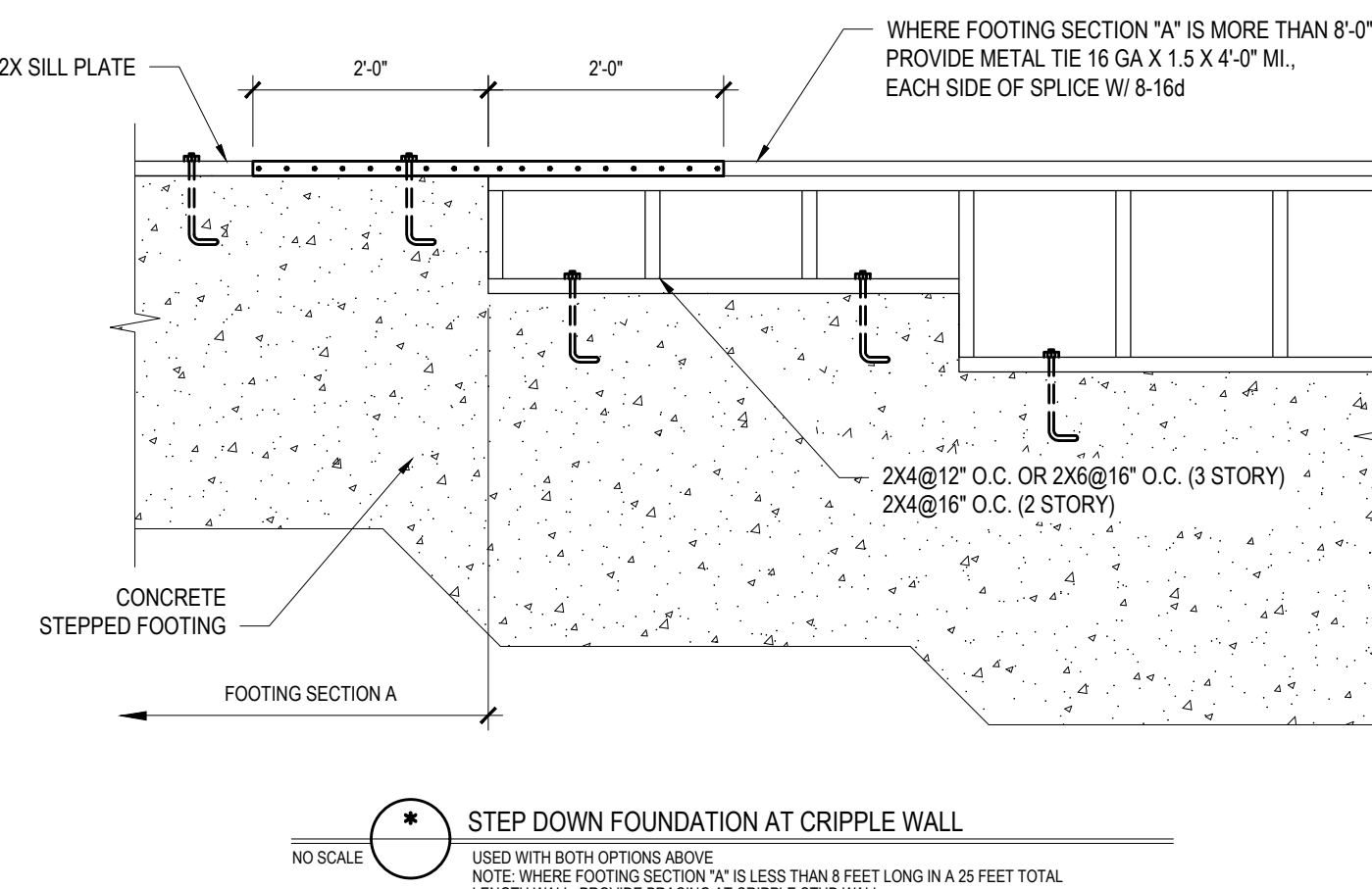
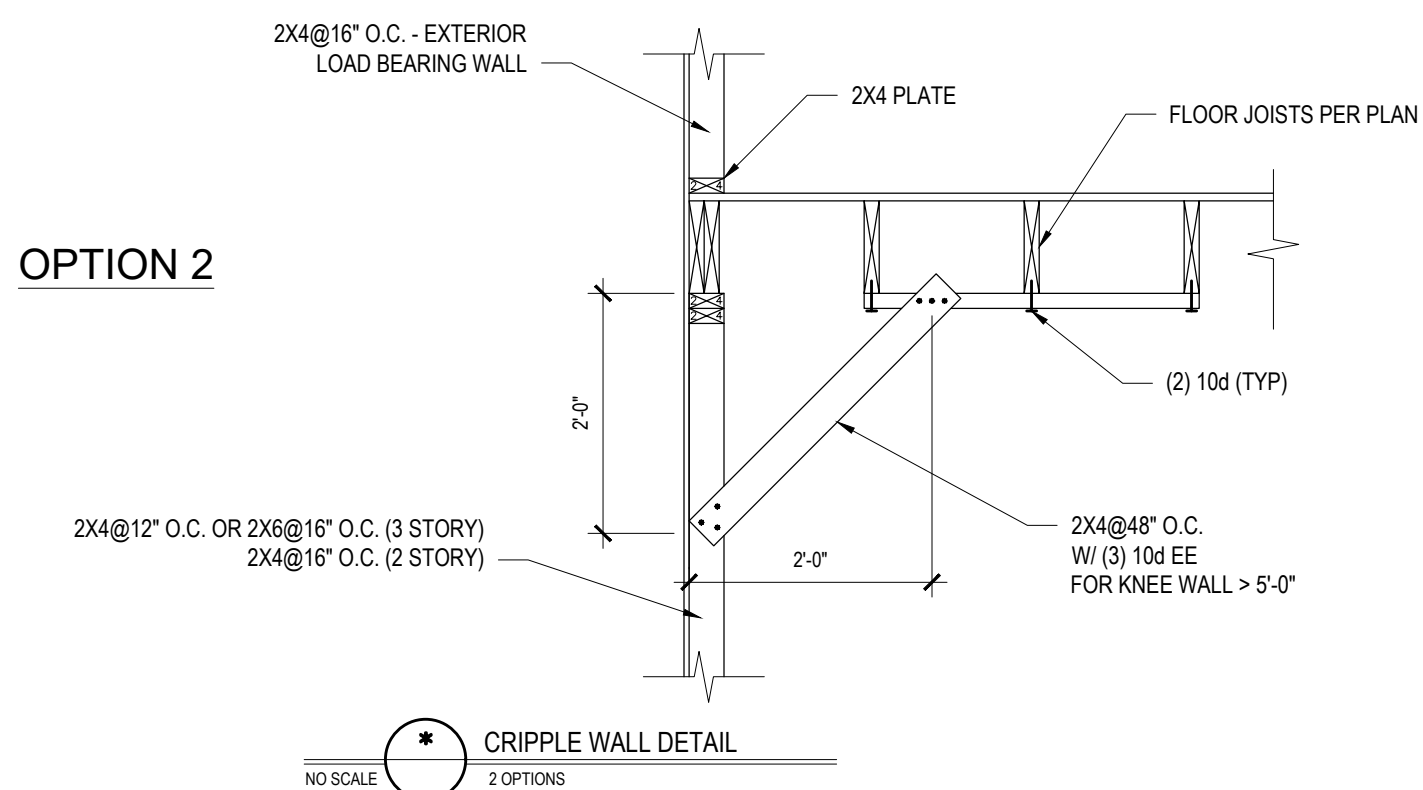


TABLE N1102.1 CLIMATE ZONES 3-5

CLIMATE ZONES	FENESTRATION U-FACTOR ^a	SKYLIGHT U-FACTOR ^b	GLAZED FENESTRATION SHGC ^{c,1-5}	CEILING ^m R-VALUE	WOOD FRAMED WALL R-VALUE	MASS WALL R-VALUE	FLOOR R-VALUE	BASEMENT WALL R-VALUE ^{6,9}	SLAB ^d R-VALUE AND DEPTH	CRAWL SPACE WALL R-VALUE ⁷
3	0.35	0.55	0.30	38 or 30 cont	15 or 13 + 2.5 ^h	5/13 or 5/10 cont	19	5/13 ¹	0	5/13
4	0.35	0.55	0.30	38 or 30 cont ¹	15 or 13 + 2.5 ^h	5/13 or 5/10 cont	19	10/15	10	10/15
5	0.35	0.55	NR	38 or 30 cont ¹	19, or 13 + 5 ^h or 15 + 3 ^h	13/17 or 13/12.5 cont	30 ⁹	10/15	10	10/19

NO SCALE

* R-VALUES ARE MINIMUM U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE.

¹ THE FENESTRATION U-FACTOR COLUMN ENCLOSED SKYLIGHTS. THE SOLAR HEAT GAIN COEFFICIENT (SHGC) COLUMN APPLIES TO ALL GLAZED FENESTRATION.

² 100% MEANS R-3 CONTINUOUS INSULATED SHEATHING ON THE INTERIOR OR EXTERIOR OF THE HOME OR IN A CAVITY INSULATION AT THE INTERIOR OF THE BASEMENT WALL OR CRAWL SPACE WALL.

³ FOR MONOLITHIC SLAB INSULATION SHALL BE APPLIED FROM THE INSPECTION GAP DOWNWARD TO THE BOTTOM OF THE FOOTING OR MINIMUM 24\"/>

ATTIC VENTILATION CALCULATION

1600 SQ. FT. OF ATTIC / 300 = 6 SQ. FT. INLETS/OUTLETS REQUIRED

- CALCULATION BASED ON VENTILATORS USED AT LEAST 3\"/>
- CHEMICAL CEILING SHALL HAVE A 1\"/>

NO SCALE

Engineers and designers shall not include construction means, methods, techniques, sequences, procedures or safety precautions. Any deviation or discrepancy on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.

Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.

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Client: **DAVID KRAKOWSKI**
Project: **KRAKOWSKI RESIDENCE**

STANDARD DETAILS

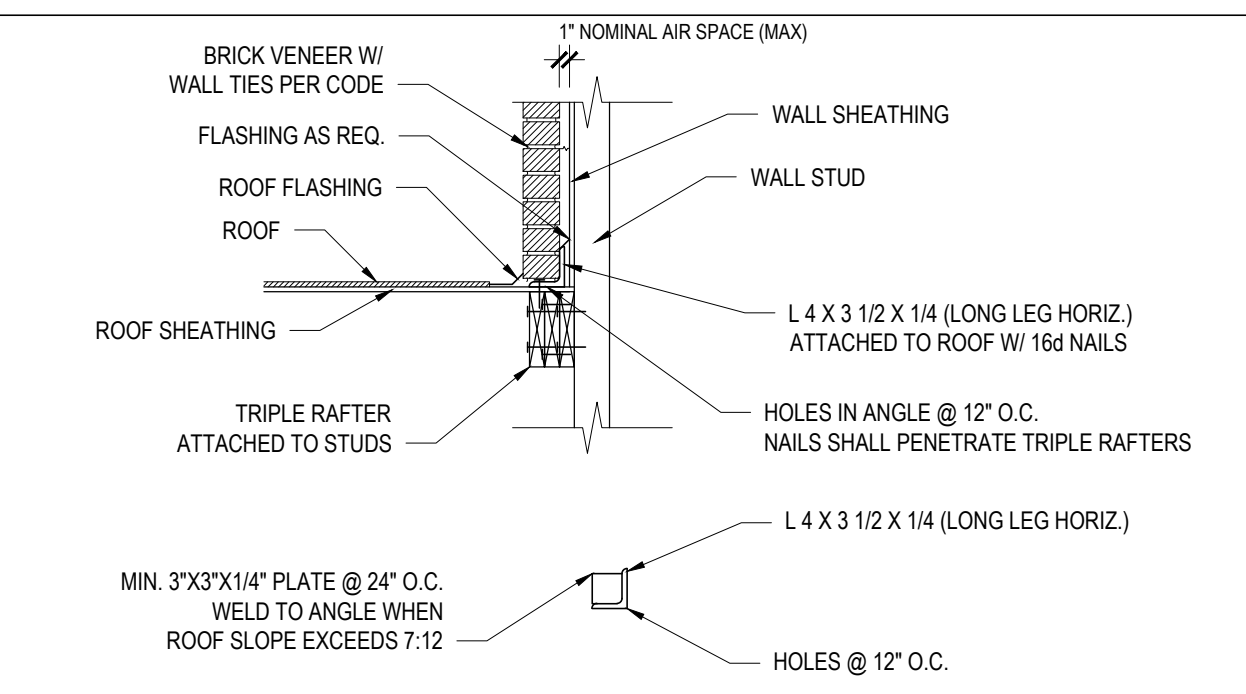
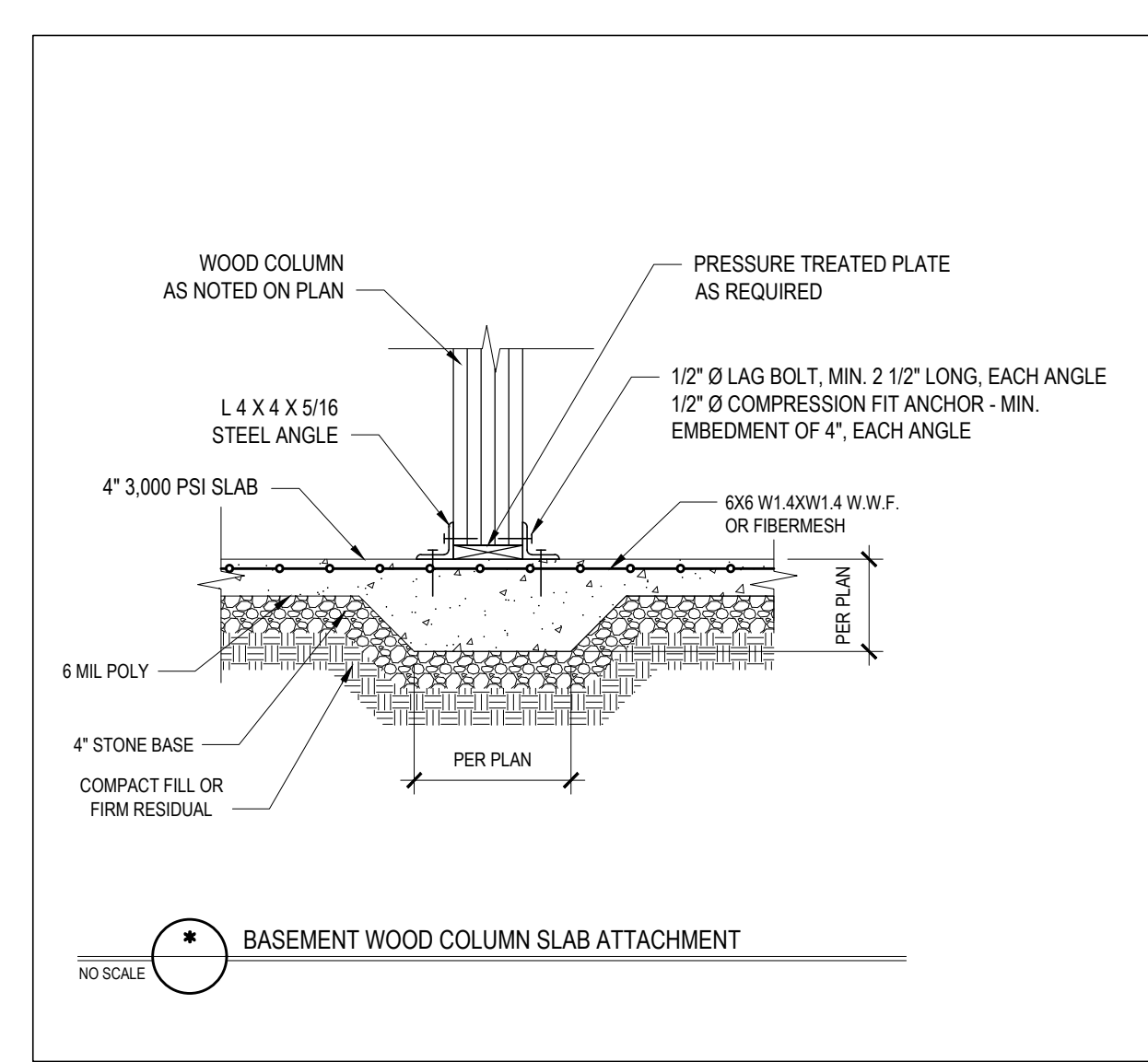
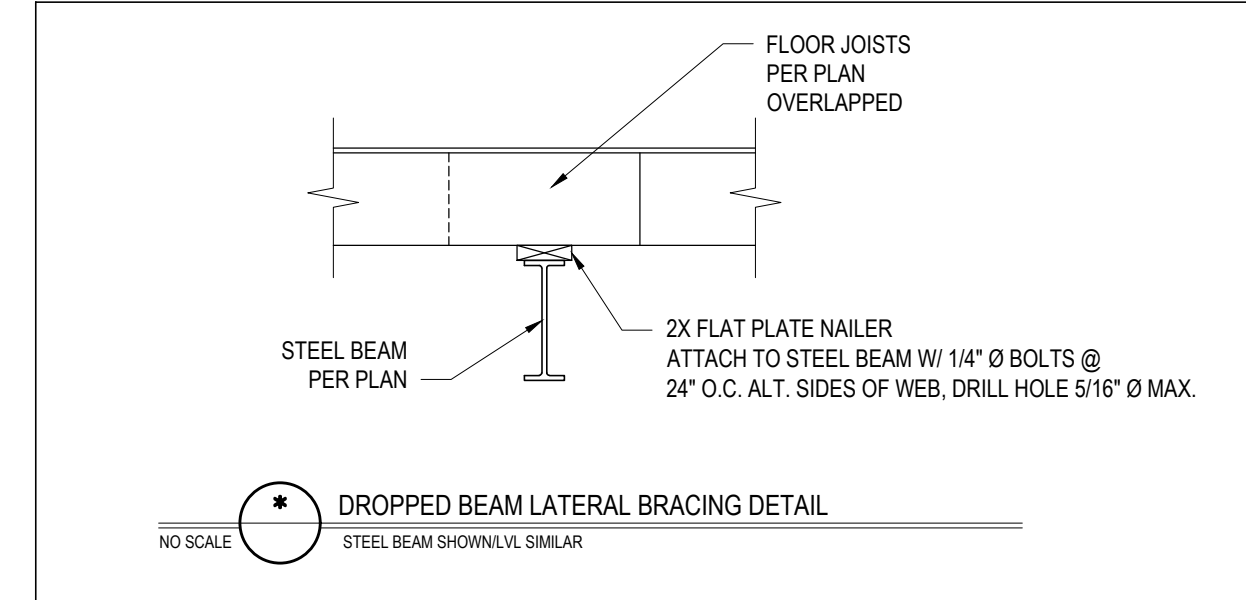
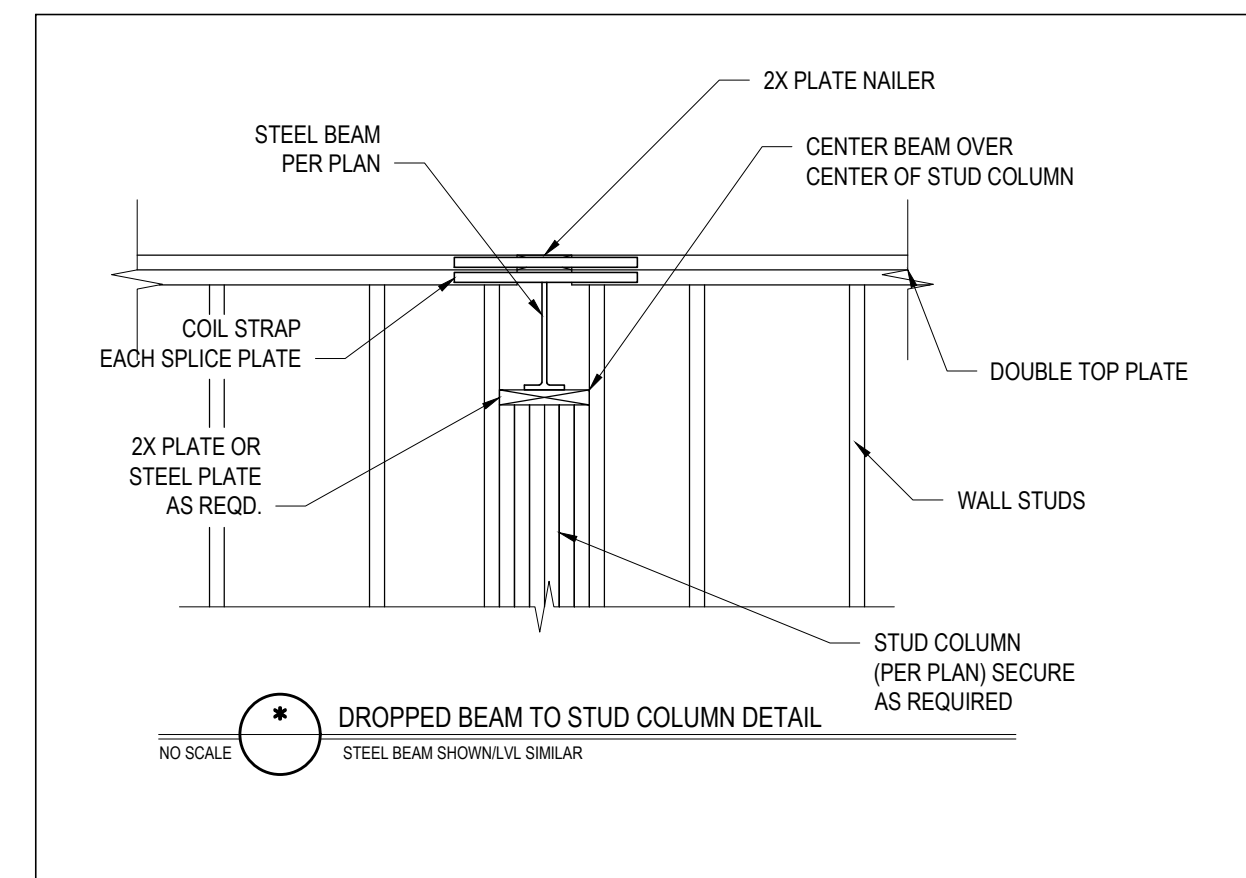
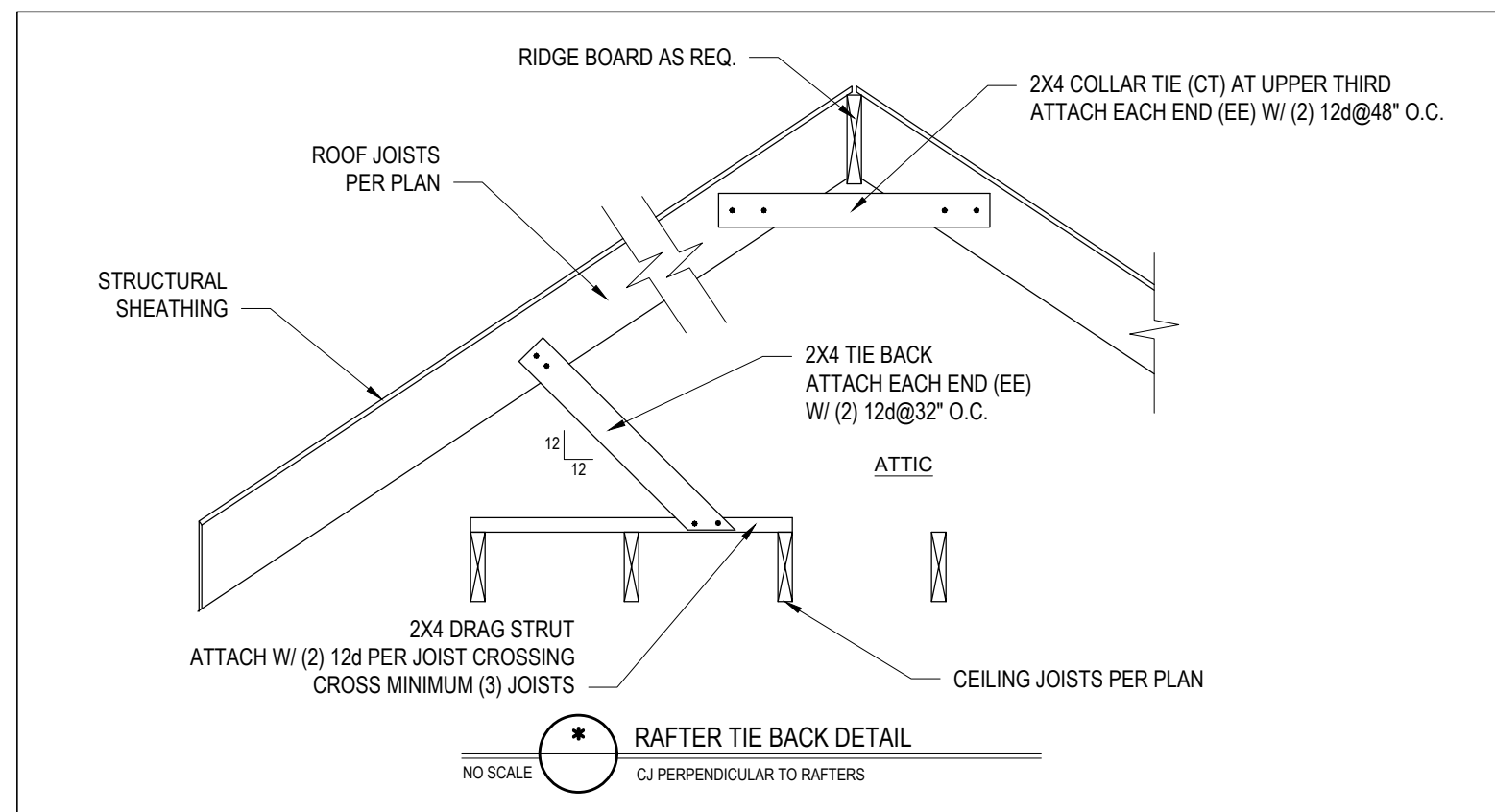
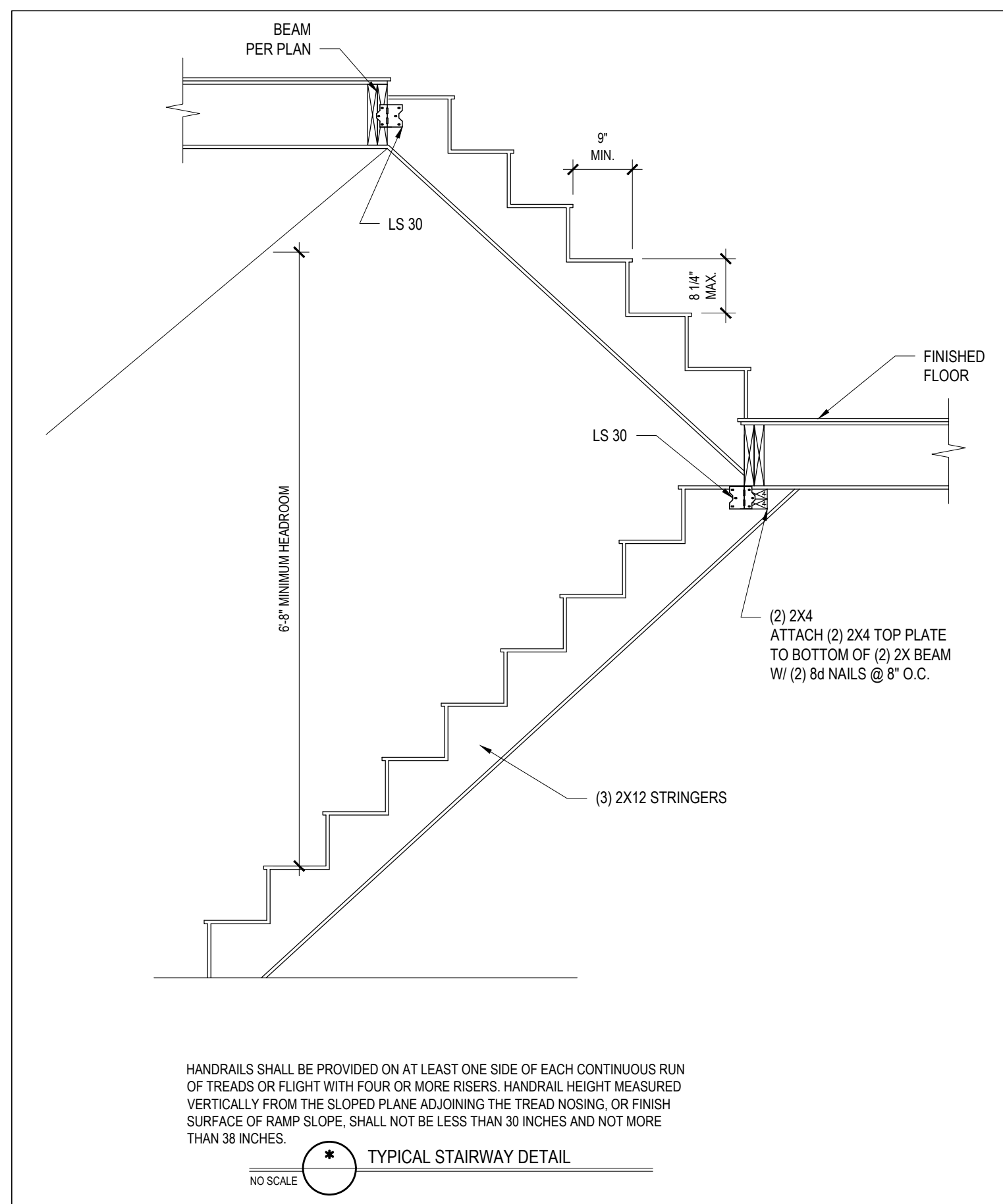
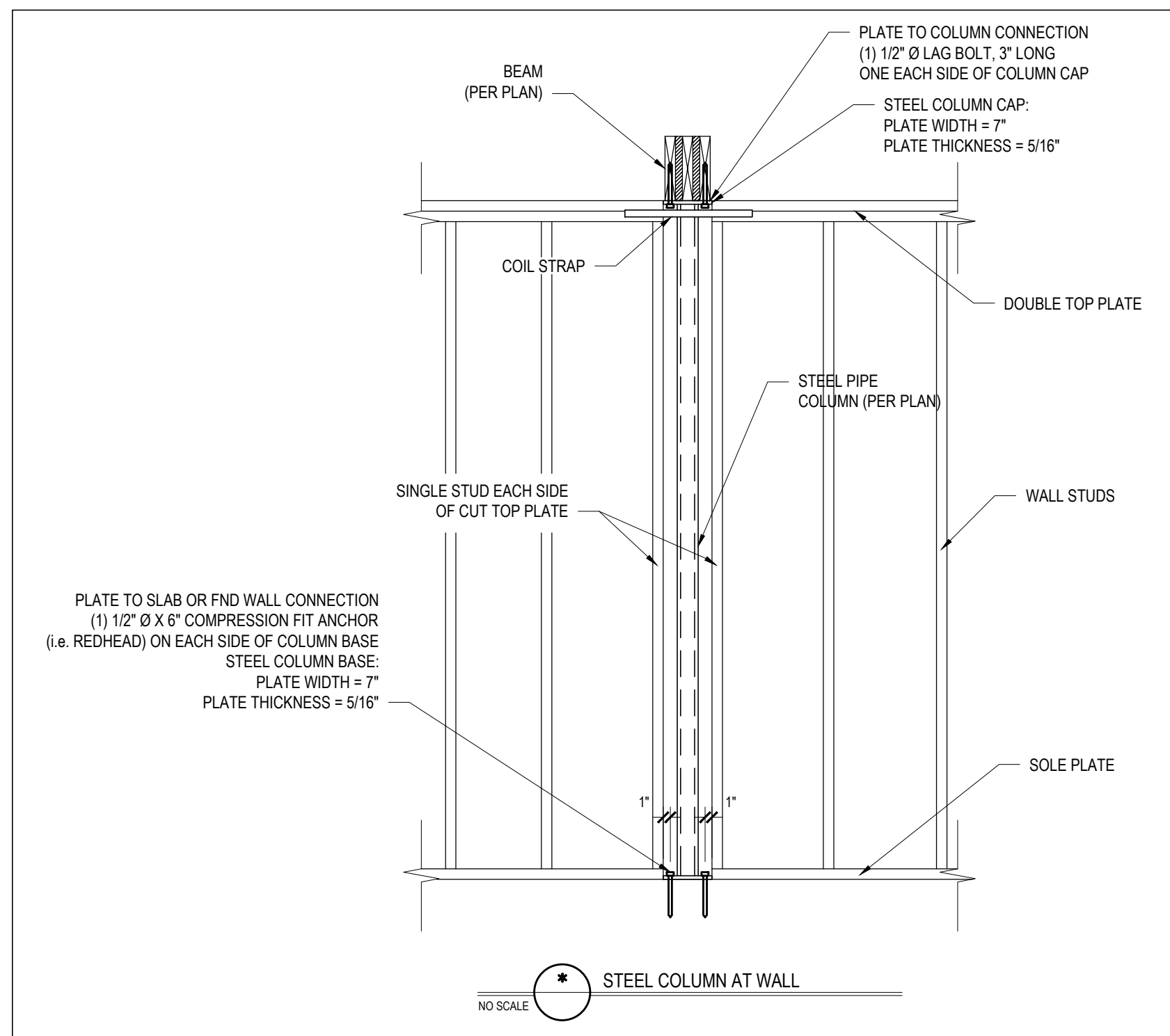
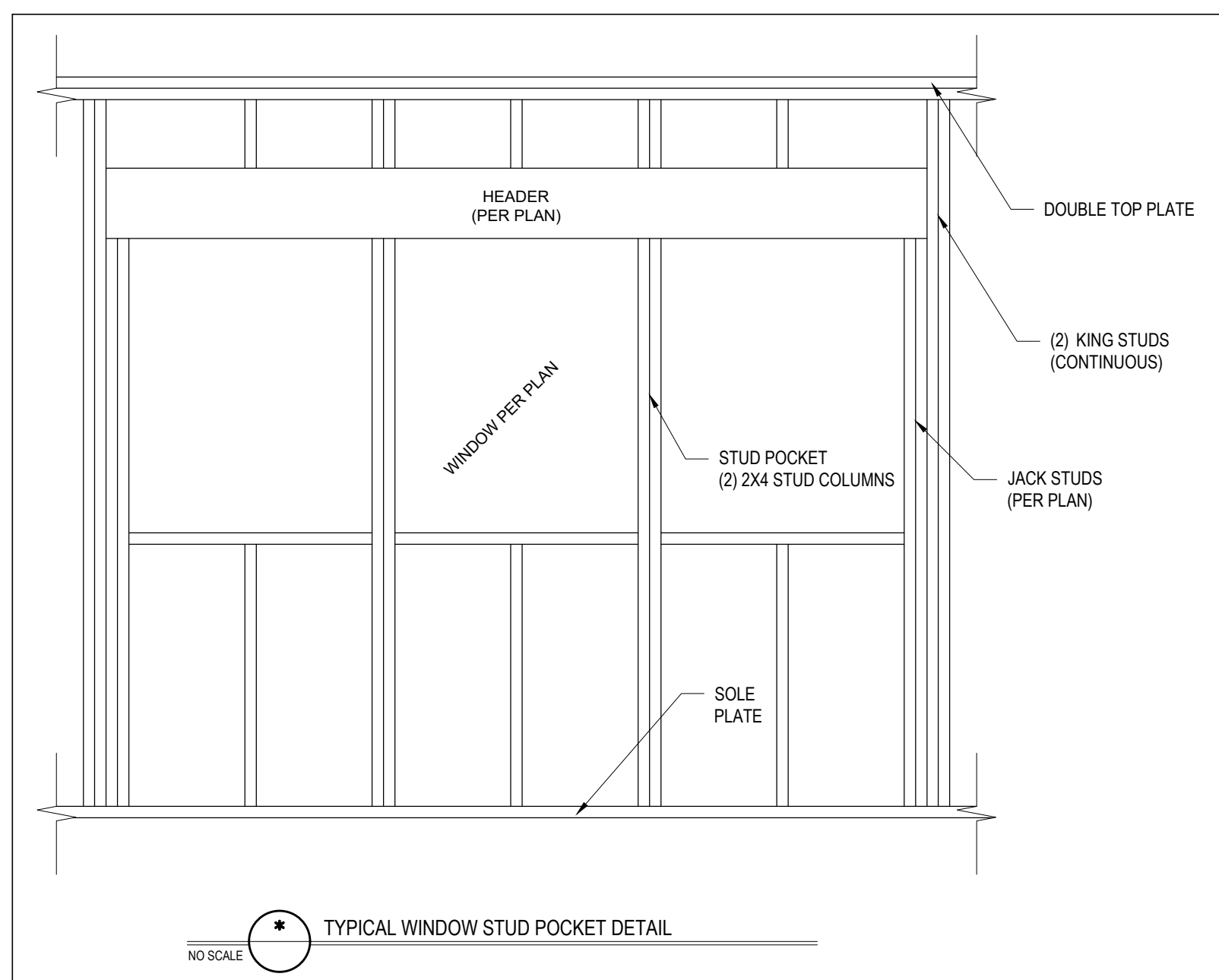
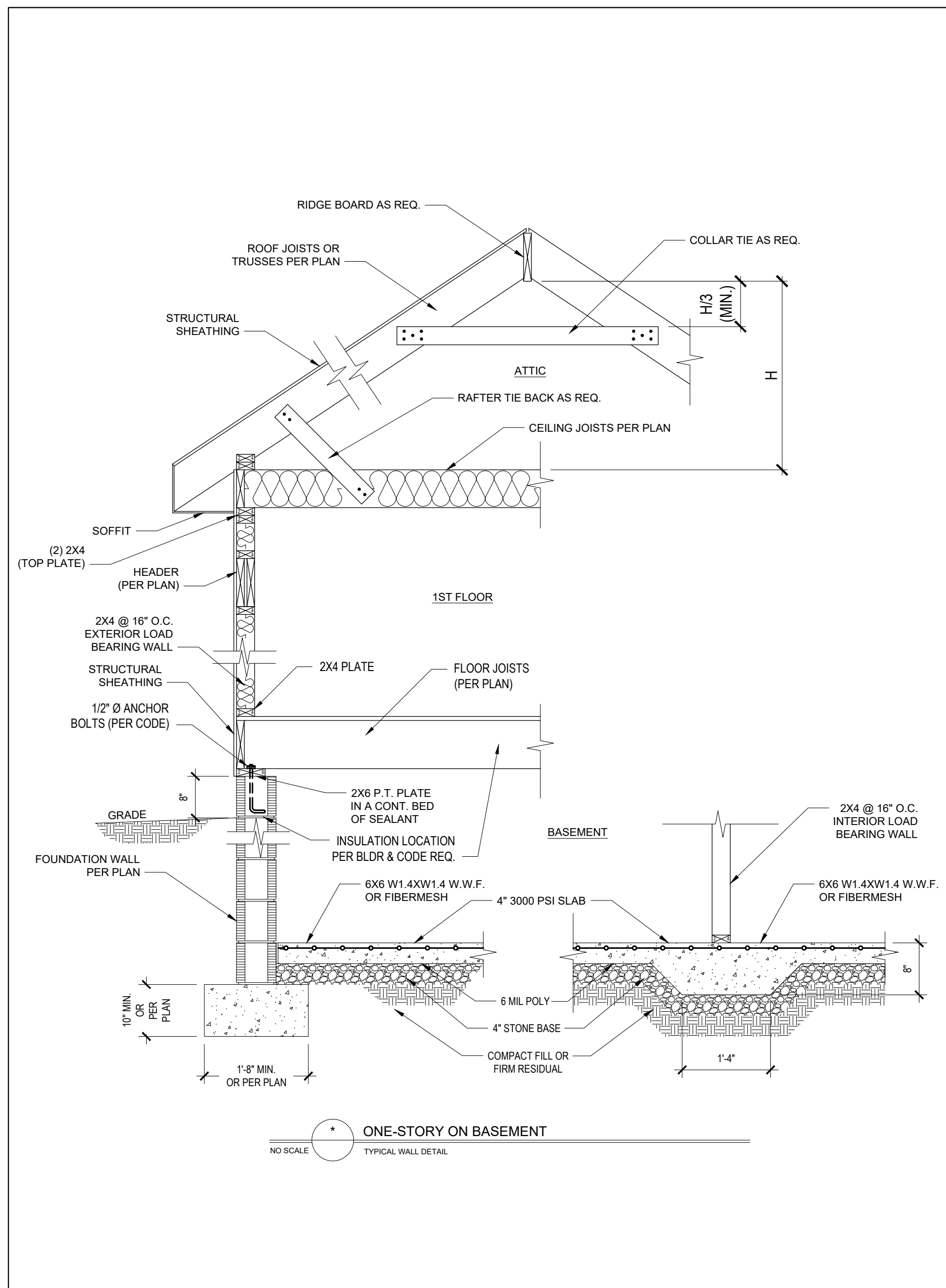
Project #: 2201-010306
Date: 9/20/2022
Engineered by: HJS
DWG. Checked by: AWL
Scale: SEE PLAN

REVISIONS

No.	Date	Remarks

Sheet Number **D1**
6 of 8

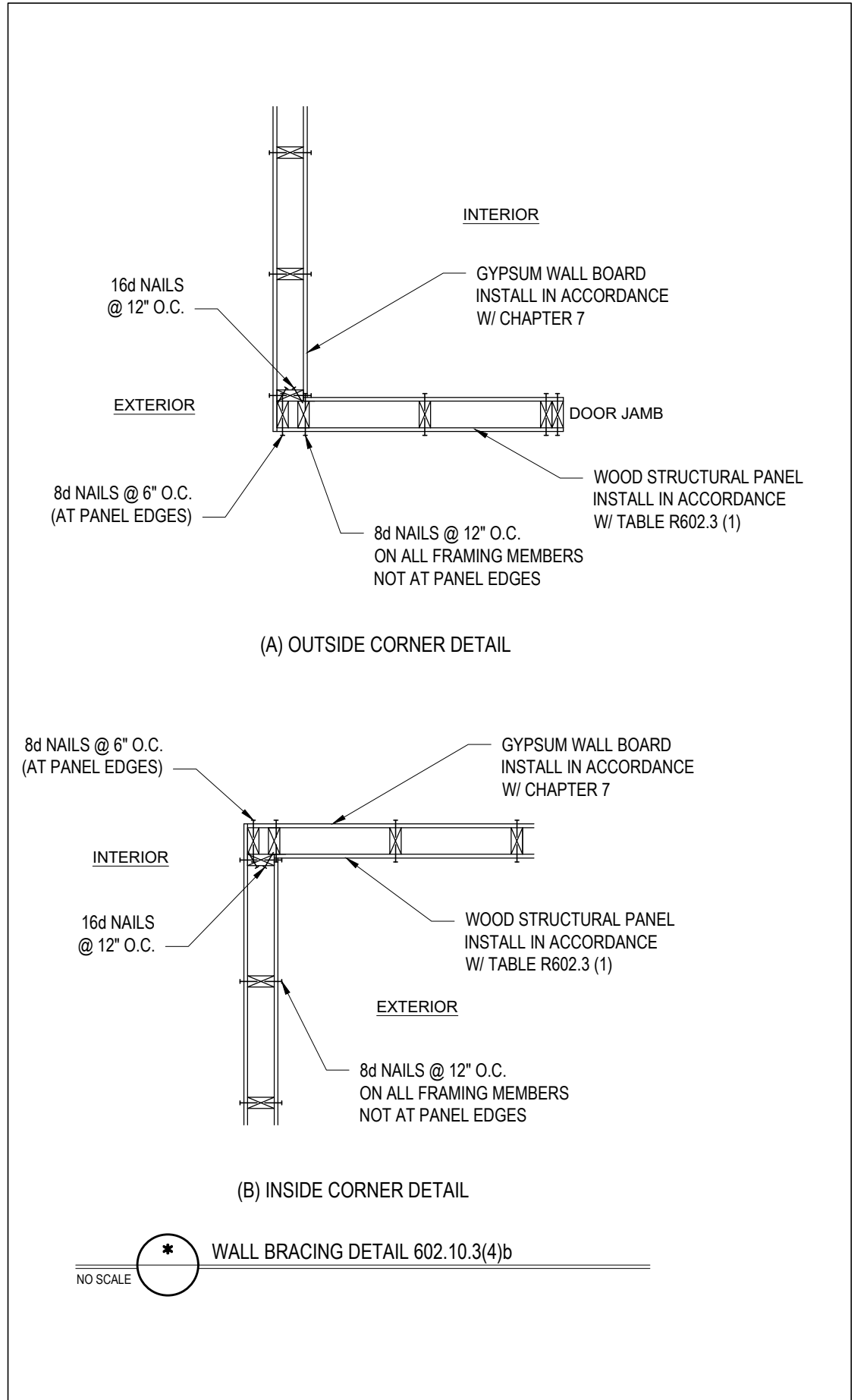
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ALLOWABLE SPANS FOR LINTELS SUPPORTING MASONRY VENEER

SIZE OF ANGLE (1,3)	NO STORY ABOVE (5)	1 STORY ABOVE (5)	2 STORIES ABOVE (5)	# OF 1/2\"/>
L 3 x 3 x 1/2	6'-0"	4'-6"	3'-0"	1
L 4 x 3 x 1/2	8'-0"	6'-0"	4'-6"	1
L 5 x 3 1/2 x 5/16	10'-0"	8'-0"	6'-0"	2
L 6 x 3 1/2 x 5/16	14'-0"	9'-6"	7'-0"	2
2L 5 x 3 1/2 x 5/16	20'-0"	12'-0"	9'-6"	4

- LONG LEG OF THE ANGLE SHALL BE PLACED IN A VERTICAL POSITION.
- DEPTH OF REINFORCING LINTELS SHALL NOT BE LESS THAN 8" AND ALL CELLS OF HOLLOW MASONRY LINTELS SHALL BE GROUTED. REINFORCING BARS SHALL EXTEND NOT LESS THAN 8" INTO THE SUPPORT.
- STEEL MEMBERS INDICATED ARE ADEQUATE TYPICAL EXAMPLES; OTHER STEEL MEMBERS MEETING STRUCTURAL DESIGN REQUIREMENTS SHALL BE PERMITTED TO BE USED.
- EITHER STEEL ANGLE OR REINFORCED LINTEL SHALL SPAN OPENING.
- SPANS OVER 4'-0" SHALL BE SHORED UP UNTIL CURED.



Engineers and architects shall not include construction means, methods, techniques, sequences, procedures or safety precautions. Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.

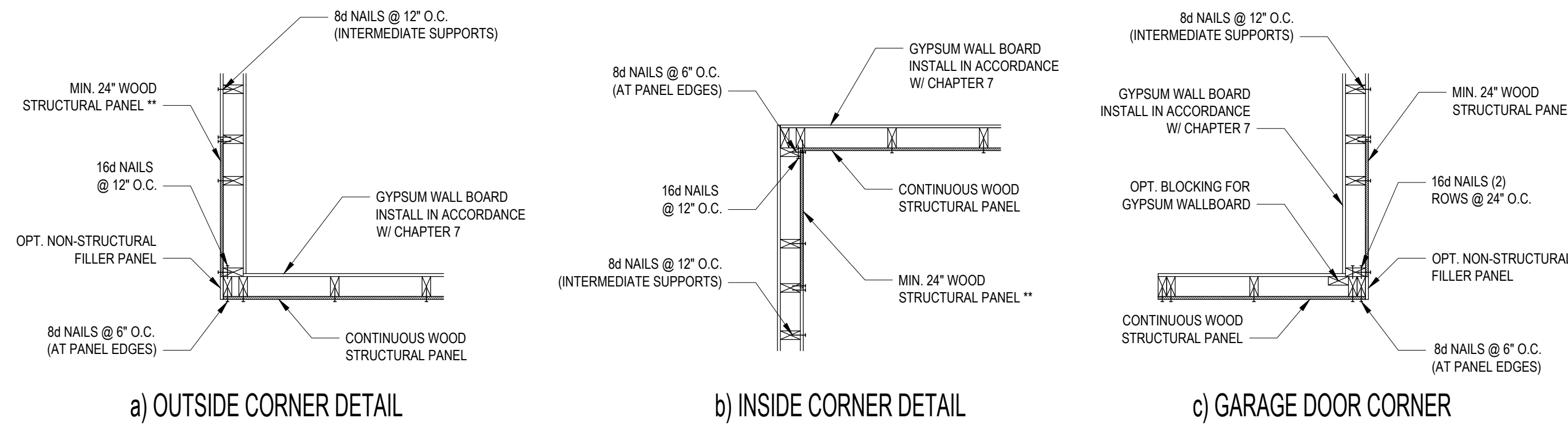
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DAVID KRAKOWSKI
 CLIENT: KRAKOWSKI RESIDENCE
 PROJECT: STANDARD DETAILS

Project #: 2201-010306
 Date: 9/20/2022
 Engineer: HJS
 DWG. Checked By: AWL
 Scale: SEE PLAN

No.	Date	Remarks

Sheet Number **D2**
 7 of 8



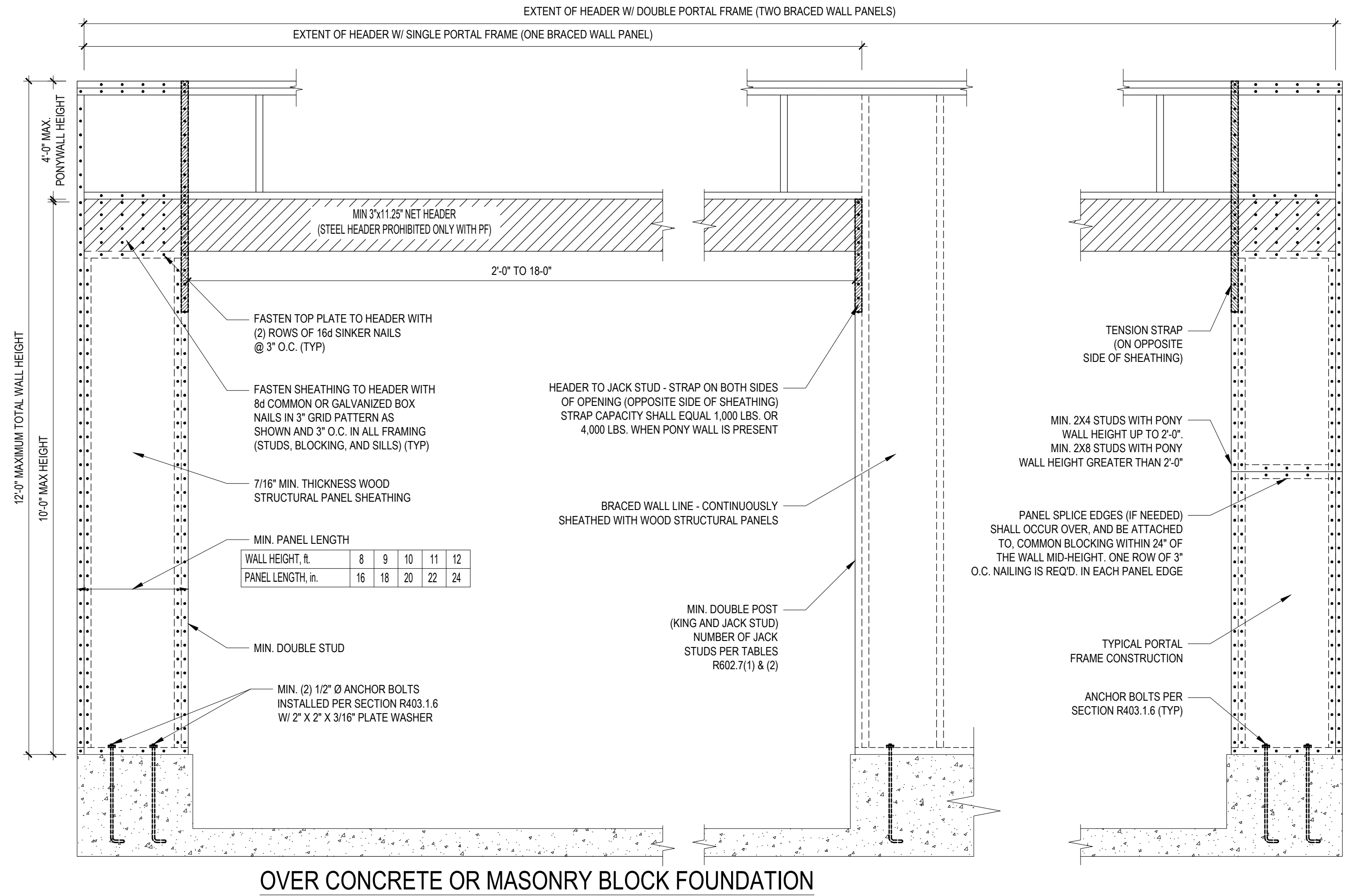
B1: TYPICAL EXTERIOR CORNER FRAMING FOR CONTINUOUS SHEATHING
NO SCALE

STRUCTURAL SHEATHING NOTES

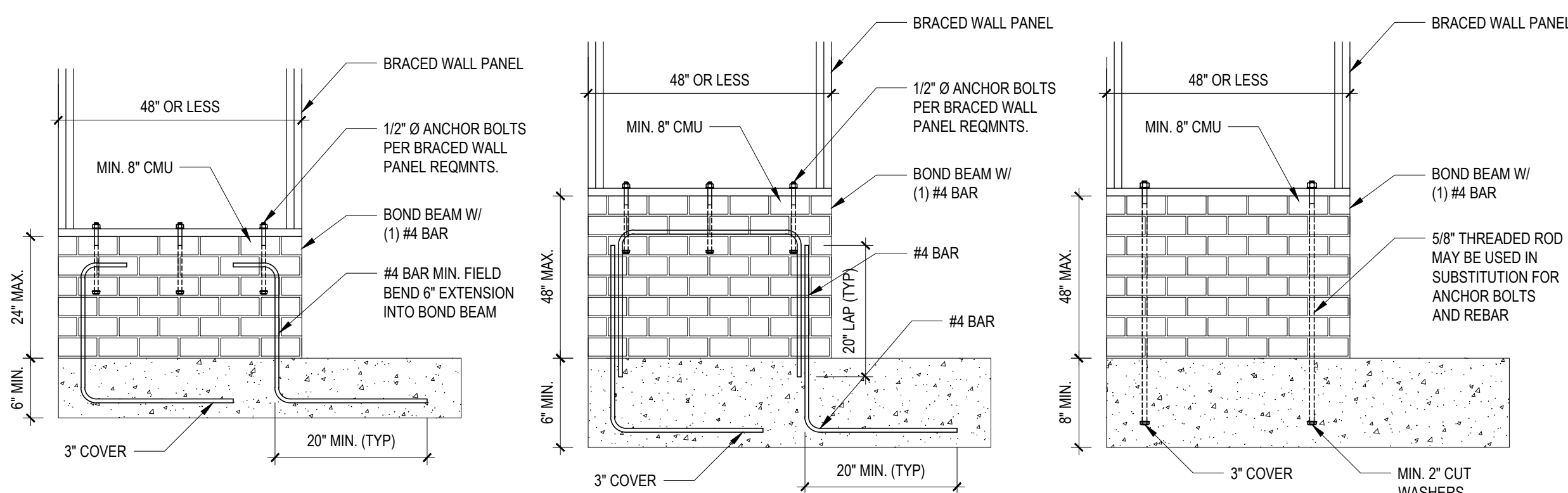
- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
- WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10.3 OF THE 2018 NCR.
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCR.
- INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO).
- 12\"/>

REQUIRED BRACED WALL PANEL CONNECTIONS				
METHOD	MATERIAL	MIN. THICKNESS	REQUIRED CONNECTION	
			@ PANEL EDGES	@ INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6\"/>	

*OR EQUIVALENT PER TABLE R702.3.5
B3: BRACE WALL PANEL CONNECTIONS
NO SCALE



B2: METHOD PF: PORTAL FRAME CONSTRUCTION
FIGURE R602.10.1



B4: MASONRY STEM WALL SUPPORTING BRACED WALL PANELS
FIGURE R602.10.4.3 OF THE 2018 NCR
NOTE: GROUT BOND BEAMS AND ALL CELLS WHICH CONTAIN REBAR, THREADED RODS AND ANCHOR BOLTS

Engineers and designers shall not be held responsible for construction methods, materials, techniques, equipment, procedures or safety precautions. Any deviation or discrepancy on plans shall be brought to the immediate attention of Tyn dall Engineering & Design, P.A. Failure to do so will void Tyn dall Engineering & Design, P.A. liability. Please review these documents carefully. Tyn dall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.



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Client: **DAVID KRAKOWSKI**
Project: **KRAKOWSKI RESIDENCE**

SHEATHING DETAILS

Project #: 2201-010306
Date: 9/20/2022
Engineered by: HJS
DWG. Checked by: AWL
Scale: SEE PLAN

REVISIONS		
No.	Date	Remarks

Sheet Number
D3
8 of 8