MEAN ROOF HGT.

28'-2"

18'-6"

÷2

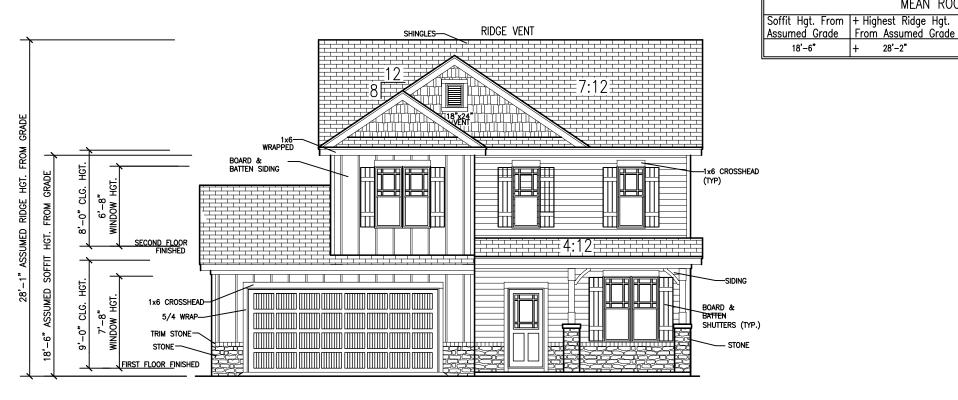
÷2

= Mean Roof Hgt.

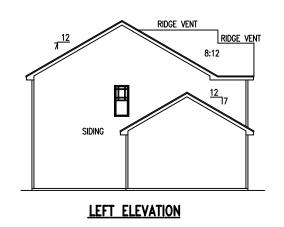
= 23'-4" Mean Roof Hgt.

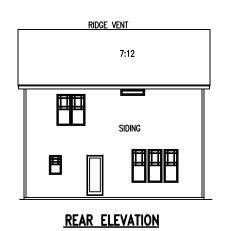
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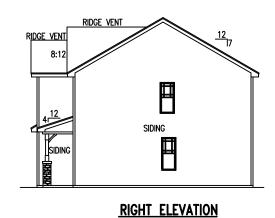




FRONT ELEVATION "C"







PROJECT #: DRB2201-0065B

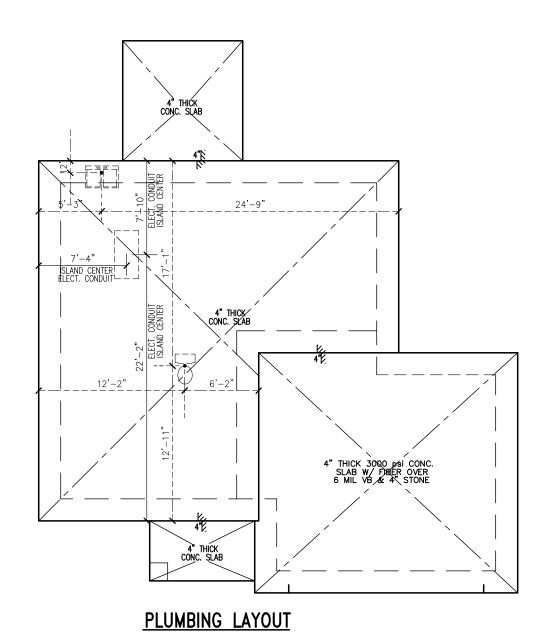
DATE: 07/13/2022

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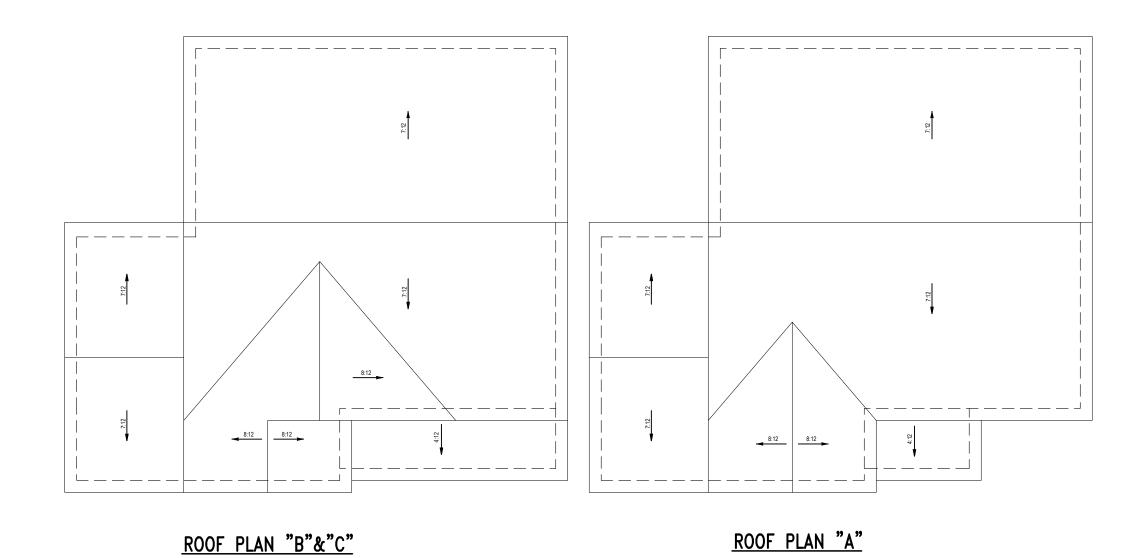
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PLUMBING





ROOF PLAN



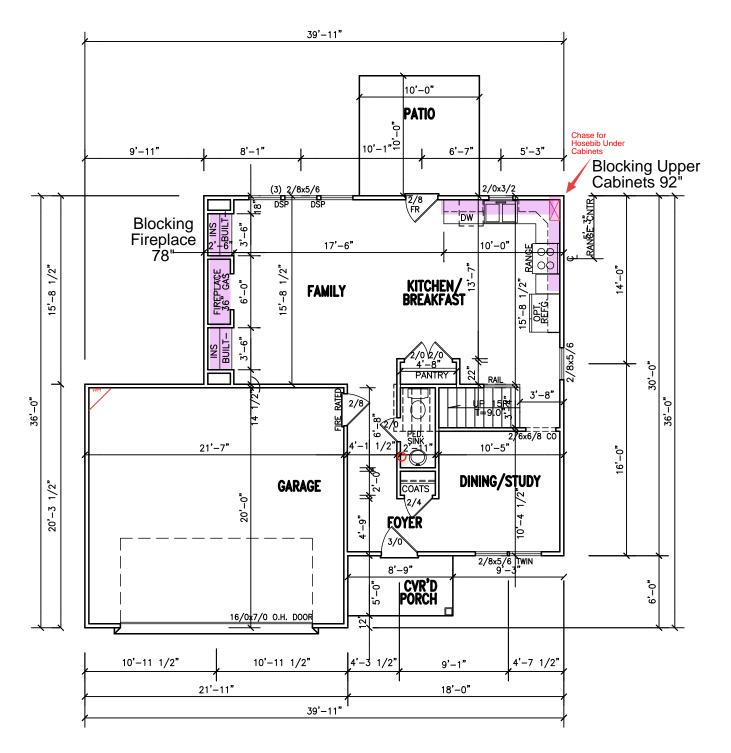


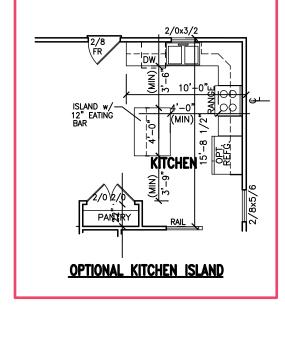
DATE: 07/13/2022 DRAWN BY:

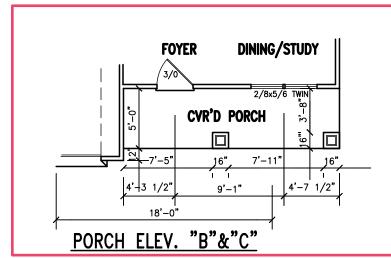
CHECKED BY:











SQUARE FOOTAGE	
1ST FLOOR HTD. 2ND FLOOR HTD.	737 942
FLOOR HTD.	1679
GARAGE PATIO ELV "A" PORCH ELV "B" PORCH ELV "C" PORCH	437 100 44 90 90
1ST FLOOR PLAN NOTE	<u>۰</u> .

1ST FLOOR PLAN NOTES:	
1) 9'-0" CLG. HGT. (9'-1 1/2" PLT. HGT.) UNLESS OTHERWISE NOTED.	
 ALL EXTERIOR WALLS FIGURED AT 4"NOMINA WIDTHS UNLESS OTHERWISE NOTED. 	U
 ALL INTERIOR WALLS FIGURED AT 3 1/2" WIDTHS UNLESS OTHERWISE NOTED. 	
4) SET WINDOWS AT 7'-8" A.F.F. UNLESS OTHERWISE NOTED.	
5) DIMENSIONS ARE TO FRAMING UNLESS OTHERWISE NOTED.	

FIRST FLOOR PLAN





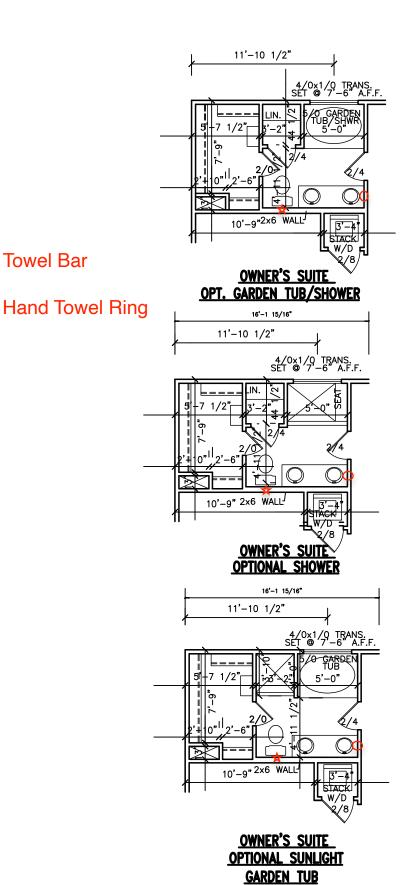
PROJECT #: DRB2201-0065B

DATE: 07/13/2022

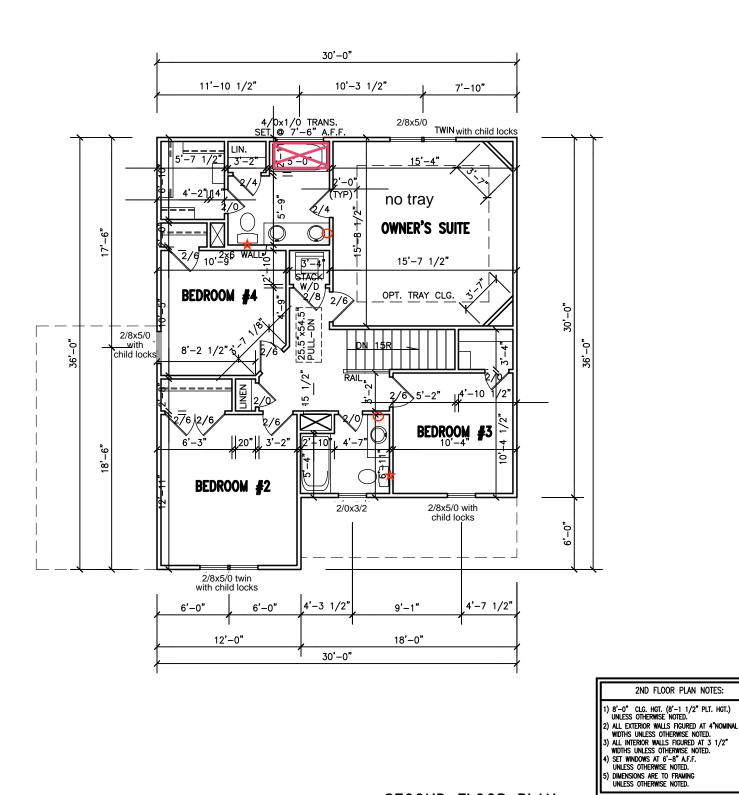
DRAWN BY: CHECKED BY:

FLOOR PLAN





Towel Bar



SECOND FLOOR PLAN

2ND FLOOR PLAN NOTES:

4TH BEDROOM OPTION

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION
	(/	(/	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

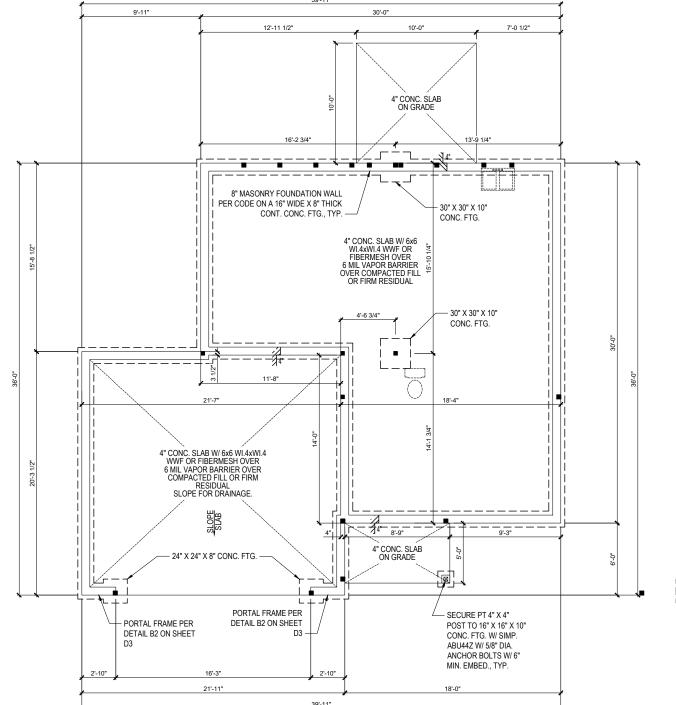
- STRUCTURAL NOTES:

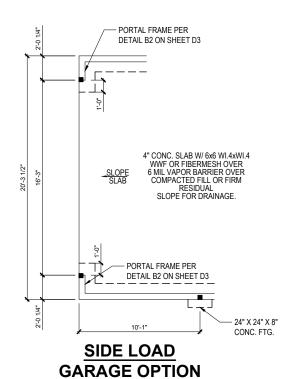
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
 OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN
- ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS IT IS THE CONTINCE TO THE STORY OF THE CONTINUE OF THE CONTIN
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- Fb = 2600 PSI, E = 1.9M PSI
- (IE. ILEVEL MICROLAM)
 ALL LISL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
 ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R802.7.5. AND
- (1) 2x4 JACK S I UD (UN.O.) AND KING S I UDUS PER I ABLE. R602.7.5, AND TOGETHER N.C. [2] 10d NAILS @ 6" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6"5", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1"5". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFOR TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION
- OF ALL WALLS OVER 10'-0" IN HEIGHT.

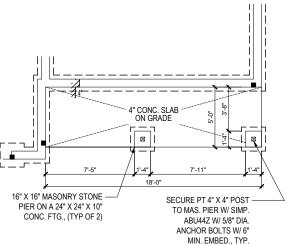
- OF ALL WALLS OVER 10-0" IN HEIGHT.
 ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
 Fy = 50 KSI MIN. (LINO)
 ALL EXTERIOR LUMBER TO BE #2 SVP PT
 ALL CONCRETE, 10 = 3000 PSI MIN.
 PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/2"O ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE
 THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS
 PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3"-0" O.C.
 FOR BASEMENTS. ANCHOR BOLT SHALL BE STACED AT 3"-0" O.C.
 FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
 MASONRY. MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS (U.N.O.) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- INC.

 MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.







PORCH ELEV. A

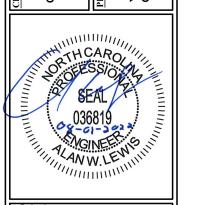
PORCH ELEV. B & C

FOUNDATION PLAN (STEM WALL)

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering





Project #: DRB2201-0065D
<u>Date:</u> 08/01/22
Engineered By: AM
DWG. Checked By: AWL
Scale: SEE PLAN
REVISIONS

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No.	Date:	Remarks
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Sheet Number

S₁B

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION
	(/	(/	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

- STRUCTURAL NOTES:
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
 OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN
- ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS. IT IS THE CONTINCE TO THE STORY OF THE CONTINUE OF THE CONTIN
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- Fb = 2600 PSI, E = 1.9M PSI
- (IE. ILEVEL MICROLAM)
 ALL LISL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
 ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R802.7.5. AND
- (1) 243 JACKS 100 (JANUC) AND KINGS 21 IOSP SEY READER REQUEZY, 3AND TOGETHER W. (2) 100 MAILS & 8° O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6°5', MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1°5', OTHERWISE REFER TO TABLES REGOZ! (A) AND REGOZ! (2) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFO. TO TABLES REGOZ! (7) BAND REGOZ! (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (1) AND STATEMENT OF THE STATEMENT OF TH
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- OF ALL WALLS OVER 10-7 IN HEIGHT.

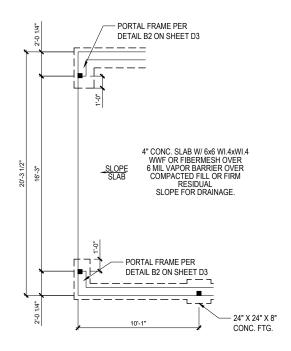
 ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
 Fy = 50 KSI MIN. (UNO)
 ALL EXTERIOR LUMBER TO BE #2 SYP PT
 ALL CONCRETE, fc = 3000 PSI MIN.
 PRESUMPTIVE BEARING CAPACITY = 2000 PSF

- 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE 112 D'ANCHOR BOLTS SPACED IN IMMANIMUM 07-0 0 CL. AND NOT MOVE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM 0F (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3:0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MACANIMUM.
- MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- INC.

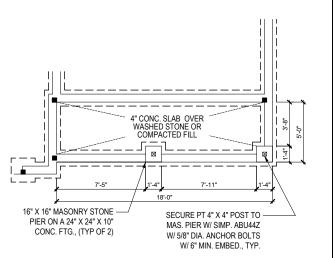
 MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

30'-0" 12'-11 1/2" 7'-0 1/2" - PT 6" X 6" POST ON 16" X 16" X 8' CONC. FTG.. TYP. @ DECK 8" MASONRY FOUNDATION WALL PER CODE ON A 16" WIDE X 8" THICK CONT. CONC. FTG., TYP. - 30" X 30" X 10" CONC. FTG. FIELD LOCATE PILASTER ON A 16" X 16" MASONRY PIER 22" X 30" X 10" CONC. ON A 30" X 30" X 10" FTG., TYP. UNO -CONC. FTG., TYP. UNO DROPPED 211 4" CONC. SLAB W/ 6x6 WI.4xWI.4 WWF OR FIBERMESH OVER
6 MIL VAPOR BARRIER OVER
COMPACTED FILL OR FIRM SLOPE FOR DRAINAGE. 4" CONC. SLAB OVER WASHED STONE OR COMPACTED FILL - 24" X 24" X 8" CONC. FTG. PORTAL FRAME PER PORTAL FRAME PER DETAIL B2 ON SHEET - SECURE PT 4" X 4" POST TO DETAIL B2 ON SHEET D3 -FND WALL W/ SIMP. ABU44Z W/ 5/8" DIA. ANCHOR BOLTS 2'-10" 21'-11"



SIDE LOAD GARAGE OPTION



PORCH ELEV. B & C

660 SOLETIOF CRAWL SPACE / 150 = 4.4 SOLETIOF RED'D VENTILATION WITHOUT CROSS VENTILATION 4.4 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 5 VENTS REQ'D (BASED ON 8" X 16" VENTS)

660 SQ. FT. OF CRAWL SPACE / 1500 = 0.44 SQ. FT. OF REQ'D VENTILATION WITH CROSS VENTILATION 0.44 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 1 VENTS REQ'D (BASED ON 8" X 16" VENTS)

- THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 11500 OF THE CRAWL SPACE. GROUND AREA WHERE THE REQUIRED OPENINGS ME PLACED SO AS TO REVOICE CROSS VENTILATE OF THE COMES. SHORT THE RESTALLATION OF OPENINGEL CONTRES SHALL ROTE REPORTING THE COMES OF THE PROPERTY OF OPENING SHALL TO AN EXPORT SHALL THE PROPERTY PARKING THE RITHER YMEN'THE CRAWL SHALL SHALL TO AN SUPER SHELL THE LIPINEL FOUNDATION WALLS MAY BE COST RECTED WITHOUT WALL SHALL TO SHALL SHALL THE PROPERTY AND ALL SHALL SHALL

FOUNDATION PLAN (CRAWL SPACE)

PORCH ELEV. A

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engi





REVISIONS	
Scale: SEE PLAN	
DWG. Checked By: AWL	
Engineered By: AM	
Date: 08/01/22	
Project #: DRB2201-0065D	

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DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	ECTION
	(/	(/	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

- STRUCTURAL NOTES:

 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
 OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN
- ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS. IT IS THE CONTINCE TO THE STORY OF THE CONTINUE OF THE CONTIN
- ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND

- ALL LVI. LUMBER 10 BE 1.75' WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2800 PSI, E = 1 9M PSI

 (I.E. ILEVEL MICROLAM)
 ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)

 ALL LODB BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w

 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER WI (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER
- TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO)
 ALL EXTERIOR LUMBER TO BE #2 SYP PT
 ALL CONCRETE, f6 = 3000 PSI MIN.

- PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 17:29 ANCHOR BOLTS SPACED AT MAXIMM OF 6*0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS FER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3"0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
- MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLUMNS (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

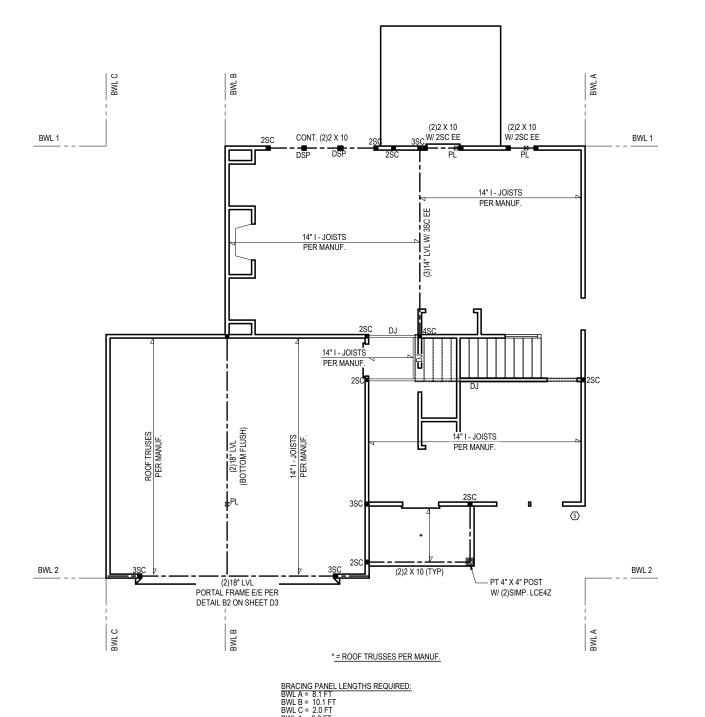
 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION.
- METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

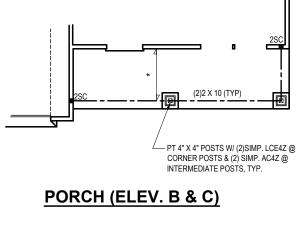
STRUCTURAL SHEATHING NOTES

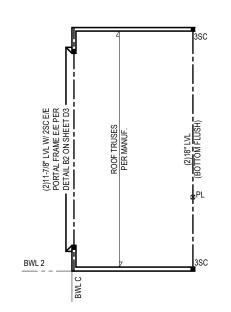
- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- LESS.

 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- 1 REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC
- INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2) 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8"-0" (ISOLATED PANELS) OR 4"-0" (CONTINUOUS SHEATHING). SECURE W 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACEO @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION
- ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ALL SPEATMABLE SYMPACES OF EATHOUR WHALE SWICEDIMIN AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8°. SHEATHING SHALL BE SECURED WITH MINIMUM GO COMMON NAUS SPACED AT 6° O.C. AT
- PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
 MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS:
 - S FOLLOWS:

 24" ADJACENT TO OPENINGS NOT MORE THAN
 67% OF WALL HEIGHT
 30" ADJACENT TO OPENINGS GREATER THAN
 67% AND LESS THAN 85% OF WALL HEIGHT.
 48" FOR OPENINGS GREATER THAN 85% OF
- WALL HEIGHT 4 SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602 10.3(4). IN LIFU OF A CORNER RETURN. ACCORDANCE WITH FIGURE R802.10 3.(4). IN LIEU OF A CORNER RETURE EITHER A MIN. 48° BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 8009 SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- (5) MINIMUM 800# HOLD-DOWN DEVICE







SIDE LOAD GARAGE OPTION

BRACING PANEL LENGTHS PROVIDED: BWL C = 6.44 FT PF BWL 2 = 33.5 FT CS-WSP

BRACING PANEL LENGTHS PROVIDED: BWL A = 27.33 FT CS-WSP BWL B = 15.71 FT CS-WSP BWL C = 20.29 FT CS-WSP BWL 1 = 16.67 FT CS-WSP

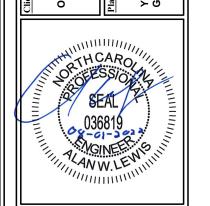
FIRST FLOOR PLAN

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

Engineers seal does not include con procedures or safety precau 'Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A.

Failure to do so will void Tyndall Engin Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendetc. presented in these documents were





	ate: 8/01/22				
	ngineered By: AM				
A	WG. Checked By: AWL				
-	<u>cale:</u> EE PLAN				
	REVISIONS				
No.	Date:	Remarks			

Sheet Number

(2)2 X 10 (2)5-1/2" LVL BWL A BWL A GIRDER TRUSS PER MANUF. **ROOF TRUSSES** PER MANUF. RWI R BWL B BRACING PANEL LENGTHS REQUIRED: BWL A = 3.2 FT BWL B = 3.2 FT BWL 1 = 3.8 FT BRACING PANEL LENGTHS PROVIDED: BWL A = 30.0 FT BWL B = 33.33 FT BWL 1 = 20.5 FT BWL 2 = 20.5 FT

SECOND FLOOR PLAN

1/8" = 1'-0" (11"X17")

1/4" = 1'-0" (24"X36")

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION
	(/	(/	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BAS	SED ON 120 MPH (E	XPOSURE B)	
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

- STRUCTURAL NOTES:
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
 OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN
- OF YOKEN CAROLINA SEATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYMPALL BUILDING SENSIONS AND SQUARE FOOTAGE PRIORS TO NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
 ALL LIMBER COLAID SECREM AND ADDITIONAL TO THE PROPERTY OF THE PRINCIPLE OF THE PRIN
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND

- ALL LVL LUMBER 10 be 1.7.6" WIDE NOMINAL EACH SINGLE MEMBER AND FD = 2800 PSI, E = 19 M PSI

 (I.E. ILEVEL MICROLAM)

 ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)

 ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R802.7.5, AND
- (1) 224 JACK SI LID (LIN.O.) AND KING SI LIDS PER I RABLE RABZE 7.5, AND TOGETHER W. (2) 10d ANLIS & 6" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6"5", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1"5". OTHERWISE REFER TO TABLES R602, 7(1) AND R602, 7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2-10 (LIV.O.) REFER TO TABLES R602, 7(1) AND R602, 7(2) FOR LACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS INVOL
- (NO)
 REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION
 OF ALL WALLS OVER 10'-0' IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
 Fy = 50 KSI MIN. (UNO)
 ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, fc = 3000 PSI MIN.
 PRESUMPTIVE BEARING CAPACITY = 2000 PSF
- 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE 1/2"O ANCHOR BOLTS SPACED AT MAXIMUM OF 6:0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3:0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9:0" (UNO) PROVIDE A MINIMUM OF 500H UPILIT'S A LATERAL CONNECTION AT TOP AND ROTTOM OF EMPIREY COLUMNS. (MIN.)
- AND BOTTOM OF PORCH COLLIMNS (U.N.O.)
- AND BOTTOM OF PORCH COLUMNS, (U.N.O.)

 PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC.

 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION. 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL

STRUCTURAL SHEATHING NOTES

- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
 WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3.
 REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- 1 REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8"-0"
 (ISOLATED PANELS) OR 4"-0" (CONTINUOUS SHEATHING).
 SECURE W 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5)
 SPACED (@ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND
 BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- (3) 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- R602-10.3 (UNO)

 ALS SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 64 COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.

 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE 18 OF LOWER.
- MINIMOM BRACED WALL PANEL LENGTHS WITH CS-WSP
 BE AS FOLLOWS:
 24" ADJACENT TO OPENINGS NOT MORE THAN
 67% OF WALL HEIGHT
 3" ADJACENT TO OPENINGS GREATER THAN
 67% AND LESS THAN 85% OF WALL HEIGHT.
 48" FOR OPENINGS GREATER THAN 85% OF
 WALL HEIGHT
- 4 SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, ACCORDANCE WITH FIGURE R802.10 3.(4). IN LIEU OF A CORNER RETURE EITHER A MIN. 48° BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800% SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- 5 MINIMUM 800# HOLD-DOWN DEVICE

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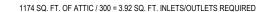
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- 1	
	Project #: DRB2201-0065D
1	Date: 08/01/22
1	Engineered By: AM
1	DWG. Checked By: AWL
	SEE PLAN
- 1	REVISIONS

Sheet Number

S3

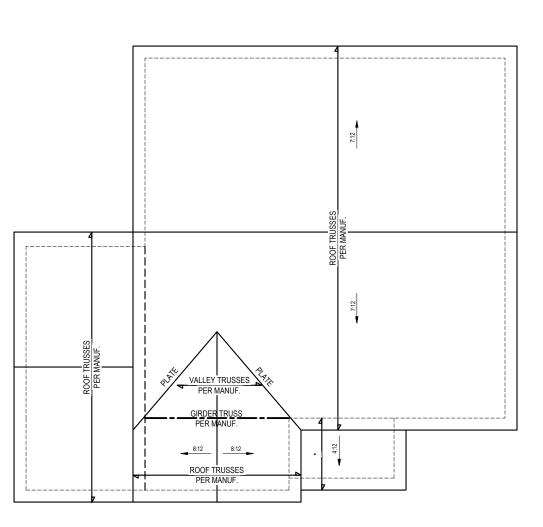


- CALCULATION BASED ON VENTILATORS USED AT LEAST 3'-0" ABOVE THE COMICE VENTS WITH THE BALANCE OF VENTILATION PROVIDED BY EAVE VENTS.
- CATHEDRAL CEILINGS SHALL HAVE A 1" MINIMUM CLEARANCE BETWEEN THE BOTTOM OF THE ROOF DECK AND THE INSULATION.



* ATTIC VENTILATION CALCULATION

VALLEY TRUSSES PER MANUF. GIRDER TRUSS PER MANUF. 8:12 ROOF TRUSSES PER MANUF.



* = ROOF TRUSSES PER MANUF

ROOF PLAN (ELEV. B & C)

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

ROOF PLAN (ELEV. A)

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

* Engineers seal does not include construction means, methods, techniques, sequences,

- means, methods, techniques, sequences, procedures or safety precaution.

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Project #: DRB2201-0065D Date: 08/01/22 Engineered By: Scale: SEE PLAN

REVISIONS

Sheet Number

S4

STRUCTURAL NOTES

ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE". IN ADDITION TO ALL LOCAL CODES AND REGULATIONS

2) DESIGN LOADS

LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION
(/	()		
	1	LL	TL
40	10	L/360	L/240
30	10	L/360	L/240
20	10	L/240	L/180
10	5	L/240	L/180
40	10	L/360	L/240
20	10	L/240	L/180
20	20	L/240	L/180
BASED ON 120 MPH (EXPOSURE B)			
SEISMIC ZONES A, B & C			
	30 20 10 40 20	30 10 20 10 10 10 5 5 40 10 20 10 20 20 BASED ON 120 MF	30 10 L/360 20 10 L/240 10 5 L/240 40 10 L/360 20 10 L/240 20 20 L/240 BASED ON 120 MPH (EXPOSURE B)

- MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE, (U.N.O.)
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT. WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON 2x10) UNO.
 ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (U.N.O.)
 ALL SL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2235 PSI, E = 1.6M PSI (U.N.O.)
 ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10. (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50.
- ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2") at **LONG, LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE.

WALL AND ROOF CLADDING VALUES:
WALL CADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE.
ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:

39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1.5/12

36.0 LBS/SQFT FOR ROOF PITCHES 1.5/12 TO 6/12 18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12

**MEAN ROOF HEIGHT 30'-0" OR LESS

- 13) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- 18) PSI COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION
- IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION.
 TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

CLIMAT ZONES		SKYLIGHT b U-FACTOR	GLAZED FENESTRATION SHGC ^{b,k}	CEILING [™] R-VALUE	WOOD FRAMED WALL R-VALUE	MASS WALL R-VALUE	FLOOR R-VALUE	BASEMENT ^{c,o} WALL R-VALUE	SLAB ^d R-VALUE AND DEPTH	CRAWL SPACE ^C WALL R-VALUE
3	0.35	0.55	0.30	38 or 30 cont	15 or 13 + 2.5 h	5/13 or 5/10 cont	19	<u>5/13</u> f	0	5/13
4	0.35	0.55	0.30	38 or 30 cont	15 or 13 + <u>2.5</u> h	5/13 or 5/10 cont	19	10/15	10	10/15
5	0.35	0.55	NR	38 or 30 cont	n 19, or 13 + 5 or 15 + 3	13/17 <u>or</u> 13/12.5 cont	30 g	10/15	10	10/19

TABLE N1102.1 CLIMATE ZONES 3-5

R-VALUES ARE MINIMUMS. U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS
OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE.

ASSIGNATION OF THE HOUSE SHARE OF REASTATON.

"UNIS MEANS AND CONTINUOUS BUILD AND READMEND OF THE WITEROO OF EXTERIOR OF THE HOME OF THE CHARLES AND ASSIGNATION SHALL BE APPLIED FROM THE INSPECTION OF DOMINIMAND TO THE BOTTOM OF THE POST OF AND AND ASSIGNATION SHALL BE APPLIED FROM THE INSPECTION OF DESTROYAND AND AND ASSIGNATION SHALL BE APPLIED FROM THE INSPECTION OF LISTS FOR THE CHARLES AND AND AND ASSIGNATION OF THE CHARLES AND ASSIGNATION OF THE POST OF THE POST

e. <u>Deleted</u> f. Basement wall insulation is not required in Warm-Humid Locations as defined by <u>Figure N1101.7</u> and <u>Table N1101.7</u>.

g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY. R-19 MINIMUM.

5. OR INSTALLAND SHY-EXH I DUTL THE PROMINE CANTE. YES WINDOW.

THE FRIST YALKE COANTY ROULDING THE SECOND VALUE SO CONTINUOUS RISULATION, SO "13-5" MEANS R-11 CANTY NIGULATION PULS R-8 INSLATED
SHEATHNG. "15-7" MEANS R-15 CANTY ROULDING THE STATE COANTY ROULDING THE COTTENS OF THE COTTENS

OR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WA

I FOR MASS WALLS, THE SECTION WALLE APPLES WHEN LINGS: THE MINISTER MASS WALLS. THE STREET HAND AS SHALL BE PROMOTED THE DESCRIPTION OF SECTION THE STREET HAND AS SHALL BE PROMOTED THE SECTION OF SE

OF THE ATTO-ROOF DEEX.

FINE THE REPORT OF THE ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF. THERE THE INSLIATION MUST FILL THE SPACE UP TO THE ARE BAFFLE.

8. THE REPORT ASS BATTS COMPRESSED AND INSTALLED IN AMOUNTA, 2.4 FRAMING CANTY IS DEEMED TO COMPLY. PREPORTASS BATTS RATTO R-19 OR HIGHER COMPRESSED

AND INSTALLED AN AZ MANUL IS NOT DEEMED TO COMPLY. M MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT

DEFINITIONS FOR COMMON ABBREVIATIONS

ALT	=	ALTERNATE	MAX	=	MAXIMUM
CANT	=	CANTILEVER	MIN	=	MINIMUM
CJ	=	CEILING JOIST	NOM	=	NOMINAL
CMU	=	CONCRETE MASONRY UNIT	O.C.	=	ON CENTER
COL	=	COLUMN	PL	=	POINT LOAD
CONC	=	CONCRETE	PT	=	PRESSURE TREATED
CONT	=	CONTINUOUS	REINF	=	REINFORCED
CT	=	COLLAR TIE	REQD	=	REQUIRED
DBL	=	DOUBLE	RJ	=	ROOF JOIST
DIA	=	DIAMETER	RS	=	ROOF SUPPORT
DJ	=	DOUBLE JOIST	SC	=	STUD COLUMN
DR	=	DOUBLE RAFTER	SCH	=	SCHEDULE
EA	=	EACH	SPEC	=	SPECIFIED
EE	=	EACH END	THK	=	THICK
FJ	=	FLOOR JOIST	TJ	=	TRIPLE JOIST
FND	=	FOUNDATION	TRTD	=	TREATED
FTG	=	FOOTING	TYP	=	TYPICAL
GALV	=	GALVANIZED	UNO	=	UNLESS NOTED OTHERWISE
HORIZ	=	HORIZONTAL	W	=	WIDE FLANGE BEAM
HT	=	HEIGHT	WWF	=	WELDED WIRE FABRIC
MANUF	=	MANUFACTURER	XJ	=	EXTRA JOIST

MAXIMUM HEIGHT OF DECK SUPPORT POSTS AS FOLLOWS:

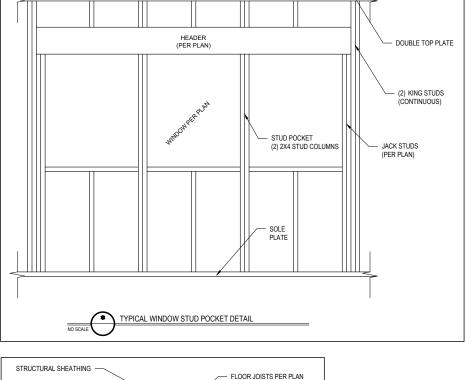
POST SIZE	MAX. POST HEIGHT**				
4 x 4	8'-0"				
6 x 6	20'-0"				
***	OVER 20'-0"				

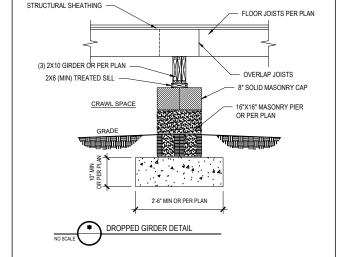
- THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS.

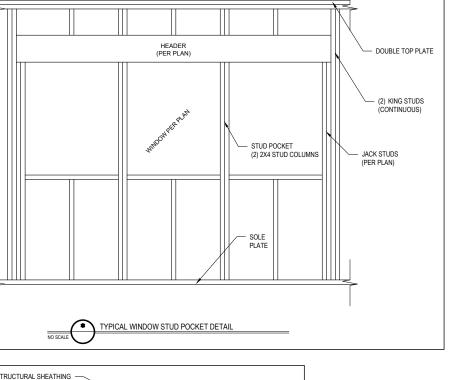
 MAXIMUM TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET
 WHICH MAY BE LOCATED AT DIFFERENT LEVELS.
- ROM TOP OF FOOTING TO BOTTOM OF GIRDER DECKS WITH POST HEIGHTS OVER 20'-0" SHALL BE DESIGNED AND
- SEALED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT.
- DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF
- THE DECK FLOOR HEIGHT IS LESS THAN 4-0" AND THE DECK IS
 ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION (4)
 ABOVE LATERAL BRACIGS IS NOT REQUIRED.
 4 x 4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN
 BOTO DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST
 AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL, KNEE BRACES SHALL BE BOLTED TO THE POST AND GIRDER WITH ONE 5/8"Ø HOT DIPPED GALVANIZED
- TO THE POST AND SIGNER WITH OWNERS OF HOT DIFFED GALVANIZED BOLT AT EACH END OF THE BRACE.
 REESTANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN ACCORDANCE WITH THE FOLLOWING:

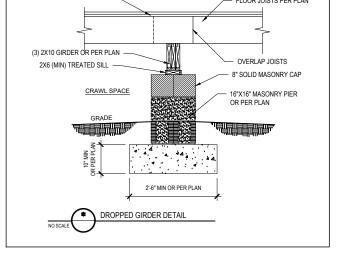
POST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER	
4 x 4	48 SQ. FT.	4'-0"	2'-6"	1'-0"	
6 x 6	120 SQ. FT.	6'-0"	3'-6"	1'-8"	

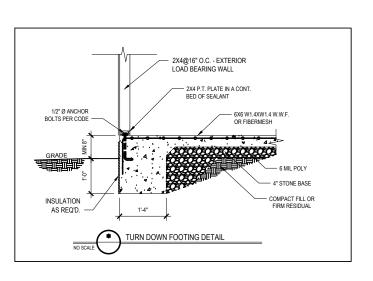
D. 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO 2x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO
 (2) PERPENDICULAR DIRECTIONS FOR FRESTANDING DECKS OR PARALLEL.
 TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS.
 THE 2x 6s SHALL BE ATTACHED TO THE POSTS WITH ONE 50% DIOT
 DIPPED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER.
 FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.











Engineers seal does not include construct procedures or safety precaution

Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engi & Design, P.A. liability. Please review these docu

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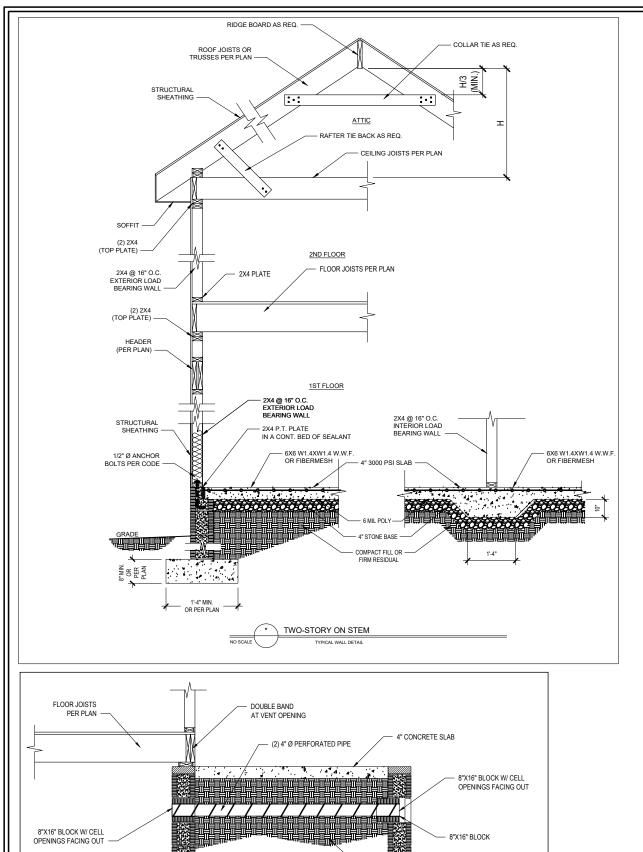


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Remarks

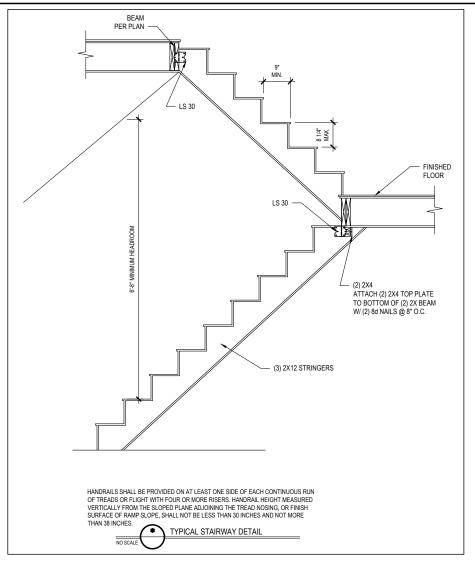
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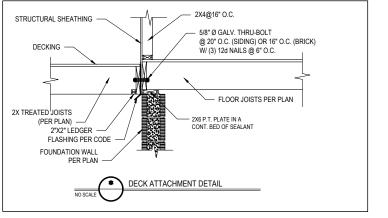
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- COMPACT FILL

FOUNDATION VENT DETAIL AT COMPACTED FILL





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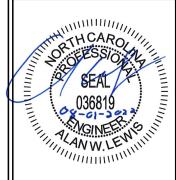
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TYNDALL ENGINEERING & DESIGN, P.A.



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Dr.

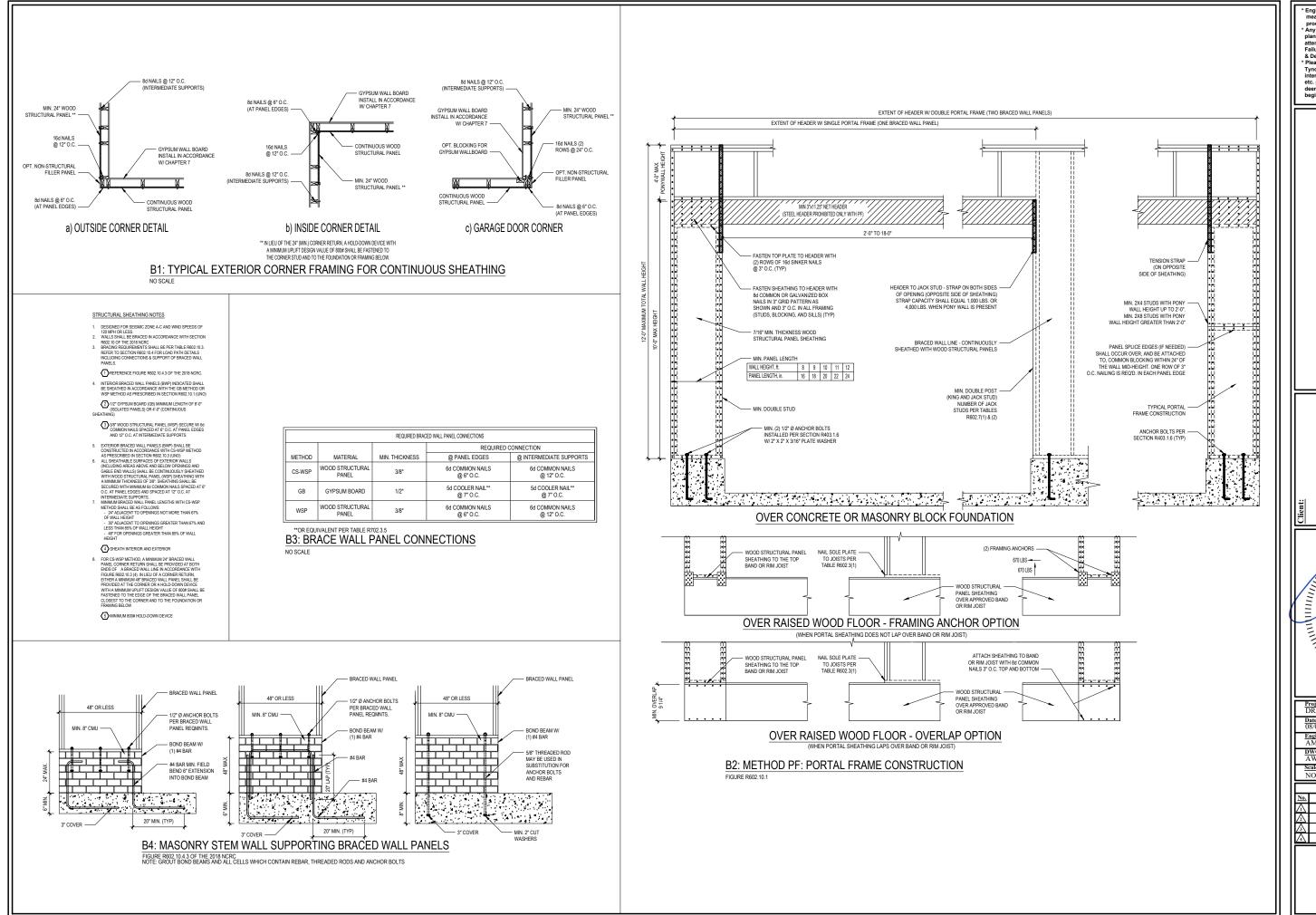


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Scale:
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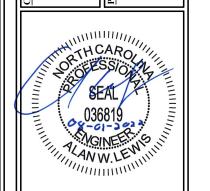
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50 Shipwash Drive = Garner = North Cavoline = 27829

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o. Date: Remarks

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