



Front - Elev A 1/8" = 1'-0"



2 Rear - Elev A 1/8" = 1'-0"

Area Schedule (Elevation A)					
Name Area					
Heated					
1st Floor	874 SF				
	874 SF				
Unheated					
Front Porch	27 SF				
	27 SF				
Under Roof	901 SF				

Under Roof

A lue Build

O M E S

on Davis Hwy, Sanford, NC 27332

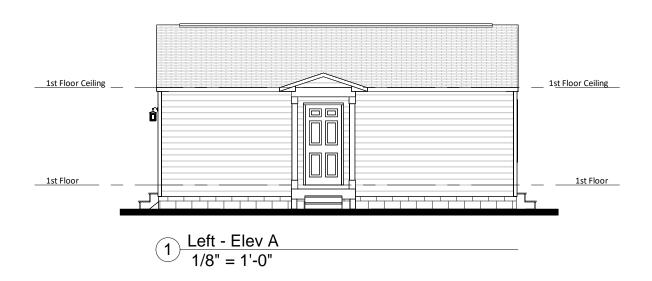
THE JUDY - Elevation A

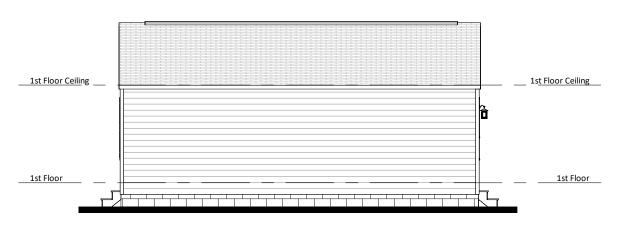
Front & Rear Elevations

Job#: 23-86-9 Address: 51 Bourbon St Fuquay Varina, NC 27526

Plan Version Date: 1-20-21

Job Version Date: 2-20-23





2 Right - Elev A 1/8" = 1'-0"

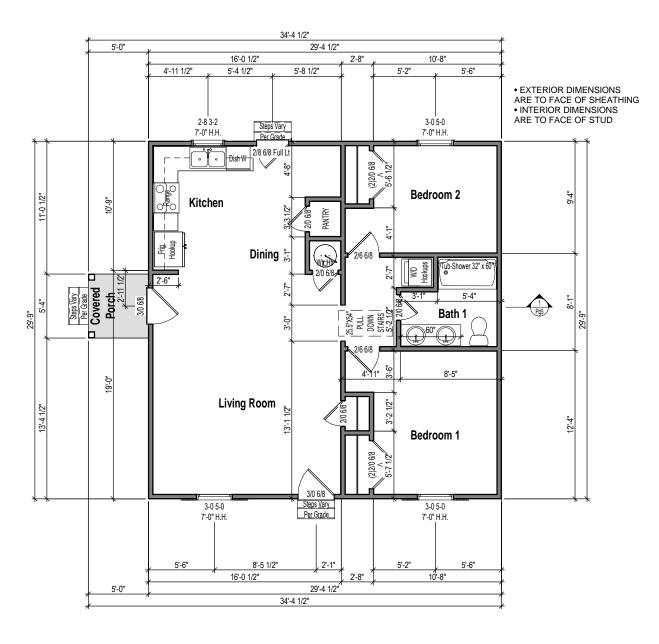
THE JUDY - Elevation A

Side Elevations

Job#: 23-86-9 Address: 51 Bourbon St Fuquay Varina, NC 27526

Plan Version Date: 1-20-21

Job Version Date: 2-20-23



1 1st Floor Plan - Elev A 1/8" = 1'-0"



THE JUDY - Elevation A

1st Floor Plan

Job#: 23-86-9 Address: 51 Bourbon St Fuquay Varina, NC 27526

Plan Version Date: 1-20-21

Job Version Date: 2-20-23

Attic Ventilation Calcs 1/300 (sq.in.)

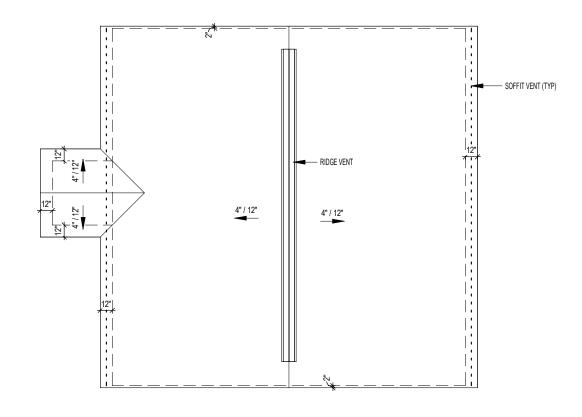
		Ventilation	Max	Min	Upper	Lower	Total	Ridge	Roof	Soffit
		Required	Upper	Upper	Ventilation	Ventilation	Ventilation	Vent	Vents	Vents
Name	Area	(sq.in.)	(sq.in.)	(sq.in.)	(sq.in.)	(sq.in.)	(sq.in.)	(In.ft.)	(ea)	(sq.ft.)
Main Roof	1075 SF	516	413	258	390	372	762	26	0	62

- CALCS BASED ON THE FOLLOWING VALUES

 Ridge Vents = 15 in² of net free area per linear foot

 Roof Vents = 50 in² of net free area per unit

 Soffit Vents = 6 in² of net free area per square foot



Roof Plan - Elev A 1/8" = 1'-0"



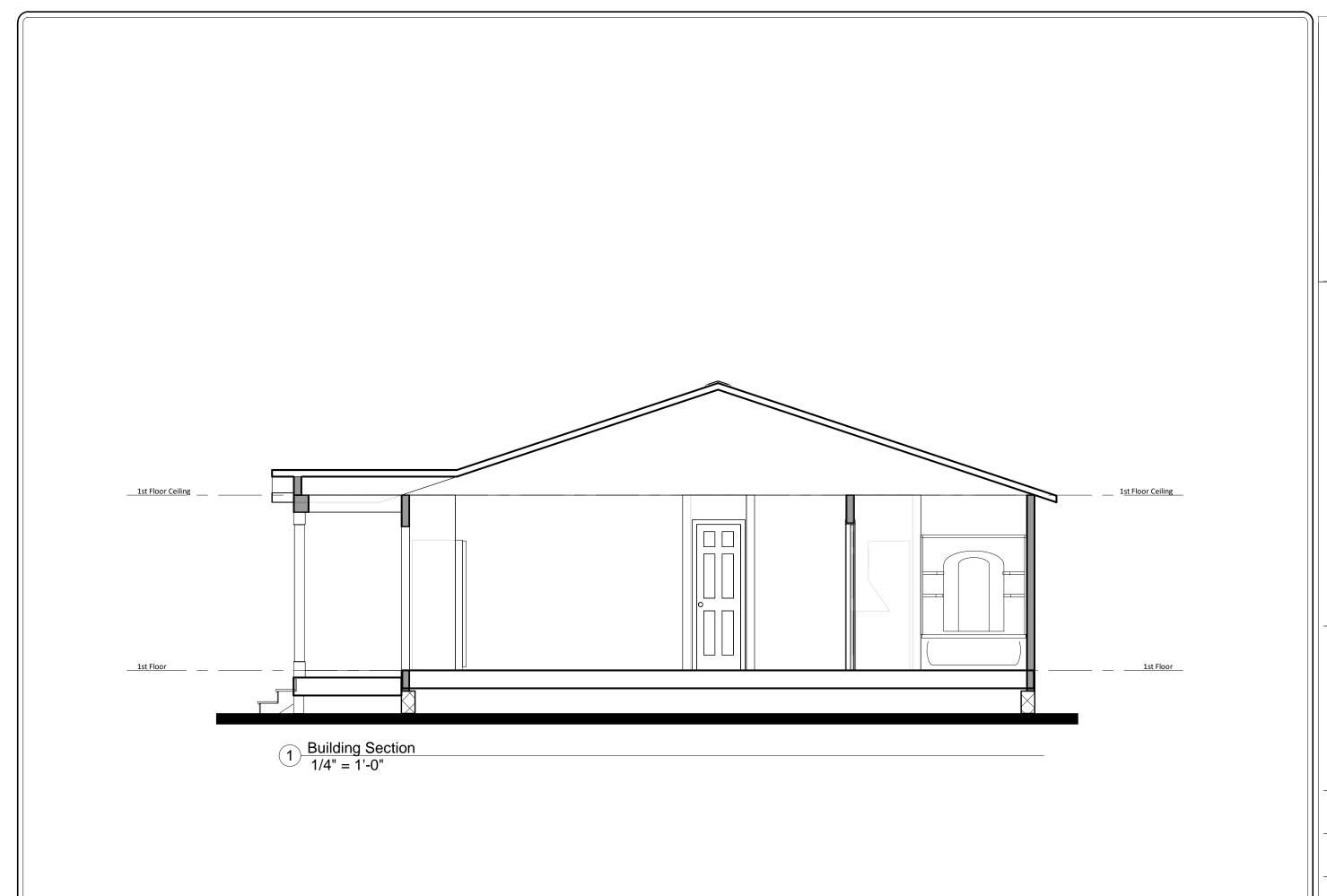
THE JUDY - Elevation A

Roof Plan

Job#: 23-86-9 Address: 51 Bourbon St Fuquay Varina, NC 27526 County: Harnett County

Plan Version Date: 1-20-21

Job Version Date: 2-20-23





THE JUDY - Elevation A

Building Section

Job#: 23-86-9 Address: 51 Bourbon St Fuquay Varina, NC 27526

Plan Version Date: 1-20-21

Job Version Date: 2-20-23

> Sheet #: Pg5

Typical Wall Section - Block Fnd 3/4" = 1'-0"



Elevation A THE JUDY -

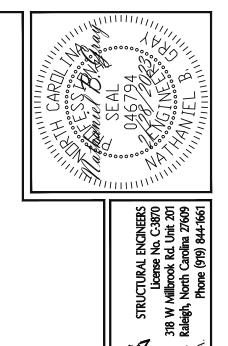
Typical Wall Section

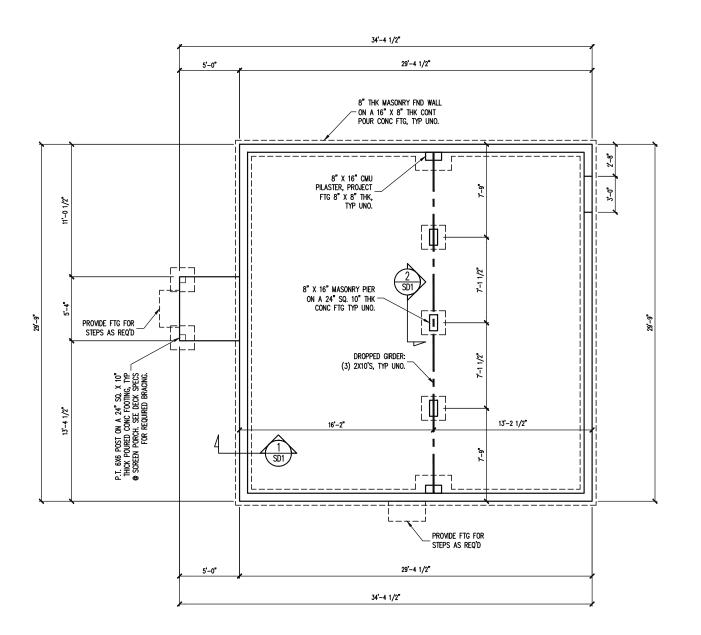
Job#: 23-86-9 Address: 51 Bourbon St Fuquay Varina, NC 27526

Plan Version Date: 1-20-21

Job Version Date: 2-20-23

Sheet #: Sec-Crawl/Block





VALUE BUILD HOMES
STRUCTURAL ADDENDUM
51 BOURBON ST.
REV # REF PROJ # TE ENC: NBG PLAN DESIGNED UNDER

2018 NORTH CAROLINA RESIDENTIAL CODE

NOTES:

-HEIGHT AND BACKFILL LIMITATIONS FOR
FOUNDATION WALLS ARE TO BE GOVERNED
BY THE NCSBC, LATEST EDITION.
REINFORCEMENT AND GROUTING SHALL BE
DETERMINED BY FINAL SITE CONDITIONS.

-BUILDER TO FIELD LOCATE CRAWLSPACE ACCESS OPENING WITH MINIMUM DIMENSIONS OF 18X24. DO NOT LOCATE ACCESS OPENING BELOW POINT LOADS FROM ABOVE WITHOUT ENGINEER APPROVAL.

FOUNDATION PLAN

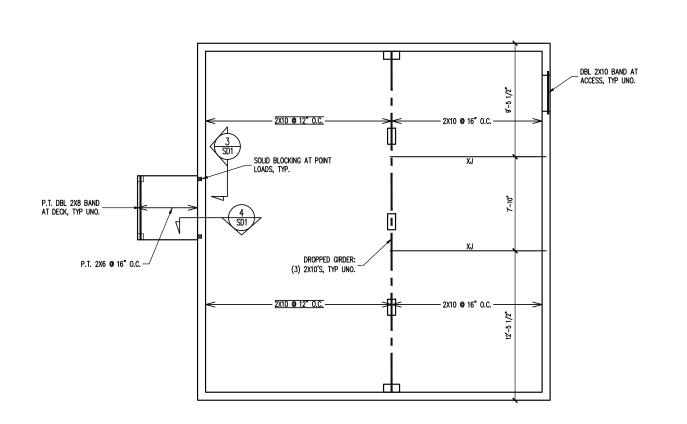
<u>1/8" = 1'-0"</u>

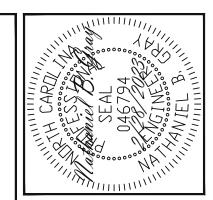
S1 1 of 6

DATE: 2/28/2023

PROJECT NO.

23-26-016 SHEET NO.





		REV # REF PROJ # DATE				
)MES	NDON	REV #				
VALUE BUILD HOMES	STRUCTURAL ADDENDUM	F1 ROLIBRON CT	SI BOOMBOIL SI:	FUOUAY VARINA, NC		
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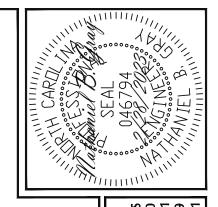
ENG: NBG DATE: 2/28/2023

PROJECT NO. 23-26-016

SHEET NO. **S2**

CRAWL SPACE FRAMING PLAN
1/8" = 1'-0"

2 of 6



STRUCTURAL E License N

VALUE BUILD HOMES
STRUCTURAL ADDENDUM
URBON ST. REV # REF PROJ #

51 BOURBON ST. FUQUAY VARINA, NC

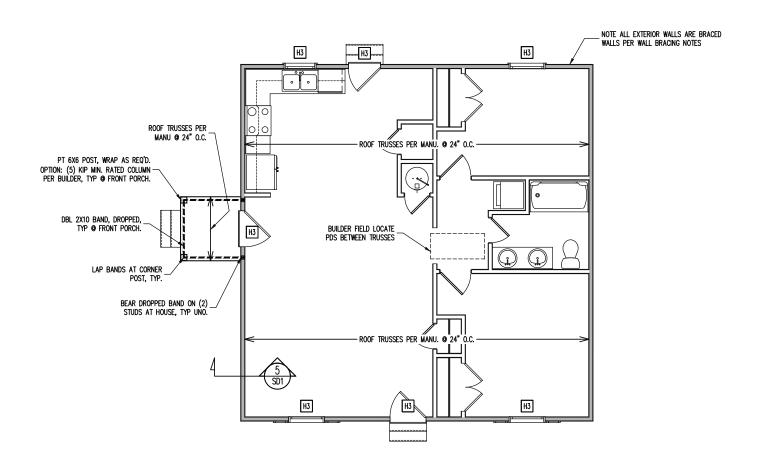
NBG DATE: 2/28/2023

PROJECT NO.

23-26-016 SHEET NO.

S3

318 W Mille Raleigh, Nor



CONSTRUCTION SPECIFICATIONS INSTANT REFERENCES

REFER TO THE CONSTRUCTION SPECIFICATIONS SECTIONS FOR THE FOLLOWING INFORMATION:

PART 1.01: CURRENT GOVERNING CODE

PART 14: STUD SUPPORT FOR BEAMS

PART 17: KING STUDS FOR EXTERIOR WALLS

SEE DETAIL / CONSTRUCTION SPECIFICATIONS SHEETS FOR I-JOISTS ALLOWABLE SUBSTITUTIONS

HEADER SCHEDULE

- H1 SINGLE 2X4 TURNED FLAT (A)
- H2 (2) 2X4'S ON SINGLE JACKS (B)
- H3 (2) 2X10'S ON SINGLE JACKS (C)
- H4 (2) 1.75" X 9.25" LVL'S ON DBL JACKS
- H5 (3) 2X10'S ON SINGLE 2X6 JACKS
- H6 (3) 1.75" X 9.25" LVL'S ON DBL 2X6 JACKS
- (A) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPENING 38" MAX.
- (B) TYPICAL FOR INTERIOR NON LOAD BEARING WALLS ONLY, ROUGH OPNG 38" TO 74" MAX.
- (C) TYPICAL FOR ALL CONDITIONS NOT LISTED IN (A) OR (B) UNO.

-HEADERS IN NON LOAD BEARING INTERIOR
WALLS ARE NOT LABELED.

WALL BRACING

SHADED WALLS:

ALL EXTERIOR STUD WALLS, EXTERIOR SIDE, ARE TO BE CONTINUOUSLY SHEATHED WITH 7/16 APA RATED OSB NAILED TO STUDS WITH 8d NAILS @ 6" O.C. AT PANEL EDGES, 12" O.C. IN PANEL FIELD.

PROVIDED CONTINUOUS SHEATHING = 116' MIN.

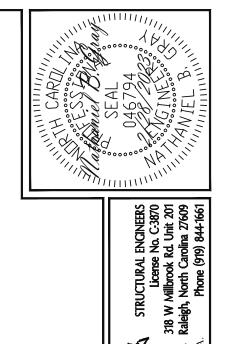
REFERENCE PART 16.02 OF CONSTRUCTION SPECIFICATIONS FOR GENERAL WIND BRACING INFORMATION.

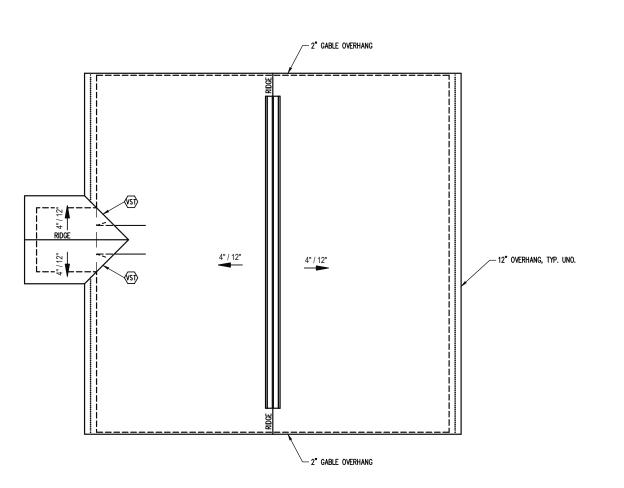
1ST FLOOR FRAMING PLAN

WALLS AND CEILING $1/8^{\circ} = 1'-0^{\circ}$

3 of 6

ENG:





TRUSS UPLIFT CONNECTORS EXPOSURE B, 115 MPH, ANY PITCH

24" O.C. MAX ROOF TRUSS SPACING

TRUSSES SHALL BE ATTACHED TO SUPPORT WALL FOR UPLIFT RESISTANCE. CONTINUOUS OSB WALL SHEATHING BELOW PROVIDES CONTINUOUS UPLIFT RESISTANCE TO FOUNDATION. ALL TRUSSES SUPPORTED BY INTERMEDIATE SUPPORT WALLS, KNEEWALLS OR BEAMS SHALL BE ATTACHED TO SUPPORTING MEMBER PER SCHEDULE BELOW.

ROOF SPAN IS MEASURED HORIZONTALLY BETWEEN FURTHEST SUPPORT POINTS.

CONNECTOR NAILING PER TABLE 602.3(1) NCRBC 2018 EDITION

(1) SIMPSON H2.5A HURRICANE CLIP TO DBL TOP PLATE OR BEAM

FRAMING NOTES

ROOF ONLY

-ROOF TRUSSES PER MANU. TYPICAL U.N.O.
-VERIFY ALL HEEL HEIGHTS, ROOF PITCHES,
AND ARCHITECTURAL OVERHANGS PRIOR TO
CONSTRUCTION

FRAMING SCHEDULE ROOF ONLY

VST VALLEY SET TRUSSES PER MANU

ROOF FRAMING PLAN

<u>1/8" = 1'-0"</u>

4 of 6

ENC:

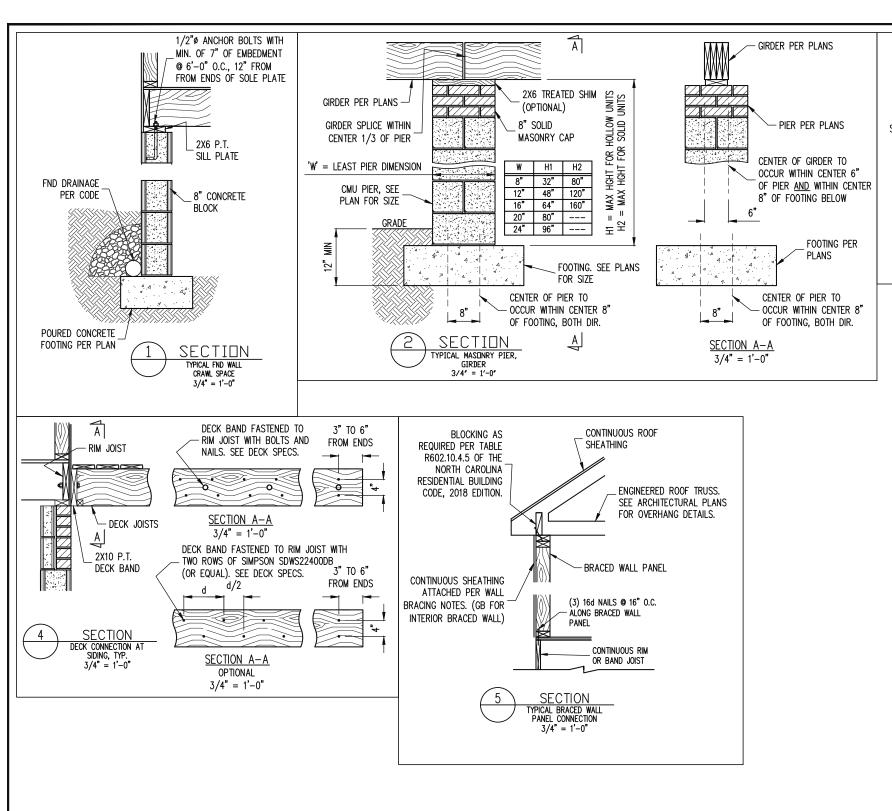
NBG DATE: 2/28/2023

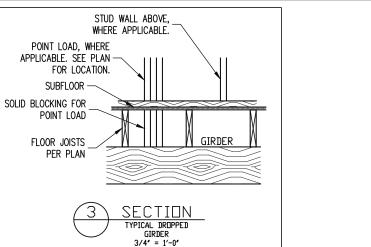
PROJECT NO.

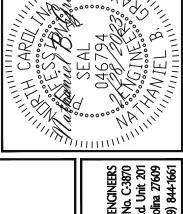
23-26-016

SHEET NO. **S4**

VALUE BUILD HOMES
STRUCTURAL ADDENDUM
51 BOURBON ST.
REV # REF PROJ # I







STRUCTURAL ENGINEE	License No. C-387	318 W Millbrook Rd. Unit 20	Raleigh, North Carolina 2760	Phone (919) 844-16
Ingineering			√ech Rale	ASSOCIATES, P.A.
	DATE			

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VALUE BUILD HOMES	STRUCTURAL ADDENDUM	TY NOR BOILD ST		FUOUAY VARINA, NC		
	SCOPE	 	3			
EN		N	BC	;		

DATE: 2/28/2023

PROJECT NO. 23-26-016

SHEET NO.

SD1

5 of 6

NOTES

THE BUILDER IS RESPONSIBLE FOR REVIEWING PLANS PRIOR TO CONSTRUCTION. THE BUILDER SHALL IMMEDIATELY CONTACT THE ENGINEER OF RECORD (EOR) BEFORE PROCEEDING IF THE FOLLOWING CONDITIONS ARE NOTED BEFORE OR DURING CONSTRUCTION:

any errors due to a failure to follow the above procedures shall not be the responsibility of the eor. Furthermore, it is the responsibility of the builder to ensure than any revisions issued by the eor are promply distributed to the

ABBREVIATIONS

FND	FOUNDATION	TJ	TRIPLE
FTG	FOOTING	TYP	TYPIC/
HDG	HOT DIPPED	TRPL	TRIPLE
	GALVANIZED	TSP	TRIPLE
100	HAMOED	1 11110	

XJ EXTRA JOIST

1 1/4" S4S

MAX POST HEIGHT

20'

E JOIST STUD POCKE

CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.

.02 DIMENSIONS SHOWN SHALL GOVERN OVER SCALE ON THESE DRAWINGS.

METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR, WHO SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND INSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.

PART 2: DESIGN LOADS

PART 1: GENERAL

2.01 DESIGN LOADS SHALL CONFORM WITH THE TABLE BELOW:

USE	LIVE LOAD (PSF)	DEAD LOAD (PSF)		PART 8: BULIS AND L
CONIES, DECKS, ATTICS WITH FIXED STAIR ESS, DWELLING UNITS INCLUDING ATTICS WITI D STAIR ACCESS, STAIRS, FIRE ESCAPES	H 40	10	8.01	BOLTS SHALL CONFORM STEEL WASHERS (ASTM MEMBERS

NED STAIN ACCESS, STAINS, TINE ESCAPES	70	10	i	
GARAGES (PASSENGER CARS ONLY)	50		8.02	LAG SCREWS SHALL SHALL BE USED F
ATTICS (NO STORAGE, LESS THAN 5' HEADROOM)	10	10	ı	NDS SPECIFICATION
ATTICS (WITH STORAGE)	20	10	ı	SCREW HEAD

Notes: — Individual stair treads are to be designed for the uniformly distributed Live Load of 40 psf or a 300 lb. Concentrated Load acting over an area of 4 SQ. Whichever produces the Greater Stress. — Builder to Verify Dead Load does not exceed 10 psf when heavy floor or roof finishes such as tile or slate are utilized. Notify engineering under these conditions.

ROOF

- 2.02 INTERIOR WALLS: 5 PSF LATERAL.
- 2.03 BASIC WIND DESIGN VELOCITY OF 120 MPH.
- 2.04 SOIL BEARING CAPACITY 2000 PSF (PRESUMPTIVE).

PART 3: STRUCTURAL STEEL

3.01 WIDE FLANGE BEAMS AND TEE SECTIONS SHALL CONFORM TO ASTM A992 MINIMUM GRADF

3.02 SQUARE AND RECTANGULAR TUBING SHALL CONFORM TO ASTM A500 GRADE B MINIMUM GRADE.

3.03 STEEL PIPE SHALL CONFORM TO ASTM A53 GRADE B, TYPE S, MINIMUM GRADE 3.04 ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 MINIMUM GRADE

THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED STRUCTURAL STEEL CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.

PART 4: WELDING

WELDING ELECTRODES SHALL BE E70XX AND ALL WELDING SHALL BE PERFORMED BY AN AWS CERTIFIED WELDER

PART 5: CONCRETE AND SLABS ON GRADE

- CAST IN PLACE CONCRETE SHALL BE OF NORMAL WEIGHT, 6% AIR ENTRAINMENT, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS TYP UNO. ALL CONCRETE, INCLUDING CONCRETE FOR FOOTINGS, IS TO BE CAST IN PLACE, TYP INIO.
- REINFORCED CAST IN PLACE CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACED I ACCORDANCE WITH THE SPECIFICATIONS OF ACI 318, LATEST EDITION.
- 5.03 SLABS ON GRADE, IF ANY, SHALL CONTAIN SYNTHETIC POLYPROPYLENE FIBRILLATED MICRO FIBERS, FIBER LENGTH 1 1/2", DOSAGE RATE 1 1/2 LBS/CU YD. SLAB TO BE PLACED ON A 6 MIL VAPOR BARRIER ON 4" MIN GRANULAR FILL ON SOIL WITH 90% MIN STANDARD PROCTOR DENSITY. VAPOR BARRIER MAY BE OMITTED FOR SLABS NOT IN ENCLOSED AREAS

PART 6: REBAR AND WIRE REINFORCEMENT

- 6.01 REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615 GRADE 60 TYP UNO
- 6.02 LAP SPLICES SHALL BE CLASS B AS DEFINED BY ACI 318, TYP UNO
- 6.03 WIRE REINFORCEMENT SHALL BE 9 GA AND SHALL CONFORM TO ASTM A1064

CONSTRUCTION SPECIFICATIONS

- CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND C55, NORMAL WEIGHT, $fM = 1.500 \ PSI \ MIN$
- .02 CLAY MASONRY UNITS SHALL CONFORM TO ASTM C62-17 GRADE SW
- 7.03 MORTAR SHALL BE TYPE S. MORTAR AND GROUT SHALL CONFORM TO ASTM C476, MIN COMPRESSIVE STRENGTH OF 2000 PSI.
- 7.04 MASONRY CONSTRUCTION SHALL CONFORM TO THE SPECIFICATIONS OF ACI 530
- LADDER WIRE REINFORCEMENT SHALL CONFORM TO ASTM A951. 6" MIN LAPS FOR CONTINUOUS WALL APPLICATIONS

PART 8: BOLTS AND LAG SCREWS

- ORM TO ASTM A307 MINIMUM GRADE TYP UNO. INSTALL STANDARD TM F844-07a) FOR THE NUT / BOLT HEAD WHEN BOLTING WOOD
- IALL CONFORM TO ANSI/ASME STANDARD B18.2.1-1981. PILOT HOLES FOR LAG SCREW INSTALLATION AND SHALL BE BORED ACCORDING TO TONS. INSTALL STANDARD STEEL WASHERS (ASTM F844-070) FOR
- 20 10 (15 FOR VAULTS) 8.03 ANCHOR RODS AND BOLTS SHALL CONFORM TO ASTM F1554—15 GRADE 36 UNO. BENT ANCHOR BOLTS SHALL HAVE A 2" MIN HOOK UNO

PART 9: DRIVEN FASTENERS

NAILS, SPIKES AND STAPLES SHALL CONFORM TO ASTM F 1667-05. NAILS ARE TO BE COMMON WIRE OR BOX

PART 10: DIMENSIONAL LUMBER

SOLID SAWN WOOD FRAMING DESIGN IS BASED ON NO. 2 SPRUCE PINE FIR \underline{OR} SYP #2 FOR JOISTS, RAFTERS, GIRDERS, BEAMS, STUDS, ETC.

PART 11: ENGINEERED LUMBER

- LVL OR PSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS: E= 1.9 X 10E6 PSI, Fb = 2600 PSI, Fv = 285 PSI, Fc = 750 PSI LSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS: E= 1.3 X 10E6 PSI, Fb = 1700 PSI, Fv = 400 PSI, Fc = 680 PSI
- LVL OR PSL MEMBERS MAY BE RIPPED FROM DEEPER MEMBERS TO MATCH THE MEMBER DEPTH SPECIFIED IN THE PLANS

PART 12: PRESSURE TREATED LUMBER

LUMBER IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA STANDARD C-15. ALL OTHER EXPOSED LUMBER SHALL BE IREATED IN ACCORDANCE WITH AWPA STANDARD C-2 OR BY ANY METHOD GIVING EQUAL PROTECTION. THE BUILDING CODE OFFICE MAY ALSO APPROVE A NATURAL DECAY RESISTANT WOOD PER SECTION 19-6(A)

PART 13: STEEL FLITCH PLATE BEAMS

FLITCH PLATE BEAMS SHALL CONSIST OF A CONTINUOUS STEEL PLATE BOLTED BETWEEN TWO PIECES OF CONTINUOUS LUMBER AS SIZED ON THE PLANS. BOLT PIECES TOCETHER USING 1/2" # BOLTS SPACED AT 24" O.C. STAGGERED TO TO BOTTOM OF THE BEAM. MAINTAIN A 2" EDGE DISTANCE. PLACE TWO BOLTS, ONE ABOVE THE OTHER, 6" ± 2"

PART 14: STUD SUPPORTS FOR BEAMS

STEEL, ENGINEERED LUMBER, AND FLITCH PLATE BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:

- 1—WHEN THE BEAM IS PERPENDICULAR TO, OR SKEWED RELATIVE TO THE WALL, THE BEAM SHALL BEAR FULL WIDTH ON THE SUPPORTING WALL INDICATED AND SHALL BE SUPPORTED BY A MINIMUM OF THREE GANGED STUDS, OR A GANGED STUD COLUMN WITH A NUMBER OF STUDS SUCH THAT THE STUD COLUMN IS AT LEAST AS MIDE AS THE TRUE WIDTH OF THE BEAM BEING SUPPORTED, WHICHEVER IS GREATER, TYP UND, FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON THE BEAM
- THE BEAM

 2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR

 A MINIMUM OF 4 1/2" ONTO THE WALL AND BE SUPPORTED BY A TRPL STUD GANGED
- 4.02 DIMENSIONAL LUMBER BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:
- 1-when the beam is perpendicular to, or skewed relative to the wall, the beam shall bear $\frac{1}{2}$ for a continuous rim joist where applicable) and shall be supported by a

GANGED STUD COLUMN THE SAME WIDTH AS THE BEAM TYP UNO. (E.G. A TRIPLE 2X10 IS TO BE SUPPORTED BY (3) STUDS), FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON THE BEAM 2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 3" ONTO THE WALL AND BE SUPPORTED BY A DBL STUD GANGED COLUMN TYP UNO.

- 14.03 Extra joists bearing on a stud wall perpendicular to or skewed relative to the beam shall be supported by one additional stud.
- STUDS THAT ARE GANGED TO FORM A COLUMN SHALL HAVE ADJACENT STUDS WITHIN THE COLUMN NAILED TOGETHER WITH ONE ROW OF 10d NAILS AT 8" O.C. (TWO ROWS OF 10d NAILS AT 8" O.C. (TWO ROWS OF 10d NAILS OF 8" O.C. 3" APART, FOR 2XIS OF 2XIS STUDIS) ALL COLUMNS SHALL BE CONTINUOUS DOWN TO THE FOUNDATION OR OTHER PROPERTY DESIGNED STRUCTURAL LEBERT SLOT AS A BEAM. COLUMNS TRANSFERRING LOADS THROUGH FLOOR LEVELS SHALL BE SOLIDLY BLOCKED FOR THE FULL WIDTH. OF THE STUD COLUMN WITHIN THE CANTY FORMED BY THE FLOOR JOISTS.

PART 15: NAILING OF MULTI PLY WOOD BEAMS

- SOLID SAWN LUMBER JOISTS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM NAILED TOCETHER WITH THREE ROWS OF 104 NAILS 9 16" O.C. FOR 2240 OR LARGER, TWO ROWS OF 104 NAILS 9 16" O.C. FOR 226, ONE ROW OF 104 NAILS 9 16" O.C. FOR 226 OR SMALLER. STAGGER ROWS 5" MIN.
- LVL MEMBERS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM FASTENED TOGETHER PER MANUFACTURERS RECOMMENDATIONS, TYP

PART 16: WALL FRAMING AND BRACING

STUD WALLS SHALL CONSIST OF 224 STUDS SPACED AT 16" O.C. UNO. STUDS SHALL BE CONTINUOUS FROM SOLE PLATE AT FLOOR TO DOUBLE TOP PLATE AT THE CEILING ROOK ON INTERMEDIATE BANDS OR PLATES SHALL CAUSE DISCONTINUITIES IN A STUD WALL EXCEPT AS REQUIRED FOR DOOR OR WINDOW OPENINGS. THE KING STUDS FOR SUCH OPENINGS SHALL BE CONTINUOUS. THE UNIO. MAX ALLOWABLE WALL HEIGHTS FOR EXTERIOR STUD WALLS, WITH SOLE PLATE AND DIB. TOP PLATE AND 7/16" OSB EXTERIOR BRACING AND GOVE 724 / 2X6 PURIOS AT 8" HEIGHT (AND AT 16" HEIGHT FOR TALL WALLS), TYP UNO: 2X4 0 16" O.C.: 11"-11/2" 2X6 0 16" O.C.: 17"-0" DBL 2X4 0 16" O.C.: 13"-4" DBL 2X6 0 16" O.C.: 21"-0"

DEL ZAA ® 16 U.C.: 13 -4 DEL ZA6 ® 16 U.C.: 21 -0

2 FOR WALL BRACING THE FOLLOWING SHALL APPLY:

-BLOCKING AT UNSUPPORTED PANEL EDGES IS REQUIRED TYP UNO.

-WALL BRACING IS BY ENDIREMED DESIGN AND NOT PRESCRIPTIVE PER SECTION
602.10 OF THE 2018 NORC. CONTINUOUS SHEATHING HAS BEEN PROVIDED, ALONG
WITH ALTERNATIVE METHODS TO INSURE THE MINIMUM INTENT OF SECTION 602.10

OF THE 2018 NORC HAS BEEN MET AND EXCEEDED.

-BRACED WALL PANELS SHALL BE FASTENED IN ACCORDANCE WITH TABLE 602.3(1) TO
PROVIDE CONTINUOUS PANEL UPLIFT RESISTANCE AND COMPLIANCE WITH MORBC
R602.3-S AND R002.11 UNLESS NOTIED OTHERWISE ON STRUCTURAL PLANS.

-MAY SUBSTITUE WSP FOR 8

-SINGLE JOIST, CONTINUOUS RIM JOIST, OR BLOCKING OF EQUAL DEPTH IS REQUIRED
ABOVE AND BELOW ALL BRACED WALLS OF PLATE OF WALL TO TOP PLATE
WITH 161 OTE NAILS ® 0.16" O.C. NAIL SOLE PLATE OF BRACED WALL TO TOP PLATE
WITH 161 OTE NAILS ® 0.16" O.C. NAIL SOLE PLATE OF BRACED WALL TO BLOCKING
BELOW WITH (3) 164 NAILS ® 16" O.C. BLOCKING AT HORIZONTAL JOINTS IN BRACED
WALL LINES ONLY REQUIRED AT SHADED WALLS, UNO.

PART 17: KING STUDS

17.01 KING STUDS FOR OPENINGS IN EXTERIOR WALLS SHALL BE AS FOLLOWS:

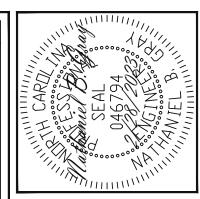
			r of Kin		
MAX OPENING WIDT	H 5'-0"	9'-0"	13'-0"	17'-0"	21'-0"
2X4	1	2	3	4	5
STUD SIZE 2X6	1	1	2	2	2
2X8	1	1	1	1	2

PART 18: SUBSTITUTIONS

MATERIAL OR MEMBER SIZE SUBSTITUTIONS OR PLAN DEVIATIONS REQUIRE THE WRITTEN AUTHORIZATION OF THE DESIGNERS, UNAUTHORIZED DEVIATIONS ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

PART 19: OWNERSHIP OF STRUCTURAL DESIGN

19.01 THE STRUCTURAL DESIGN OF THIS PLAN IS THE PROPERTY OF ENGINEERING TECH THE STRUCKED BY THE SE PLANS ARE FOR THE ONE THIS CHEET THE PROPERTY OF ENGINEERING ICCH AND FOR THE CLIENT USITED. ETA ASSUMES NO LIABILITY FOR THESE PLANS IF THEY ARE REPRODUCED, IN WHOLE OR IN PART, FOR CONSTRUCTION AT ANY OTHER LOCATION WITHOUT WRITTEN PERMISSION FROM ETA.



STRUCTURAL ENGINEERS	License No. C-3870	318 W Millbrook Rd. Unit 201	Raleigh, North Carolina 27609	Phone (919) 844-1661
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VALUE BUILD HOMES	SCOPE STRUCTURAL ADDENDUM	TO NOR BOLLBRON ST		FUOUAY VARINA. NC		
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ENC: NBG DATE: 2/28/2023

SPEC

6 of 6

PROJECT NO. 23-26-016 SHEET NO.

B. BOTH B.E. BOTH ENDS RTWN RETWEEN) THE WORKING PLANS DO NOT BEAR THE SEAL OF THE EOR CAST IN PLACE CONCRETE 2) THE PLANS CONTAIN DISCREPANT OR INCOMPLETE INFORMATION LVL LAMINATED VENEER CS CONTINUOUS SHEATHING DIA DIAMETER LUMBER NTS NOT TO SCALE DBL DOUBLE
DJ DOUBLE JOIST
DSP DBL STUD POCKET O.C. ON CENTER PARALLEL STRAND EQ EQUAL EA EACH FLG FLANGE PT PRESSURE TREATED THE EOR DOES NOT PERFORM FENESTRATION OR VENTING CALCULATIONS OR ANY OTHER CALCULATIONS THAT ARE NOT DIRECTLY RELATED TO STRUCTURAL ENGINEERING. QJ QUAD JOIST SP STUD POCKET
SQ SQUARE ROOF AND FLOOR TRUSSES TO BE DESIGNED BY AN ENGINEER REGISTERED BY THE STATE. FINAL FLOOR TRUSS DRAWING SHOULD BE SUBMITTED TO THE EOR FOR REVIEW DECK SPECIFICATIONS A DECK IS AN EXPOSED EXTERIOR WOOD FLOOR STRUCTURE WHICH MAY BE ATTACHED TO JOIST SPAN DECKING 12" O.C. 16" O.C. 1" S4S 1" T&G

24" O.C.

32" O.C.

POST SIZE

6X6

ENGINEERED

MAXIMUM HEIGHT OF DECK SUPPORT POSTS IS AS FOLLOWS:

NOTES: 1) THIS TARLE IS RASED ON NO. 2 TREATED SOLITHERN PINE POSTS.

3) POST HEIGHT IS FROM TOP OF FOOTING TO BOTTOM OF GIRDER.

DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THE FOLLOWING

R AXA WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLLIMN IN BOTH DIRECTIONS.

C. FOR FREE STANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL

POST SIZE TRIBUT. AREA POST HEIGHT EMB. DEPTH CONC. DIAM.

4'-0" 6'-0"

D. 2X6 DIAGONAL VERTICAL CROSS BRACING SHALL BE PROVIDED IN TWO PERPENDICULAR

THE POSTS WITH ONE - 5/8" Ø BOLT AT EACH END OF THE BRACE.

1) ALL NAILS AND BOLTS ARE TO BE HOT DIPPED GALVANIZED. 2) MINIMUM EDGE DISTANCE FOR BOLTS IS 2 1/2"

DIRECTIONS FOR FREE STANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE BRACES SHALL BE ATTACHED TO

3) NAILS MUST PENETRATE THE SUPPORTING STRUCTURE BAND A MINIMUM OF 1 1/2".

STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN CONCRETE IN ACCORDANCE WITH THE FOLLOWING:

THE ENDS TO THE GIRDER AND THE POST WITH ONE - 5/8" BOLT

2) THIS TABLE IS BASED ON A MAXIMUM TRIBUTARY AREA OF 128 SQ. FT.

when the Deck Floor height is less than $4-0^\circ$ and the Deck is attached to the structure in accordance with Section 4, Lateral Bracing is not required.

BETWEEN 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE ATTACHED AT

- A STRUCTURE OR BE FREE STANDING. ROOFED PORCHES, OPEN OR SCREENED IN, MAY BE CONSTRUCTED USING THESE PROVISIONS.
- SUPPORT POSTS SHALL BE SUPPORTED BY A FOOTING.
- WHEN ATTACHED TO A STRUCTURE. THE STRUCTURE TO WHICH ATTACHED SHALL HAVE A TREATED WOOD BAND FOR THE LENGTH OF THE DECK, OR CORROSION RESISTANT FLASHING REALIZE WOOD BAND YOU HE LENGTH OF THE DECK, OF CONTROL RESISTANT PLASHAND
 SHALL BE USED TO PREVENT MOSTURE FROM COMING IN CONTACT WITH THE UNTREATED
 FRAMING OF THE STRUCTURE. THE DECK BAND AND THE STRUCTURE BAND SHALL BE
 CONSTRUCTED IN CONTACT WITH EACH OTHER EXCEPT AT BRICK VENEER AND WHERE PLYWOOD SHEATHING IS REQUIRED AND PROPERLY FLASHED. SIDING SHALL NOT BE INSTALLED BETWEEN THE STRUCTURE AND THE DECK BAND. IF ATTACHED TO A BRICK STRUCTURE, NEITHER FLASHING NOR A TREATED BAND FOR THE BRICK STRUCTURE IS REQUIRED. IN ADDITION, THE TREATED DECK BAND SHALL BE CONSTRUCTED IN CONTACT
- WHEN THE DECK IS SUPPORTED AT THE STRUCTURE BY ATTACHING THE DECK TO THE STRUCTURE, THE FOLLOWING ATTACHMENT SCHEDULES SHALL APPLY FOR ATTACHING THE DECK BAND TO THE STRUCTURE:
- A. ALL STRUCTURES EXCEPT BRICK STRUCTURES

l		JOIST LENGTH			
l		UP TO 8' MAX.	UP TO 16' MAX.		
	REQUIRED FASTENERS	(2) ROWS OF 12d NAILS @ 8" O.C. OR	ONE- 5/8" Ø BOLT @ 20" O.C. AND (3) ROWS OF 12d NAILS @ 6" O.C. OR TWO ROWS OF SIMPSON SDWS22400DB @ d = 16" O.C. STAGGERED		

A . BRICK VENEER STRUCTURES

	JOIST LENGTH	
	UP TO 8' MAX.	UP TO 16' MAX.
REQUIRED FASTENERS		ONE- 5/8" Ø BOLT @ 16" O.C.

- IF THE DECK BAND IS SUPPORTED BY A 1/2" MINIMUM MASONRY LEDGE ALONG THE
- OTHER MEANS OF SUPPORT, SUCH AS JOIST HANGERS, MAY BE USED TO CONNECT DECK

- FOUNDATION WALL, 5/8" Ø BOLTS SPACED @ 48" O.C. MAY BE USED FOR SUPPORT.
- JOISTS TO A TREATED STRUCTURE BAND GIRDERS SHALL BEAR DIRECTLY ON POSTS OR BE BE CONNECTED TO THE SIDES OF POSTS
- FLOOR DECKING SHALL BE NO. 2 GRADE TREATED SOUTHERN PINE OR EQUIVALENT. THE MINIMUM FLOOR DECKING THICKNESS SHALL BE AS FOLLOWS: